

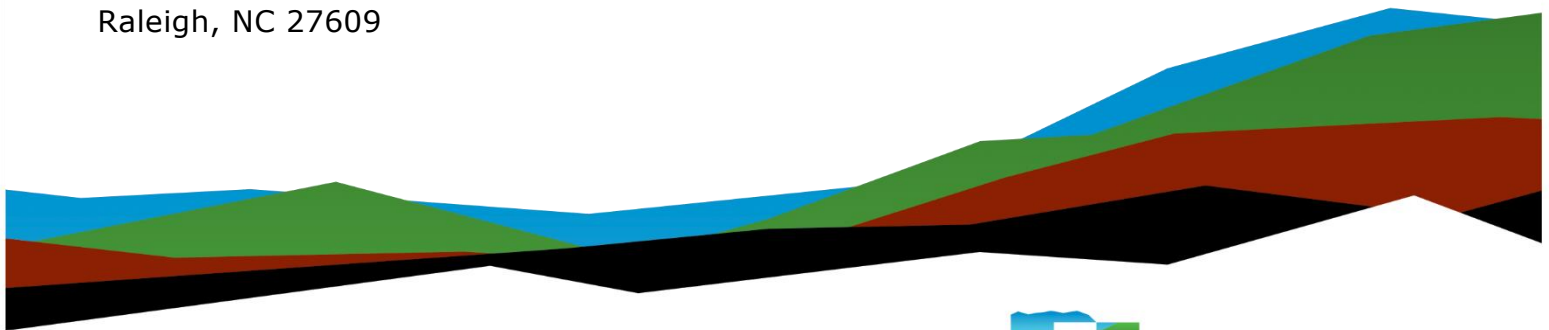
S-13-296 (Old Creek Road) Bridge Replacement over Blackwell Mill Stream

Chesterfield County, SC Geotechnical Baseline Report

August 21, 2023 (rev1) | SCDOT Project ID: P041957
Terracon Project No.: 7323P100

Prepared for:

HNTB Corporation
343 E. Six Forks Road, Suite 200
Raleigh, NC 27609





521 Clemson Road
Columbia, SC 29229
P (803) 741-9000
[Terracon.com](https://www.terracon.com)

August 21, 2023 (rev1)

HNTB Corporation
343 E. Forks Road, Suite 200
Raleigh, NC 27609

Attn: Mr. Spencer Franklin, PE, Senior Vice President
P: 919-546-8997

Re: Geotechnical Baseline Report
S-13-296 Bridge Replacement over Blackwell Mill Stream
Chesterfield County, South Carolina
SCDOT Project ID.: P041957
Terracon Project No.: 7323P100

Dear Mr. Franklin:

Terracon Consultants Inc. (Terracon) has completed the exploration, testing and limited engineering analysis services for the above referenced project. The services were conducted in general accordance with our Task Order Number 001, dated May 25, 2023.

Introduction

HNTB Corporation (HNTB) has contracted Terracon to perform subsurface exploration, laboratory testing and very preliminary engineering recommendations for the replacement of the S-13-296 bridge over Blackwell Mill Stream in Chesterfield County, South Carolina. The proposed bridge intends to replace the existing one. The results of subsurface exploration and laboratory testing have been separately presented in a Geotechnical Subsurface Data Report (GSDR). For convenience, those data are also provided here in this Geotechnical Baseline Report (GBR) along with a characterization of the subsurface conditions for the project. Very preliminary geotechnical recommendations are associated with the requested scope of study and are included in this GBR. This GBR was prepared in general accordance with the 2022 SCDOT Geotechnical Design Manual (GDM) and Preconstruction Design Memorandum (PCDM) 11 - Supplemental Design Criteria for Low Volume Bridge Replacement Projects.

Project Description

The project site is located at the S-13-296 (Old Creek Road) crossing over Blackwell Mill Stream in Chesterfield County, South Carolina. Site location and exploration plans are presented in Appendix A of this report. Based on the conceptual plans by HNTB dated 6/9/2023, the replacement bridge will be constructed on essentially the same alignment as the current bridge. The existing bridge is a multi-span structure supported by deep foundations. The conceptual plans show that the replacement bridge will be a single-span structure supported by deep foundations.

Geotechnical Testing

The geotechnical exploration for this project was performed between June 9 and June 23, 2023. The results of our field work and our associated laboratory testing are included in Appendices A and B.

Field Exploration

Our field exploration consisted of the following:

- Two (2) Standard Penetration Test (SPT) Borings (S-13-296-1 and S-13-296-2)
- One (1) offset boring of S-13-296-1 for bulk sample collection

The tests were performed at the approximate locations as approved by SCDOT. A description of our testing methods and graphical logs outlining the soil conditions at each test location are presented in Appendix A. The test locations were established in the field by Terracon and surveyed by Construction Support Services, LLC after completion.

Laboratory Testing

The following laboratory tests were performed on the soil samples collected at the site.

- Eleven (11) Natural Moisture Content Tests
- Three (3) Atterberg Limits Tests
- Seven (7) Fines Content Tests
- One (1) Grain Size Test
- Four (4) Grain Size Tests with Hydrometer
- One (1) Remolded, Consolidated-Undrained (CU) Triaxial Compression Test with Pore Pressure Readings
- One (1) Standard Proctor Test
- One (1) Corrosivity Suite Tests (pH, chloride content, sulfate content, and resistivity tests)

The general scope of the laboratory testing frequency was determined by the SCDOT. The laboratory testing assignment was performed by our engineers. The laboratory procedures and results of the laboratory tests are presented in Appendix B.

Subsurface Conditions

Regional Geology

The bridge site is located on route S-13-296, approximately 5 miles south of the town of McBee in Chesterfield County, South Carolina. The site is located in the Upper Coastal Plain Physiographic Province of South Carolina. The Upper Coastal Plain Subunit is bounded by the Piedmont Unit to the west and the Middle Coastal Plain Subunit to the east, and formed during the Paleogene, Neogene and late Cretaceous Periods. Based on mapping, the soils underlying the site are primarily those of the Cretaceous Period (65 MYA to 135 MYA).

The Upper Coastal Plain subunit is formed of older, generally well-consolidated layers of sands, silts, or clays that were deposited by marine or fluvial action during a period of retreating ocean shoreline. Due to their age, sediments exposed at the ground surface are often heavily eroded. Ridges and hills are either capped by terrace gravels or wind-deposited sands. Younger alluvial soils may mask these sediments in swales or stream valleys.

Soil and Rock Stratification

The soils encountered at this site consist of fill in the upper 5 to 9 feet, followed by alluvial soils consisting of sands and silty sands to about 12 to 17 feet below ground surface. Both the existing fill and alluvium were generally loose to very loose. Below the alluvium, coastal plain soils were encountered and continued to the termination of the borings at 100 feet below the roadway surface. The coastal plain horizon can be generally subdivided into two layers – hard clay and medium dense to very dense sand. A summary of subsurface strata found during subsurface exploration is provided in the table below.

Geology	Approximate Elevation of Layer Bottom (ft, NAVD88)	USCS Soil Type	Measured Field N Value	Plasticity Index	Fines Content
Asphalt and Stone Base	212	--	--	--	--
Fill	203 to 208	SP, SM	5 to 8	--	13 to 15
Alluvium	196 to 201	SP, SM	2 to 5	--	5 to 26
Coastal Plain – Clays	145 to 156	MH, CL	5 to 100+	24 to 25	66 to 99

Geology	Approximate Elevation of Layer Bottom (ft, NAVD88)	USCS Soil Type	Measured Field N Value	Plasticity Index	Fines Content
Coastal Plain – Sands	PMDE ¹	SC, SP	3 to 59	--	3 to 8

1. PMDE = Present to Maximum Depth Explored

Design and Construction Considerations

Foundations

Steel H-piles driven into the high consistency Coastal Plain clays are anticipated to be feasible for the proposed bridge end bents. Assuming redundant piles, Table 9-3 GDM 2022 allows using a resistance factor of 0.5 for redundant piles with wave equation, and 0.65 for redundant piles with PDA and calibrated wave equation. Considering the very loose to loose existing fill and alluvium above the much higher consistency Coastal Plain soils, it is anticipated that foundations will be installed after the approach embankment construction. If for any reason embankment fill will be placed after installing foundation piles, the pile design must account for any downdrag loads subjected to the piles.

We have observed relatively consistent depth to the high consistency clay layer, as seen in **Soil and Rock Stratification**. Therefore, we expect relatively consistent tip elevations at each end bent. Resistance of piles driven to practical refusal in these materials may be limited by their structural resistance. Therefore, likely reinforced pile tips will be required to adequately penetrate these materials. Pile drivability using the wave equation should be performed along with estimating stresses during driving and, in general, verifying the ability of the Contractor's selected hammer to drive the piles to the desired penetration while preventing overstressing.

Corrosion and Deterioration

Corrosion testing was performed on a composite sample obtained from split spoons in the upper 10 feet. Corrosion testing included pH, resistivity, chlorides, and sulfates content as summarized in Table below. Corrosion test results are included in Appendix B.

Geotechnical Baseline Report

S-13-296 BRO Blackwell Mill Stream | Chesterfield County, SC

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Corrosion Test	Results Bent 2, Boring S-13-296-2 Composite Sample from 1 to 11 feet	Indication of Corrosivity ¹
pH	6.3	Less than 5.5
Resistivity	53,000 ohm-cm	Less than 2,000 ohm-cm
Chloride	49 ppm	Greater than 500 ppm
Sulfate	59 ppm	Greater than 1,000 ppm

1. AASHTO LRFD bridge design specifications, Ninth Edition 2020, Section 10.7.5.

Based on the criteria for electro-chemical properties in the GDM Section 7.18, the electro-chemical classification of the project site is non-aggressive. Interpretation of these data should be communicated with the project's structural engineer.

Embankment Construction

Based on the conceptual plans by HNTB, cut excavation is expected in front of the end bents and up to 5 feet of fill behind the end bent. Bulk samples were obtained near End Bent 1 from the top 5 feet of existing embankment material. Per our scope, the bulk sample was tested for soil classification and was also remolded to 95% of the Standard Proctor prior to being tested under CU Triaxial Compression. Test results are presented in Appendix B and summarized in the table below.

Sample No.	Station	Offset (ft)	Sample Depth (ft)	USCS Soil Type	Compaction		Shear Strength ¹	
					Optimum Moisture (%)	Max Dry Density (pcf)	c', c (psf)	φ', φ (°)
S-13-296-1	23+36.56	4.25 L	0 – 5	SM	11.5	121.8	245, 3715	33, 11

1. Based on a maximum deviator stress failure criterion

Geotechnical Baseline Report

S-13-296 BRO Blackwell Mill Stream | Chesterfield County, SC

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Closure

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

Terracon Consultants, Inc.

Phillip A. Morrison, P.E.
Senior Engineer
SC Registration No. 17275

Abdul Q. Fekrat, PhD, P.E.
Project Engineer
SC Registration No. 38531

Reviewed by Terracon's Authorized Project Reviewer: David J. Corley, P.E.

Appendix A

Field Exploration

- Exhibit A-1 – Site Location Map
- Exhibit A-2 – Exploration Plan
- Exhibit A-3 – Subsurface Profile
- Exhibit A-4 – Summary of Boring Data
- Exhibit A-5 – GeoScoping Form (2 Pages)
- Exhibit A-6 – Field Exploration Description (2 Pages)
- Exhibit A-7 – Soil Description Terms
- Exhibit A-8 – Soil/Rock Symbols
- Exhibit A-9 – Boring Logs (6 Pages)
- Exhibit A-10 – Grout Logs (3 Pages)

Note: All exhibits are one page unless noted above.



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT
INTENDED FOR CONSTRUCTION PURPOSES

TOPOGRAPHIC MAP IMAGE COURTESY OF THE U.S. GEOLOGICAL SURVEY
QUADRANGLES INCLUDE: HORNSBORO, SC (1/1/1983) and MT CROGHAN, NC
(1/1/1983).




DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT
INTENDED FOR CONSTRUCTION PURPOSES


AERIAL PHOTOGRAPHY PROVIDED BY
MICROSOFT BING MAPS



Figure 1.1 displays eight soil types and their typical color and texture, arranged in two rows:

- Row 1:**
 - Asphalt:** Solid black.
 - Silty Sand:** Yellow with vertical black lines.
 - Poorly-graded Sand:** Yellow with small black dots.
 - Clayey Sand:** Yellow with diagonal black lines.
- Row 2:**
 - Lean Clay:** Green with diagonal black lines.
 - Sandy Lean Clay:** Yellow with diagonal black lines.
 - Poorly-graded Sand with Silt:** Yellow with small black dots and a vertical blue line.
 - Poorly-graded Sand with Clay:** Yellow with small black dots and a vertical green line.

 Water Level Reading at time of drilling.

 Water Level Reading after drilling.

Project No.: 7323P100
Scale:
File Name:



EXHIBIT

A-3

Summary of Boring Data – Exhibit A-4

S-13-296 BRO Blackwell Mill Stream | Chesterfield County, SC

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Summary of Boring Data

Boring No.	Ground Elevation ft.	Test Depth ft.	Northing	Easting	Latitude	Longitude	Station	Offset
S-13-296-1	212.4	100.0	934110.42	2215537.34	34.398661	-80.285439	23+36.56	4.25 L
S-13-296-2	213.3	100.0	934006.75	2215606.24	34.398375	-80.285213	24+60.62	5.83 R

Note: A bulk sample was collected near S-13-296-1.

GeoScoping Form

PROJECT INFORMATION	
Project ID: P041957	Date of Trip: 6/9/2023
County: Chesterfield	Location: McBee
Rd/Route: S-13-296	Local Name: Rudolph Sikes
Attendees: J. Cerceo	

EXISTING BRIDGE INFORMATION	
Bridge Length: 60 feet 6 inches	Bridge Width: 27 Feet 8 inches
Superstructure Type: Concrete framing and decking	Substructure Type: Concrete Piles
Begin Bridge Sta.: 23+80	End Bridge Sta.: 24+40.5
Begin Bridge Embankment Sta. ¹ 22+80	End Bridge Embankment Sta. ¹ : 35+40.5
Structure Number: 04976	Posted Weight Limit: 34 Tons
Crossing: Blackwell Mill Stream	Skew: 0 degrees
Latitude: 34.39875° N	Longitude: 80.25856° W
Existing Fill Height: N/A	Approximate Existing Slope Angle: N/A

¹Begin and End Bridge Embankment 100 feet down station or up station from bridge, respectively

EXISTING ROADWAY EMBANKMENT INFORMATION	
Begin Project Sta.: 17+50	Begin Bridge Embankment Sta. ¹ 22+80
Accessibility Issues: None	
Ground Cover: Asphalt Pavement	
Existing Fill Height: N/A	Approximate Existing Slope Angle: N/A
Local Development (undeveloped, developed residential, developed commercial, developed industrial, etc.): Rural, Residential	
Topography (level, flat, rolling, steep, hillside, valley, swamp, gully, etc.): Graded slope to swamp like pond	
Traffic Control Necessary (Y/N):	
No	
Surface Soil: N/A	Muck (Y/N): No
Exposed Rock (Y/N): No	In Stream Bed (Y/N): No
	In Banks (Y/N): No
Wetlands On-Site (Y/N): Yes	Wetlands Adjacent (Y/N): Yes
Depth FG to Water: < 1 foot	Water Depth: 6 feet
Depth to Existing Ground: < 1 foot	
Scour Condition at EB: Water flow under approach slab	Scour Condition at IB: N/A
End Bridge Embankment Sta. ¹ : 29+29	End Project Sta.: 30+00
Accessibility Issues: None	
Ground Cover: Asphalt Pavement	
Existing Fill Height: N/A	Approximate Existing Slope Angle: N/A
Local Development (undeveloped, developed residential, developed commercial, developed industrial, etc.): Undeveloped	
Topography (level, flat, rolling, steep, hillside, valley, swamp, gully, etc.): Rural, residential	
Traffic Control Necessary (Y/N): No	
Surface Soil: NA	Muck (Y/N): No
Exposed Rock (Y/N): No	In Stream Bed (Y/N): No
	In Banks (Y/N): No
Wetlands On-Site (Y/N): Yes	Wetlands Adjacent (Y/N): Yes (Stream)
Depth FG to Water: < 1 Foot	Water Depth: < 1 foot
Depth to Existing Ground: < 1 foot	
Scour Condition at EB: Water Flow under Approach Slab	Scour Condition at IB: N/A

GeoScoping Form

UTILITIES INFORMATION
Attached:
Above Ground/ Overhead: Overhead power lines run along the west side of the roadway
Underground:

COMMENTS
The channel under the bridge has silted up essentially blocking the natural flow of the stream. As a result, the soils behind the end bents, under the approach slabs have scoured out. Were unable to determine the total amount of scour during our site visit and field exploration.

Instructions:

1. Attach boring location plan for bridge and roadway.
2. Attach all photographs taken, photographs to be labeled as to direction looking in and what is being depicted.
3. Fill out GeoScoping Form as completely as possible, using additional sheets as necessary to describe site conditions.
4. If representative of GEC on site during GeoScoping, include GEC representative's name and contact number in Attendees block.

Field Exploration Description

Overview

The testing locations were proposed to and approved by SCDOT and located in the field by Terracon using measurements from existing structures shown on the provided drawings. The borings were surveyed by Construction Support Services, LLC after testing and drilling was complete. The locations as shown in the Exploration Plan are shown to the scale indicated.

A field log of each test location was prepared by our engineer. The final boring logs included with this report represent the engineer's description of the encountered conditions modified as necessary based on laboratory test results of the individual samples.

Soil Test Borings (STB)

All boring and sampling operations were conducted in general accordance with the following procedures:

- SCDOT Geotechnical Design Manual 2022
- Preconstruction Design Memorandum (PCDM) 11 - Supplemental Design Criteria for Low Volume Bridge Replacement Projects
- ASTM D5783, "Standard Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geo-environmental Exploration"
- ASTM D6151, "Standard Practice for Using Hollow-Stem Augers for Geotechnical Exploration and Soil Sampling
- ASTM D1586 "Test Method for Penetration Test and Split-Barrel Sampling of Soils"
- ASTM D4220 "Standard Practices for Preserving and Transporting Soil"

Each soil test boring was advanced using rotary wash drilling techniques. The initial sampling program is summarized in the following table:

Test ID	Total Depth	Interval of Continuous Sampling
S-13-296-1	100 feet or 10 feet rock coring	0 to 10 feet
S-13-296-2	100 feet or 10 feet rock coring	0 to 10 feet
S-13-296-1 Bulk	5 feet	Bulk Sample

Soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-barrel sampler, also known as a standard split-spoon. The sampler is advanced into the soil a total of 18 to 24 inches by striking the drill rod using a 140-pound automatic hammer falling 30 inches. The number of blows required to advance the sampler for each of three to four, 6-inch increments is recorded. The sum of the number of blows for the second and third

Exhibit A-6

S-13-296 BRO Blackwell Mill Stream | Chesterfield County, SC

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increments is called the "Standard Penetration Value", or N-value (N_{meas} , blows per foot). The N-value, when properly evaluated, is an index to the soil strength.

