

S-46-998 Bridge Replacement over Wildcat Creek

York County, SC

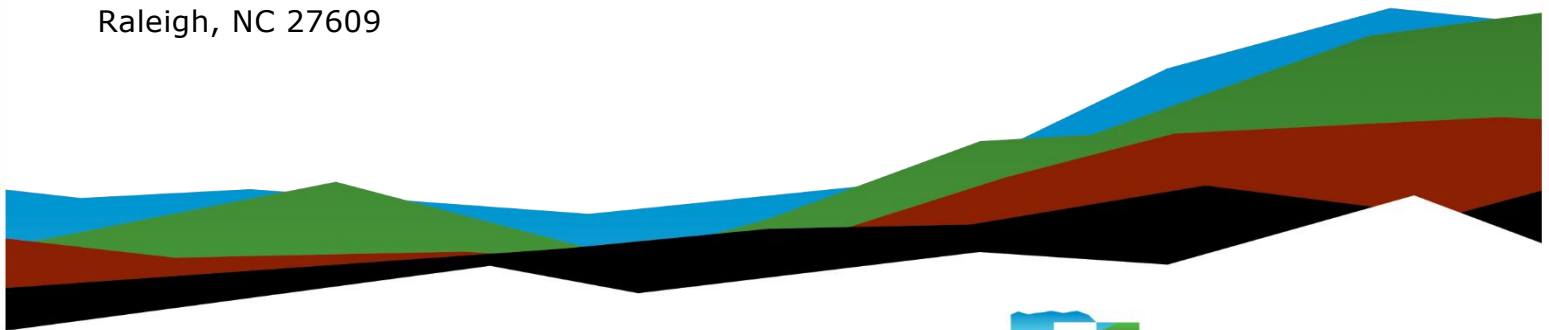
Geotechnical Baseline Report

August 21, 2023 (rev1) | SCDOT Project ID: P041172

Terracon Project No.: 7323P100

Prepared for:

HNTB Corporation
343 E. Six Forks Road, Suite 200
Raleigh, NC 27609



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521 Clemson Road
Columbia, SC 29229
P (803) 741-9000
[Terracon.com](https://www.terracon.com)

August 21, 2023 (rev1)

HNTB Corporation
343 E. Forks Road, Suite 200
Raleigh, NC 27609

Attn: Mr. Spencer Franklin, PE, Senior Vice President
P: 919-546-8997

Re: Geotechnical Baseline Report
S-46-998 Bridge Replacement over Wildcat Creek
York County, South Carolina
SCDOT Project ID.: P041172
Terracon Project No.: 7323P100

Dear Mr. Franklin:

Terracon Consultants Inc. (Terracon) has completed the exploration, testing and limited engineering analysis services for the above referenced project. The services were conducted in general accordance with our Task Order Number 001, dated May 25, 2023.

Introduction

HNTB Corporation (HNTB) has contracted Terracon to perform subsurface exploration, laboratory testing, and very preliminary engineering recommendations for the replacement of the S-46-998 bridge over Wildcat Creek in York County, South Carolina. The proposed bridge intends to replace the existing one. The results of subsurface exploration and laboratory testing has been separately presented in a Geotechnical Subsurface Data Report (GSDR). For convenience, those data are also provided here in this Geotechnical Baseline Report (GBR) along with a characterization of the subsurface conditions for the project. Very preliminary geotechnical recommendations are associated with the requested scope of study and are included in this GBR. This GBR was prepared in general accordance with the 2022 SCDOT Geotechnical Design Manual (GDM).

Project Description

The project site is located at the S-46-998 (E. Robertson Road) crossing over Wildcat Creek in York County, South Carolina. Site location and exploration plans are presented in Appendix A of this report. Based on the conceptual plans by HNTB dated 6/22/2023, the

replacement bridge will be constructed on essentially the same alignment as the current bridge. The existing bridge is a multi-span structure supported by deep foundations. The conceptual plans show that the replacement bridge will also be a multi-span structure supported by deep foundations.

Geotechnical Testing

The geotechnical exploration for this project was performed between June 10 and June 14, 2023. The results of our field work and our associated laboratory testing are included in Appendices A and B.

Field Exploration

Our field exploration consisted of the following:

- Two (2) Standard Penetration Test (SPT) Borings (S-46-998-1 and S-46-998-2)
- One (1) offset boring of S-46-998-1 for bulk sample collection
- One (1) Downhole Shear Wave Velocity Test (DHT-1) located in Boring S-46-998-1
- Two (2) Cone Penetration Test soundings (S-46-998-1C and S-46-998-2C)

The tests were performed at the approximate locations as approved by SCDOT. A description of our testing methods and graphical logs outlining the soil conditions at each test location are presented in Appendix A. The test locations were established in the field by Terracon and surveyed by Construction Support Services, LLC after completion.

Laboratory Testing

The following laboratory tests were performed on the soil samples collected at the site.

- Nine (9) Natural Moisture Content Tests
- Five (5) Atterberg Limits Tests
- Seven (7) Fines Content Tests
- Four (4) Grain Size Tests with Hydrometer
- One (1) Remolded, Consolidated-Undrained (CU) Triaxial Compression Test with Pore Pressure Readings
- One (1) Standard Proctor Test
- One (1) Corrosivity Suite Tests (pH, chloride content, sulfate content, and resistivity tests)

The general scope of the laboratory testing frequency was determined by the SCDOT. The laboratory testing assignment was performed by our engineers. The laboratory procedures and results of the laboratory tests are presented in Appendix B.

Subsurface Conditions

Regional Geology

The bridge site is located on route S-46-998, approximately 3.5 miles southwest of the city of Rock Hill in York County, South Carolina. The site is located in the Piedmont Physiographic Province of South Carolina. The Piedmont Unit is bounded by the Blue Ridge Unit to the west and the Upper Coastal Plain Subunit to the east. The bedrock underlying the site mainly consists of diorite rock of Cambrian or Neoproterozoic (485.4 MYA to 1 BYA). The bridge end bents and approach embankments contain fill overlying alluvial soils and residuum – very dense granular or hard cohesive Intermediate Geomaterials (IGM) – resting on bedrock.

Soil and Rock Stratification

The soils encountered at this site consist of fill in the upper 10 to 15 feet, followed by alluvial soils of silty sand to sandy silt to about 22 feet below the roadway surface. The alluvial soils are followed by the residual IGM materials classifying as sand and silt. Rock was encountered at a depth of 41 feet below ground surface at Boring S-46-998-1 and at a depth of 20 feet at Boring S-46-998-2. The RQD of the sampled bedrock ranged from 0% to 38%. The rock was found to be jointed, with wide discontinuity that are very closely spaced and have irregular joint shape. The joints contained no infilling and had a slightly rough surface. A summary of subsurface strata found during subsurface exploration is provided in the table below.

Geology	Approximate Elevation of Layer Bottom (ft, NAVD88)	USCS Soil Type	Measured Field N Value	Plasticity Index	Fines Content	REC / RQD
Asphalt and Stone Base	525.0	--	--	--	--	--
Fill	512 to 513	CL, CH, MH	0 to 12	16 to 26	54 to 74	--
Alluvium	503 to 506	ML, SM	0 to 8	--	12 to 83	--
Residuum (IGM)	484 to 506	SP-SM, ML	100+	--	7+	--
Rock	PMDE ¹	--	--	--	--	0 - 100% / 0 - 38%

1. PMDE = Present to Maximum Depth Explored

Seismic Conditions

According to SCDOT Seismic Design Specifications for Highway Bridges version 2.0, the proposed bridge will be an Operational Classification II (OC II). Per SCDOT GDM 2022, the proposed bridge shall be designed to meet the performance limits for an OCII bridge.

Acceleration Design Response Spectrum (ADRS)

The shear wave and compression wave velocity results, as measured at Boring S-46-998-1 using downhole seismic tests, were provided to SCDOT. SCDOT used these velocity measurements to develop Acceleration Design Response Spectrum (ADRS) curves by determining the seismic hazard and evaluating the local site effects on the response spectra.

SCDOT provided "3-Point Acceleration Design Response Spectrum" curves along with a table that included pseudo-spectral accelerations (PSA) for 5% critical damping and at selected frequencies, consistent with a Geologically Realistic (B-C Boundary) condition (shear wave velocity, $V_s = 2,500$ feet per second). PSA values were provided for the:

- Functional Evaluation Earthquake (FEE): 15% probability of exceedance in 75 years
- Safety Evaluation Earthquake (SEE): 3% probability of exceedance in 75 years

The table below provides the maximum considered earthquake peak ground acceleration (PGA), the short period acceleration (S_{DS}), and one-second period acceleration (S_{D1}) for the FEE and SEE earthquakes at the ground surface. A copy of the "3-Point Acceleration Design Response Spectrum" provided by SCDOT is included in Appendix A.

Seismic Design Parameter	FEE	SEE
PGA	0.06g	0.12g
S_{DS}	0.15g	0.28g
S_{D1}	0.01g	0.03g

Design and Construction Considerations

Foundations

Driven steel H-piles are anticipated to be feasible for the proposed bridge end bents. Assuming redundant piles, Table 9-3 GDM 2022 allows using a resistance factor of 0.5 for a single redundant pile with wave equation, and 0.65 for a single redundant pile with PDA and calibrated wave equation. Appropriate group effect should be considered as necessary per GDM Chapter 16. According to the conceptual bridge plans by HNTB dated 6/22/2023, new

fill is not anticipated at the end bent embankment. If plans change and new fill will be needed, foundations should be installed after the approach embankment construction to avoid any potential downdrag issues. The pile design must account for any downdrag loads subjected to the piles, should new fill be placed after installing foundation piles.

Drilled shafts are anticipated to be feasible for the proposed bridge interior bent. Assuming redundant drilled shafts, Table 9-4 GDM 2022 allows using a resistance factor of 0.60 (both side resistance and end bearing) for a single redundant drilled shaft in rock. It is assumed that the drilled shaft will be cased to the top of rock and the side resistance along the casing length will not be considered in estimating axial resistances. Appropriate group effects should be considered as necessary per GDM Chapter 16.

We observed variability in top of rock and thickness of IGM, as seen in **Soil and Rock Stratification**. Therefore, we expect variability in tip elevations at each bent location. Resistance of piles driven to practical refusal in IGM or rock will be limited by their structural resistance. Therefore, reinforced pile tips will likely be required to penetrate to IGM and rock. Pile drivability using the wave equation should be performed along with estimating stresses during driving and, in general, verifying the ability of the Contractor's selected hammer to drive the piles to the desired penetration while preventing overstressing.

Corrosion and Deterioration

Corrosion testing was performed on a composite sample obtained from split spoons in the upper 10 feet. Corrosion testing included pH, resistivity, chlorides, and sulfates content as summarized in Table below. Corrosion test results are included in Appendix B.

Corrosion Test	Results Bent 1 Boring S-46-998-2 Composite Sample from 0.0 to 10.0 feet	Indication of Corrosivity ¹
pH	5.9	Less than 5.5
Resistivity	2,300 ohm-cm	Less than 2,000 ohm-cm
Chloride	96 ppm	Greater than 500 ppm
Sulfate	18 ppm	Greater than 1,000 ppm

1. AASHTO LRFD bridge design specifications, Ninth Edition 2020, Section 10.7.5.

Based on the criteria for electro-chemical properties in the GDM Section 7.18, the electro-chemical classification of the project site is non-aggressive. Interpretation of these data should be communicated with the project's structural engineer.

Embankment Construction

Based on the conceptual plans by HNTB, cut excavation with benching is expected in front of the end bents. A bulk sample was obtained near End Bent 3 from the top 5 feet of existing embankment material. Per our scope the bulk sample was tested for soil classification and was also remolded to 95% of the Standard Proctor prior to being tested under CU Triaxial Compression. Test results are presented in Appendix B and are summarized in the Table below.

Sample No.	Station	Offset (ft)	Sample Depth (ft)	USCS Soil Type	Compaction		Shear Strength	
					Optimum Moisture (%)	Max Dry Density (pcf)	c', c (psf)	ϕ' , ϕ (°)
S-46-998-1	36+44.95	7.29 R	0-5	CL	17.0	105.9	130, 230	31, 12

1. Based on a maximum deviator stress failure criterion.

Closure

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

Terracon Consultants, Inc.

Phillip A. Morrison, P.E.
Senior Engineer
SC Registration No. 17275



Abdul Q. Fekrat, PhD, P.E.
Project Engineer
SC Registration No. 38531

Reviewed by Terracon's Authorized Project Reviewer: David J. Corley, P.E.

Appendix A

Field Exploration

- Exhibit A-1 – Site Location Map
- Exhibit A-2 – Exploration Plan
- Exhibit A-3 – Subsurface Profile
- Exhibit A-4 – Summary of Boring Data
- Exhibit A-5 – GeoScoping Form (2 Pages)
- Exhibit A-6 – Field Exploration Description (3 Pages)
- Exhibit A-7 – Soil/Rock Description Terms (2 Pages)
- Exhibit A-8 – Soil/Rock Symbols
- Exhibit A-9 – Boring Logs (3 Pages)
- Exhibit A-10 – CPT Sounding Logs (2 Pages)
- Exhibit A-11 – Geophysical Testing Results
- Exhibit A-12 – Grout Logs (4 Pages)
- Exhibit A-13 – Rock Core Photograph Logs (2 pages)

Note: All exhibits are one page unless noted above.

Site Location Map – Exhibit A-1

S-46-998 RBO Wildcat Creek Creek | York County, SC
Terracon Project No. 7323P100 | SCDOT Project ID: P041172

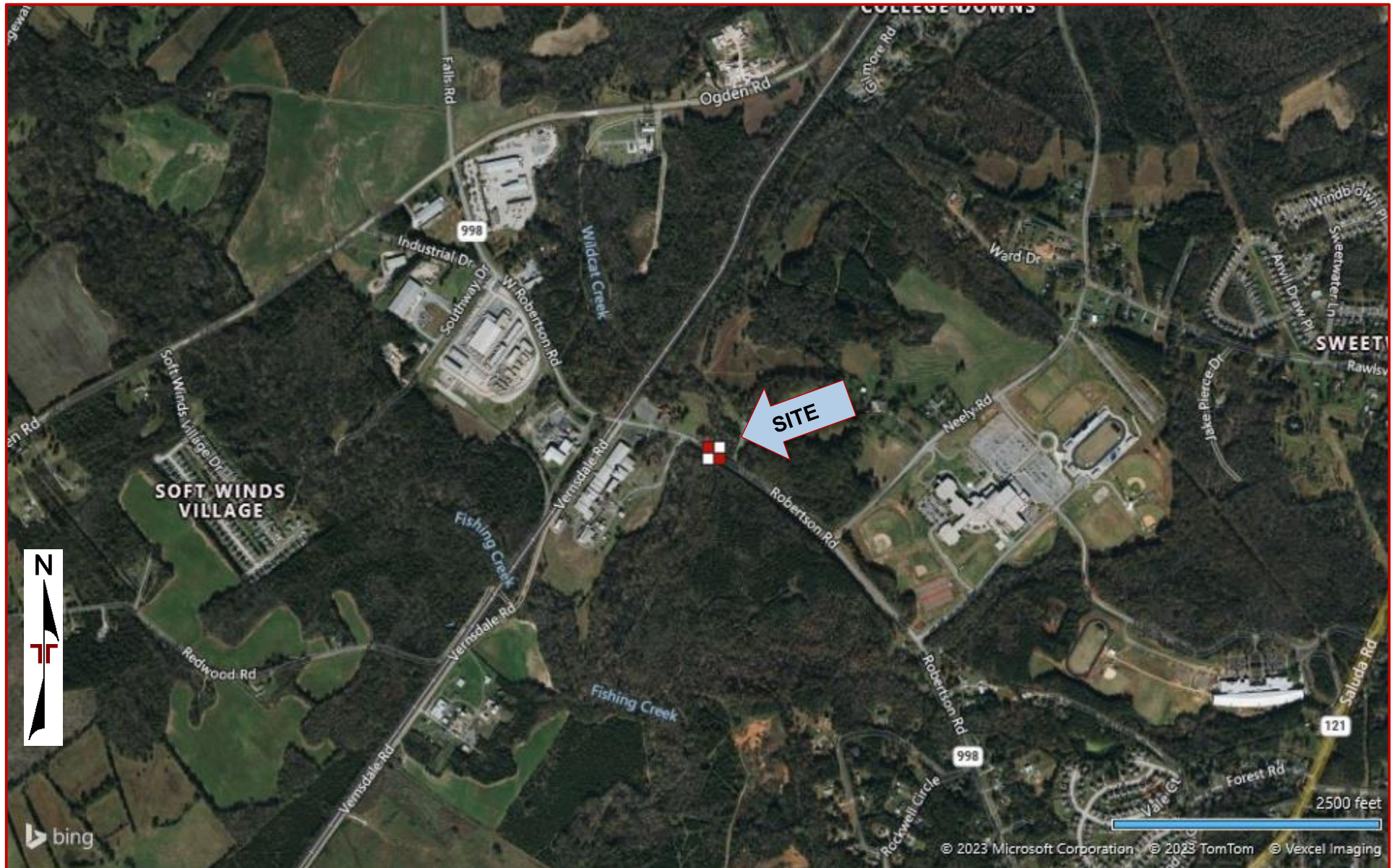


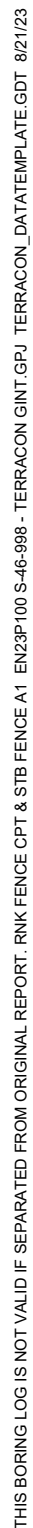
DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT
INTENDED FOR CONSTRUCTION PURPOSES

TOPOGRAPHIC MAP IMAGE COURTESY OF THE U.S. GEOLOGICAL SURVEY
QUADRANGLES INCLUDE: HORNSBORO, SC (1/1/1983) and MT CROGHAN, NC
(1/1/1983).