Soil classification provides a general guide to the engineering properties of various soil types and enables the engineer to apply his experience to current situations. In our exploration, samples obtained during drilling operations are examined and visually classified by a geotechnical engineer using the procedures outlined in ASTM D2487 - Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System). Laboratory testing was also performed on select split-spoon samples to evaluate index properties for further classification. The soils are described according to color, texture, and relative density or consistency (based on standard penetration resistance). The designations shown on the logs are described in the 2022 SCDOT Geotechnical Design Manual, Chapter 6.

As practical, groundwater readings were collected from each of the soil test borings after 24 hours. These water levels are indicated on the boring logs. The borings were advanced using mud rotary drilling techniques. As the drilling method introduces water into the borehole, time-of-drilling water levels may not be reliable.

At the conclusion of the work, the boreholes were backfilled with the drill cuttings and clean sand. The upper 20 feet of the boreholes were grouted with a cement bentonite grout and capped with cold-patch asphalt.

SOIL DESCRIPTION TERMS

Relative Density/Consistency Terms

<u>Relative Density</u> ¹			<u>Consistency</u> ²		
Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Unconfined Compression Strength (q _u) (tsf)	SPT Blow Count
Very Loose	0 to 15%	4 and less	Very Soft	0.25 and less	2 and less
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	51 and more	Very Stiff	2.01 to 4.00	16 to 30
			Hard	4.01 and more	31 and more

Moisture Condition

<u>Descriptive Term</u>	<u>Criteria</u>
Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually in coarse-grained soils below the water table

Color

Describe the sample color while sample is still moist.

Angularity¹

<u>Descriptive Term</u>	<u>Criteria</u>
Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces.
Subangular	Particles are similar to angular description but have rounded edges.
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges.
Rounded	Particles have smoothly curved sides and no edges.

HCl Reaction³

<u>Descriptive Term</u>	<u>Criteria</u>
None Reactive	No visible reaction
Weakly Reactive	Some reaction, with bubbles forming slowly
Strongly Reactive	Violent reaction, with bubbles forming immediately

Cementation³

<u>Descriptive Term</u>	<u>Criteria</u>
Weakly Cemented	Crumbles or breaks with handling or little finger pressure
Cemented	Crumbles or breaks with considerable finger pressure
Strongly Cemented	Will not crumble or break with finger pressure

Particle-Size Range¹

<u>Gravel</u>	Diameter, mm	Sieve Size	<u>Sand</u>	Diameter, mm	Sieve Size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	¾ inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4.00 to 4.76	#10 to #4

Primary Soil Type^{1, 2}

The primary soil type will be shown in all capital letters.

USCS Soil Designation

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

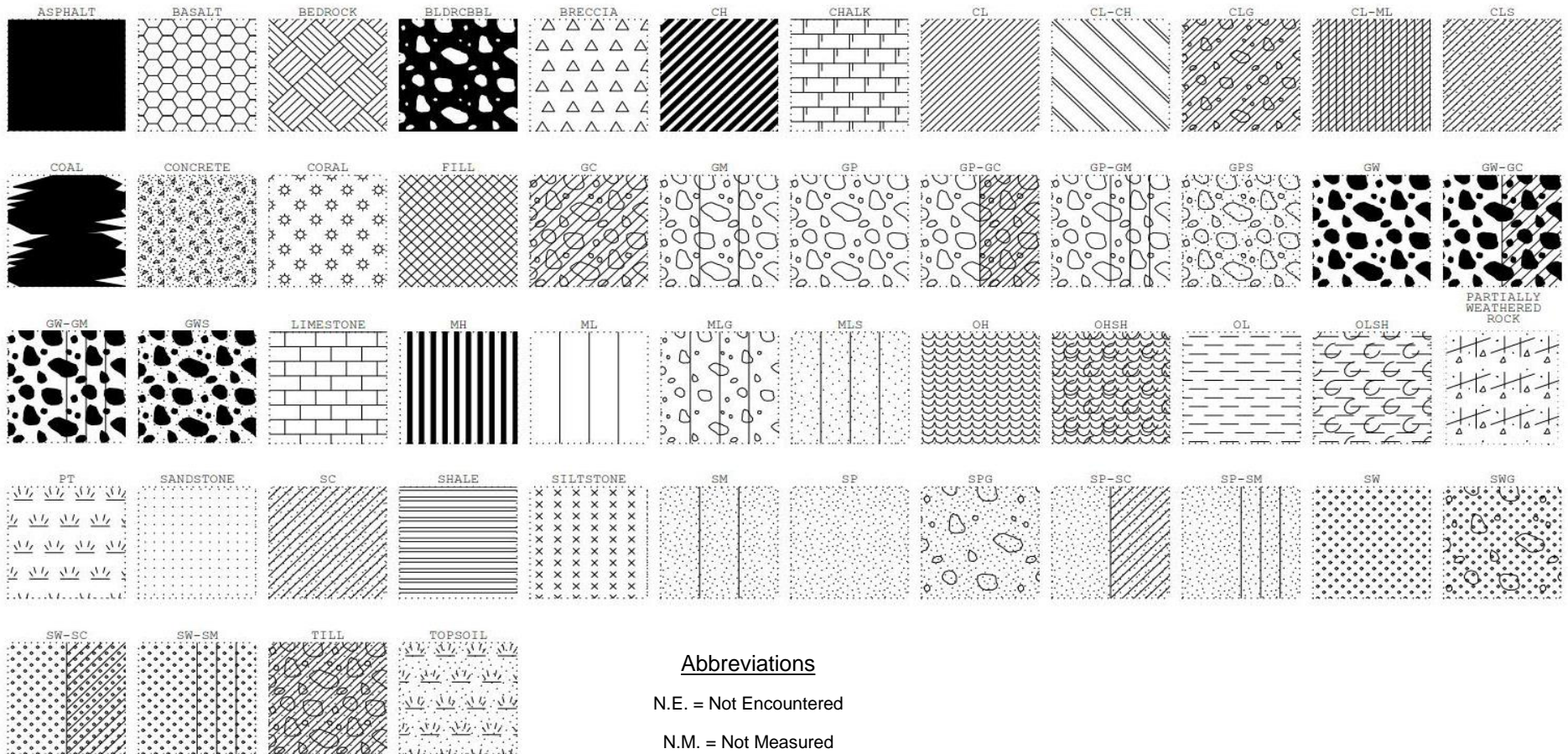
AASHTO Soil Designation

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

¹Applies to coarse-grained soils (major portion retained on No. 200 sieve)

²Applies to fine-grained soils (major portion passing No. 200 sieve)

³Use as required



Project Manager:	PAM
Drawn by:	KJZ
Checked by:	PAM
Approved by:	DJC

Project No.	7323P100
Scale:	N.T.S.
File Name:	Soil – Rock – Log
Date:	Jul 2023



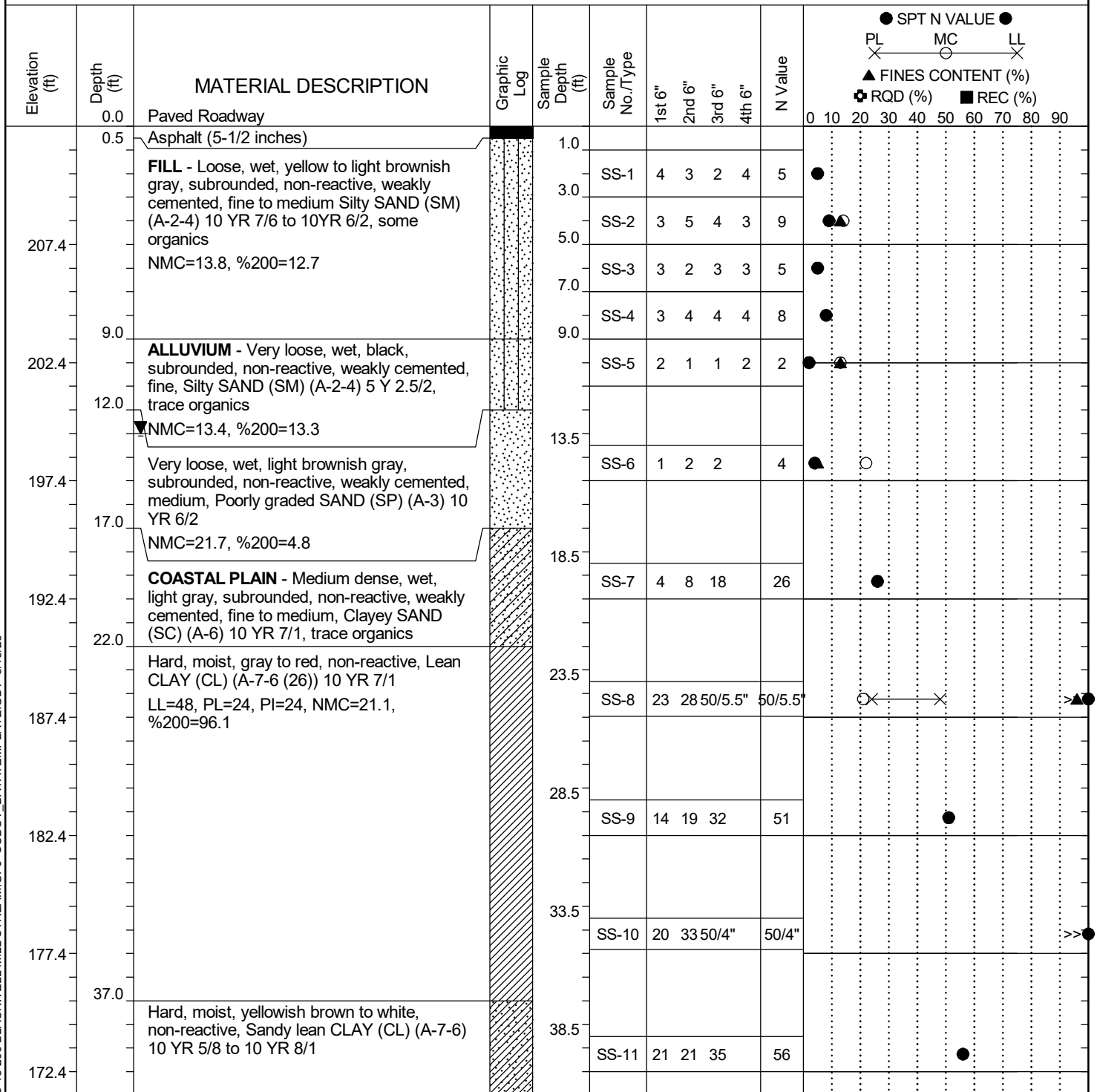
521 Clemson Road
PH. (803) 741-9000
Columbia, SC 29229
FAX. (803) 741-9900

SOIL AND ROCK SYMBOLS

Exhibit A-8

SCDOT Soil Test Log

Project ID:	P041957	County:	Chesterfield	Boring No.:	S-13-296-1
Site Description:	S-13-296 BRO Blackwell Mill Stream			Route:	S-13-296
Eng./Geo.:	K. Figate	Boring Location:	23+36.56	Offset:	4.25 L
Elev.:	212.4 ft	Latitude:	34.398661	Longitude:	-80.285439
Date Started:	6/14/2023				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	0 ft
Date Completed:	6/23/2023				
Bore Hole Diameter (in):	3	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	DR543	Drill Method:	WR	Hammer Type:	Automatic
Energy Ratio:	93.5%				
Core Size:		Driller:	C.Costner	Groundwater:	TOB N.M. 24HR 13 ft



LEGEND

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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-13-296 BLACKWELL MILL STREAM.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

Project ID:	P041957	County:	Chesterfield	Boring No.:	S-13-296-1
Site Description:	S-13-296 BRO Blackwell Mill Stream			Route:	S-13-296
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Liner Used:	Y (N)				
Drill Machine:	DR543	Drill Method:	WR	Hammer Type:	Automatic
Energy Ratio:	93.5%				
Core Size:		Driller:	C.Costner	Groundwater:	TOB N.M. 24HR 13 ft

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd 6"	3rd 6"	4th 6"	N Value	<div> ● SPT N VALUE ● PL X MC X LL X ▲ FINES CONTENT (%) + RQD (%) ■ REC (%) </div>
167.4	43.5				SS-12	44	40	50/4"	50/4"		>>●
162.4	48.8				SS-13	23	50/5"	50/5"			>>●
157.4	53.5				SS-14	47	40	44	84		●
152.4	57.0	Medium dense, moist, pinkish white, subrounded, non-reactive, strongly cemented, fine to medium, Clayey SAND (SC) (A-2-6) 7.5 YR 8/2			SS-15	7	7	12	19		●
147.4	62.0	Hard, moist, pinkish gray, non-reactive, Sandy lean CLAY (CL) (A-6) 7.5 YR 7/2			SS-16	12	12	21	33		●
142.4	67.0	Medium dense, pinkish gray, subrounded, non-reactive, strongly to weakly cemented, fine to medium, Clayey SAND (SC) (A-2-6) 7.5 YR 7/2			SS-17	8	9	12	21		●
137.4	72.0	Dense to very dense, wet, reddish brown to white, subrounded, weakly cemented, fine to medium, Poorly graded SAND with silt (SP-SM) (A-3) 2.5 YR 5/3 to 10 YR 8/1			SS-18	17	23	36	59		●
132.4	78.5				SS-19	15	15	24	39		●

LEGEND

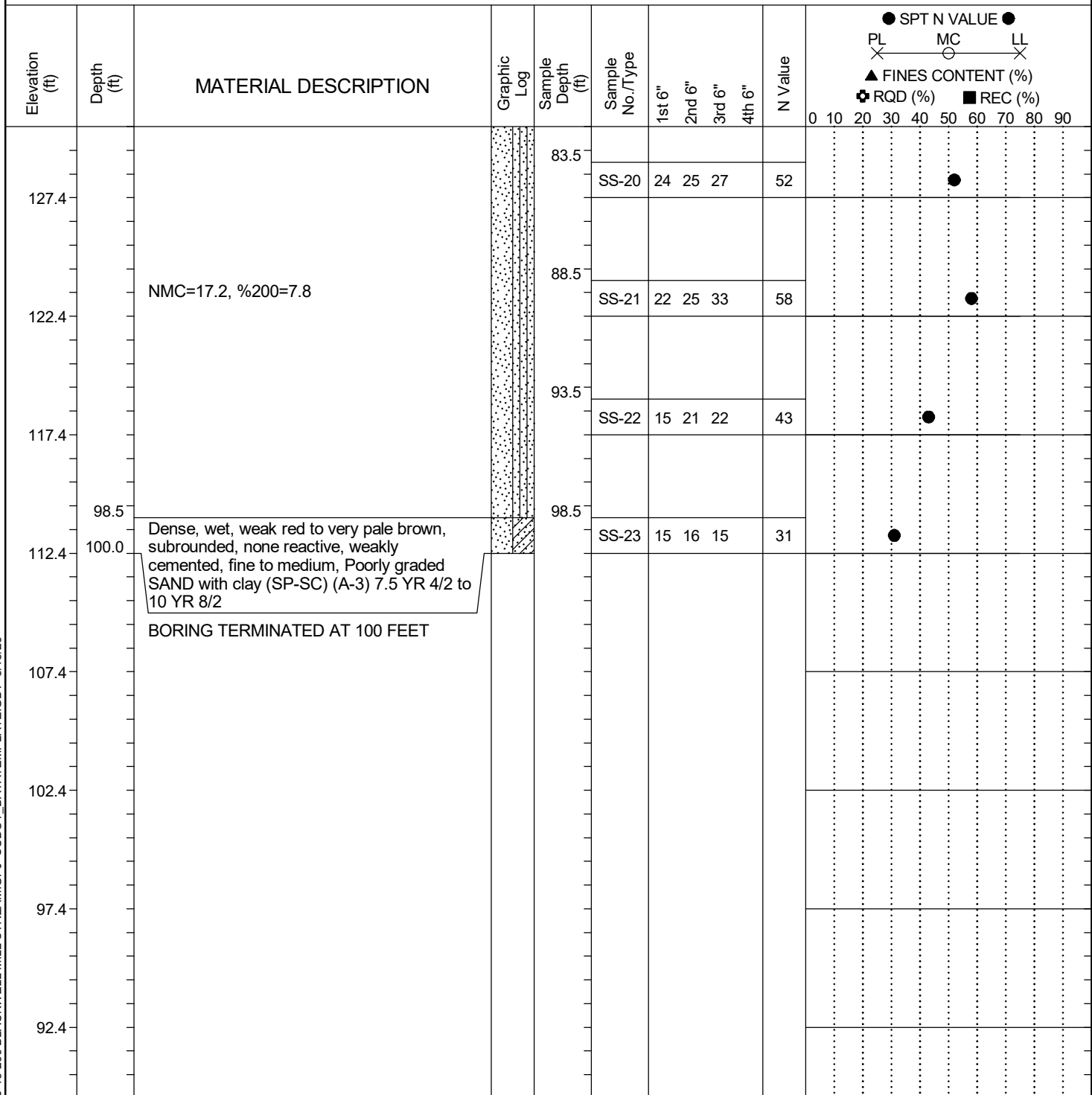
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-13-296 BLACKWELL MILL STREAM.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

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Date Completed:	6/23/2023				
Bore Hole Diameter (in):	3	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	DR543	Drill Method:	WR	Hammer Type:	Automatic
Energy Ratio:	93.5%				
Core Size:		Driller:	C.Costner	Groundwater:	TOB N.M. 24HR 13 ft



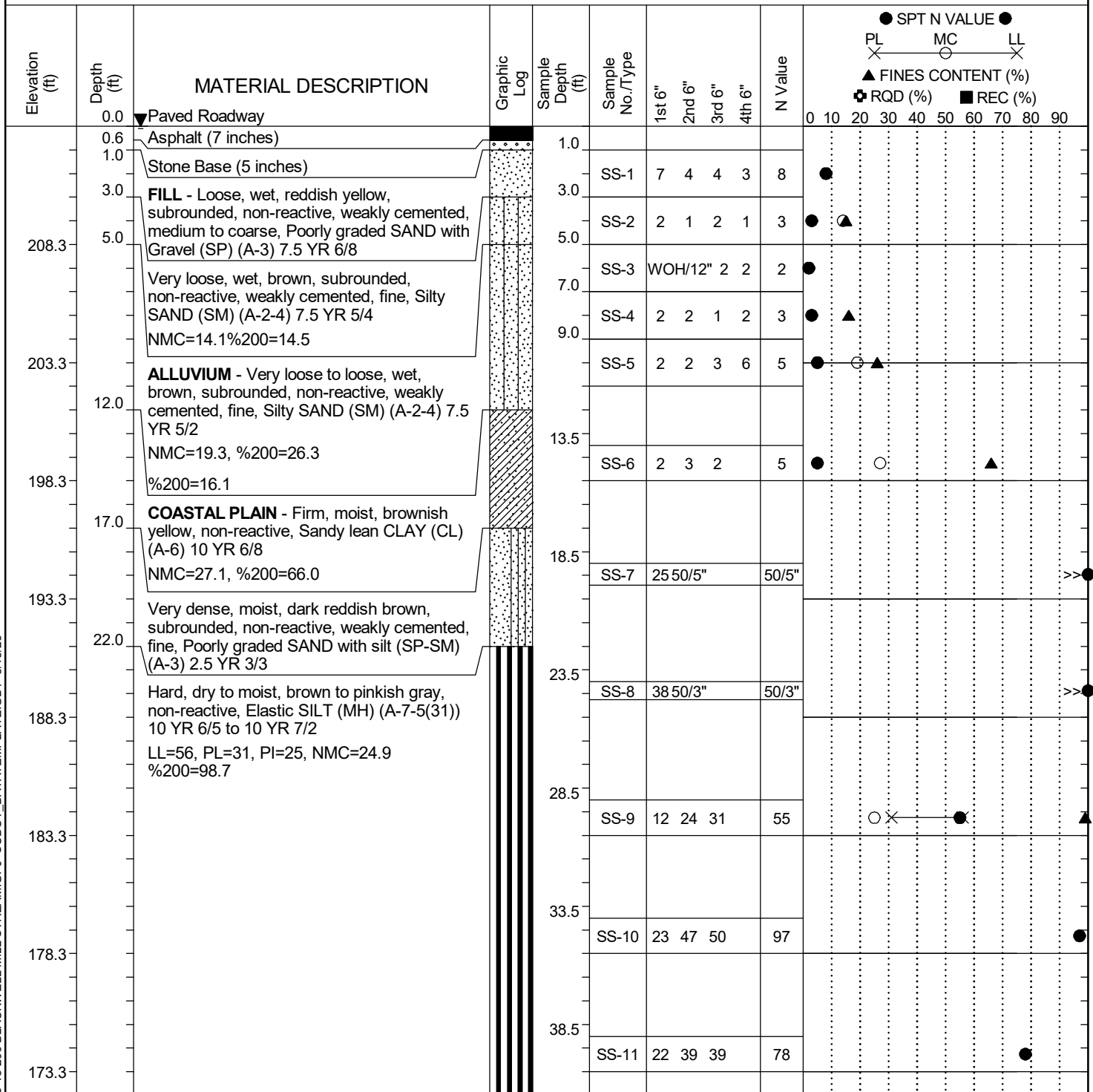
LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-13-296 BLACKWELL MILL STREAM.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

Project ID:	P041957	County:	Chesterfield	Boring No.:	S-13-296-2
Site Description:	S-13-296 BRO Blackwell Mill Stream			Route:	S-13-296
Eng./Geo.:	Z. McIntosh	Boring Location:	24+60.62	Offset:	5.83 R
Elev.:	213.3 ft	Latitude:	34.398375	Longitude:	-80.285213
Date Started:	06/09/2023				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	0 ft
Date Completed:	6/23/2023				
Bore Hole Diameter (in):	3	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	DR543	Drill Method:	WR	Hammer Type:	Automatic
Energy Ratio:	93.5%				
Core Size:		Driller:	C.Costner	Groundwater:	TOB N.M. 24HR 0, slight artesian flow



LEGEND

Continued Next Page

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-13-296 BLACKWELL MILL STREAM.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

Project ID:	P041957	County:	Chesterfield	Boring No.:	S-13-296-2
Site Description:	S-13-296 BRO Blackwell Mill Stream			Route:	S-13-296
Eng./Geo.:	Z. McIntosh	Boring Location:	24+60.62	Offset:	5.83 R
Elev.:	213.3 ft	Latitude:	34.398375	Longitude:	-80.285213
Date Started:	06/09/2023				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	0 ft
Date Completed:	6/23/2023				
Bore Hole Diameter (in):	3	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)	Drill Machine:	DR543	Drill Method:	WR
Hammer Type:	Automatic	Energy Ratio:	93.5%		
Core Size:		Driller:	C.Costner	Groundwater:	TOB N.M.
24HR	0, slight artesian flow				

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd 6"	3rd 6"	4th 6"	N Value	<div> ● SPT N VALUE ● PL X MC X LL X ▲ FINES CONTENT (%) ⊕ RQD (%) ■ REC (%) </div>
168.3	42.0	Hard, dry to moist, light gray to darkish red brown, non-reactive, Lean CLAY with sand (CL) (A-7-6) 7.5 YR 7/1 to 2.5 YR 3/3		43.5	SS-12	21	32	44		76	
163.3	52.0	Hard, dry to moist, pinkish gray, non-reactive, Lean CLAY (CL) (A-7) 7.5 YR 7/2		48.5	SS-13	50/4"				50/4"	>>●
158.3	57.0	Medium dense, moist to wet, pinkish gray, subrounded, non-reactive, moderately cemented, fine to coarse, Clayey SAND (SC) (A-2-6) 7.5 YR 7/2		53.5	SS-14	23	50/5"			50/5"	>>●
153.3	62.0	Medium dense, moist to wet, pinkish gray, subrounded, non-reactive, moderately cemented, fine to coarse, Clayey SAND (SC) (A-2-6) 7.5 YR 7/2		58.5	SS-15	2	6	6		12	●
148.3	72.0	Medium dense, moist to wet, white, subrounded, non-reactive, weakly cemented, fine to coarse, Well graded SAND with clay (SW-SC) (A-3) 7.5 YR 8/1		63.5	SS-16	6	9	8		17	●
143.3	77.0	Very loose, wet, light pink, subrounded, non-reactive, weakly cemented, fine to coarse, Poorly graded SAND (SP) (A-3) 5 R 8/2 NMC=22.2, %200=3.0		68.5	SS-17	6	12	17		29	●
138.3				73.5	SS-18	3	2	1		3	● ○
133.3		Medium dense to dense, wet, white to light red, subrounded, non-reactive, weakly cemented, fine to coarse, Poorly graded SAND (SP) (A-3) 5 YR 8/2 to 10 R 7/6		78.5	SS-19	11	21	18		39	●

LEGEND

Continued Next Page

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-13-296 BLACKWELL MILL STREAM.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

Project ID:	P041957	County:	Chesterfield	Boring No.:	S-13-296-2
Site Description:	S-13-296 BRO Blackwell Mill Stream			Route:	S-13-296
Eng./Geo.:	Z. McIntosh	Boring Location:	24+60.62	Offset:	5.83 R
Elev.:	213.3 ft	Latitude:	34.398375	Longitude:	-80.285213
Date Started:	06/09/2023				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	0 ft
Date Completed:	6/23/2023				
Bore Hole Diameter (in):	3	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	DR543	Drill Method:	WR	Hammer Type:	Automatic
Energy Ratio:	93.5%				
Core Size:		Driller:	C.Costner	Groundwater:	TOB N.M. 24HR 0, slight artesian flow

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd 6"	3rd 6"	4th 6"	N Value	<div> <div> ● SPT N VALUE ● </div> <div> PL X MC X LL X </div> <div> ▲ FINES CONTENT (%) </div> <div> ⊕ RQD (%) ■ REC (%) </div> </div>
128.3				83.5	SS-20	4	8	13		21	
123.3				88.5	SS-21	13	15	18		33	
118.3				93.5	SS-22	9	11	7		18	
113.3	100.0	BORING TERMINATED AT 100 FEET		98.5	SS-23	7	12	11		23	
108.3											
103.3											
98.3											
93.3											

LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

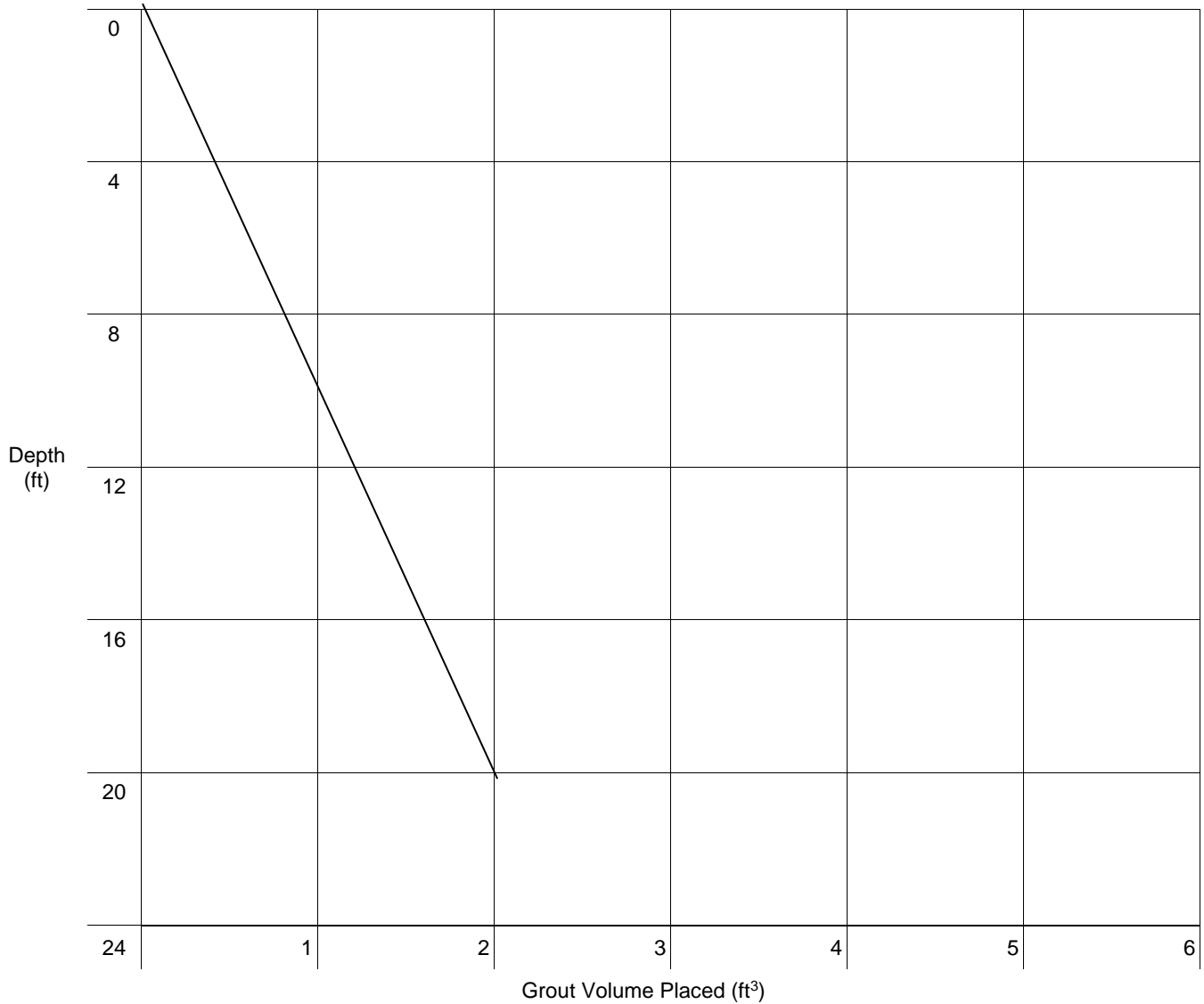
SC.DOT SCDOT S-13-296 BLACKWELL MILL STREAM.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-13-296- BRO Blackwell Mill Stream		Test Hole No.:	S-13-296-1
Project ID:	P041957		Station:	23+36.56
Consultant Firm:	Terracon Consultants, Inc.		Offset:	4.25 L
Grouted By (Driller's Name):	Costner	Date	6/23/23	
Notes:	Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water			

Grout Curve

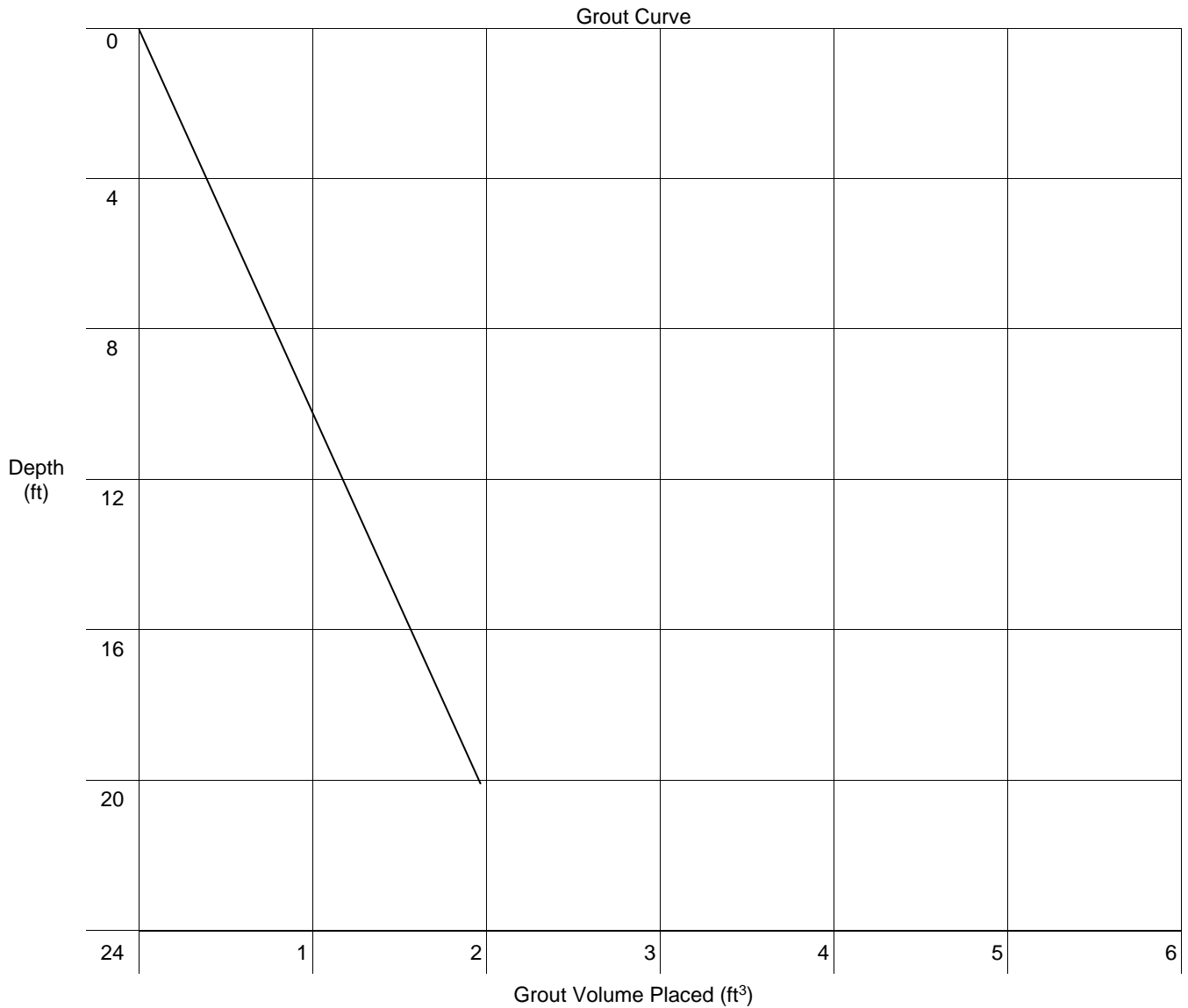


Number of Bags On-Site	20	ea.
Depth of Test Hole Grouted	20	ft.
Diameter of Test Hole	0.33	ft.
Area of Test Hole	0.09	ft²
Volume of Test Hole	1.74	ft³
Volume of Casing (If applicable)	-	ft³
Theoretical Volume of Test Hole	1.74	ft³
Number of Bags Used	2.5	ea.
Volume Placed	2	ft³



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-13-296- BRO Blackwell Mill Stream		Test Hole No.:	S-13-296-2
Project ID:	P041957		Station:	24+60.62
Consultant Firm:	Terracon Consultants, Inc.		Offset:	5.83 R
Grouted By (Driller's Name):	Costner	Date	6/23/23	
Notes:	Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water			