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT
INTENDED FOR CONSTRUCTION PURPOSES

AERIAL PHOTOGRAPHY PROVIDED BY
MICROSOFT BING MAPS



Summary of Boring Data – Exhibit A-4

S-46-998 BRO Wildcat Creek | York County, SC

Terracon Project No. 7323P100 | SCDOT Project ID: P041172



Summary of Boring Data

Boring No.	Ground Elevation ft.	Test Depth ft.	Northing	Easting	Latitude	Longitude	Station	Offset
S-46-998-1	525.6	57.0	1112052.369	1979179.091	34.889627	-81.069424	36+44.95	7.29 R
S-46-998-2	526.5	33.0	1111994.255	1979261.380	34.889468	-81.069149	35+44.22	5.98 R
S-46-998-1C	526.5	18.9	1112118.398	1979090.676	34.889809	-81.069719	37+54.52	13.13 R
S-46-998-2C	526.4	19.8	1111996.098	1979259.071	34.889473	-81.069157	35+47.17	6.19 R

Note: A bulk sample was collected near S-46-998-2.

GeoScoping Form

PROJECT INFORMATION	
Project ID: P041172	Date of Trip: 6/14/2023
County: York	Location: Rock Hill
Rd/Route: S-46-998	Local Name: E. Robertson Road
Attendees: A. Beaty	

EXISTING BRIDGE INFORMATION	
Bridge Length: 77 feet	Bridge Width: 22 Feet
Superstructure Type: Concrete framing and decking	Substructure Type: Timber Piles
Begin Bridge Sta.: 35+39	End Bridge Sta.: 36+59
Begin Bridge Embankment Sta. ¹ : 34+66	End Bridge Embankment Sta. ¹ : 37+42
Structure Number: 05643	Posted Weight Limit: N/A
Crossing: Wildcat Creek	Skew: 10 degrees
Latitude: 34.8895414° N	Longitude: 80.0692732° W
Existing Fill Height: 13 feet	Approximate Existing Slope Angle: 2H:1V

¹Begin and End Bridge Embankment 100 feet down station or up station from bridge, respectively

EXISTING ROADWAY EMBANKMENT INFORMATION	
Begin Project Sta.: 32+50	Begin Bridge Embankment Sta. ¹ : 34+66
Accessibility Issues: None	
Ground Cover: Asphalt Pavement with grassed shoulders	
Existing Fill Height: 13	Approximate Existing Slope Angle: 2H:1V
Local Development (undeveloped, developed residential, developed commercial, developed industrial, etc.): Developed Residential	
Topography (level, flat, rolling, steep, hillside, valley, swamp, gully, etc.): Rolling	
Traffic Control Necessary (Y/N):	
Yes	
Surface Soil: Clay	Muck (Y/N): No
Exposed Rock (Y/N): No	In Stream Bed (Y/N): No
	In Banks (Y/N): Yes
Wetlands On-Site (Y/N): Yes (Stream)	Wetlands Adjacent (Y/N): No
Depth FG to Water: 13 Feet	Water Depth: 2 Feet
Depth to Existing Ground: 15 feet	
Scour Condition at EB: None observed	Scour Condition at IB: None Observed
End Bridge Embankment Sta. ¹ : 37+42	End Project Sta.: 39+00
Accessibility Issues: None	
Ground Cover: Asphalt Pavement with grassed shoulders	
Existing Fill Height: 13 feet	Approximate Existing Slope Angle: 2H:1V
Local Development (undeveloped, developed residential, developed commercial, developed industrial, etc.): Developed Residential	
Topography (level, flat, rolling, steep, hillside, valley, swamp, gully, etc.): Rolling	
Traffic Control Necessary (Y/N): Yes	
Surface Soil: Clay	Muck (Y/N): No
Exposed Rock (Y/N): No	In Stream Bed (Y/N): No
	In Banks (Y/N): Yes
Wetlands On-Site (Y/N): Yes (Stream)	Wetlands Adjacent (Y/N): No
Depth FG to Water: 13 Feet	Water Depth: 2 Feet
Depth to Existing Ground: 15 Feet	
Scour Condition at EB: None	Scour Condition at IB: None Observed

GeoScoping Form

UTILITIES INFORMATION	
Attached:	
Above Ground/ Overhead:	Power linse along north side of the bridge
Underground:	

COMMENTS	

Instructions:

1. Attach boring location plan for bridge and roadway.
2. Attach all photographs taken, photographs to be labeled as to direction looking in and what is being depicted.
3. Fill out GeoScoping Form as completely as possible, using additional sheets as necessary to describe site conditions.
4. If representative of GEC on site during GeoScoping, include GEC representative's name and contact number in Attendees block.

Exhibit A-6

S-46-998 BRO Wildcat Creek | York County, SC

August 21, 2023 (rev1) | Terracon Project No. 7323P100 | SCDOT Project ID: P041172



Field Exploration Description

Overview

The testing locations were proposed to and approved by SCDOT and located in the field by Terracon using measurements from existing structures shown on the provided drawings. The borings were surveyed by Construction Support Services, LLC after testing and drilling was complete. The locations as shown in the Exploration Plan are shown to the scale indicated.

A field log of each test location was prepared by our engineer. The final boring logs included with this report represent the engineer's description of the encountered conditions modified as necessary based on laboratory test results of the individual samples.

Soil Test Borings (STB)

All boring and sampling operations were conducted in general accordance with the following procedures:

- SCDOT Geotechnical Design Manual 2022
- Preconstruction Design Memorandum (PCDM) 11 - Supplemental Design Criteria for Low Volume Bridge Replacement Projects
- ASTM D5783, "Standard Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geo-environmental Exploration"
- ASTM D6151, "Standard Practice for Using Hollow-Stem Augers for Geotechnical Exploration and Soil Sampling
- ASTM D1586 "Test Method for Penetration Test and Split-Barrel Sampling of Soils"
- ASTM D4220 "Standard Practices for Preserving and Transporting Soil"
- ASTM D2113 "Standard Practice for Rock Core Drilling and Sampling of Rock for Site Exploration"
- ASTM D5079 "Standard Practices for Preserving and Transporting Rock Core Samples"

Each soil test boring was advanced using rotary wash drilling techniques. The initial sampling program is summarized in the following table:

Test ID	Total Depth	Interval of Continuous Sampling
S-46-998-1	100 feet or 15 feet rock coring	0 to 10 feet
S-46-998-2	100 feet or 10 feet rock coring	0 to 40 feet
S-29-292-1 Bulk	5 feet	Bulk Sample
S-29-292-1C	60 feet or refusal	CPT - No sampling
S-29-292-2C	60 feet or refusal	CPT - No sampling

Exhibit A-6

S-46-998 BRO Wildcat Creek | York County, SC

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Soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-barrel sampler, also known as a standard split-spoon. The sampler is advanced into the soil a total of 18 to 24 inches by striking the drill rod using a 140-pound automatic hammer falling 30 inches. The number of blows required to advance the sampler for each of three to four, 6-inch increments is recorded. The sum of the number of blows for the second and third increments is called the "Standard Penetration Value", or N-value (N_{meas} , blows per foot). The N-value, when properly evaluated, is an index to the soil strength.

Soil classification provides a general guide to the engineering properties of various soil types and enables the engineer to apply his experience to current situations. In our exploration, samples obtained during drilling operations are examined and visually classified by a geotechnical engineer using the procedures outlined in ASTM D2487 - Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System). Laboratory testing was also performed on select split-spoon samples to evaluate index properties for further classification. The soils are described according to color, texture, and relative density or consistency (based on standard penetration resistance). The designations shown on the logs are described in the 2022 SCDOT Geotechnical Design Manual, Chapter 6.

The borings were advanced either to the planned drilling depth at which they were terminated, or to refusal of the drilling equipment. Select borings were continued below this depth using diamond bit rock coring techniques. NQ2 sized cores were recovered from the borehole. The rock recovery ratios (REC, percentage of the total core run), Rock Quality Designation (RQD, percentage of the total core run of pieces greater than 4 inches) were recorded along with a description of the rock. An explanation of the rock descriptions shown on the logs is provided in the SCDOT GDM Chapter 6. Photos of the recovered rock core specimens are provided in the Rock Core Photograph Log.

As practical, groundwater readings were collected from each of the soil test borings after 24 hours. These water levels are indicated on the boring logs. The borings were advanced using mud rotary drilling techniques. As the drilling method introduces water into the borehole, time-of-drilling water levels may not be reliable.

At the conclusion of the work, the boreholes and sounding holes were backfilled with the drill cuttings and clean sand. The upper 20 feet of those in the embankments were grouted with a cement bentonite grout and capped with cold-patch asphalt.

Cone Penetration Test (CPT) Soundings

Cone Penetration Test soundings were conducted in accordance with ASTM D5778 *Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils*.

Downhole Shear Wave Velocity Test (DHT)

One downhole seismic test was performed in a cased borehole drilled for this project. After the test boring was completed, the boring was filled with a fluid water/cement/bentonite grout and then a threaded PVC pipe casing (capped at the bottom end) was inserted into the borehole, providing a uniform bond between the soil and pipe exterior.

The downhole seismic test consisted of placing two downhole triaxial geophones at selected depth intervals in the borehole casing. The geophone was connected to a recording device (Seismic Source Daq Link 5 Seismograph) at the surface and clamped to the side of the casing at the selected test depth. The geophones are equipped with a spring-arm that is released at the bottom of the boring. The spring expands and forces the geophone against the casing wall. The interval between each geophone and each test depth was 3 feet for the entire depth of the cased borehole. An instrumented hammer was then used to strike a steel plate with cleats at the bottom (often called a shear wave golf shoe) that penetrated the ground and prevented sliding when struck. The steel plate was oriented to generate horizontal shear waves (SH) at the surface. An additional plate was also struck to better produce compression waves. The horizontal distance was measured and the plate was set exactly 10 feet from the borehole. The recorder was set to record the arrival times of the shear waves at the geophone locations. At least 15 blows (5 in each direction on the golf shoe, and 5 on the steel plate) were struck for each test depth to electronically stack and polarize the observed data, and to increase the signal-to-noise ratio. The data was stored on computer disks for processing and computation. The geophone was raised to the next depth interval and the process was repeated.

Shear Wave Velocity Test Results shows the downhole shear wave velocity and compressive wave velocity test results. The data was evaluated using the Fixed Interval method. S-wave arrival times using the Interval method were picked based on the onset of the signal (first break) as observed in the software package TomTime by GeoTom.

SOIL DESCRIPTION TERMS

Relative Density/Consistency Terms

<u>Relative Density</u> ¹			<u>Consistency</u> ²		
Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Unconfined Compression Strength (q _u) (tsf)	SPT Blow Count
Very Loose	0 to 15%	4 and less	Very Soft	0.25 and less	2 and less
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	51 and more	Very Stiff	2.01 to 4.00	16 to 30
			Hard	4.01 and more	31 and more

Moisture Condition

<u>Descriptive Term</u>	<u>Criteria</u>
Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually in coarse-grained soils below the water table

Color

Describe the sample color while sample is still moist.

Angularity¹

<u>Descriptive Term</u>	<u>Criteria</u>
Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces.
Subangular	Particles are similar to angular description but have rounded edges.
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges.
Rounded	Particles have smoothly curved sides and no edges.

HCl Reaction³

<u>Descriptive Term</u>	<u>Criteria</u>
None Reactive	No visible reaction
Weakly Reactive	Some reaction, with bubbles forming slowly
Strongly Reactive	Violent reaction, with bubbles forming immediately

Cementation³

<u>Descriptive Term</u>	<u>Criteria</u>
Weakly Cemented	Crumbles or breaks with handling or little finger pressure
Cemented	Crumbles or breaks with considerable finger pressure
Strongly Cemented	Will not crumble or break with finger pressure

Particle-Size Range¹

<u>Gravel</u>	Diameter, mm	Sieve Size	<u>Sand</u>	Diameter, mm	Sieve Size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	¾ inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4.00 to 4.76	#10 to #4

Primary Soil Type^{1, 2}

The primary soil type will be shown in all capital letters.

USCS Soil Designation

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

AASHTO Soil Designation

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

¹Applies to coarse-grained soils (major portion retained on No. 200 sieve)

²Applies to fine-grained soils (major portion passing No. 200 sieve)

³Use as required

DESCRIPTION OF ROCK PROPERTIES

WEATHERING

Fresh	Rock fresh, crystals bright, few joints may show slight staining. Rock rings under hammer if crystalline.
Very slight	Rock generally fresh, joints stained, some joints may show thin clay coatings, crystals in broken face show bright. Rock rings under hammer if crystalline.
Slight	Rock generally fresh, joints stained, and discoloration extends into rock up to 1 in. Joints may contain clay. In granitoid rocks some occasional feldspar crystals are dull and discolored. Crystalline rocks ring under hammer.
Moderate	Significant portions of rock show discoloration and weathering effects. In granitoid rocks, most feldspars are dull and discolored; some show clayey. Rock has dull sound under hammer and shows significant loss of strength as compared with fresh rock.
Moderately Severe	All rock except quartz discolored or stained. In granitoid rocks, all feldspars dull and discolored and majority show kaolinization. Rock shows severe loss of strength and can be excavated with geologist's pick.
Severe	All rock except quartz discolored or stained. Rock "fabric" clear and evident, but reduced in strength to strong soil. In granitoid rocks, all feldspars kaolinized to some extent. Some fragments of strong rock usually left.
Very severe	All rock except quartz discolored or stained. Rock "fabric" discernible, but mass effectively reduced to "soil" with only fragments of strong rock remaining.
Complete	Rock reduced to "soil". Rock "fabric" not discernible or discernible only in small, scattered locations. Quartz may be present as dikes or stringers.

HARDNESS (for engineering description of rock – not to be confused with Moh's scale for minerals)

Very hard	Cannot be scratched with knife or sharp pick. Breaking of hand specimens requires several hard blows of geologist's pick.
Hard	Can be scratched with knife or pick only with difficulty. Hard blow of hammer required to detach hand specimen.
Moderately hard	Can be scratched with knife or pick. Gouges or grooves to ¼ in. deep can be excavated by hard blow of point of a geologist's pick. Hand specimens can be detached by moderate blow.
Medium	Can be grooved or gouged 1/16 in. deep by firm pressure on knife or pick point. Can be excavated in small chips to pieces about 1-in. maximum size by hard blows of the point of a geologist's pick.
Soft	Can be gouged or grooved readily with knife or pick point. Can be excavated in chips to pieces several inches in size by moderate blows of a pick point. Small thin pieces can be broken by finger pressure.
Very soft	Can be carved with knife. Can be excavated readily with point of pick. Pieces 1-in. or more in thickness can be broken with finger pressure. Can be scratched readily by fingernail.

Joint, Bedding, and Foliation Spacing in Rock^a

Spacing	Joints	Bedding/Foliation
Less than 2 in.	Very close	Very thin
2 in. – 1 ft.	Close	Thin
1 ft. – 3 ft.	Moderately close	Medium
3 ft. – 10 ft.	Wide	Thick
More than 10 ft.	Very wide	Very thick

^aSpacing refers to the distance normal to the planes, of the described feature, which are parallel to each other or nearly so.

Rock Quality Designation (RQD)^a

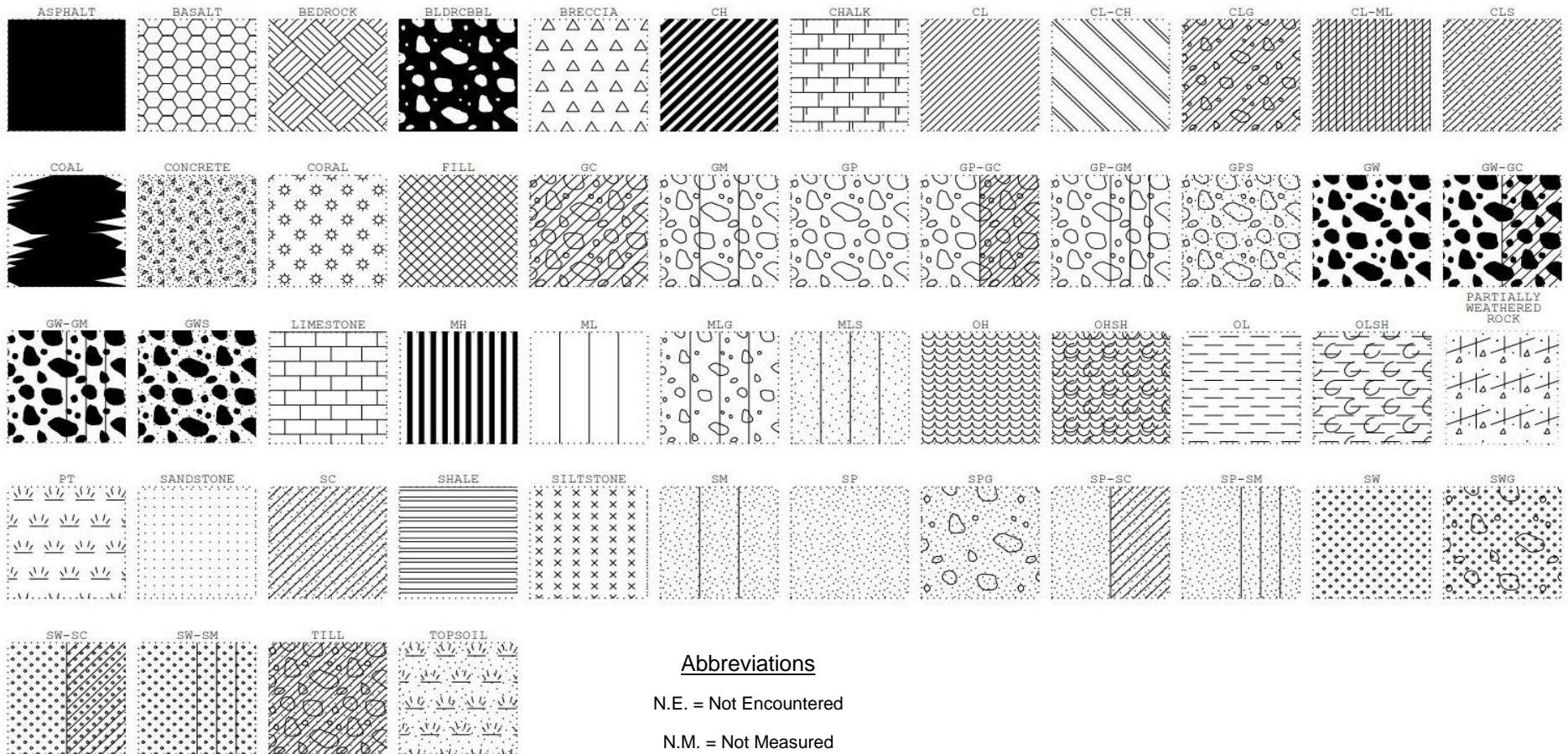
RQD, as a percentage	Diagnostic Description
Exceeding 90	Excellent
90 – 75	Good
75 – 50	Fair
50 – 25	Poor
Less than 25	Very poor

^aRQD (given as a percentage) = length of core in pieces 4 in. and longer/length of run.

Joint Openness Descriptors

Openness	Descriptor
No Visible Separation	Tight
Less than 1/32 in.	Slightly open
1/32 to 3/8 in.	Moderately open
1/8 to 3/8 in.	Open
3/8 in. to 0.1 ft.	Moderately wide
Greater than 0.1 ft.	Wide

References: American Society of Civil Engineers. Manuals and Reports on Engineering Practice - No. 56. Subsurface Investigation for Design and Construction of Foundations of Buildings. New York: American Society of Civil Engineers, 1976. U.S. Department of the Interior, Bureau of Reclamation, Engineering Geology Field Manual.



Project Manager:	PAM
Drawn by:	KJZ
Checked by:	PAM
Approved by:	DJC

Project No.	7323P100
Scale:	N.T.S.
File Name:	Soil – Rock – Log
Date:	Jul 2023



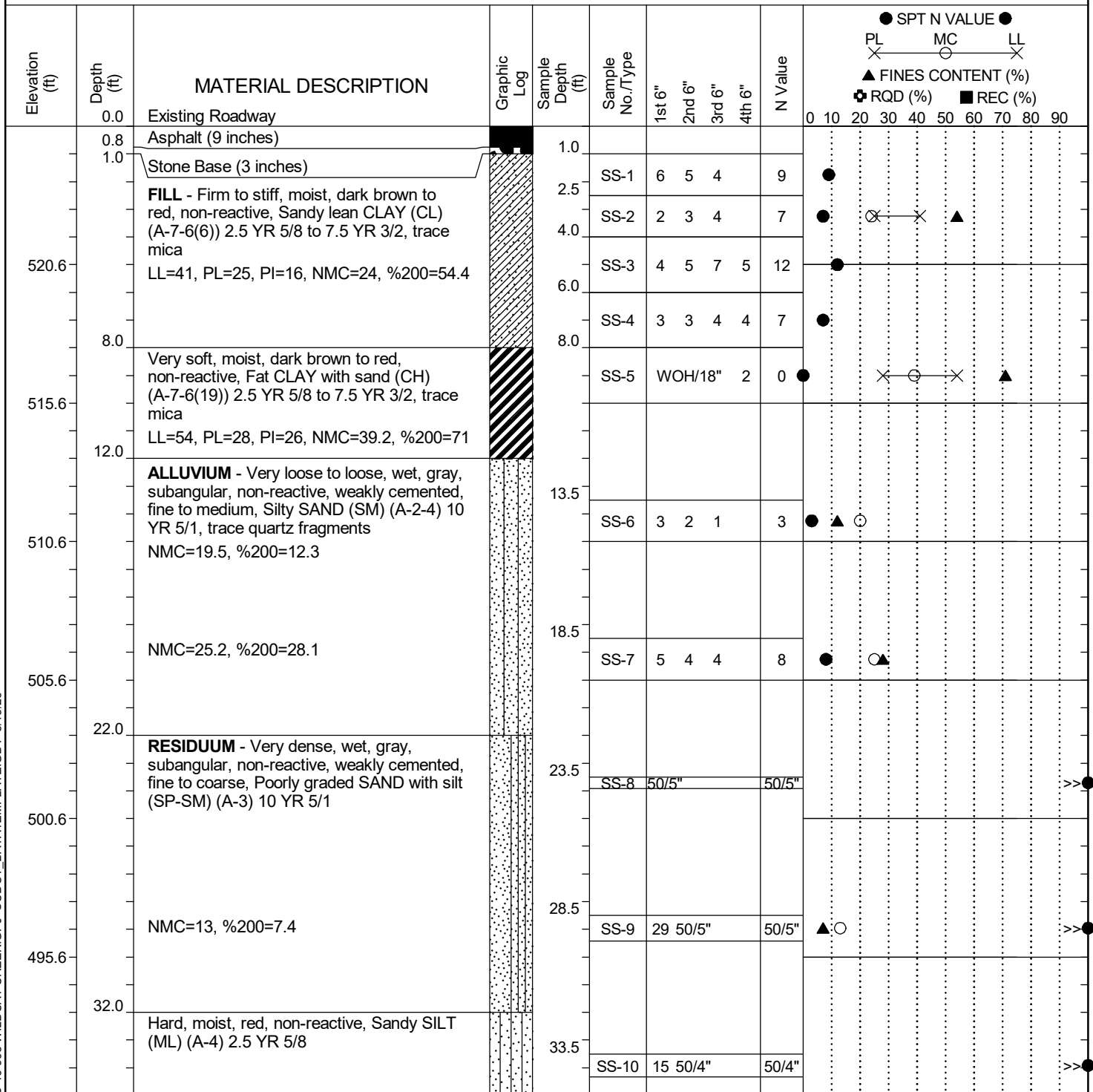
521 Clemson Road
PH. (803) 741-9000
Columbia, SC 29229
FAX. (803) 741-9900

SOIL AND ROCK SYMBOLS

Exhibit A-8

SCDOT Soil Test Log

Project ID:	P041172	County:	YORK	Boring No.:	S-46-998-1
Site Description:	S-46-998 BRO Wildcat Creek			Route:	S-46-998
Eng./Geo.:	A. Beaty	Boring Location:	36+44.95	Offset:	7.29 R
Elev.:	525.6 ft	Latitude:	34.889627	Longitude:	-81.069424
Date Started:	6/14/2023				
Total Depth:	57 ft	Soil Depth:	41 ft	Core Depth:	16 ft
Date Completed:	6/14/2023				
Bore Hole Diameter (in):	4	Sampler Configuration	Liner Required: Y (N)		Liner Used: Y (N)
Drill Machine:	DR1109	Drill Method:	RW/RC	Hammer Type:	Automatic
Energy Ratio:	93.9%				
Core Size:	NQ2	Driller:	S. Truesdale	Groundwater:	TOB N.M.
24HR	N.M.				



LEGEND

Continued Next Page

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-46-998 WILDCAT CREEK.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

Project ID:	P041172	County:	YORK	Boring No.:	S-46-998-1
Site Description:	S-46-998 BRO Wildcat Creek	Route:	S-46-998		
Eng./Geo.:	A. Beaty	Boring Location:	36+44.95	Offset:	7.29 R
Elev.:	525.6 ft	Latitude:	34.889627	Longitude:	-81.069424
Date Started:	6/14/2023				
Total Depth:	57 ft	Soil Depth:	41 ft	Core Depth:	16 ft
Date Completed:	6/14/2023				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	DR1109	Drill Method:	RW/RC	Hammer Type:	Automatic
Energy Ratio:	93.9%				
Core Size:	NQ2	Driller:	S. Truesdale	Groundwater:	TOB N.M.
24HR	N.M.				

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd 6"	3rd 6"	4th 6"	N Value	<div> <div> ● SPT N VALUE ● </div> <div> PL X MC X LL X </div> <div> ▲ FINES CONTENT (%) </div> <div> ⊕ RQD (%) ■ REC (%) </div> </div>
485.6	41.0	AUGER REFUSAL AT 41 FEET - SET CASING SET AND BEGIN CORING		38.5	SS-11	9	13 50/2"			50/2"	>>●
480.6		DIORITE , black and white, medium grained, slightly weathered, strong rock, -, J, W, No, No, Ir, VC, SR, %REC=10, RQD=0, GSI=20, 2.8 min/ft, #8 bit, 350 psi down pressure		41.0	SS-12		50/0"			50/0"	>>●
		%REC=5, RQD=0, GSI=20, 1.9 min/ft, #8 bit, 350 psi down pressure		45.0	NQ-1						
475.6		%REC=80, RQD=38, GSI=60, RMR=49, qc=17,860 psi, 1.8 min/ft, #8 bit, 350 psi down pressure		50.0	NQ-2						
470.6		%REC=16, RQD=0, GSI=20, 2.9 min/ft, #8 bit, 350 psi down pressure		55.0	NQ-3						
465.6	57.0	CORING TERMINATED AT 57 FEET AND SET DHT CASING			NQ-4						
460.6											

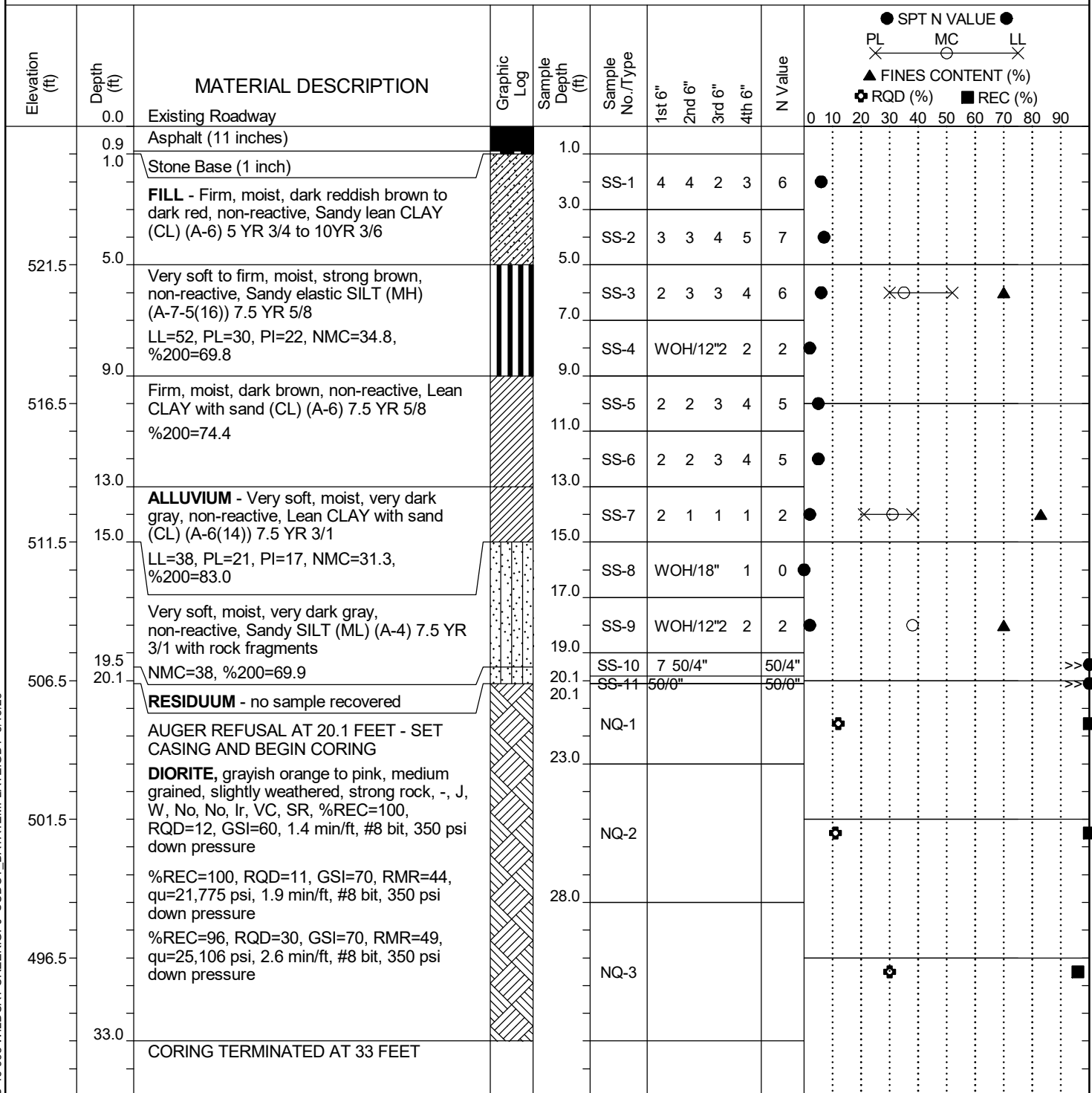
LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT SCDOT S-46-998 WILDCAT CREEK.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