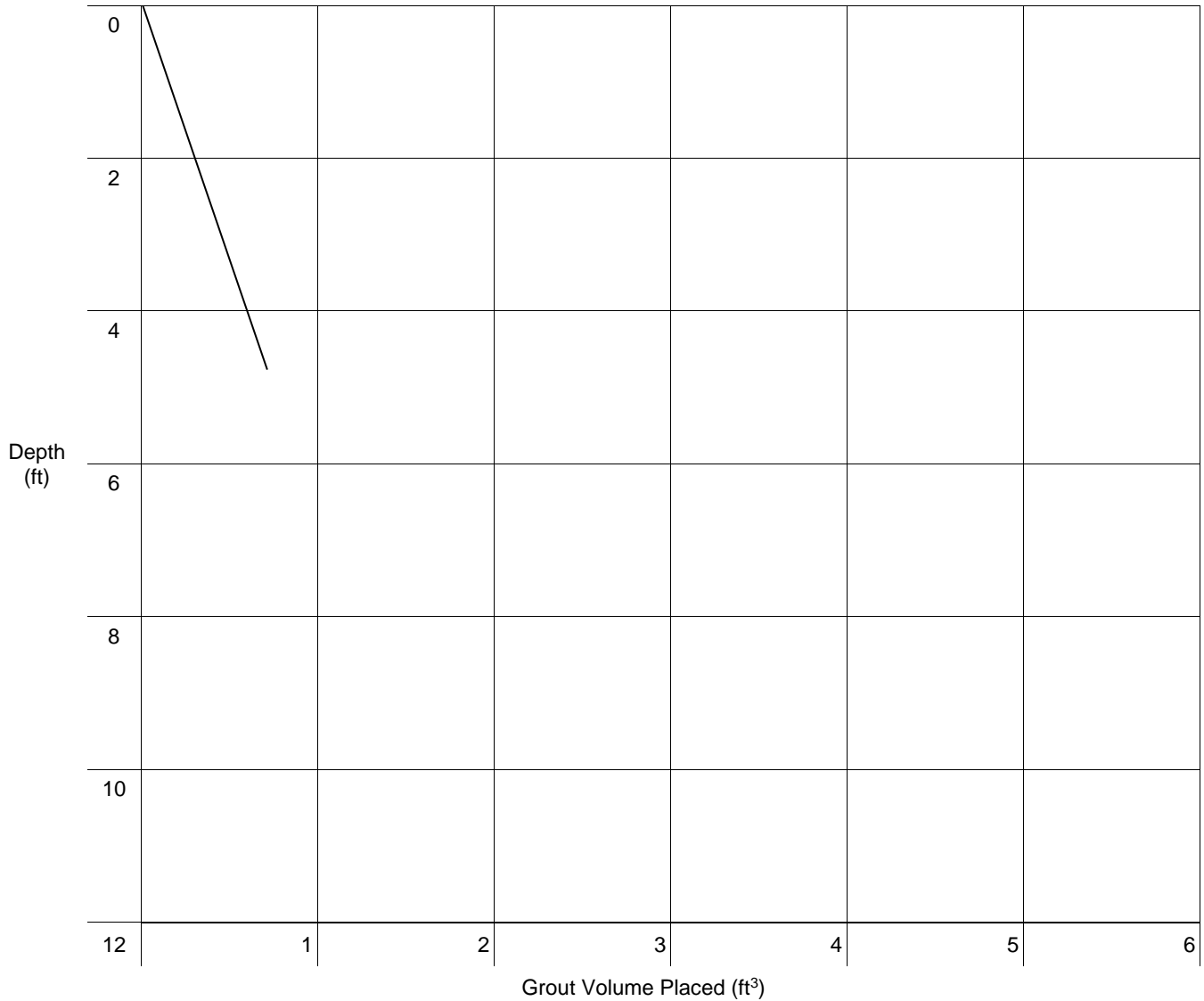
Number of Bags On-Site	20	ea.
Depth of Test Hole Grouted	20	ft.
Diameter of Test Hole	0.33	ft.
Area of Test Hole	0.09	ft²
Volume of Test Hole	1.74	ft³
Volume of Casing (If applicable)	-	ft³
Theoretical Volume of Test Hole	1.74	ft³
Number of Bags Used	2.5	ea.
Volume Placed	2	ft³



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-13-296- BRO Blackwell Mill Stream		Test Hole No.:	S-13-296-1
Project ID:	P041957		Station:	(BULK)
Consultant Firm:	Terracon Consultants, Inc.		Offset:	23+36.56
Grouted By (Driller's Name):	Costner	Date	6/23/23	4.25 L
Notes:	Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water			

Grout Curve



Number of Bags On-Site	20	ea.
Depth of Test Hole Grouted	5	ft.
Diameter of Test Hole	0.5	ft.
Area of Test Hole	0.2	ft ²
Volume of Test Hole	1.0	ft ³
Volume of Casing (If applicable)	-	ft ³
Theoretical Volume of Test Hole	1.0	ft ³
Number of Bags Used	1.5	ea.
Volume Placed	0.7	ft ³

Appendix B

Laboratory Testing

Exhibit B-1 – Laboratory Testing Description
Summary of Laboratory Data
Laboratory Data Sheets (15 Pages)

Note: All exhibits are one page unless noted above.

Exhibit B-1

S-13-296 BRO Blackwell Mill Stream | Chesterfield County, SC
August 21, 2023 (rev1) | Terracon Project No. 7323P100 | SCDOT Project ID: P041957



Laboratory Testing Description

The samples collected during the field exploration were taken to our laboratory for additional testing. The laboratory testing scope was developed by the SCDOT and laboratory assignment was performed by Terracon. The laboratory tests were conducted on selected soil samples from the borings and the bulk sample locations. The test results are presented in this appendix.

The laboratory test results were used to confirm the soil descriptions presented on the boring logs in Appendix A. Laboratory tests were performed in general accordance with the applicable ASTM, AASHTO, SCDOT or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

■	Moisture Content	AASHTO T265/(ASTM D2216)
■	Atterberg Limits	AASHTO T89/T90(ASTM D4318)
■	Wash 200	AASHTO T11/(ASTM D1140)
■	Triaxial Shear CU w/ PP	AASHTO T297/(ASTM D4767)
■	Grain Size Distribution	ASTM D6913
■	Hydrometer	ASTM D7928
■	Corrosion Series	AASHTO D422
		AASHTO T289/ASTM G51
		AASHTO T290/ASTM C1580
		AASHTO T291

SUMMARY OF LABORATORY RESULTS

PAGE 1 OF 1

BORING ID	Depth (Ft.)	Soil Classification USCS & AASHTO	Liquid Limit	Plastic Limit	Plasticity Index	% Fines	% Gravel	% Sand	% Silt	% Clay	Water Content (%)
S-13-296 Bulk	0-5	SILTY SAND(SM) / A-2-4 (0)	NP	NP	NP	20.1					10.1
S-13-296-1	3-5					12.7					13.8
S-13-296-1	9-11					13.3	0.5	86.2	8.2	5.1	13.4
S-13-296-1	13.5-15					4.8					21.7
S-13-296-1	23.5-25	LEAN CLAY(CL) / A-7-6 (26)	48	24	24	96.1	0.0	3.9			21.1
S-13-296-1	88.5-90					7.8					17.2
S-13-296-2	3-5					14.5					14.1
S-13-296-2	7-9					16.1	1.1	82.8	6.9	9.2	
S-13-296-2	9-11					26.3	1.2	72.5	17.0	9.3	19.3
S-13-296-2	13.5-15					66.0	0.1	33.9	32.7	33.4	27.1
S-13-296-2	28.5-30	ELASTIC SILT(MH) / A-7-5 (31)	56	31	25	98.7					24.9
S-13-296-2	73.5-75					3.0					22.2
PROJECT: S-13-296 BRO Blackwell Mill Stream			<div>Terracon</div> <div>521 Clemson Rd Columbia, SC</div>					PROJECT NUMBER: 7323P100			
SITE: Chesterfield County, SC								CLIENT: NHTB			
			PH. 803-741-9000 FAX. 803-741-9900								



INDEX PROPERTIES VERSUS DEPTH

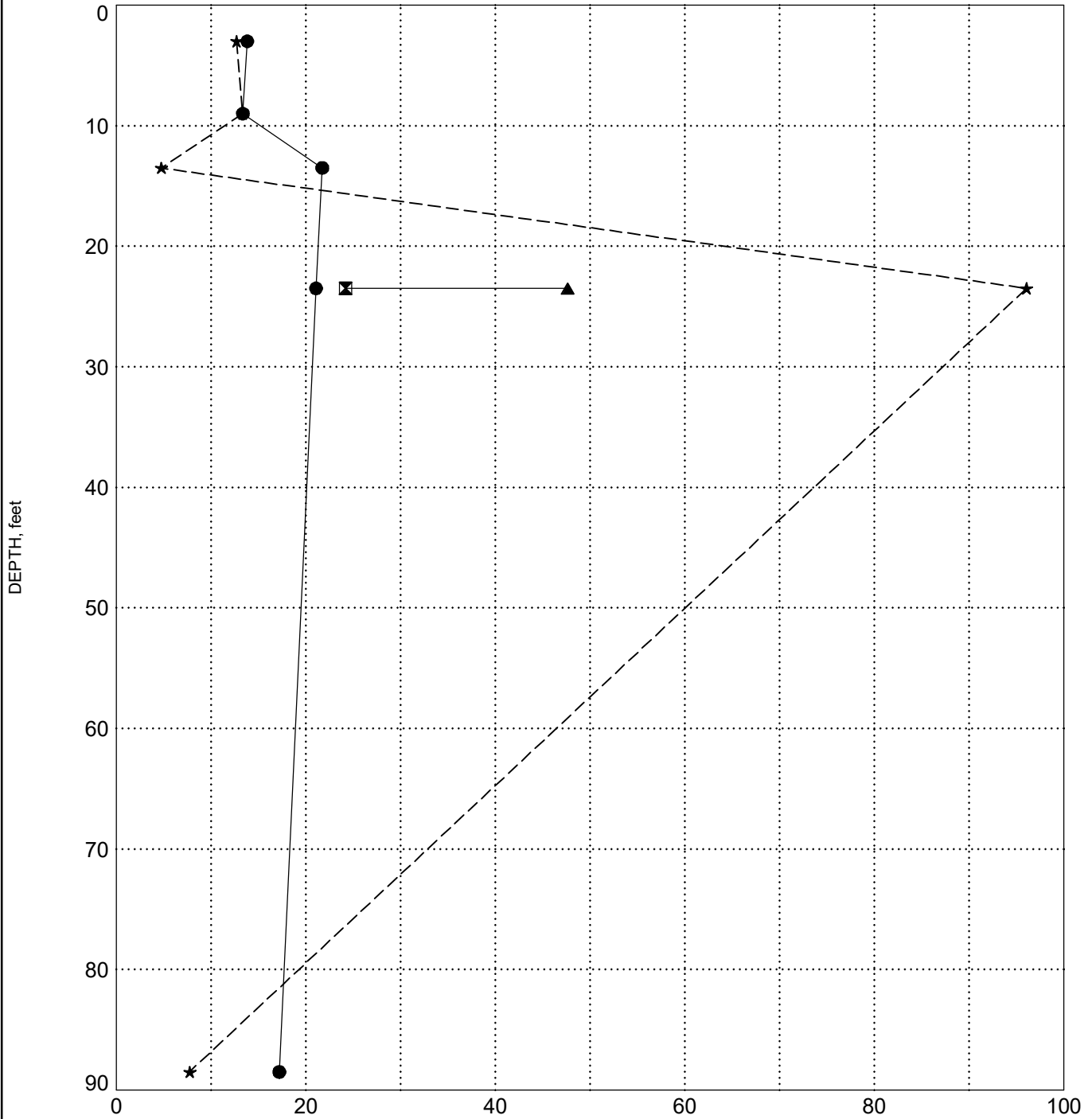
PROJECT ID P041957

PROJECT NAME S-13-296 BRO Blackwell Mill Stream

PROJECT COUNTY Chesterfield

SURFACE ELEVATION: 212.4

BORING S-13-296-1



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines



INDEX PROPERTIES VERSUS DEPTH

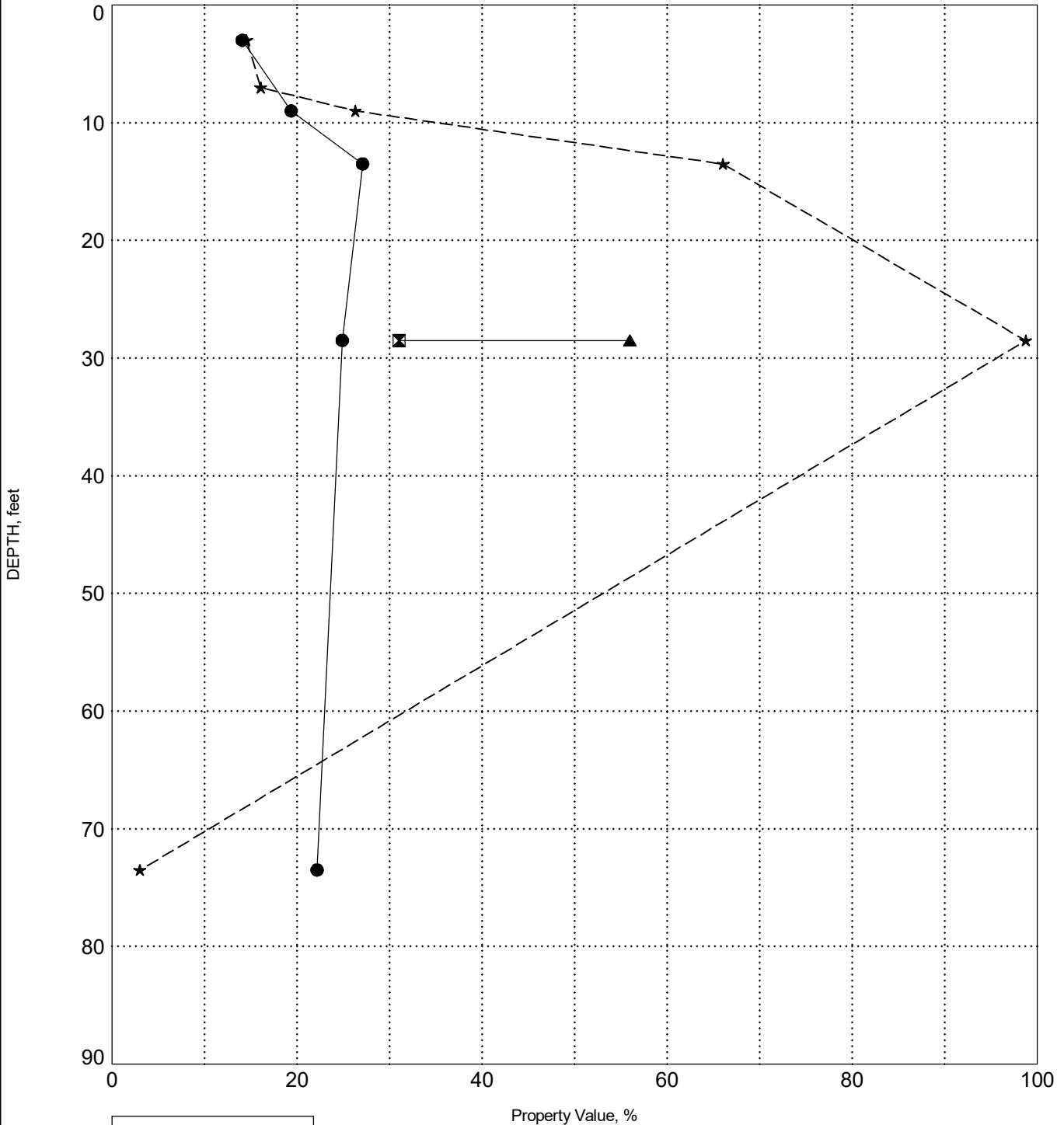
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PROJECT NAME S-13-296 BRO Blackwell Mill Stream

PROJECT COUNTY Chesterfield

SURFACE ELEVATION: 213.3

BORING S-13-296-2

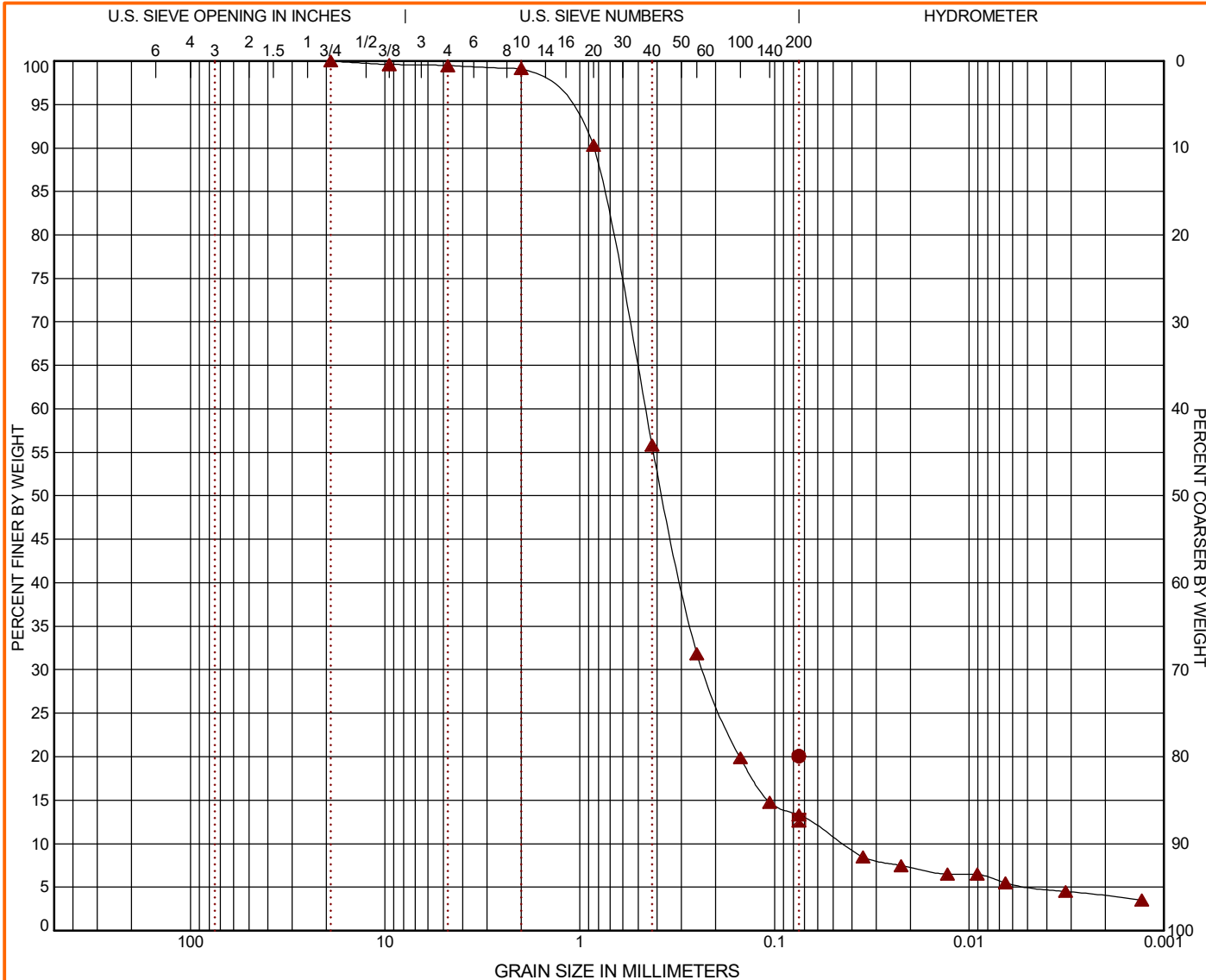


LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-13-296 BLACKWELL BRANCH.GPJ TERRACON_DATATEMPLATE.GDT 8/18/23



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
● S-13-296 Bulk	0 - 5					20.1		SM
☒ S-13-296-1	3 - 5					12.7		
▲ S-13-296-1	9 - 11	0.0	0.5	86.2	8.2		5.1	

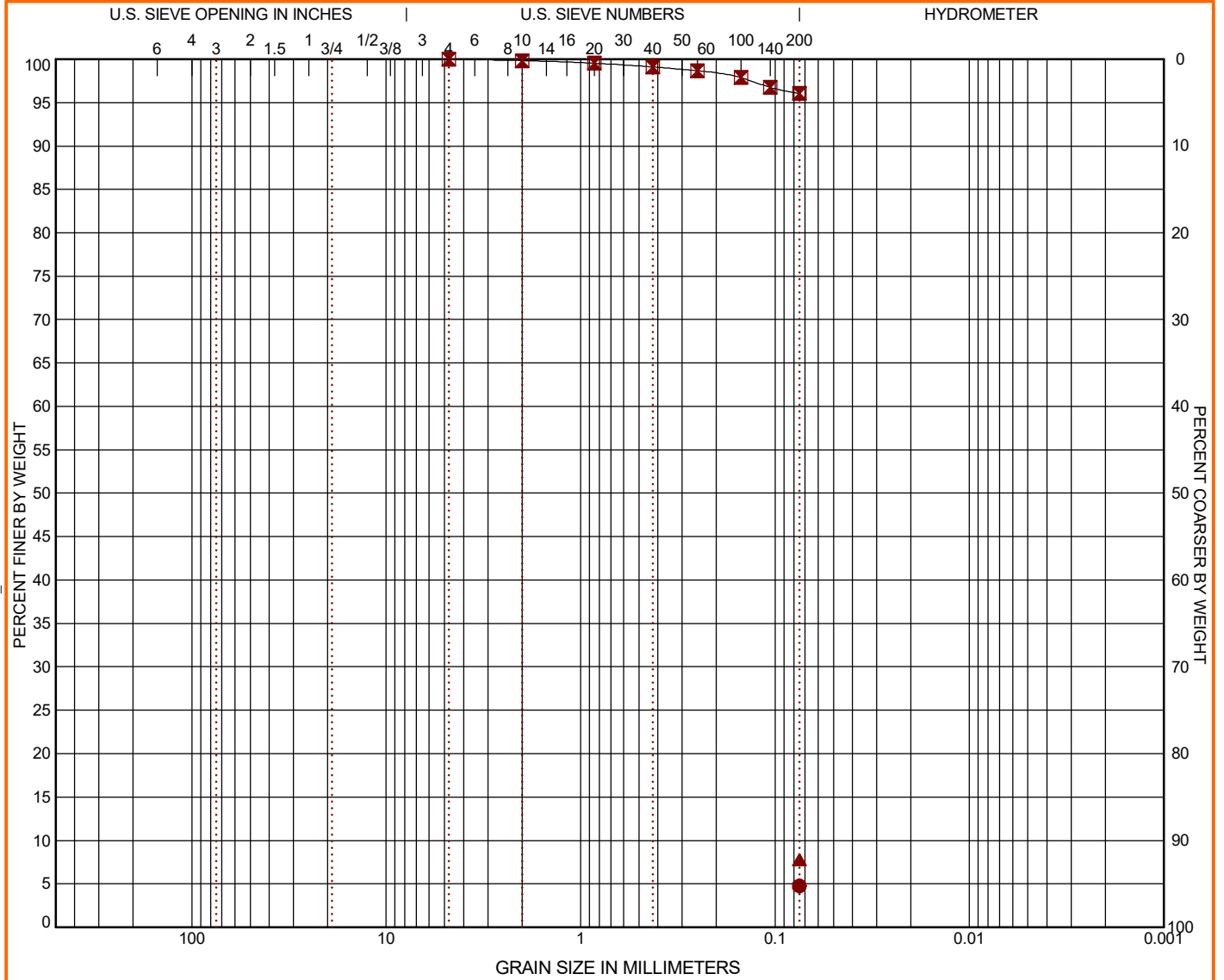
GRAIN SIZE				Sieve				SOIL DESCRIPTION			
	●	☒	▲	#200	% Finer	#200	% Finer	3/4"	3/8"	#4	#10
D ₆₀			0.462		20.06		12.69	100.0	99.61	99.48	99.14
D ₃₀			0.231					99.82	99.48	99.14	90.34
D ₁₀			0.044					55.82	31.85	19.88	14.78
COEFFICIENTS											
	●	☒	▲								
C _c			2.60								
C _u			10.39								

PROJECT: S-13-296 BRO Blackwell Mill Stream	<p>521 Clemson Rd Columbia, SC</p>	PROJECT NUMBER: 7323P100
SITE: Chesterfield County, SC		CLIENT: NHTB

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-13-296 BLACKWELL BRANCH.GPJ TERRACON.DATATEMPLATE.GDT 8/18/23



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
● S-13-296-1	13.5 - 15					4.8		
☒ S-13-296-1	23.5 - 25	0.0	0.0	3.9		96.1		CL
▲ S-13-296-1	88.5 - 90					7.8		

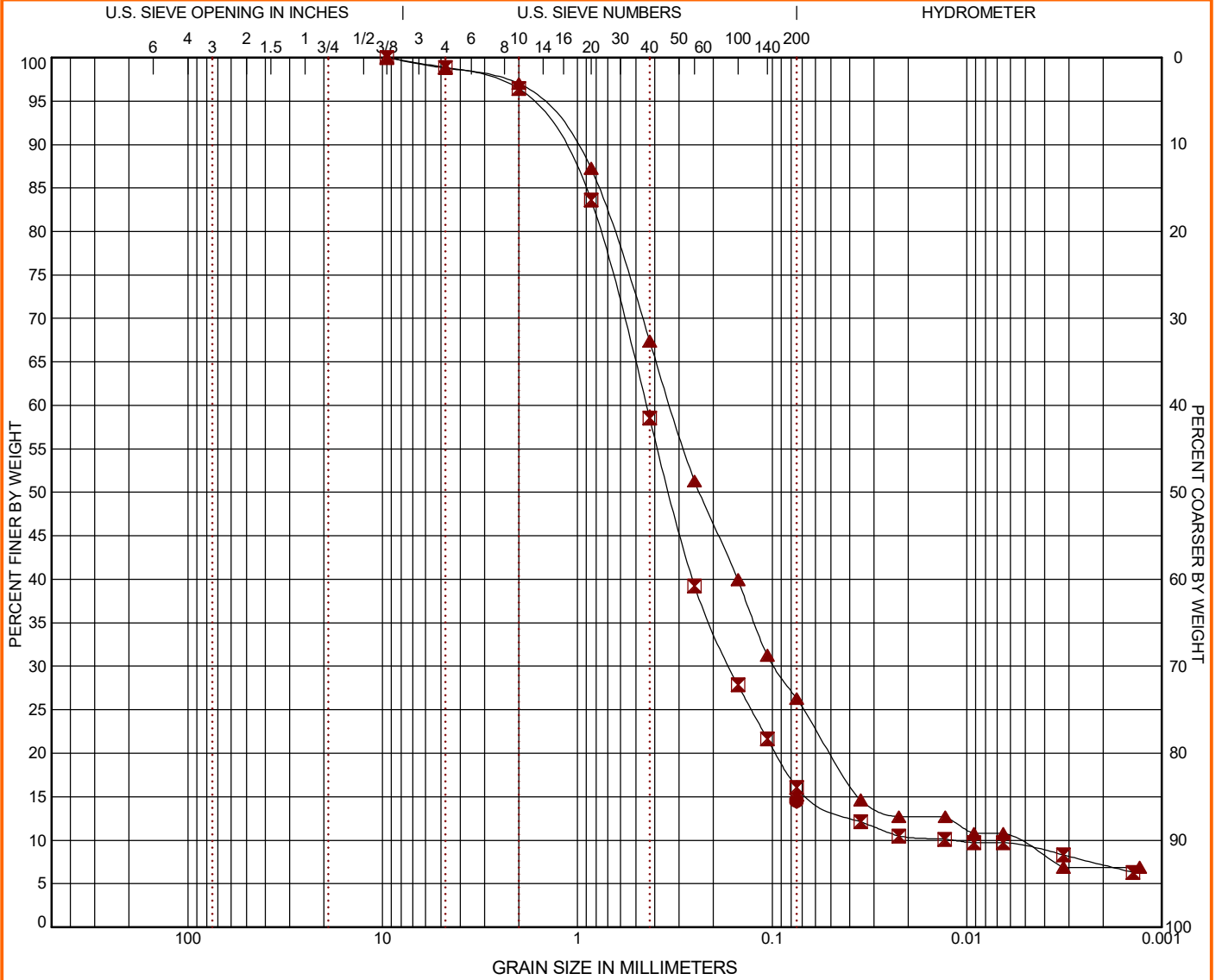
GRAIN SIZE				SOIL DESCRIPTION			
	●	☒	▲	Sieve	% Finer	Sieve	% Finer
D ₆₀				#200	4.76	#200	7.76
D ₃₀				#4	100.0		
D ₁₀				#10	99.83		
				#20	99.53		
				#40	99.11		
				#60	98.67		
				#100	97.88		
				#140	96.76		
				#200	96.05		
COEFFICIENTS				REMARKS			
	●	☒	▲				
C _c							
C _u							

PROJECT: S-13-296 BRO Blackwell Mill Stream	<p>521 Clemson Rd Columbia, SC</p>	PROJECT NUMBER: 7323P100
SITE: Chesterfield County, SC		CLIENT: NHTB

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-13-296 BLACKWELL BRANCH.GPJ TERRACON.DATATEMPLATE.GDT 8/18/23



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
● S-13-296-2	3 - 5					14.5		
■ S-13-296-2	7 - 9	0.0	1.1	82.8	6.9		9.2	
▲ S-13-296-2	9 - 11	0.0	1.2	72.5	17.0		9.3	

GRAIN SIZE			
	●	■	▲
D ₆₀		0.443	0.333
D ₃₀		0.165	0.097
D ₁₀		0.012	0.006
COEFFICIENTS			
	●	■	▲
C _c		5.18	4.99
C _u		37.28	58.90

Sieve	% Finer	Sieve	% Finer	Sieve	% Finer
● #200	14.54	■ 3/8"	100.0	▲ 3/8"	100.0
		■ #4	98.87	▲ #4	98.8
		■ #10	96.42	▲ #10	97.02
		■ #20	83.64	▲ #20	87.25
		■ #40	58.52	▲ #40	67.41
		■ #60	39.26	▲ #60	51.29
		■ #100	27.86	▲ #100	39.95
		■ #140	21.67	▲ #140	31.29
		■ #200	16.07	▲ #200	26.29

SOIL DESCRIPTION	
●	
■	
▲	
REMARKS	
●	
■	
▲	

PROJECT: S-13-296 BRO Blackwell Mill Stream

SITE: Chesterfield County, SC

Terracon
521 Clemson Rd
Columbia, SC

PROJECT NUMBER: 7323P100

CLIENT: NHTB

ASTM D422 / ASTM C136



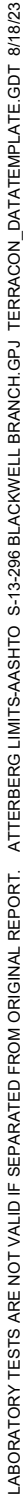
SILT OR CLAY

USCS

CLIENT: NHTB

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-13-296 BLACKWELL BRANCH.GPJ TERRACON DATATEMPLATE.GDT 8/18/23

ASTM D4318



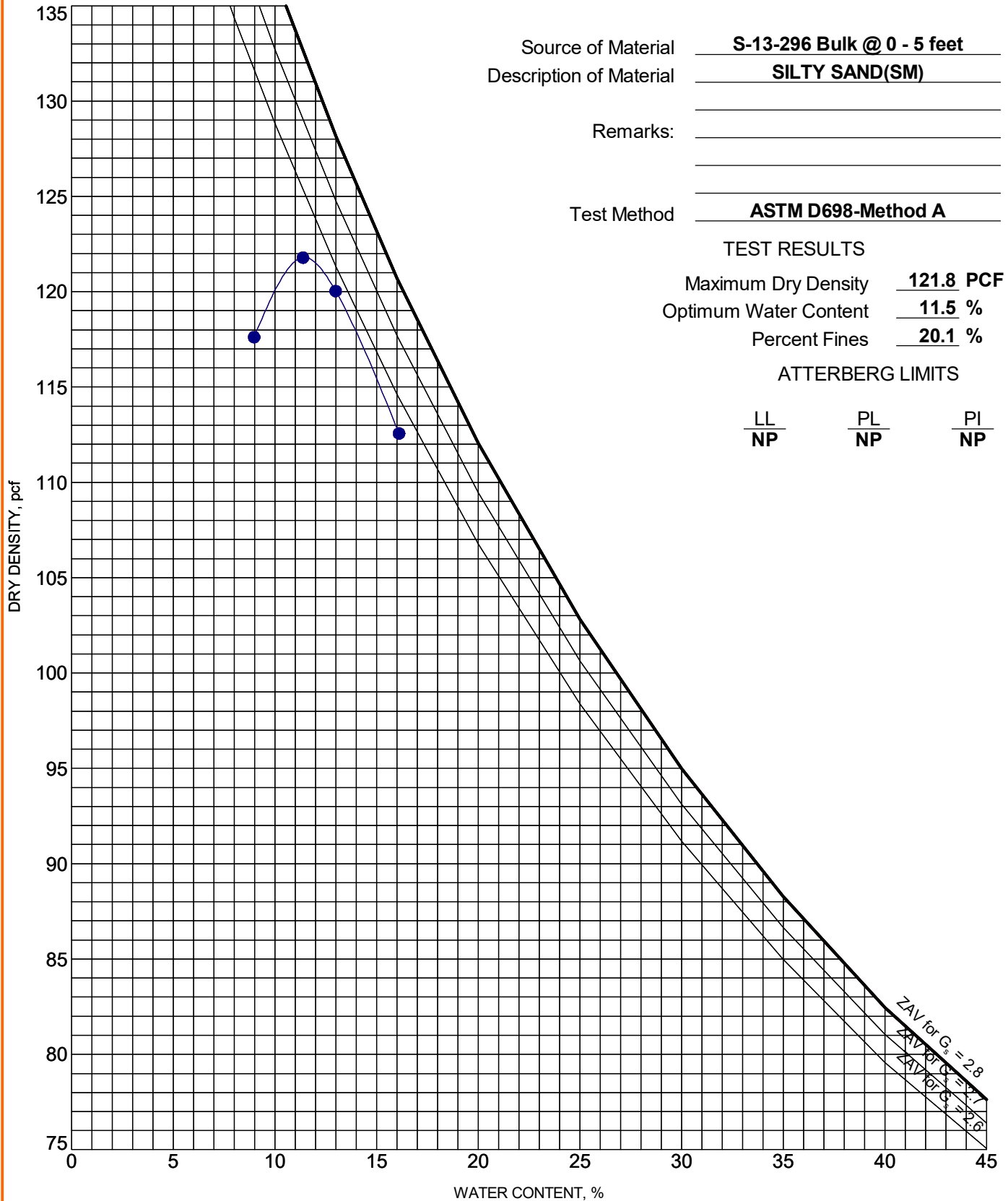
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LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT.

MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. COMPACTION - V2 S-13-296 BLACKWELL BRANCH.GPJ TERRACON_DATATEMPLATE.GDT 8/18/23



PROJECT: S-13-296 BRO Blackwell Mill Stream

SITE: Chesterfield County, SC

Terracon
521 Clemson Rd
Columbia, SC

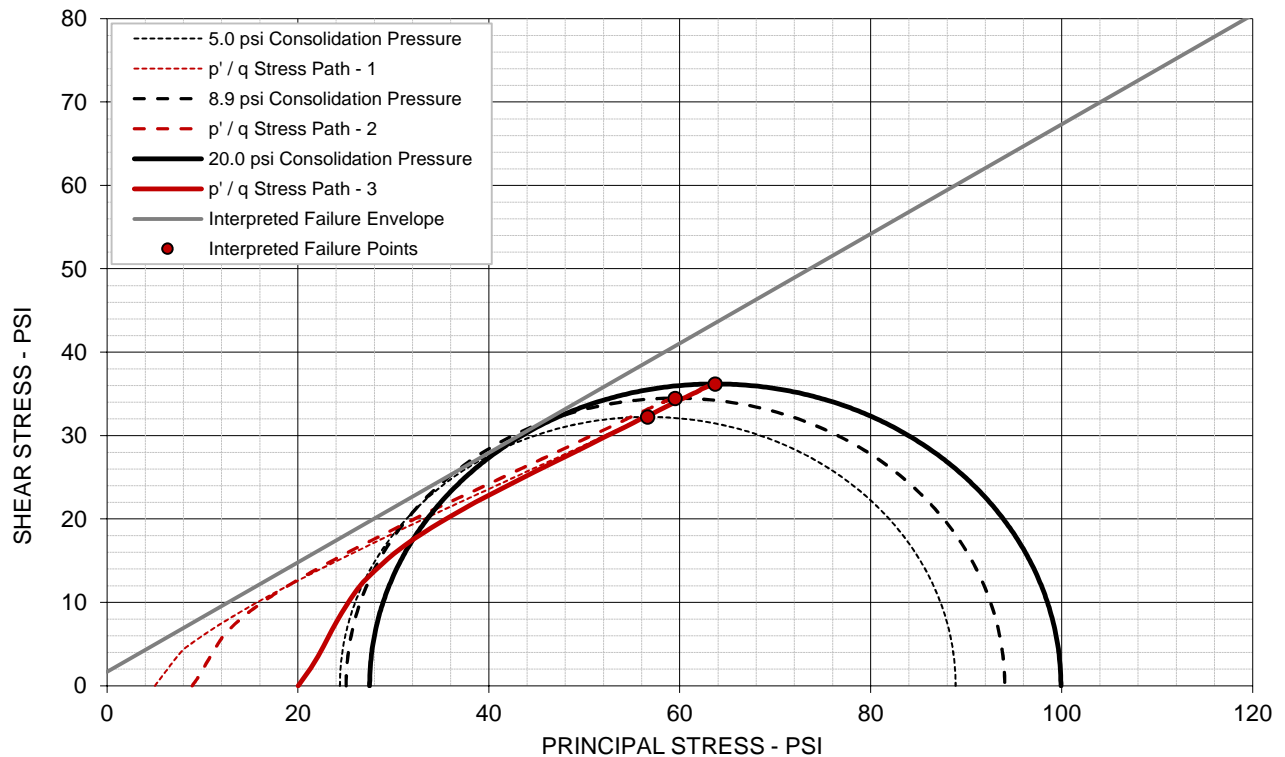
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CLIENT: NHTB