SCDOT Soil Test Log

Project ID:	P041172	County:	YORK	Boring No.:	S-46-998-2
Site Description:	S-46-998 BRO Wildcat Creek			Route:	S-46-998
Eng./Geo.:	A. Beaty	Boring Location:	35+44.22	Offset:	5.98 R
Elev.:	526.5 ft	Latitude:	34.889468	Longitude:	-81.069149
Date Started:	6/10/2023				
Total Depth:	33 ft	Soil Depth:	20 ft	Core Depth:	13 ft
Date Completed:	6/11/2023				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	CMESSAX	Drill Method:	RW	Hammer Type:	Automatic
Energy Ratio:	84.4%				
Core Size:	NQ2	Driller:	J. Cain	Groundwater:	TOB N.M.
24HR	14 ft				



LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

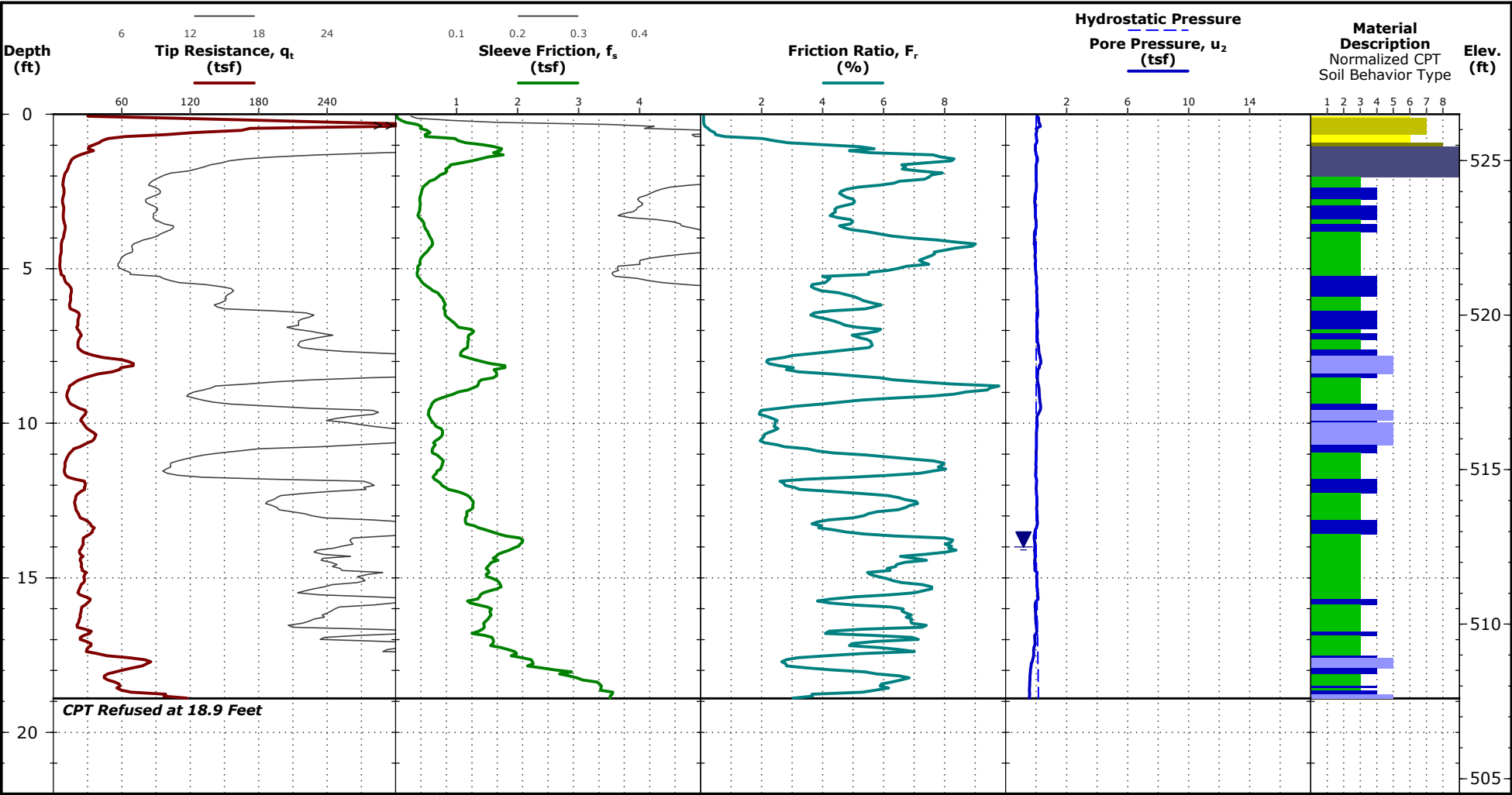
SC.DOT SCDOT S-46-998 WILDCAT CREEK.GPJ SCDOT_DATATEMPLATE.GDT 8/18/23

CPT Sounding ID S-46-998-1C

Elevation: 526.5 (ft)

Latitude: 34.889809° Longitude: -81.069719°
Station: 37+54.52 Offset: R 13.13

CPT Started: 6/27/2023
CPT Completed: 6/27/2023



See [Exploration and Testing Procedures](#) for a description of field and laboratory procedures used and additional data, if any.
See [Supporting Information](#) for explanation of symbols and abbreviations.

Notes

Test Location: See [Exploration Plan](#)

CPT Equipment

CPT Rig: TG 73-200
Operator: AF/BR
CPT sensor calibration reports available upon request
Probe No. 5617 with net area ratio of 0.84
 u_2 pore pressure transducer location
Manufactured by Geotech A.B.- Calibrated 5/31/2023
Tip and sleeve areas of 10 cm² and 150 cm²
Ring friction reducer with O.D. of 2 in

Water Level Observation

▼ 14 ft estimated water depth
(used in normalizations and correlations)

Normalized Soil Behavior Type (Robertson 1990)

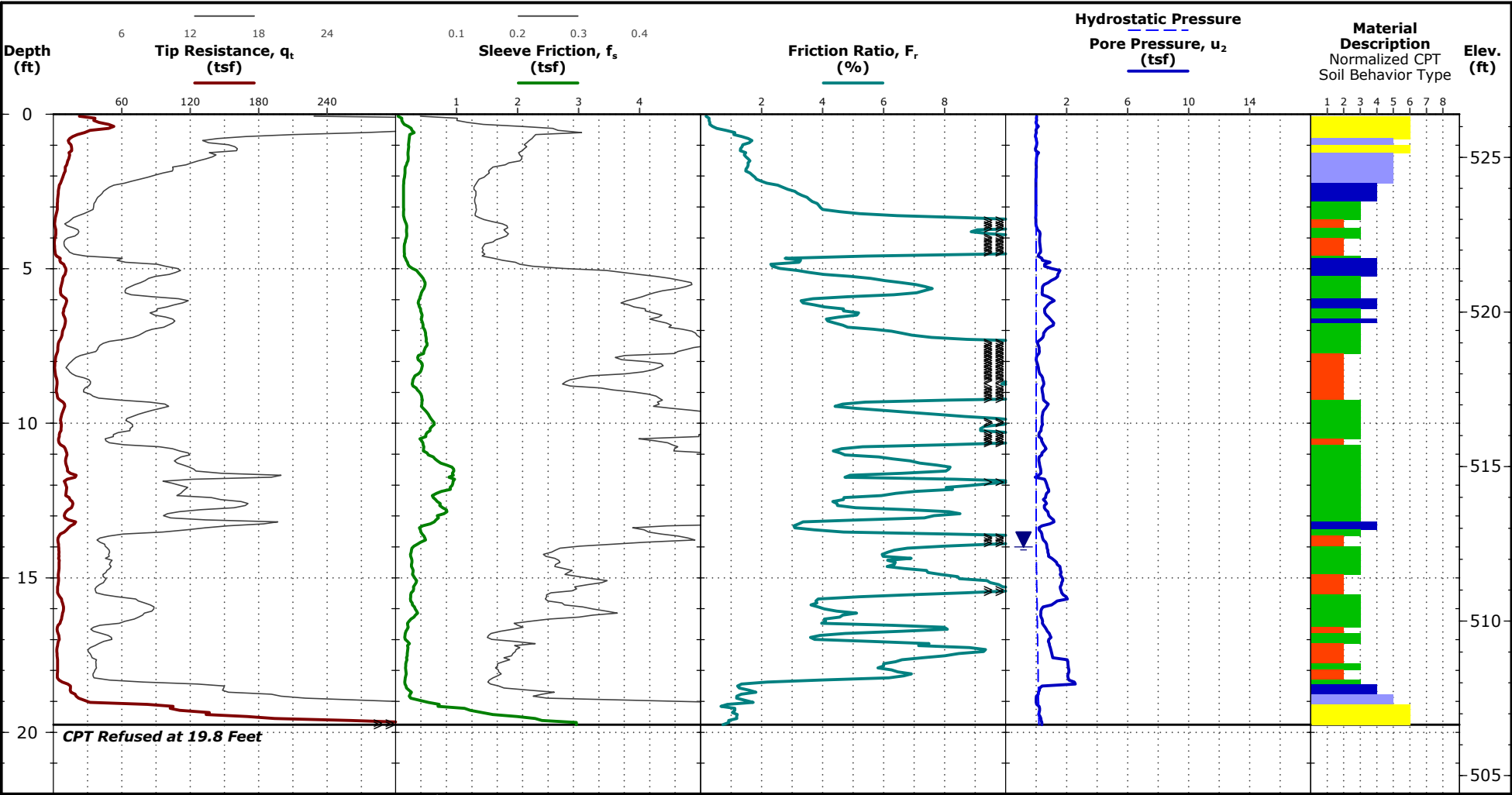
- 1 Sensitive, fine grained
- 2 Organic soils - clay
- 3 Clay - silty clay to clay
- 4 Silt mixtures - clayey silt to silty clay
- 5 Sand mixtures - silty sand to sandy silt
- 6 Sands - clean sand to silty sand
- 7 Gravelly sand to dense sand
- 8 Very stiff sand to clayey sand
- 9 Very stiff fine grained

CPT Sounding ID S-46-998-2C

Elevation: 526.4 (ft)

Latitude: 34.889473° Longitude: -81.069157°
Station: 35+47.17 Offset: R 6.19

CPT Started: 6/27/2023
CPT Completed: 6/27/2023



See [Exploration and Testing Procedures](#) for a description of field and laboratory procedures used and additional data, if any.
See [Supporting Information](#) for explanation of symbols and abbreviations.

Notes

Test Location: See [Exploration Plan](#)

CPT Equipment

CPT Rig: TG 73-200
Operator: AF/BR
CPT sensor calibration reports available upon request
Probe No. 5617 with net area ratio of 0.84
 u_2 pore pressure transducer location
Manufactured by Geotech A.B.- Calibrated 5/31/2023
Tip and sleeve areas of 10 cm² and 150 cm²
Ring friction reducer with O.D. of 2 in

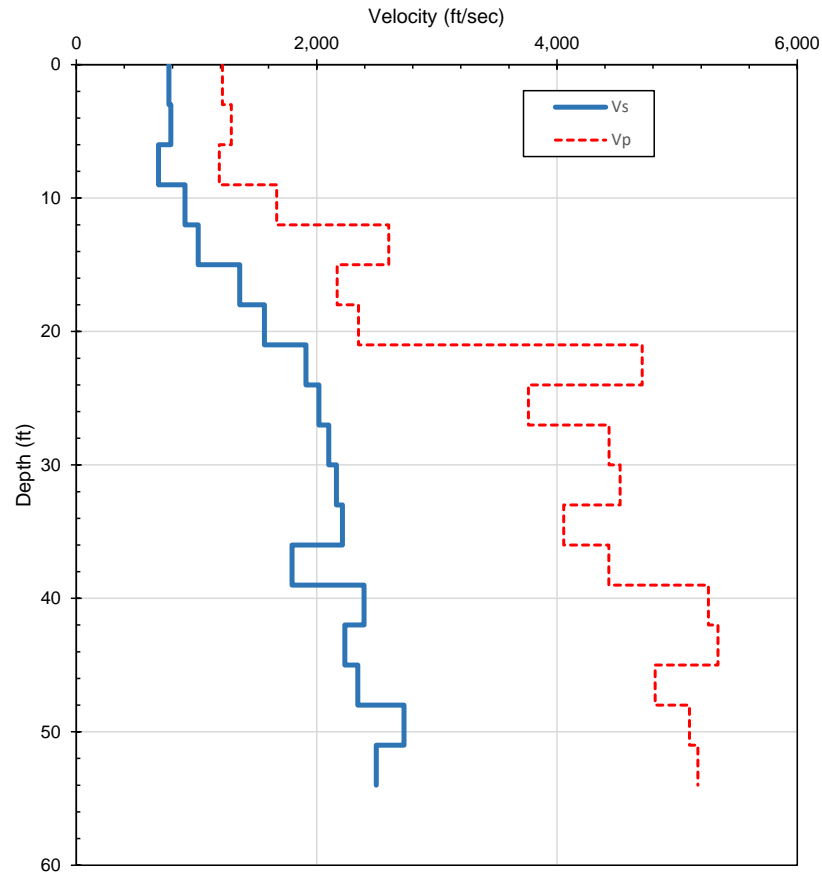
Water Level Observation

▼ 14 ft estimated water depth
(used in normalizations and correlations)

Normalized Soil Behavior Type (Robertson 1990)

- 1 Sensitive, fine grained
- 2 Organic soils - clay
- 3 Clay - silty clay to clay
- 4 Silt mixtures - clayey silt to silty clay
- 5 Sand mixtures - silty sand to sandy silt
- 6 Sands - clean sand to silty sand
- 7 Gravelly sand to dense sand
- 8 Very stiff sand to clayey sand
- 9 Very stiff fine grained

Downhole Seismic Velocity Fixed Interval Method



Depth (ft)	Vp (ft/sec)	Vs (ft/sec)	Δt (ft)	Δt (sec)
4	1,217	770	4	0.00519
7	1,290	787	3	0.00381
10	1,190	684	3	0.00439
13	1,668	904	3	0.00332
16	2,600	1,015	3	0.00296
19	2,170	1,359	3	0.00221
22	2,348	1,566	3	0.00192
25	4,709	1,911	3	0.00157
28	3,763	2,018	3	0.00149
31	4,434	2,101	3	0.00143
34	4,526	2,165	3	0.00139
37	4,056	2,215	3	0.00135
40	4,431	1,796	3	0.00167
43	5,260	2,394	3	0.00125
46	5,340	2,235	3	0.00134
49	4,818	2,342	3	0.00128
52	5,102	2,727	3	0.00110
55	5,173	2,497	3	0.00120
Sum of Data Over Profile				
			55	0.03886
Weighted Average Shear Wave Velocity Over Profile			1,415 ft/sec	

Project Mgr: DC

Prepared by: RK

Checked by: PM

Approved by:

Project No.
7323P100

Scale: NA

Date:
7/20/2023



Consulting Engineers and Scientists

521 Clemson Road
Ph: (803) 741-9000

Columbia, South Carolina
Fax: (803) 741-9900

GEOPHYSICAL TESTING RESULTS

DOWNHOLE SEISMIC TEST

S-46-998 Bridge Replacement over Wildcat Creek
YORK COUNTY, SOUTH CAROLINA

P041172

TEST
NO.

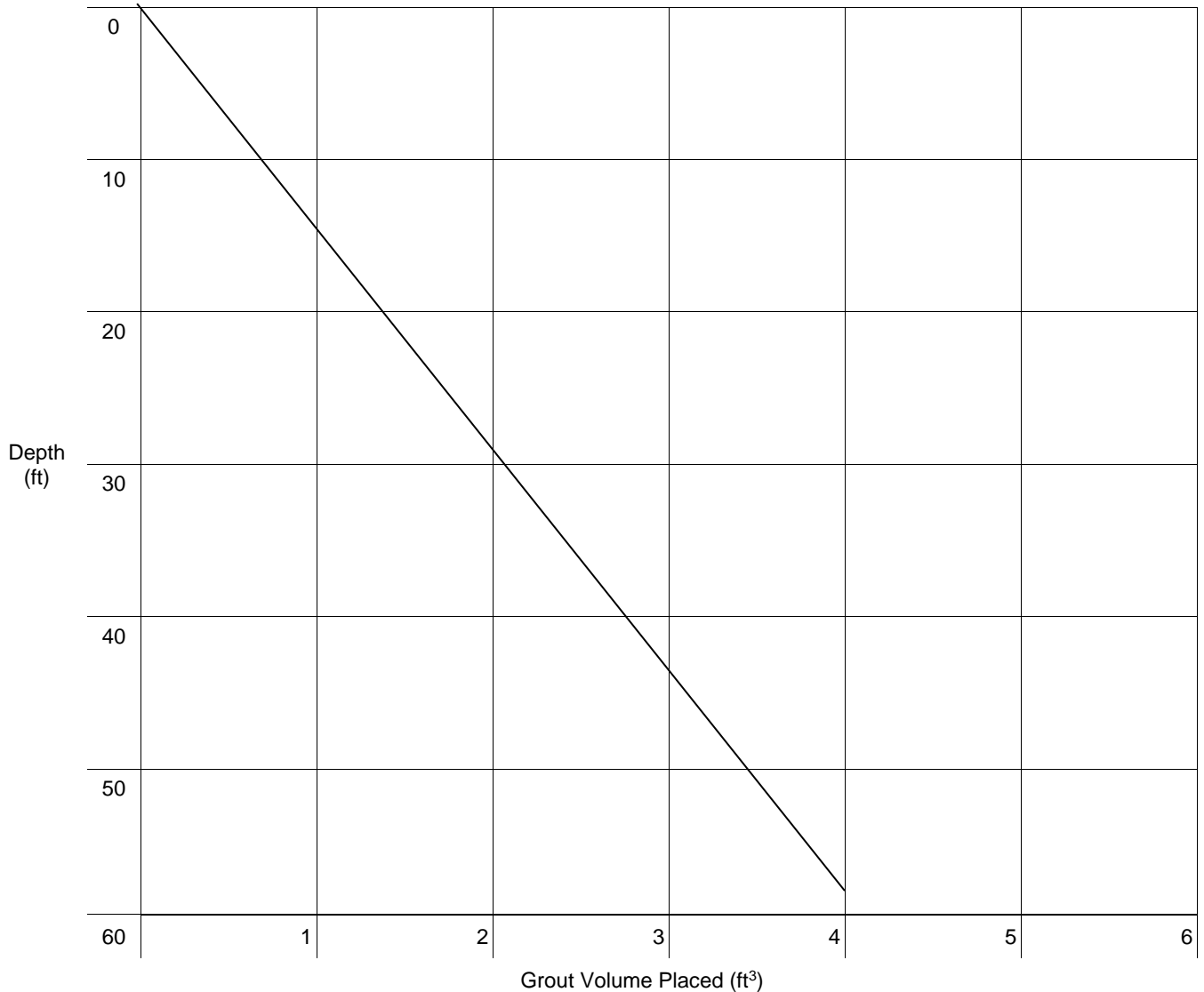
S-46-
998-1



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-46-998 BRO Wildcat Creek		Test Hole No.:	S-46-998-1
Project ID:	P041172		Station:	36+44.95
Consultant Firm:	Terracon Consultants, Inc.		Offset:	7.29 R
Grouted By (Driller's Name):	Truesdale	Date	6/14/23	
Notes:	Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water			

Grout Curve



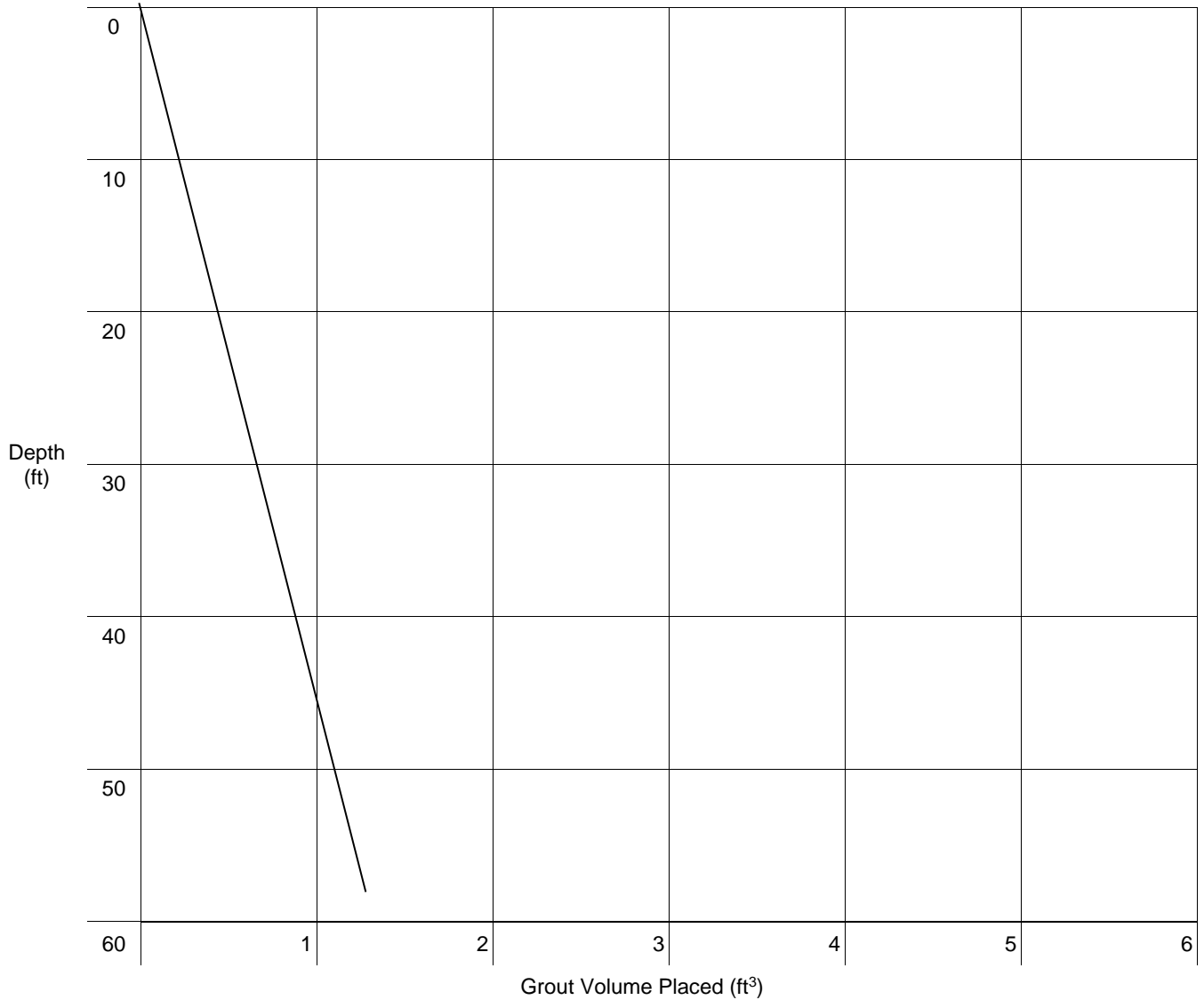
Number of Bags On-Site	20	ea.
Depth of Test Hole Grouted	57	ft.
Diameter of Test Hole	0.33	ft.
Area of Test Hole	0.09	ft²
Volume of Test Hole	5.1	ft³
Volume of Casing (If applicable)	1.6	ft³
Theoretical Volume of Test Hole	3.5	ft³
Number of Bags Used	5	ea.
Volume Placed	4	ft³



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name: S-46-998 BRO Wildcat Creek
Project ID: P041172 Test Hole No.: S-46-998-1
Consultant Firm: Terracon Consultants, Inc. Station: 36+44.95
Grouted By (Driller's Name): Truesdale Date 7/15/23 Offset: 7.29R
Notes: Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water

Grout Curve



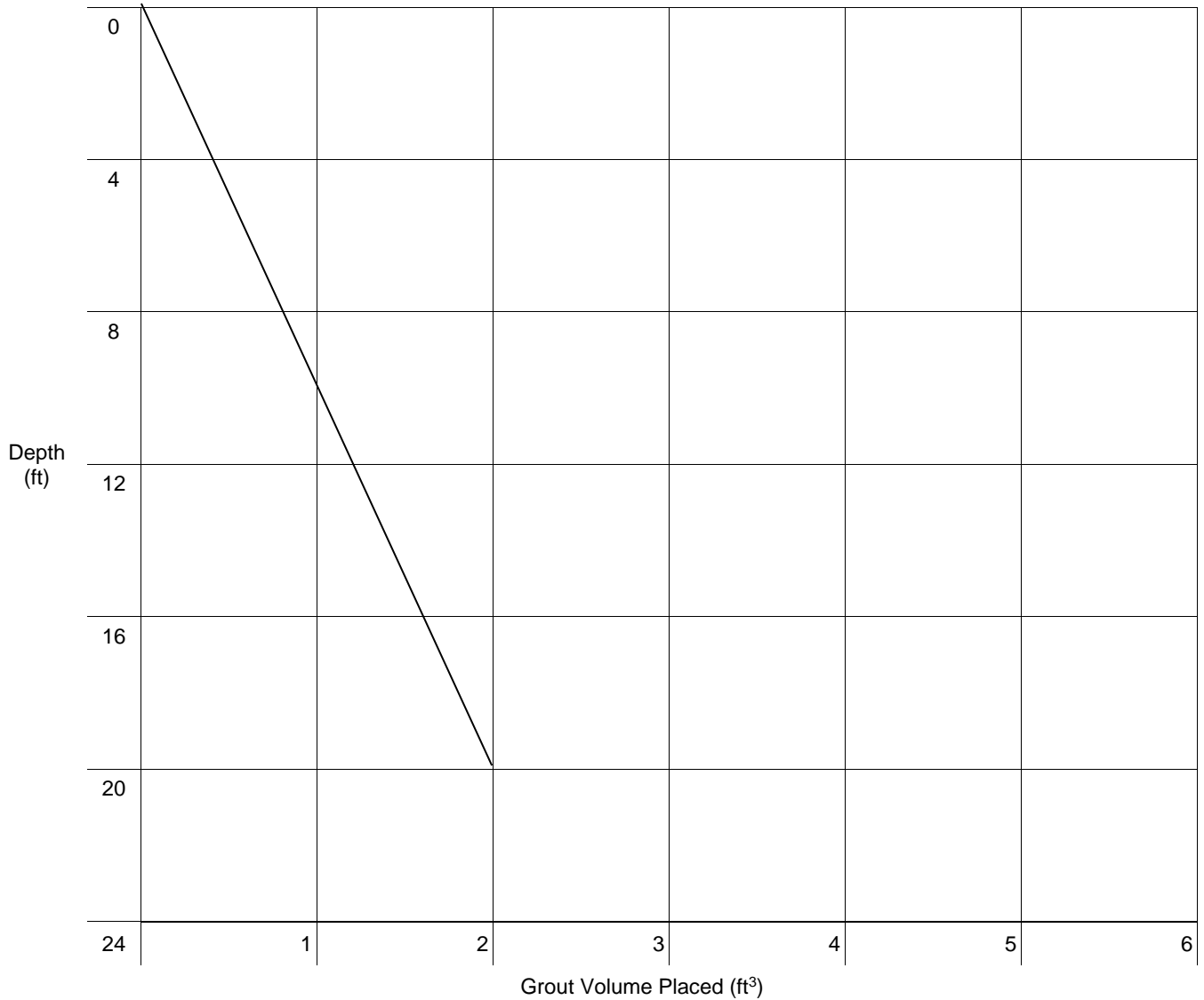
Number of Bags On-Site	<u>20</u>	ea.
Depth of Test Hole Grouted	<u>57</u>	ft.
Diameter of Test Hole	<u>0.17</u>	ft.
Area of Test Hole	<u>0.022</u>	ft²
Volume of Test Hole	<u>1.25</u>	ft³
Volume of Casing (If applicable)	<u>-</u>	ft³
Theoretical Volume of Test Hole	<u>1.25</u>	ft³
Number of Bags Used	<u>2</u>	ea.
Volume Placed	<u>1.3</u>	ft³



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-46-998 BRO Wildcat Creek		Test Hole No.:	S-46-998-2
Project ID:	P041172		Station:	35+44.22
Consultant Firm:	Terracon Consultants, Inc.		Offset:	5.98 R
Grouted By (Driller's Name):	Truesdale	Date	6/11/23	
Notes:	Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water			

Grout Curve

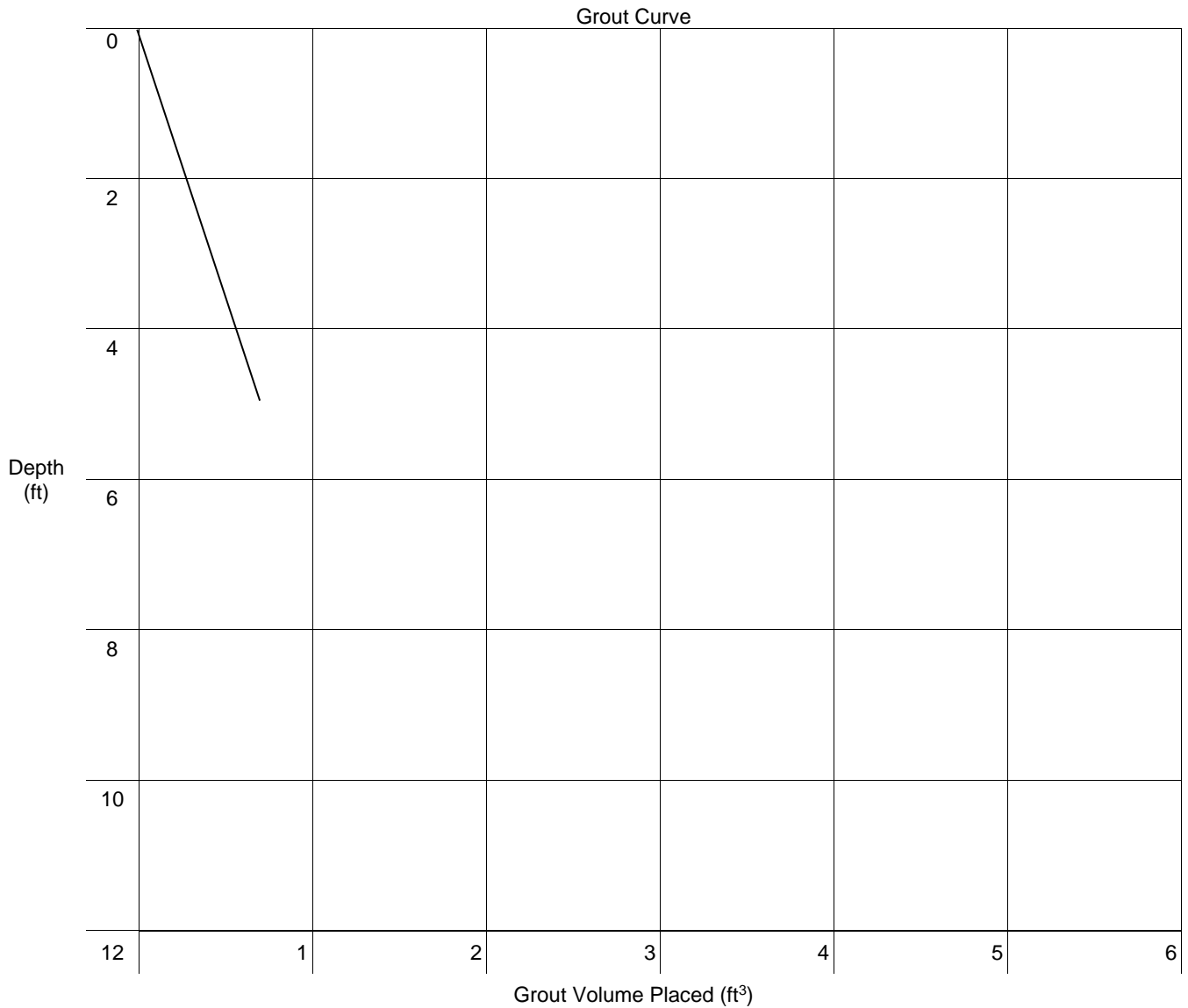


Number of Bags On-Site	20	ea.
Depth of Test Hole Grouted	20	ft.
Diameter of Test Hole	0.33	ft.
Area of Test Hole	0.09	ft²
Volume of Test Hole	1.74	ft³
Volume of Casing (If applicable)	-	ft³
Theoretical Volume of Test Hole	1.74	ft³
Number of Bags Used	2.5	ea.
Volume Placed	2	ft³



GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-46-998 BRO Wildcat Creek	Test Hole No.:	S-46-998-2
Project ID:	P041172	Station:	(BULK)
Consultant Firm:	Terracon Consultants, Inc.	Offset:	35+44.22
Grouted By (Driller's Name):	Truesdale	Date	6/11/23
Notes:	Mix design: 1 pound cement, 1 pound bentonite, 6 pounds water		9.67R



Number of Bags On-Site	20	ea.
Depth of Test Hole Grouted	5	ft.
Diameter of Test Hole	0.5	ft.
Area of Test Hole	0.2	ft ²
Volume of Test Hole	1.0	ft ³
Volume of Casing (If applicable)	-	ft ³
Theoretical Volume of Test Hole	1.0	ft ³
Number of Bags Used	1.5	ea.
Volume Placed	0.7	ft ³



S-46-998-1, NQ-1, NQ-2, NQ-3 and NQ-4



S-46-998-2, NQ-1 and NQ-2



S-46-998-2, NQ-3

Appendix B

Laboratory Testing

Exhibit B-1 – Laboratory Testing Description
Summary of Laboratory Data
Laboratory Data Sheets (24 Pages)

Note: All exhibits are one page unless noted above.

Exhibit B-1

S-46-998 BRO Wildcat Creek | York County, SC

August 21, 2023 (rev1) | Terracon Project No. 7323P100 | SCDOT Project ID: P041172



Laboratory Testing Description

The samples collected during the field exploration were taken to our laboratory for additional testing. The laboratory testing scope was developed by the SCDOT and laboratory assignment was performed by Terracon. The laboratory tests were conducted on selected soil samples from the borings and the bulk samples locations. The test results are presented in this appendix.

The laboratory test results were used to confirm the soil descriptions presented on the boring logs in Appendix A. Laboratory tests were performed in general accordance with the applicable ASTM, AASHTO, SCDOT or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

■ Moisture Content	AASHTO T265/(ASTM D2216)
■ Atterberg Limits	AASHTO T89/T90(ASTM D4318)
■ Wash 200	AASHTO T11/(ASTM D1140)
■ Triaxial Shear CU w/ PP	AASHTO T297/(ASTM D4767)
■ Grain Size Distribution	ASTM D6913
■ Hydrometer	ASTM D7928
■ Corrosion Series	AASHTO D422
	AASHTO T289/ASTM G51
	AASHTO T290/ASTM C1580
	AASHTO T291

SUMMARY OF LABORATORY RESULTS

PAGE 1 OF 1

BORING ID	Depth (Ft.)	Soil Classification USCS & AASHTO	Liquid Limit	Plastic Limit	Plasticity Index	% Fines	% Gravel	% Sand	% Silt	% Clay	Water Content (%)
S-46-998-1	2.5-4	SANDY LEAN CLAY(CL) / A-7-6 (6)	41	25	16	54.4					24.0
S-46-998-1	8-10	FAT CLAY with SAND(CH) / A-7-6 (19)	54	28	26	71.0	0.8	28.2	21.9	49.1	39.2
S-46-998-1	13.5-15					12.3	0.0	87.7	5.6	6.7	19.5
S-46-998-1	18.5-20					28.1					25.2
S-46-998-1	28.5-29.4					7.4					13.0
S-46-998-2	5-7	SANDY ELASTIC SILT(MH) / A-7-5 (16)	52	30	22	69.8					34.8
S-46-998-2	9-11					74.4	0.3	25.3	27.5	46.8	
S-46-998-2	13-15	LEAN CLAY with SAND(CL) / A-6 (14)	38	21	17	83.0	0.0	17.0	39.8	43.2	31.3
S-46-998-2	17-19					69.9					38.0
S-46-998-2 Bulk	0-5	SANDY LEAN CLAY(CL) / A-6 (7)	38	19	19	53.9					23.3
PROJECT: S-46-998 BRO Wildcat Creek			<div>Terracon</div> <div>521 Clemson Rd Columbia, SC</div>					PROJECT NUMBER: 7323P100			
SITE: York County, SC								CLIENT: NHTB			
			PH. 803-741-9000 FAX. 803-741-9900								



INDEX PROPERTIES VERSUS DEPTH

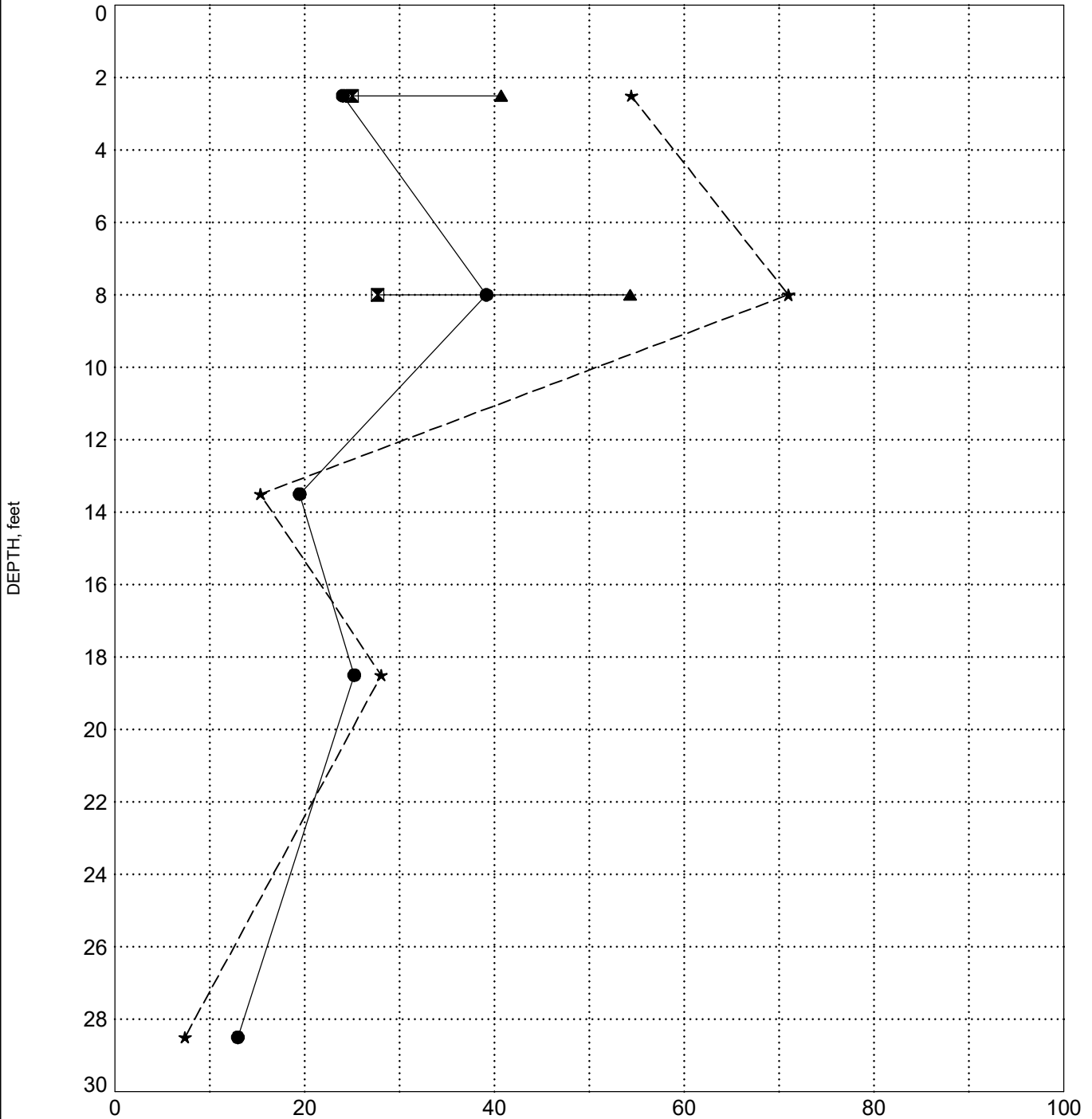
PROJECT ID P041172

PROJECT NAME S-46-998 BRO Wildcat Creek

PROJECT COUNTY YORK

SURFACE ELEVATION: 525.6

BORING S-46-998-1



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines



INDEX PROPERTIES VERSUS DEPTH

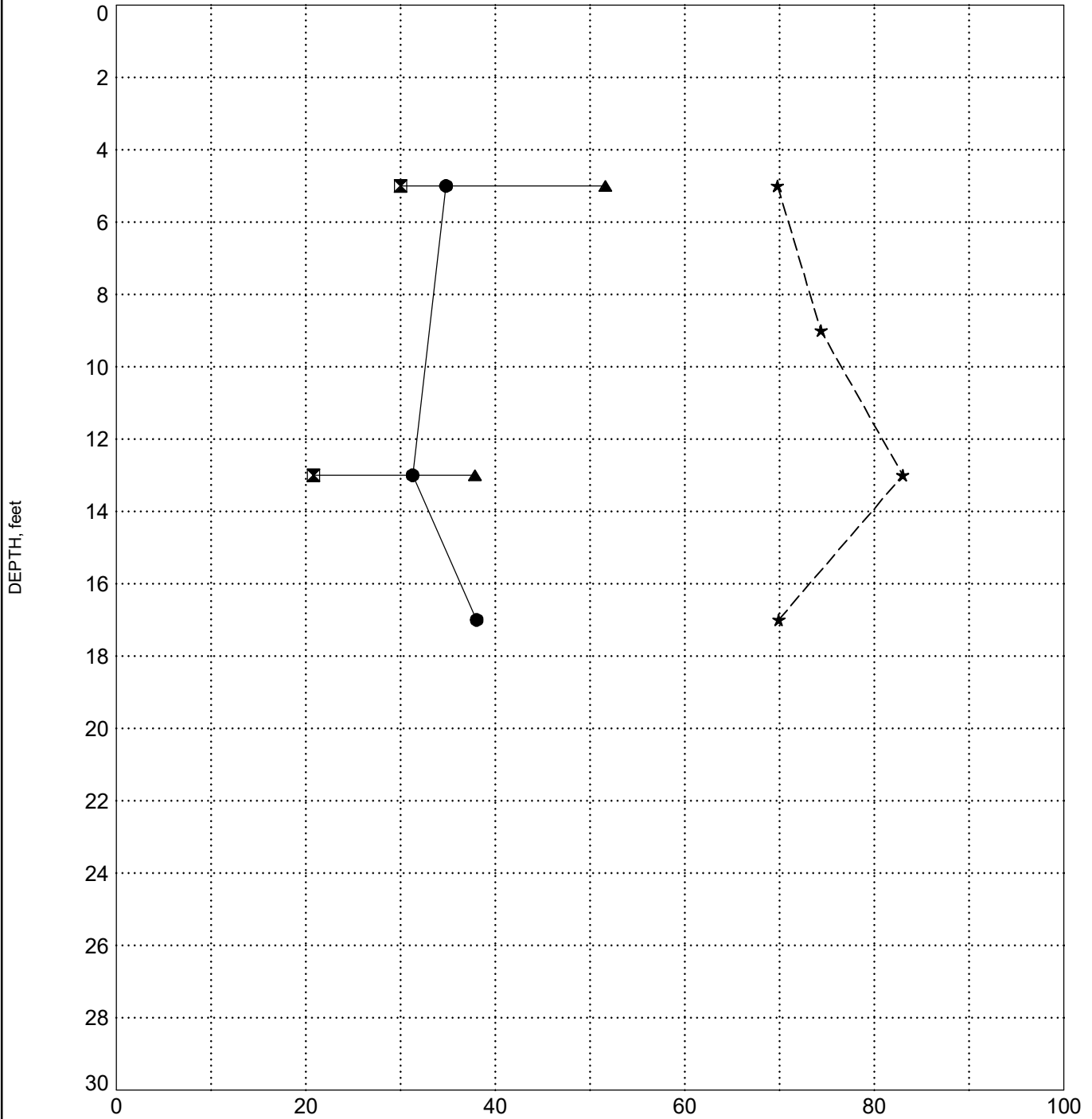
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PROJECT NAME S-46-998 BRO Wildcat Creek