ICU TRIAXIAL COMPRESSION TEST

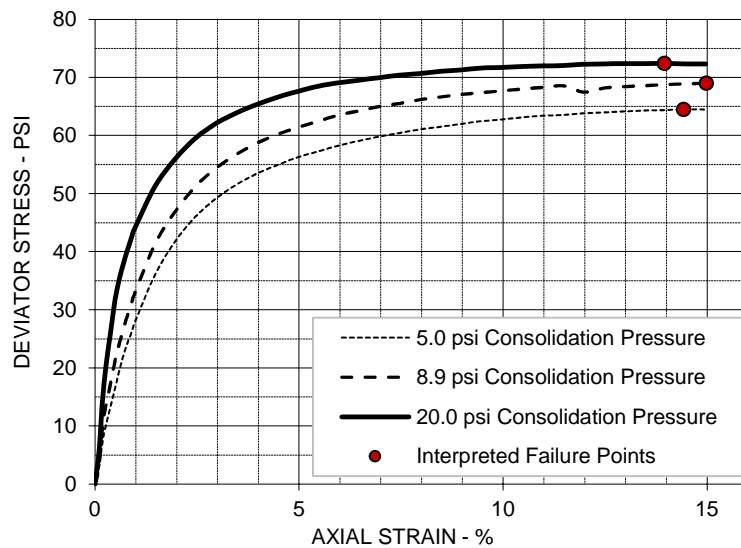
ASTM D4767 / AASHTO T297

Failure Criteria: Max Deviator Stress



EFFECTIVE STRESS PARAMETERS

$\phi' = 33.3$ deg $c' = 1.7$ psi



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	11.6	11.6	11.6
Dry Density - pcf	115.7	115.6	115.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	6.00	6.00

AT TEST

Final Moisture - %	15.0	14.9	14.7
Dry Density - pcf	115.7	115.9	116.2
Calculated Diameter (in.)	2.85	2.85	2.85
Height - inches	5.99	5.98	5.98
Effect. Consol. Stress - psi	5.0	8.9	20.0
Failure Stress - psi	64.49	68.99	72.40
Total Pore Pressure - psi	60.6	63.9	72.5
Strain Rate - %/min.	0.0497	0.0499	0.0498
Failure Strain - %	14.4	15.0	14.0
σ_1' Failure - psi	88.88	94.02	99.91
σ_3' Failure - psi	24.39	25.04	27.50

TEST DESCRIPTION

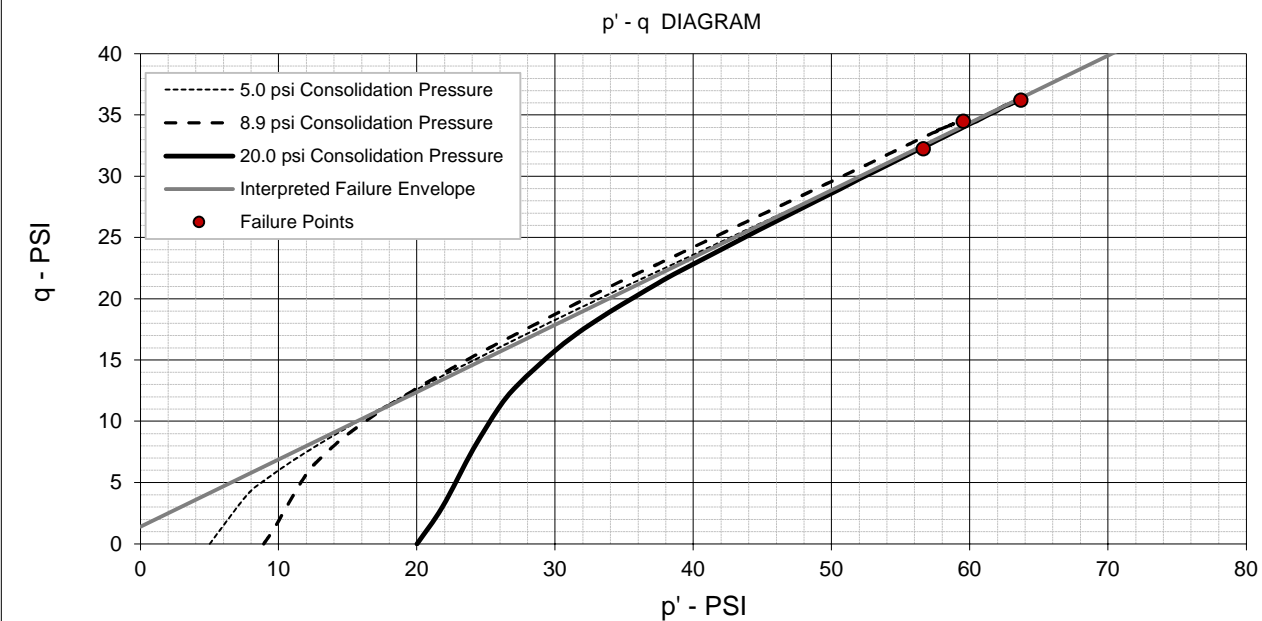
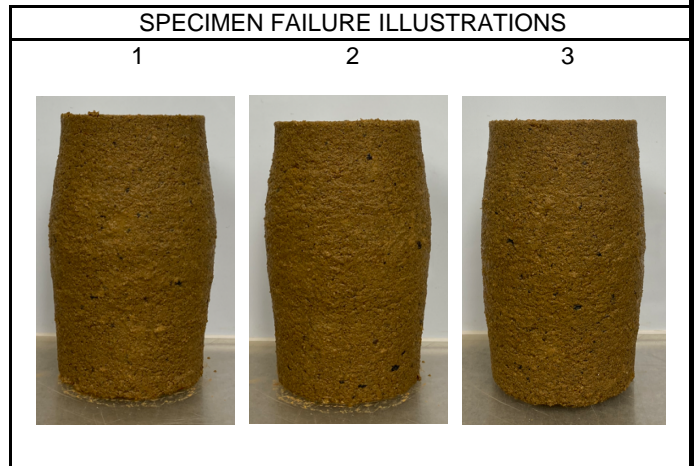
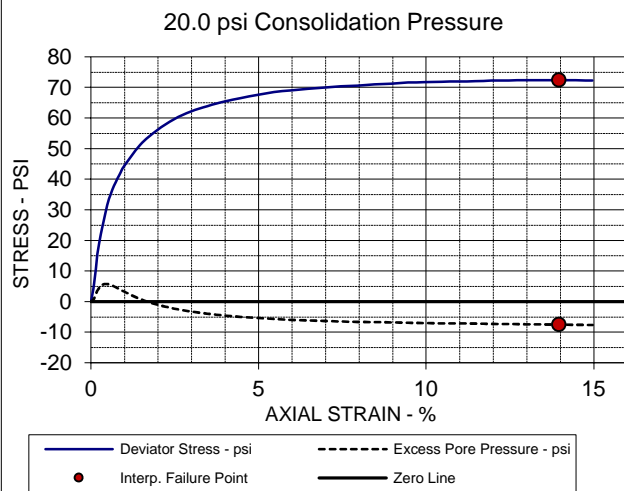
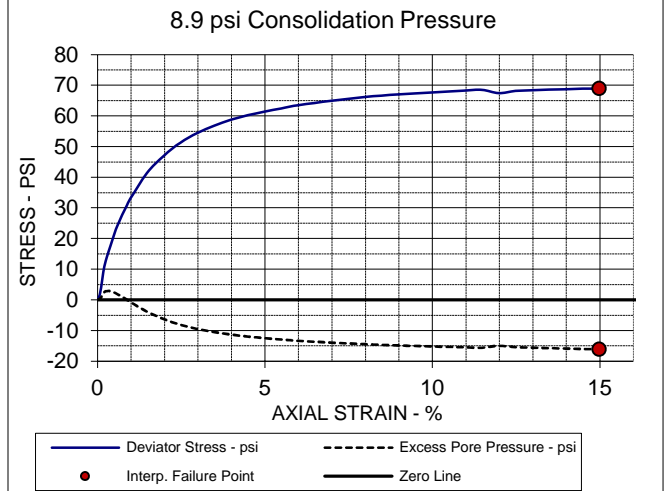
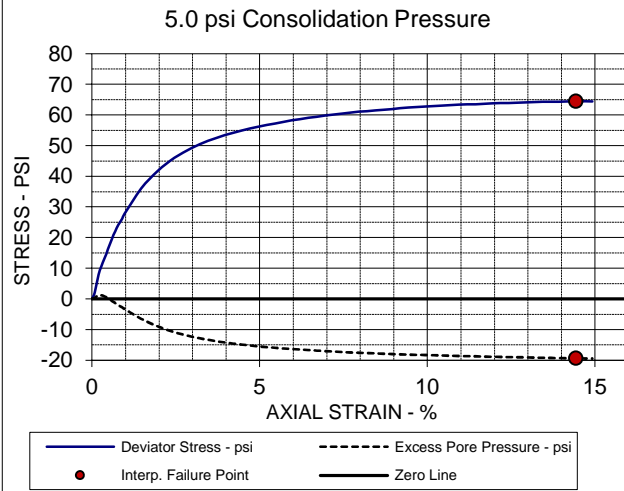
ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION
 SAMPLE TYPE: Remolded
 DESCRIPTION: Silty Sand (SM) / A-2-4 (0)
 SAMPLE ID: S-13-296-1 Bulk 0-5'
 SPECIFIC GRAVITY: 2.65
 LL: NP PL: NP PI: NP Percent -200: 20.1%
 Remarks: Remolded to 95% of the Standard Proctor


PROJECT INFORMATION

PROJECT: S-13-296 BRO Blackwell Mill Stream
 LOCATION: Chesterfield County, SC 7323P100
 PROJECT #: HNTB
 CLIENT: 07/05/23
 DATE:

521 Clemson Road
 Columbia, SC

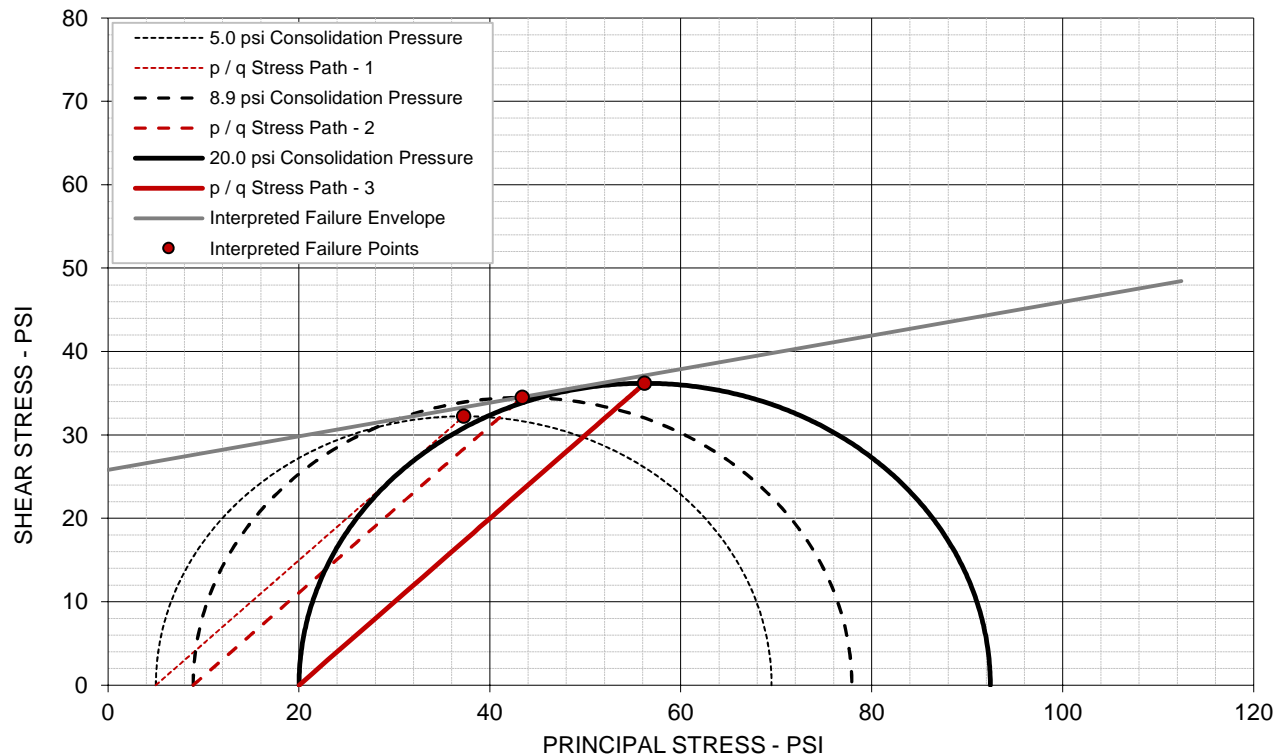




EFFECTIVE STRESS PARAMETERS		R ² = 0.98	α = 28.8 deg	a = 1.4 psi
PROJECT: S-13-296 BRO Blackwell Mill Stream			ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION TEST	
LOCATION: Chesterfield County, SC			CLIENT: HNTB	
SAMPLE ID: S-13-296-1 Bulk 0-5'			<div>521 Clemson Road Columbia, SC</div> <div></div>	
DESCRIPTION: Silty Sand (SM) / A-2-4 (0)				

ICU TRIAXIAL COMPRESSION TEST ASTM D4767 / AASHTO T297

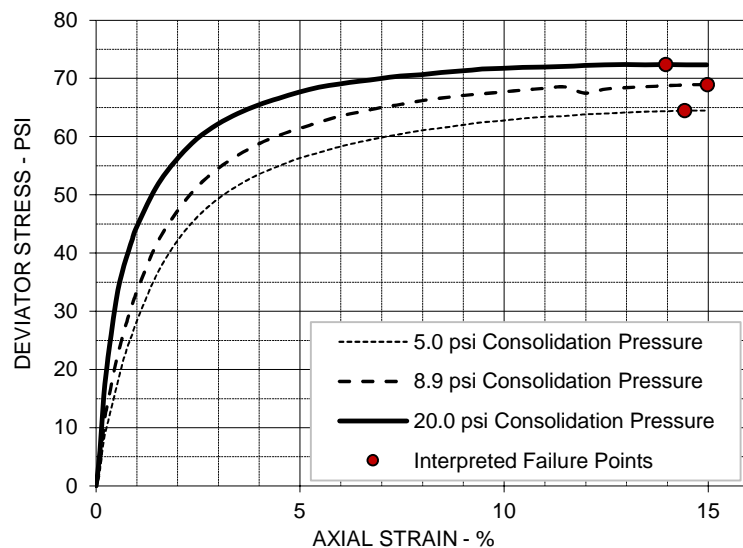
Failure Criteria: Max Deviator Stress



TOTAL STRESS PARAMETERS

$\phi = 11.4$ deg

$c = 25.8$ psi



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	11.6	11.6	11.6
Dry Density - pcf	115.7	115.6	115.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	6.00	6.00

AT TEST

Final Moisture - %	15.0	14.9	14.7
Dry Density - pcf	115.7	115.9	116.2
Calculated Diameter (in.)	2.85	2.85	2.85
Height - inches	5.99	5.98	5.98
Effect. Consol. Stress - psi	5.0	8.9	20.0
Failure Stress - psi	64.49	68.99	72.40
Total Pore Pressure - psi	60.6	63.9	72.5
Strain Rate - %/min.	0.0497	0.0499	0.0498
Failure Strain - %	14.4	15.0	14.0
σ_1 Failure - psi	69.50	77.90	92.41
σ_3 Failure - psi	5.01	8.92	20.01

TEST DESCRIPTION

ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION
 SAMPLE TYPE: Remolded
 DESCRIPTION: Silty Sand (SM) / A-2-4 (0)
 SAMPLE ID: S-13-296-1 Bulk 0-5'
 SPECIFIC GRAVITY: 2.65
 LL: NP PL: NP PI: NP Percent -200: 20.1%
 Remarks: Remolded to 95% of the Standard Proctor

PROJECT INFORMATION

PROJECT: S-13-296 BRO Blackwell Mill Stream
 LOCATION: Chesterfield County, SC 7323P100
 PROJECT #: HNTB
 CLIENT: 07/05/23
 DATE:

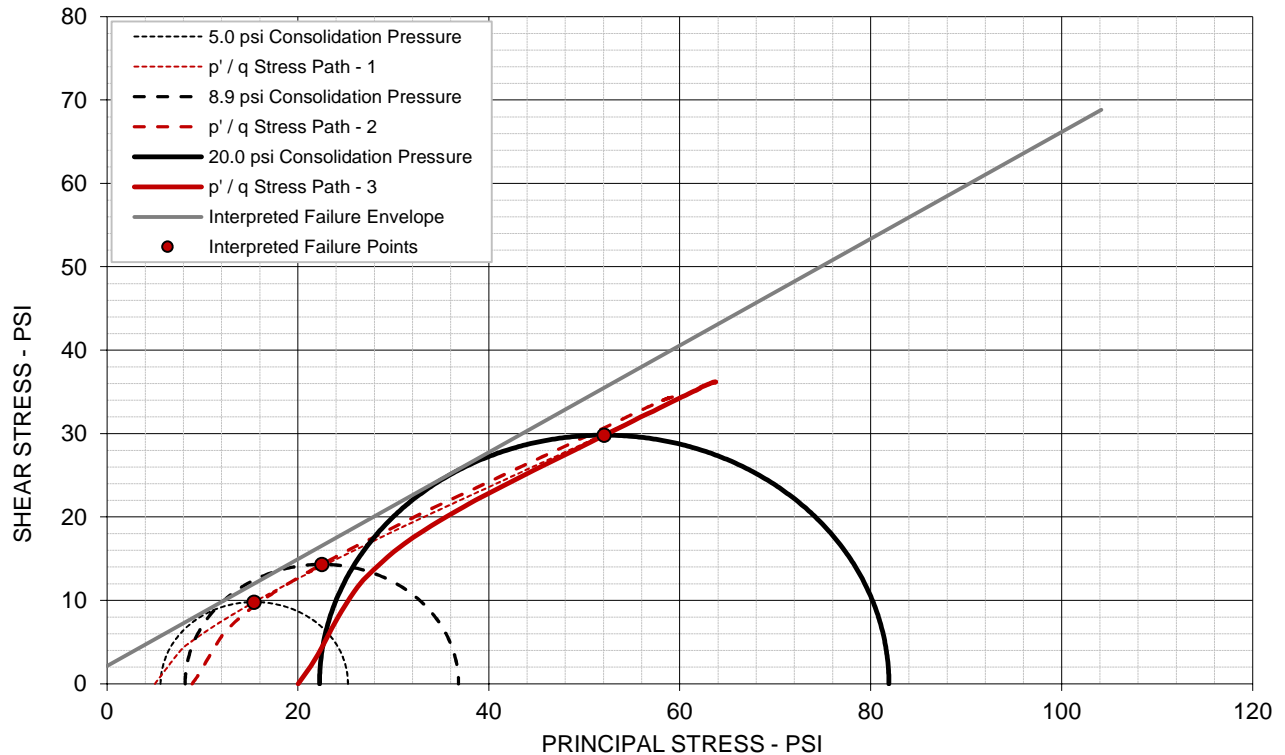
521 Clemson Road
 Columbia, SC



ICU TRIAXIAL COMPRESSION TEST

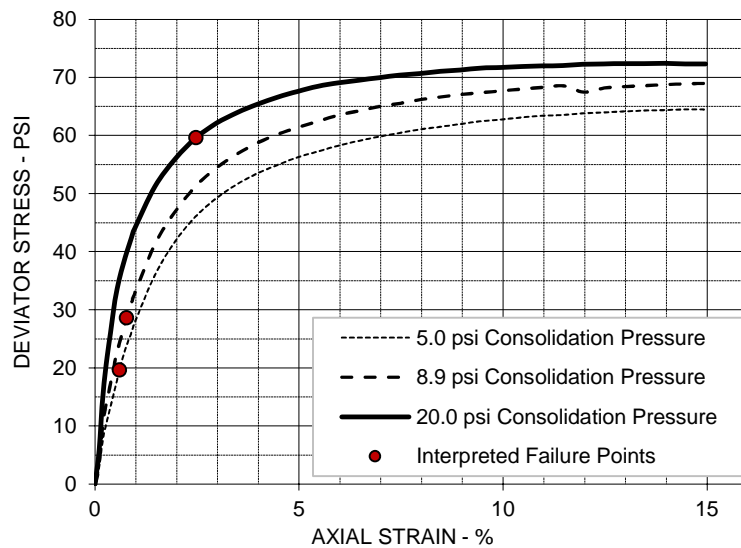
ASTM D4767 / AASHTO T297

Failure Criteria: Max Obliquity (s1': s3')



EFFECTIVE STRESS PARAMETERS

$\phi' = 32.6$ deg $c' = 2.1$ psi



SPECIMEN NO.	1	2	3
INITIAL			
Moisture Content - %	11.6	11.6	11.6
Dry Density - pcf	115.7	115.6	115.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	6.00	6.00
AT TEST			
Final Moisture - %	15.0	14.9	14.7
Dry Density - pcf	115.7	115.9	116.2
Calculated Diameter (in.)	2.85	2.85	2.85
Height - inches	5.99	5.98	5.98
Effect. Consol. Stress - psi	5.0	8.9	20.0
Failure Stress - psi	19.62	28.64	59.64
Total Pore Pressure - psi	79.4	80.7	77.8
Strain Rate - %/min.	0.0497	0.0499	0.0498
Failure Strain - %	0.6	0.8	2.5
σ_1' Failure - psi	25.23	36.82	81.89
σ_3' Failure - psi	5.61	8.18	22.26

TEST DESCRIPTION

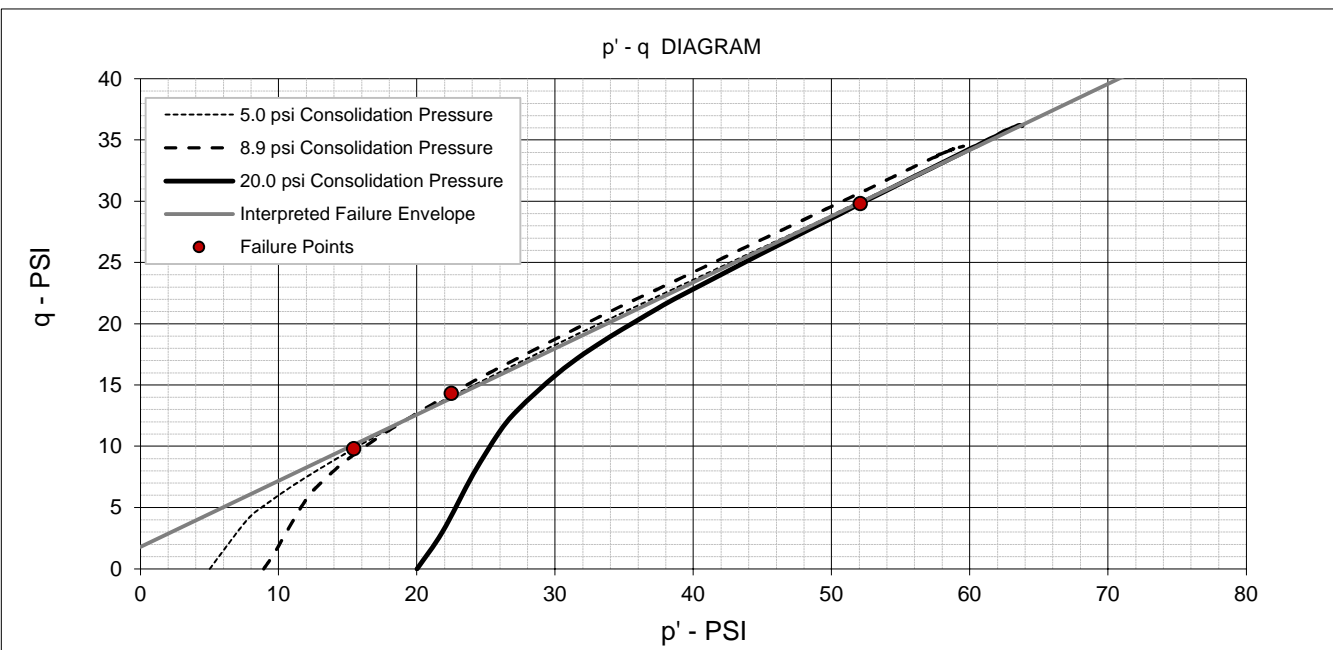
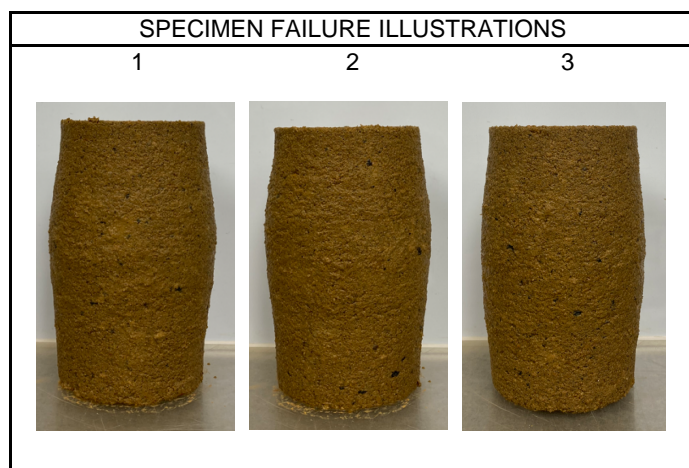
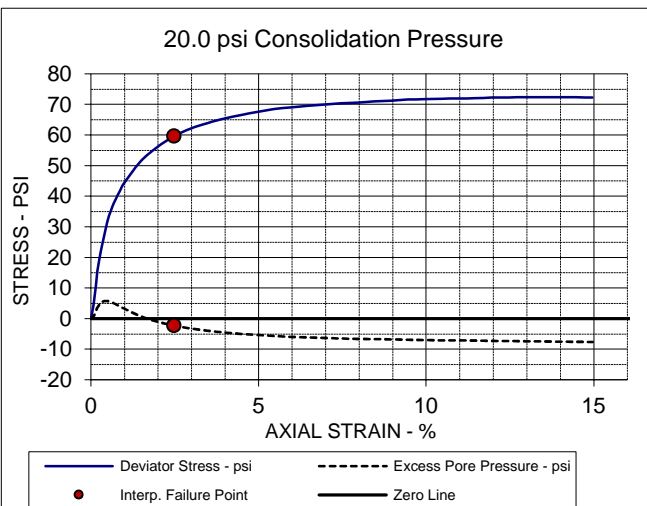
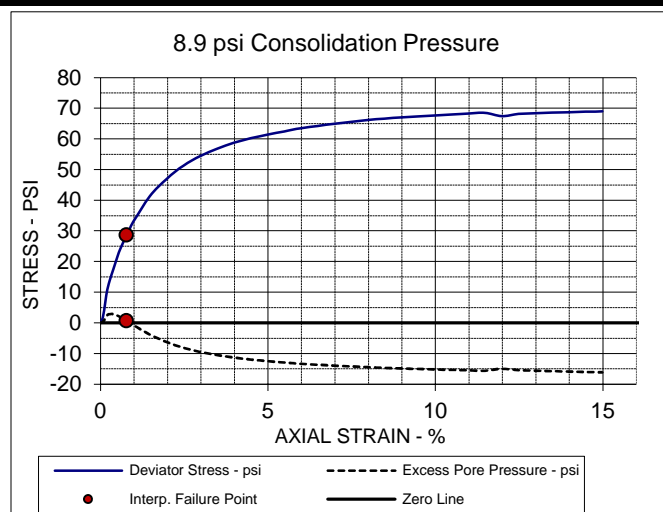
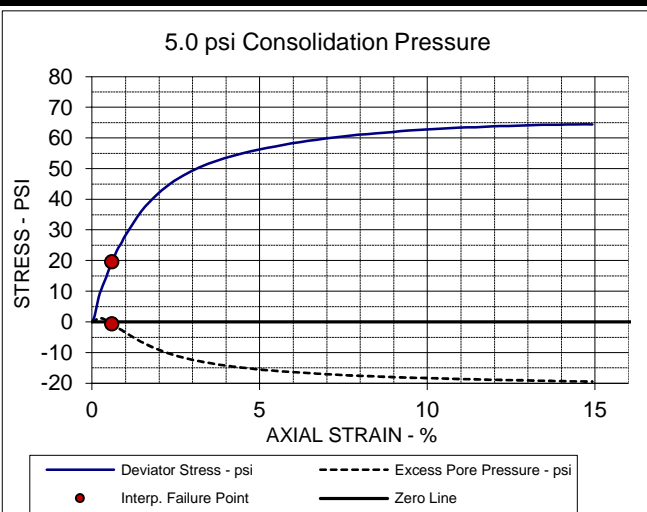
ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION
 SAMPLE TYPE: Remolded
 DESCRIPTION: Silty Sand (SM) / A-2-4 (0)
 SAMPLE ID: S-13-296-1 Bulk 0-5'
 SPECIFIC GRAVITY: 2.65
 LL: NP PL: NP PI: NP Percent -200: 20.1%
 Remarks: Remolded to 95% of the Standard Proctor


PROJECT INFORMATION

PROJECT: S-13-296 BRO Blackwell Mill Stream
 LOCATION: Chesterfield County, SC 7323P100
 PROJECT #: HNTB
 CLIENT: 07/05/23
 DATE:

521 Clemson Road
 Columbia, SC

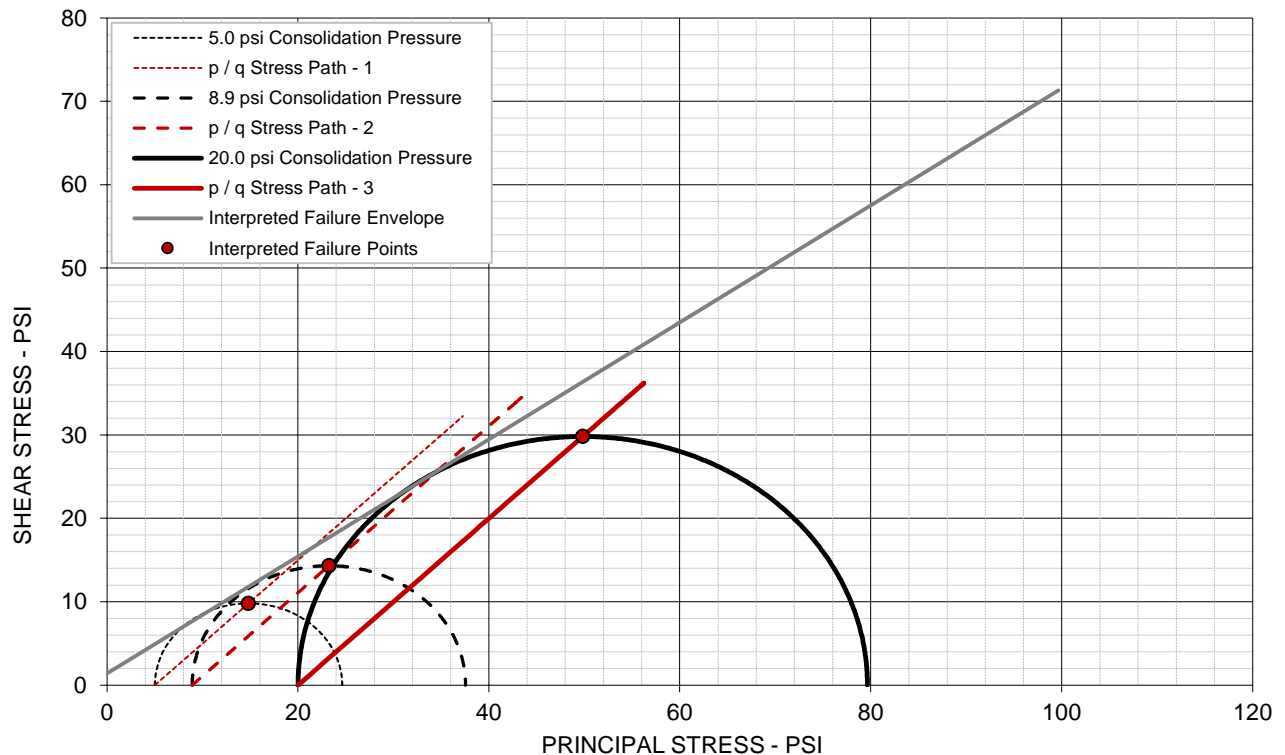




EFFECTIVE STRESS PARAMETERS		R ² = 1.00	α = 28.4 deg	a = 1.8 psi
PROJECT: S-13-296 BRO Blackwell Mill Stream			ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION TEST	
LOCATION: Chesterfield County, SC			CLIENT: HNTB	
SAMPLE ID: S-13-296-1 Bulk 0-5'			<div>521 Clemson Road Columbia, SC</div> <div></div>	
DESCRIPTION: Silty Sand (SM) / A-2-4 (0)				

ICU TRIAXIAL COMPRESSION TEST ASTM D4767 / AASHTO T297

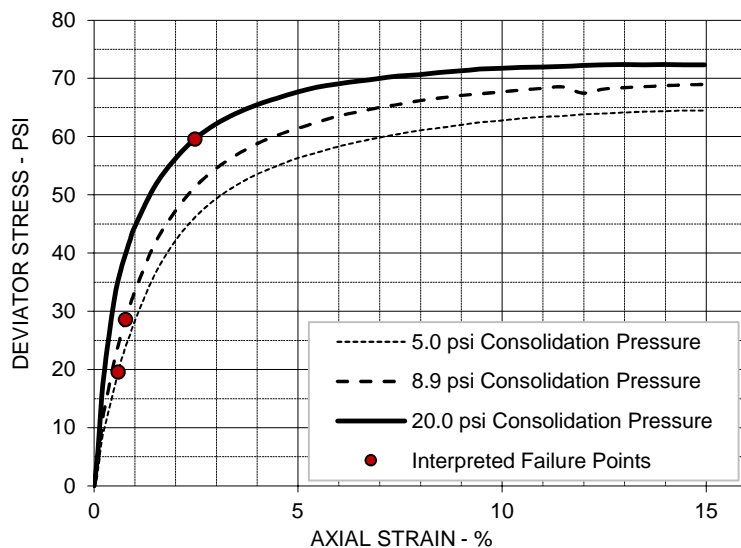
Failure Criteria: Max Obliquity (s1': s3')



TOTAL STRESS PARAMETERS

$\phi = 35.0$ deg

c = 1.4 psi



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	11.6	11.6	11.6
Dry Density - pcf	115.7	115.6	115.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	6.00	6.00

AT TEST

Final Moisture - %	15.0	14.9	14.7
Dry Density - pcf	115.7	115.9	116.2
Calculated Diameter (in.)	2.85	2.85	2.85
Height - inches	5.99	5.98	5.98
Effect. Consol. Stress - psi	5.0	8.9	20.0
Failure Stress - psi	19.62	28.64	59.64
Total Pore Pressure - psi	79.4	80.7	77.8
Strain Rate - %/min.	0.0497	0.0499	0.0498
Failure Strain - %	0.6	0.8	2.5
σ_1 Failure - psi	24.63	37.55	79.64
σ_3 Failure - psi	5.01	8.92	20.01

TEST DESCRIPTION

ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION

SAMPLE TYPE: Remolded

DESCRIPTION: Silty Sand (SM) / A-2-4 (0)

SAMPLE ID: S-13-296-1 Bulk 0-5'

SPECIFIC GRAVITY: 2.65

LL: NP PL: NP PI: NP Percent -200: 20.1%

Remarks: Remolded to 95% of the Standard Proctor

PROJECT INFORMATION

PROJECT: S-13-296 BRO Blackwell Mill Stream

LOCATION: Chesterfield County, SC 7323P100

PROJECT #: HNTB

CLIENT: 07/05/23

DATE:

521 Clemson Road
Columbia, SC



Client

HNTB North Carolina PC
Raleigh, NC

Project

S-13-296 BRO Blackwell Mill Stream
7323P100

Date Received: 6/27/2023

Results from Corrosion Testing

Sample Location	S-13-296-2
Sample Depth (ft.)	1'-11'

pH Analysis, ASTM G 51	6.31
------------------------	------

Water Soluble Sulfate (SO ₄), ASTM D516-07 (mg/kg)	59
---	----

Chlorides, APHA 4500-Cl ⁻ E, (mg/kg)	49
---	----

Resistivity (Saturated), ASTM G 57, (ohm-cm)	53000
--	-------

Analyzed By: Kyle Lemcke
Laboratory Manager

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

Appendix C

Supporting Documents

Rig Calibration Report (5 Pages)

Note: All exhibits are one page unless noted above.

SPT Automatic Hammer Energy Measurement Report

Drill Rig Model: CME 45C
Serial Number: 406484
Terracon Drill Rig Asset Number: DR#543
July 29, 2022



Prepared for:
Terracon Consultants, Inc.
Columbia Exploration Services

Prepared by:
Terracon Consultants, Inc.
Exploration Services Group

July 29, 2022

Terracon Consultants, Inc.
521 Clemson Rd,
Columbia, SC 29229

Attn: Mr. Phillip Morris
E: phillip.morrison@terracon.com

Re: SPT Automatic Hammer Energy Measurement Report
Terracon Drill Rig DR#543; CME 45C
Terracon Project Number: DUXX0500

Dear Mr. Phillip Morrison:

This report provides the Energy Transfer Ratio (ETR) for the SPT automatic hammer found on drill rig model CME 45C; Terracon Drill Rig Asset Number DR#543 (Serial Number: 406484).

Table 1: Hammer Measurement Summary

Drill Rig Model	Serial No.	Drill Rig Year	Drill Rig No.	Energy Transfer Ratio (ETR)	Hammer Efficiency Correction (Ce)
CME 45C	406484	2018	DR#543	93.5% ± 4.5%	1.56

If you have any questions concerning this summary, or if we may be of further service, please contact us.

Sincerely,
Terracon Consultants, Inc.

Smith,
James P.
James Smith
National Exploration Manager

Digitally signed by Smith, James P.
DN: cn=Smith, James P., o=Terracon
Consultants, Inc., email=j.smith@terracon.com
Reason: I have signed this document and
I agree with its contents.
Date: 2022.08.10 17:46:58 -0400

Jerry Salsgiver, P.E.
Assistant National Manager

Attachments:
Exhibit A: Measurement Information
Exhibit B: PDA SPT Analyzer Results

Terracon Consultants, Inc. 10841 S. Ridgeview Road Olathe, KS 66061
P (407) 446 2527 terracon.com

terracon.com

Terracon

Environmental Facilities Geotechnical Materials

Environmental Facilities Geotechnical Materials

Exhibit A Measurement Information

MEASUREMENT INFORMATION

ITEM	DESCRIPTION
Drill Rig Identification	Drill Rig Model: CME 45C Drill Rig Year: 2018 Terracon Drill Rig Asset No.: DR#543; Serial No. 406484
Drill Rig Owner	Terracon Consultants, Inc. - Columbia, SC
Drill Rig Operator	Aaron Bowen; Columbia Exploration
Testing Date	07/28/2022
Testing Location	Columbia, SC
Boring Identification	B-1
Hammer Type	140 pounds (automatic)
Boring Method	Rotary Wash
Drill Rods	AWJ 1 3/4" outside diameter 3/16" wall thickness
Testing Equipment	2-foot AWJ rod instrumented w/ 2 strain gauges and 2 accelerometers Model SPT Analyzer™ (PDA)
ASTM Methods Used	ASTM D1586, Standard Test Method for Standard Penetration Test and Split-Barrel Sampling of Soils ASTM D4633-16, Standard Method for Energy Measurement for Dynamic Penetrometers
Personnel	Jim Smith - National Exploration Manager - Terracon Consultants, Inc.

Exhibit B

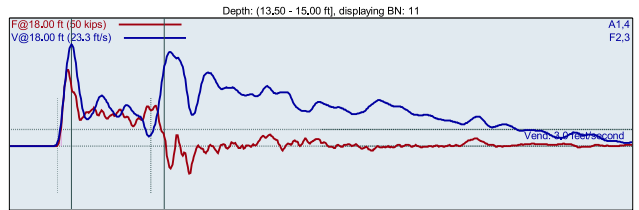
PDA SPT ANALYZER RESULTS

Pile Dynamics, Inc.
SPT Analyzer Results

Page 1 of 5
PDA-S Ver. 2018.24 - Printed: 7/29/2022

DU-543-406484
Jim Smith
AR: 1.20 in²
LE: 18.00 ft
WS: 16807.9 f/s

13.5-15.1
Test date: 7/28/2022
SP: 0.492 k/ft³
EM: 30000 ksi



F2 : [648AWJ1] 226,21 PDICAL (1) FF1
F3 : [648AWJ2] 225,58 PDICAL (1) FF1

A1 (PR): [K4484] 353,907 mv/6.4v/5000g (1) VF1
A4 (PR): [K4483] 410,187 mv/6.4v/5000g (1) VF1

FMX: Maximum Force
VMX: Maximum Velocity
BPM: Blows/Minute

EFV: Maximum Energy
ETR: Energy Transfer Ratio - Rated

BL#	BC /6"	FMX kips	VMX ft/s	BPM	EFV ft-lb	ETR %
1	2	28	18.4	1.9	279	79.8
2	2	30	18.4	51.6	276	78.9
3	2	31	18.9	51.6	305	87.1
4	2	30	18.3	51.6	309	88.2
5	9	30	18.4	51.7	319	91.2
6	9	29	19.0	51.5	318	91.0
7	9	29	18.5	51.3	305	87.0
8	9	29	18.4	51.2	313	89.3
9	9	30	18.0	51.0	317	90.5
10	9	29	18.6	51.1	317	90.5
11	9	30	18.5	51.0	302	86.1
12	9	30	19.3	51.0	312	88.1
13	9	30	18.8	51.0	321	91.6
Average		30	18.6	51.3	312	89.2
Std Dev		1	0.3	0.3	6	1.8
Maximum		31	19.3	51.7	321	91.6
Minimum		29	18.0	51.0	302	86.1

N-value: 11

Sample Interval Time: 14.02 seconds.

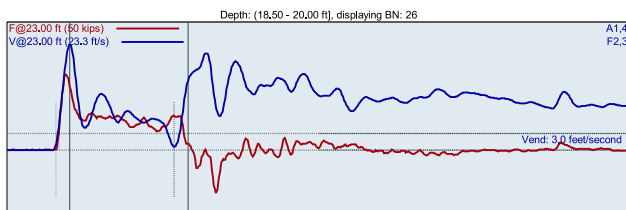
Responsive ■ Resourceful ■ Reliable

Pile Dynamics, Inc.
SPT Analyzer Results

Page 2 of 5
PDA-S Ver. 2018.24 - Printed: 7/29/2022

DU-543-406484
Jim Smith
AR: 1.20 in²
LE: 23.00 ft
WS: 16807.9 f/s

13.5-15.1
Test date: 7/28/2022
SP: 0.492 k/ft³
EM: 30000 ksi



F2 : [648AWJ1] 226,21 PDICAL (1) FF1
F3 : [648AWJ2] 225,58 PDICAL (1) FF1

A1 (PR): [K4484] 353,907 mv/6.4v/5000g (1) VF1
A4 (PR): [K4483] 410,187 mv/6.4v/5000g (1) VF1

BL#	BC /6"	FMX kips	VMX ft/s	BPM	EFV ft-lb	ETR %
14	5	29	18.1	1.9	322	91.9
15	5	29	19.0	55.9	320	91.3
16	5	30	18.7	55.3	315	90.3
17	5	30	19.2	55.4	334	95.6
18	5	29	19.5	55.7	317	90.5
19	5	29	19.0	55.8	316	90.3
20	5	29	19.4	55.1	320	91.3
21	5	29	18.8	55.9	320	91.5
22	5	30	19.3	55.2	324	92.5
23	5	29	19.2	55.5	320	91.6
24	5	29	19.0	55.6	314	89.7
25	5	30	18.7	55.4	330	94.2
26	5	29	19.2	55.0	310	88.5
27	5	29	18.5	55.2	301	86.0
28	5	30	18.4	54.6	311	88.6
Average		29	19.0	55.3	317	90.4
Std Dev		0	0.3	0.4	8	2.2
Maximum		30	19.4	55.9	330	94.2
Minimum		29	18.4	54.6	301	86.0

N-value: 10

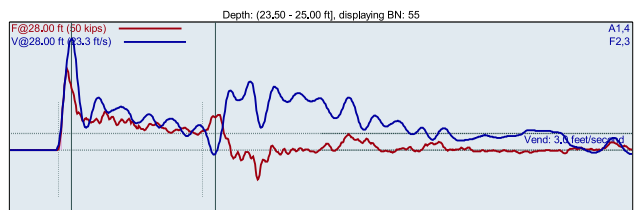
Sample Interval Time: 15.13 seconds.

Pile Dynamics, Inc.
SPT Analyzer Results

Page 3 of 5
PDA-S Ver. 2018.24 - Printed: 7/29/2022

DU-543-406484
Jim Smith
AR: 1.20 in²
LE: 28.00 ft
WS: 16807.9 f/s

13.5-15.1
Test date: 7/28/2022
SP: 0.492 k/ft³
EM: 30000 ksi



F2 : [648AWJ1] 226,21 PDICAL (1) FF1
F3 : [648AWJ2] 225,58 PDICAL (1) FF1

A1 (PR): [K4484] 353,907 mv/6.4v/5000g (1) VF1
A4 (PR): [K4483] 410,187 mv/6.4v/5000g (1) VF1

BL#	BC /6"	FMX kips	VMX ft/s	BPM	EFV ft-lb	ETR %
29	12	1	0.4	1.9	3	0.9
30	12	30	19.1	63.1	340	97.2
31	12	29	20.1	55.5	357	101.9
32	12	29	20.1	55.2	364	104.0
33	12	10	7.3	78.6	55	15.7
34	12	31	20.4	41.0	361	103.2
35	12	31	19.8	57.1	346	98.8
36	12	31	20.0	55.3	342	97.6
37	12	30	19.6	55.6	342	97.8
38	12	31	19.9	55.6	332	94.8
39	12	31	19.8	55.5	337	96.3
40	12	31	19.9	55.5	336	95.9
41	9	31	19.6	55.6	345	98.4
42	9	31	20.1	55.2	335	95.6
43	9	31	19.6	55.6	341	97.4
44	9	31	20.2	55.4	341	97.3
45	9	32	19.5	55.4	341	97.5
46	9	31	19.9	55.5	349	99.7
47	9	31	19.8	55.3	344	98.1
48	9	31	19.9	55.5	346	98.9
49	9	31	19.9	55.6	341	97.6
50	8	31	19.9	55.2	347	99.2
51	8	31	20.0	55.6	338	96.6
52	8	31	20.1	55.2	341	97.4
53	8	32	20.1	55.4	352	100.7
54	8	32	19.8	55.8	348	99.3
55	8	32	20.3	55.1	345	98.5
56	8	32	20.0	55.6	347	99.2
57	8	31	20.0	55.5	340	97.1

Average	31	19.9	55.4	344	98.1
Std Dev	0	0.2	0.2	4	1.2
Maximum	32	20.3	55.8	352	100.7
Minimum	31	19.5	55.1	335	95.6
N-value: 17					

Sample Interval Time: 30.24 seconds.

Summary of SPT Test Results

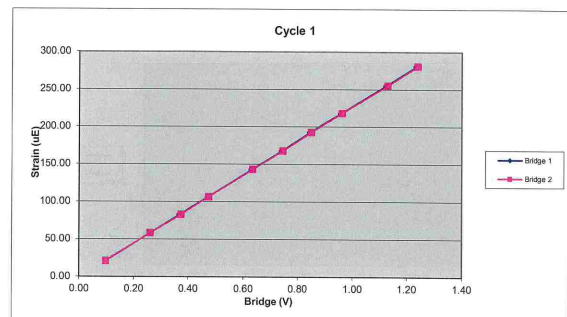
Project: DU-543-406484, Test Date: 7/28/2022						EFV: Maximum Energy		
FMX: Maximum Force						ETR: Energy Transfer Ratio - Rated		
VMX: Maximum Velocity								
BPM: Blows/Minute								
Blow	Blow	N	N50	Average	Average	Average	Average	Average
Length	Applied	Value	Value	FMX	VMX	BPM	EFV	ETR
ft	/ft			kips	ft/s	bpm	ft-lb	%
18.00	2-2-0	11	17	30	18.6	51.3	312	89.2
23.00	3-5-0	10	15	29	19.0	55.3	317	90.4
28.00	12-0-0	17	26	31	19.9	55.4	344	98.1
Overall Average Values:				30	19.3	54.2	327	93.3
Standard Deviation:				1	0.7	1.9	16	4.5
Overall Maximum Value:				32	20.3	55.9	352	100.7
Overall Minimum Value:				29	18.0	51.0	301	86.0



648AWJ		Cycle 1		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	799.99	21.12	0.10	0.10
3	2111.63	58.22	0.26	0.26
4	2997.39	82.70	0.37	0.37
5	3848.07	106.26	0.47	0.47
6	5131.83	143.07	0.63	0.63
7	6017.79	167.81	0.74	0.75
8	6872.07	192.74	0.85	0.85
9	7783.57	218.15	0.96	0.96
10	9136.93	255.02	1.12	1.13
11	10026.70	280.73	1.24	1.24

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8120.30	Force Calibration (lb/V)	8089.75
Offset	-4.24	Offset	-2.24
Correlation	0.999998	Correlation	0.999995
Strain Calibration (µE/V)	228.56	Strain Calibration (µE/V)	227.70
Offset	-1.57	Offset	-1.51
Correlation	0.999991	Correlation	0.999983

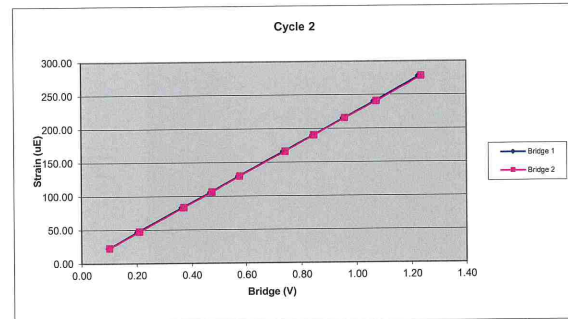
Force Strain Calibration	
EA (Kips)	35527.98
Offset	51.69
Correlation	0.999986



Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	805.54	22.23	0.10	0.10
3	1679.81	47.04	0.20	0.21
4	2989.11	83.03	0.37	0.37
5	3830.62	105.81	0.47	0.47
6	4658.00	129.50	0.57	0.58
7	5984.74	165.81	0.74	0.74
8	6848.87	189.76	0.84	0.84
9	7747.90	215.15	0.95	0.96
10	8674.21	240.08	1.07	1.07
11	9994.82	277.48	1.23	1.24

Bridge 1	Bridge 2
Force Calibration (lb/V)	8127.14
Offset	10.37
Correlation	0.999997
Strain Calibration (µE/V)	225.29
Offset	0.36
Correlation	0.999990

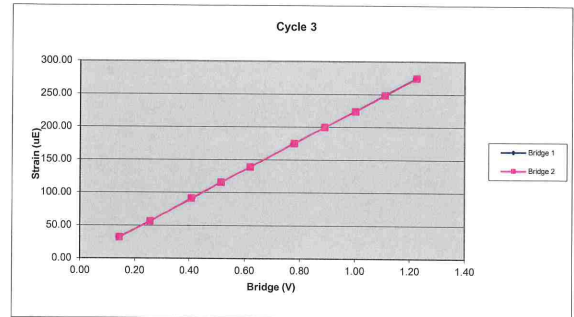
Force Strain Calibration	
EA (Kips)	36073.41
Offset	-2.66
Correlation	0.999993



Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1153.24	31.90	0.14	0.14
3	2056.55	56.28	0.26	0.26
4	3310.19	91.18	0.41	0.41
5	4155.51	115.51	0.51	0.51
6	5035.81	139.16	0.62	0.62
7	6303.78	175.10	0.78	0.78
8	7221.91	199.87	0.89	0.89
9	8120.94	223.92	1.00	1.00
10	9001.15	248.68	1.11	1.11
11	9931.66	274.33	1.22	1.23

Bridge 1	Bridge 2
Force Calibration (lb/V)	8132.32
Offset	-20.37
Correlation	0.999998
Strain Calibration (µE/V)	224.79
Offset	-0.57
Correlation	0.999984

Force Strain Calibration	
EA (Kips)	36175.62
Offset	0.42
Correlation	0.999984



Bridge Excitation (V) 5
Shunt Resistor (ohm) 60.4k

Calibration Factors	648AWJ	
Bridge 1 (µE/V)	226.21	225.58
EA Factor (Kips)	35925.67	1.20

Calibrated by: *Ant. J. J.*
Calibrated Date: 3/3/2022

Pile Dynamics Inc
30725 Aurora Rd
Solon, OH 44139

Traceable to N.I.S.T.

Accelerometer Calibration Certificate Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.
Calibration performed on 26Oct2021

Serial No: K4483 Temperature: 22.1 °C
Model: PR Humidity: 45%
Calibrated on: Channel 3 on 8G 5161 LE

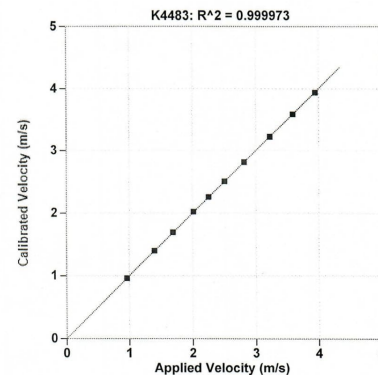
PDA CALIBRATION FACTOR
410.2 mv/5000g
(82.0 µV/g)
R²: 0.999973 [Chip programmed]

Ref Acc 1: 69096I Cal on: 27Jan2021
978 g's/volt
Ref Acc 2: 69132I Cal on: 09Feb2021
960 g's/volt

Operator: William Johnson

William Johnson
Signed

Reference accelerometer calibrations are traceable to the United States National Institute of Standards and Technology (NIST).



Reference Velocity m/s	S/N K4483 Velocity m/s
0.964	0.962
1.399	1.401
1.691	1.700
2.014	2.022
2.254	2.257
2.507	2.508
2.815	2.814
3.226	3.220
3.590	3.591
3.947	3.941

Maximum Acceleration: 874 g's

Accelerometer Calibration Certificate
Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.
Calibration performed on 25Jan2022

Serial No: K4484 Temperature: 19.3 °C
Model: PR Humidity: 30%
Calibrated on: Channel 4 on 8G 5161 LE

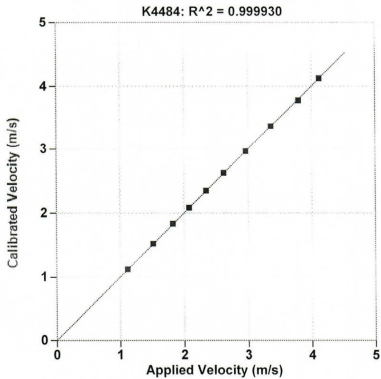
PDA CALIBRATION FACTOR
353.9 mv/5000g
(70.8 µv/g)
R^2: 0.999930 [Chip programmed]

Ref Acc 1: 69132! Cal on: 09Feb2021
960 g's/volt
Ref Acc 2: 69096! Cal on: 27Jan2021
978 g's/volt

Operator: William Johnson

Signed

Reference accelerometer calibrations are traceable to the United States National Institute of Standards and Technology (NIST).



Reference Velocity m/s	S/N K4484 Velocity m/s
1.117	1.124
1.518	1.523
1.823	1.835
2.078	2.080
2.344	2.349
2.616	2.624
2.963	2.962
3.360	3.357
3.794	3.778
4.121	4.122

Maximum Acceleration: 916 g's