PROJECT COUNTY YORK

SURFACE ELEVATION: 526.5

BORING S-46-998-2



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines

ASTM D422 / ASTM C136



SILT OR CLAY

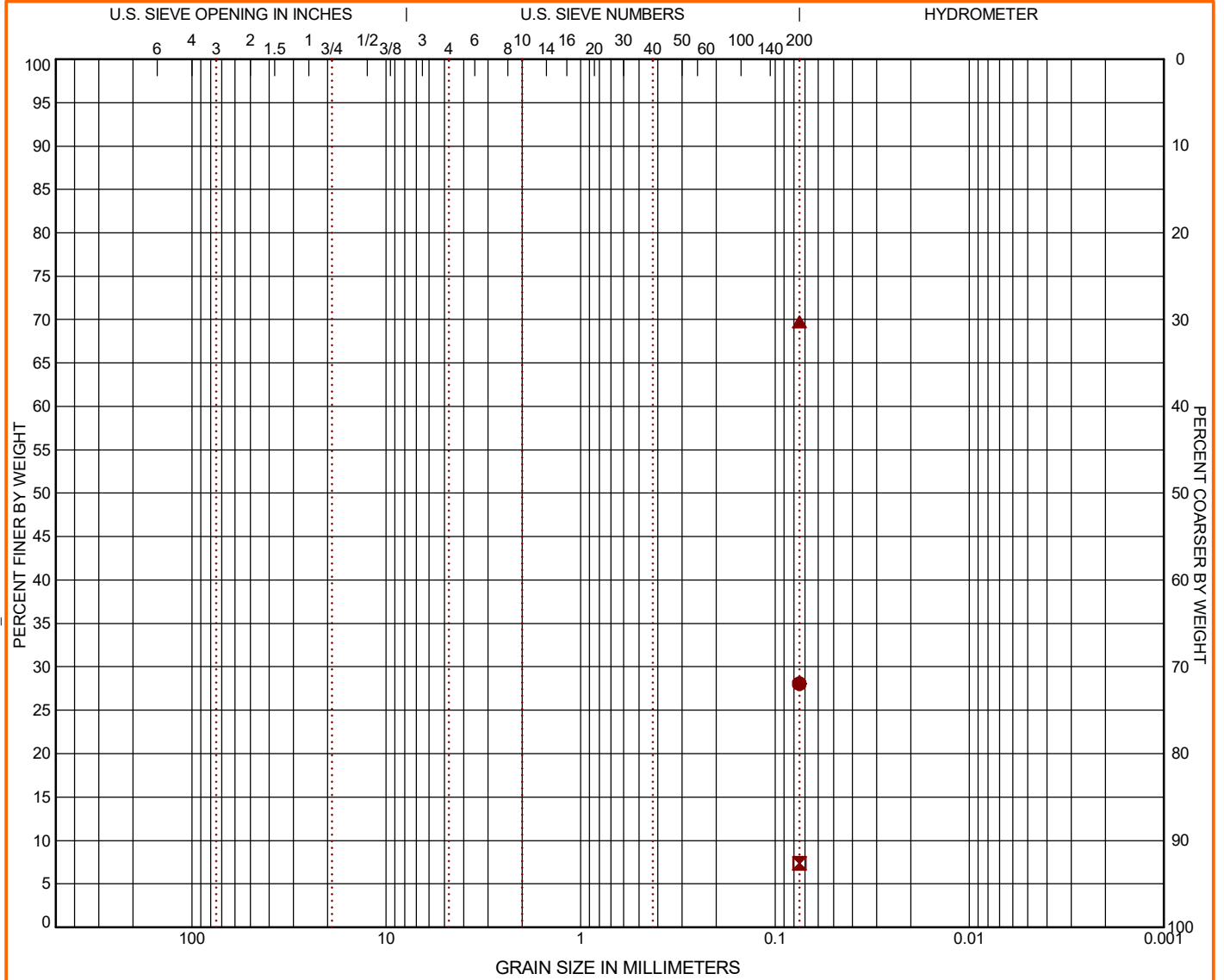
USCS

CLIENT: NHTB

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136

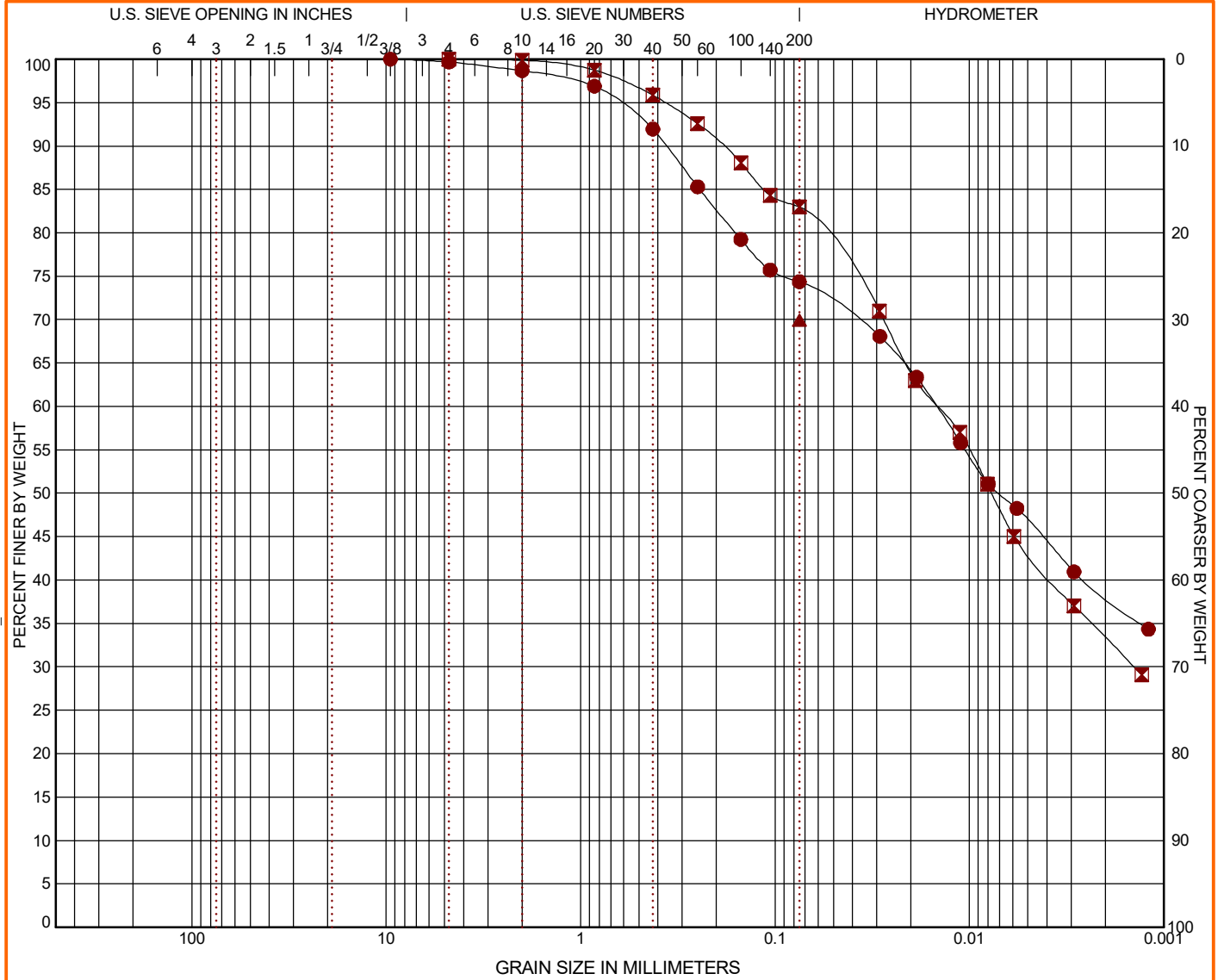
LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-46-998 WILDCAT CREEK.GPJ TERRACON_DATATEMPLATE.GDT 8/18/23



GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-46-998 WILDCAT CREEK.GPJ TERRACON_DATATEMPLATE.GDT 8/18/23



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
● S-46-998-2	9 - 11	0.0	0.3	25.3	27.5		46.8	
✕ S-46-998-2	13 - 15	0.0	0.0	17.0	39.8		43.2	CL
▲ S-46-998-2	17 - 19					69.9		

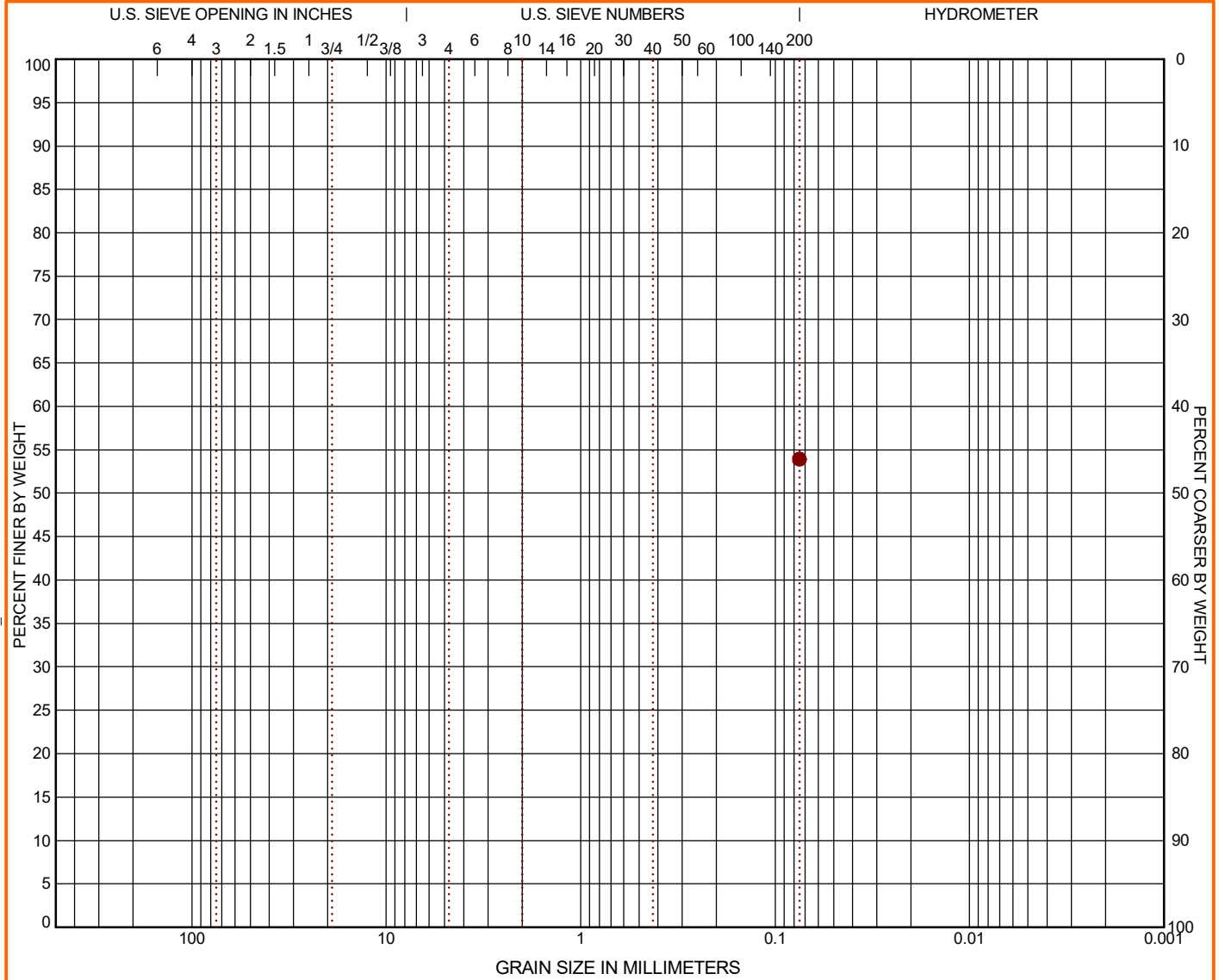
GRAIN SIZE				SOIL DESCRIPTION			
	●	✕	▲	Sieve	% Finer	Sieve	% Finer
D ₆₀	0.015	0.015		3/8"	100.0	#4	100.0
D ₃₀		0.001		#4	99.67	#10	99.87
D ₁₀				#10	98.67	#20	98.75
				#20	96.89	#40	95.86
				#40	91.95	#60	92.58
				#60	85.29	#100	88.08
				#100	79.25	#140	84.3
				#140	75.71	#200	83.0
				#200	74.36		
COEFFICIENTS				REMARKS			
	●	✕	▲				
C _c				●			
C _u				✕	A-6 (14)		
				▲			

PROJECT: S-46-998 BRO Wildcat Creek	<p>521 Clemson Rd Columbia, SC</p>	PROJECT NUMBER: 7323P100
SITE: York County, SC		CLIENT: NHTB

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: AASHTO DESC-1 S-46-998 WILDCAT CREEK.GPJ TERRACON_DATATEMPLATE.GDT 8/18/23



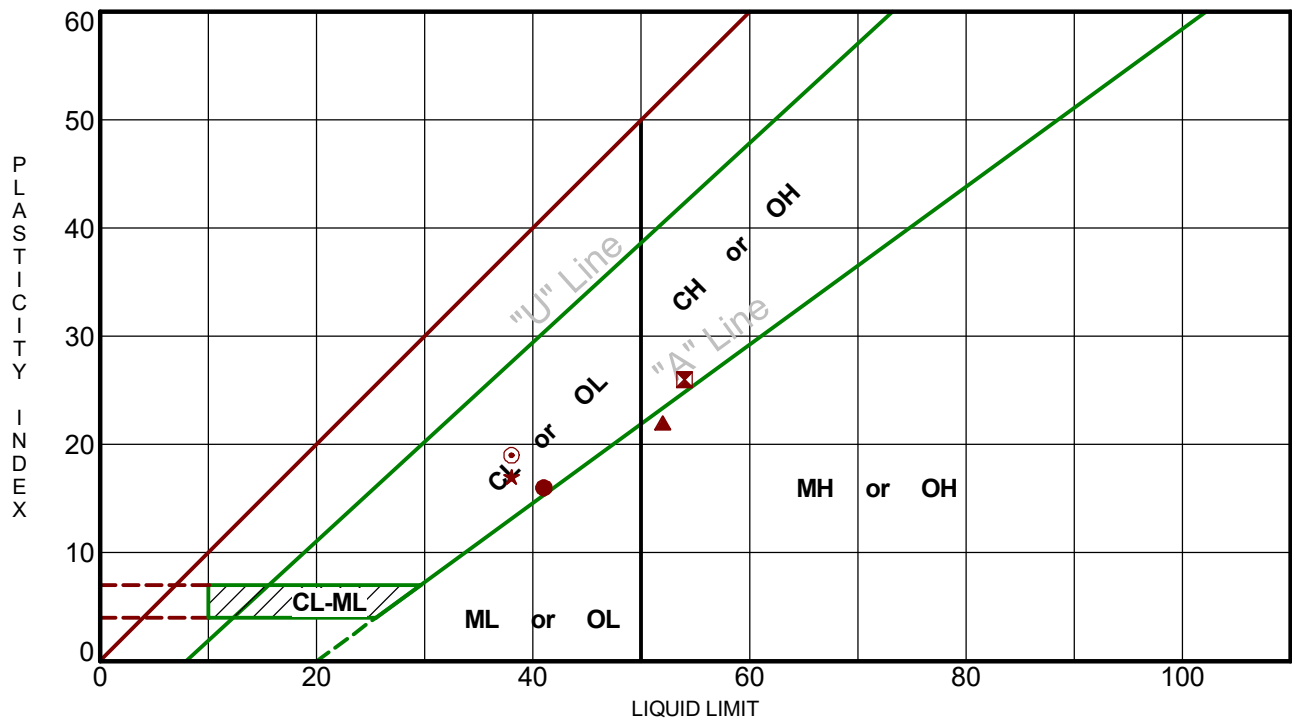
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
● S-46-998-2 Bulk	0 - 5					53.9		CL

GRAIN SIZE				SOIL DESCRIPTION			
	●			Sieve	% Finer	Sieve	% Finer
D ₆₀				#200	53.93	●	A-6 (7)
D ₃₀							
D ₁₀							
COEFFICIENTS				REMARKS			
	●			●			
C _c							
C _u							

PROJECT: S-46-998 BRO Wildcat Creek	<p>521 Clemson Rd Columbia, SC</p>	PROJECT NUMBER: 7323P100
SITE: York County, SC		CLIENT: NHTB

ASTM D4318

[illegible]

PROJECT: S-46-998 BRO Wildcat Creek

SITE: York County, SC

Terracon
521 Clemson Rd
Columbia, SC

PROJECT NUMBER: 7323P100

CLIENT: NHTB

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. ATTERBERG LIMITS-AASHTO S46-998 WILDCAT CREEK.GPJ TERRACON DATATEMPLATE.GDT 8/18/23

MOISTURE-DENSITY RELATIONSHIP

ASTM D698/D1557

Source of Material S-46-998-2 Bulk @ 0 - 5 feet

Description of Material SANDY LEAN CLAY(CL)

Remarks:

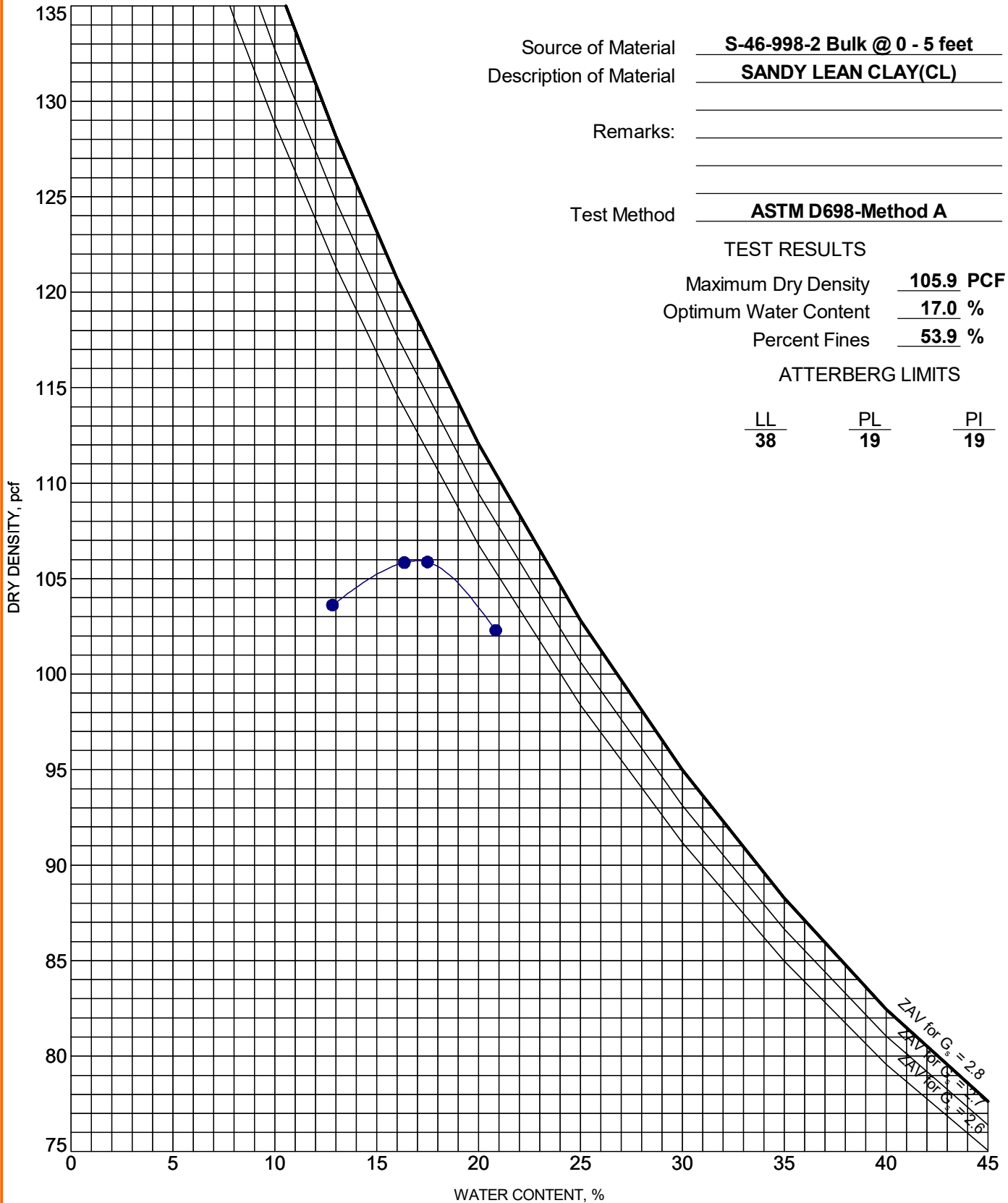
Test Method ASTM D698-Method A

TEST RESULTS

Maximum Dry Density 105.9 PCF
Optimum Water Content 17.0 %
Percent Fines 53.9 %

ATTERBERG LIMITS

LL	PL	PI
<u>38</u>	<u>19</u>	<u>19</u>



PROJECT: S-46-998 BRO Wildcat Creek

SITE: York County, SC

Terracon
521 Clemson Rd
Columbia, SC

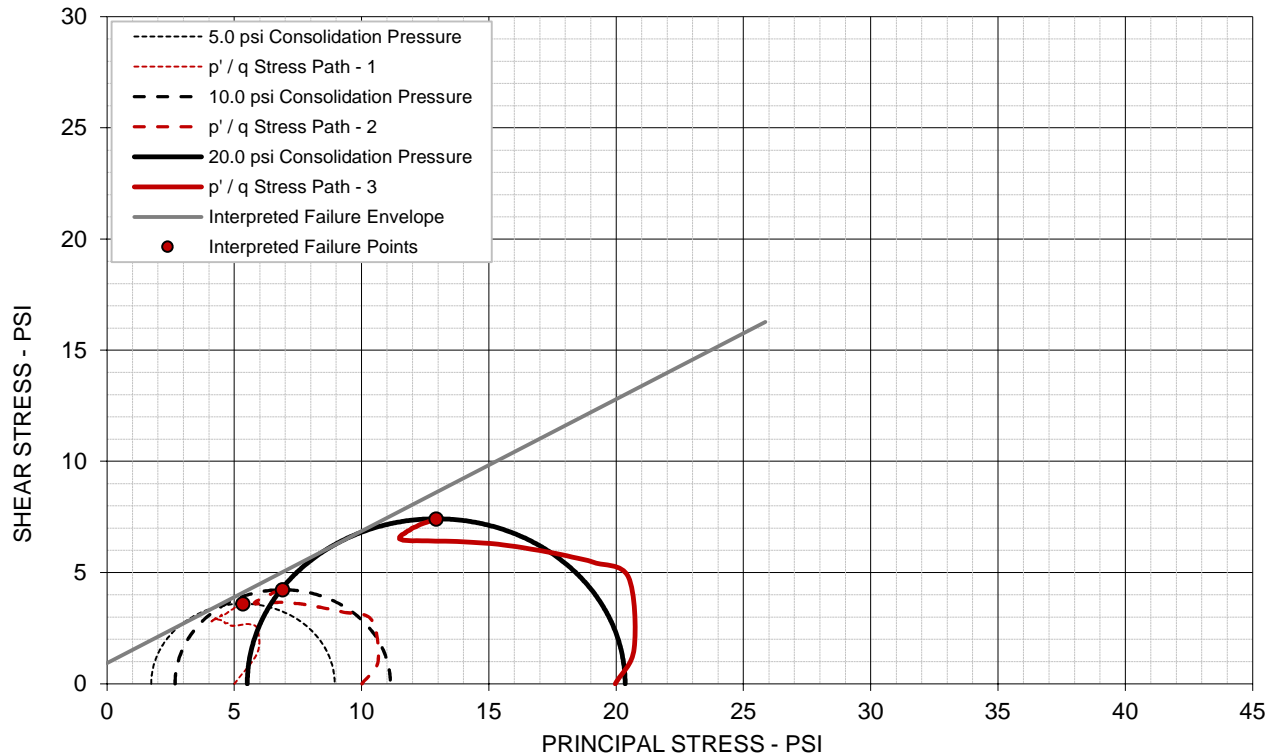
PROJECT NUMBER: 7323P100

CLIENT: NHTB

ICU TRIAXIAL COMPRESSION TEST

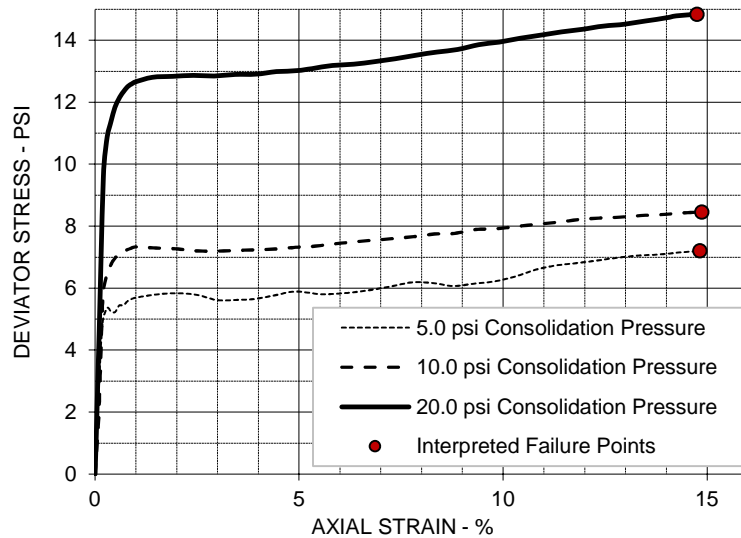
ASTM D4767 / AASHTO T297

Failure Criteria: Max Deviator Stress



EFFECTIVE STRESS PARAMETERS

$\phi' = 30.7$ deg $c' = 0.9$ psi



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	17.0	17.0	17.0
Dry Density - pcf	100.6	100.6	100.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	5.99	6.00

AT TEST

Final Moisture - %	22.9	22.2	20.9
Dry Density - pcf	101.0	101.8	105.2
Calculated Diameter (in.)	2.83	2.83	2.81
Height - inches	5.94	5.91	5.88
Effect. Consol. Stress - psi	5.0	10.0	20.0
Failure Stress - psi	7.20	8.46	14.84
Total Pore Pressure - psi	83.3	87.4	94.5
Strain Rate - %/min.	0.0335	0.0336	0.0339
Failure Strain - %	14.8	14.9	14.8
σ_1' Failure - psi	8.94	11.13	20.35
σ_3' Failure - psi	1.74	2.67	5.51

TEST DESCRIPTION

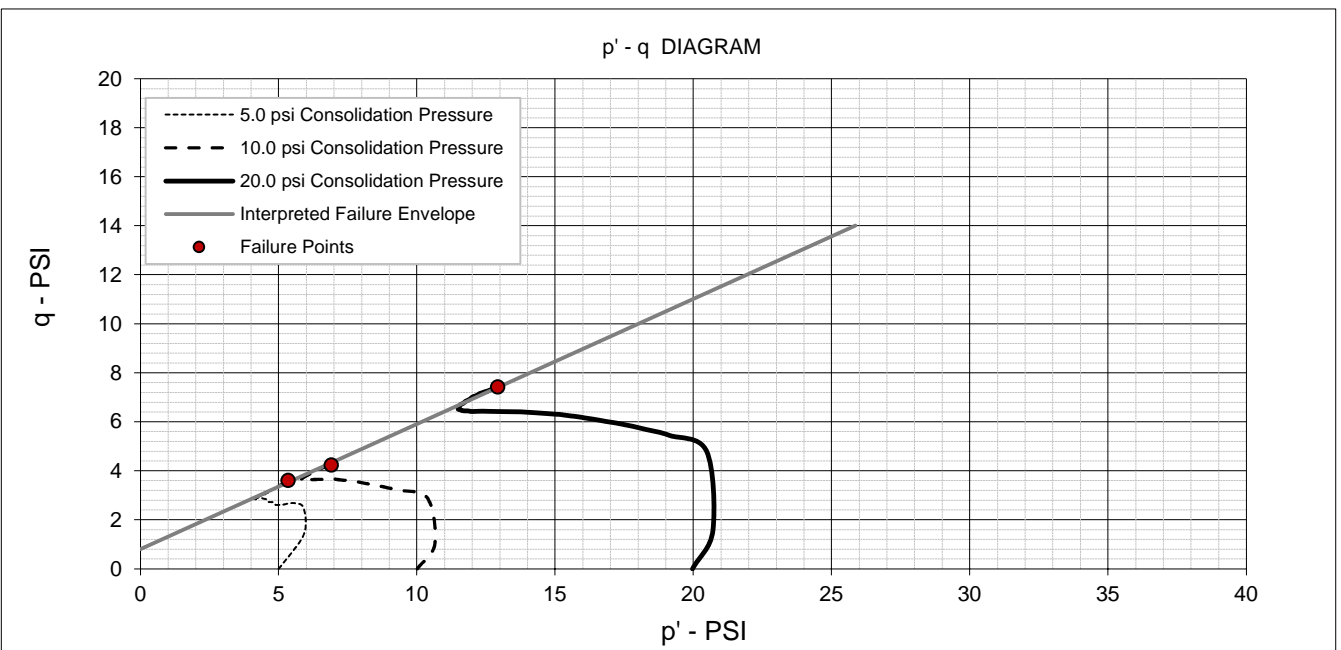
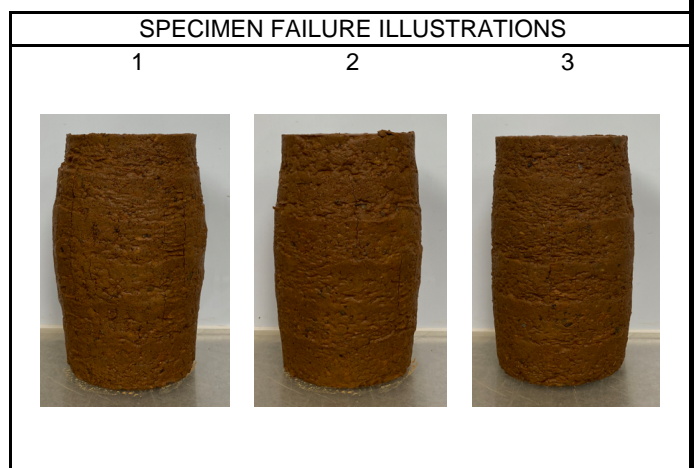
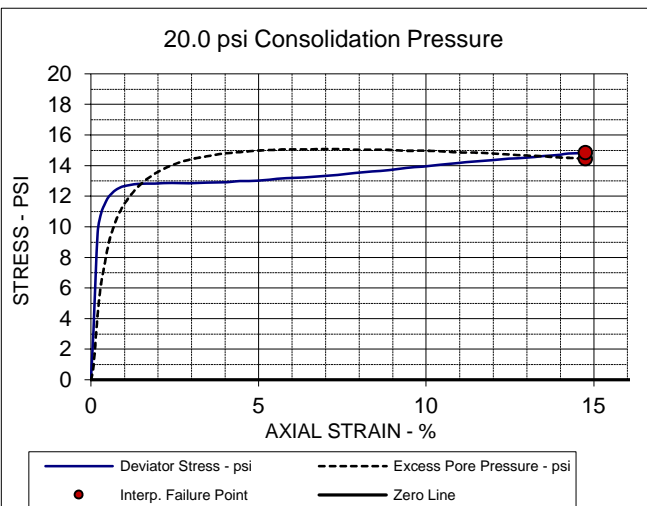
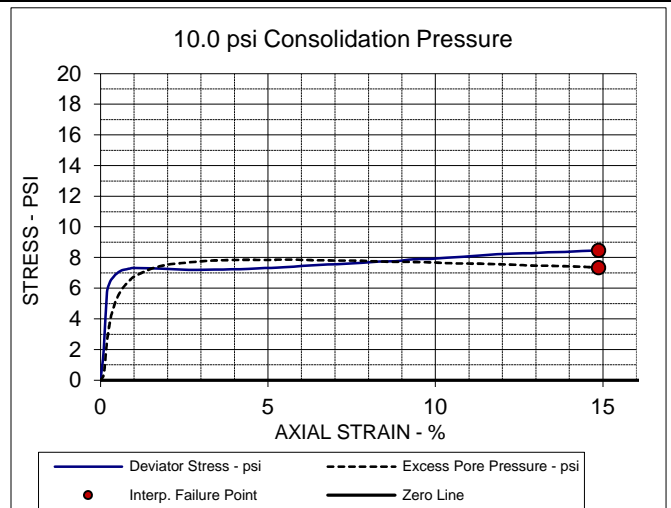
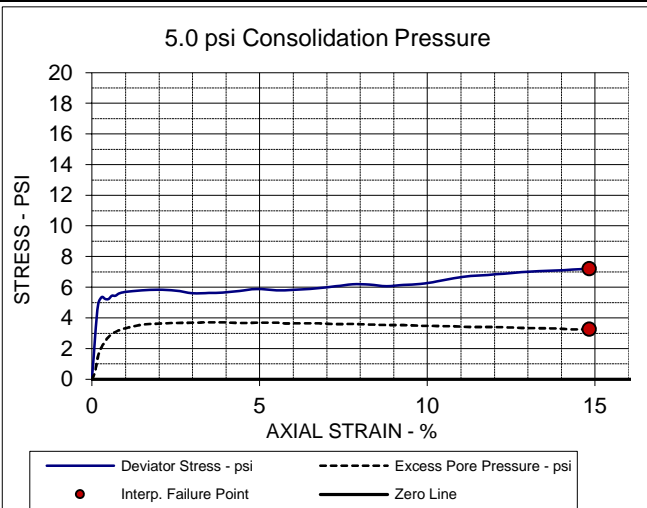
ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION
 SAMPLE TYPE: Remolded
 DESCRIPTION: Sandy Lean Clay (CL) / A-6 (7)
 SAMPLE ID: S-46-998-2 Bulk 0-5'
 SPECIFIC GRAVITY: 2.65
 LL: 38 PL: 19 PI: 19 Percent -200: 53.9%
 Remarks: Remolded to 95% of the Standard Proctor


PROJECT INFORMATION

PROJECT: S-46-998 BRO Wildcat Creek
 LOCATION: York County, SC 7323P100
 PROJECT #: HNTB
 CLIENT: 07/12/23
 DATE:

521 Clemson Road
 Columbia, SC



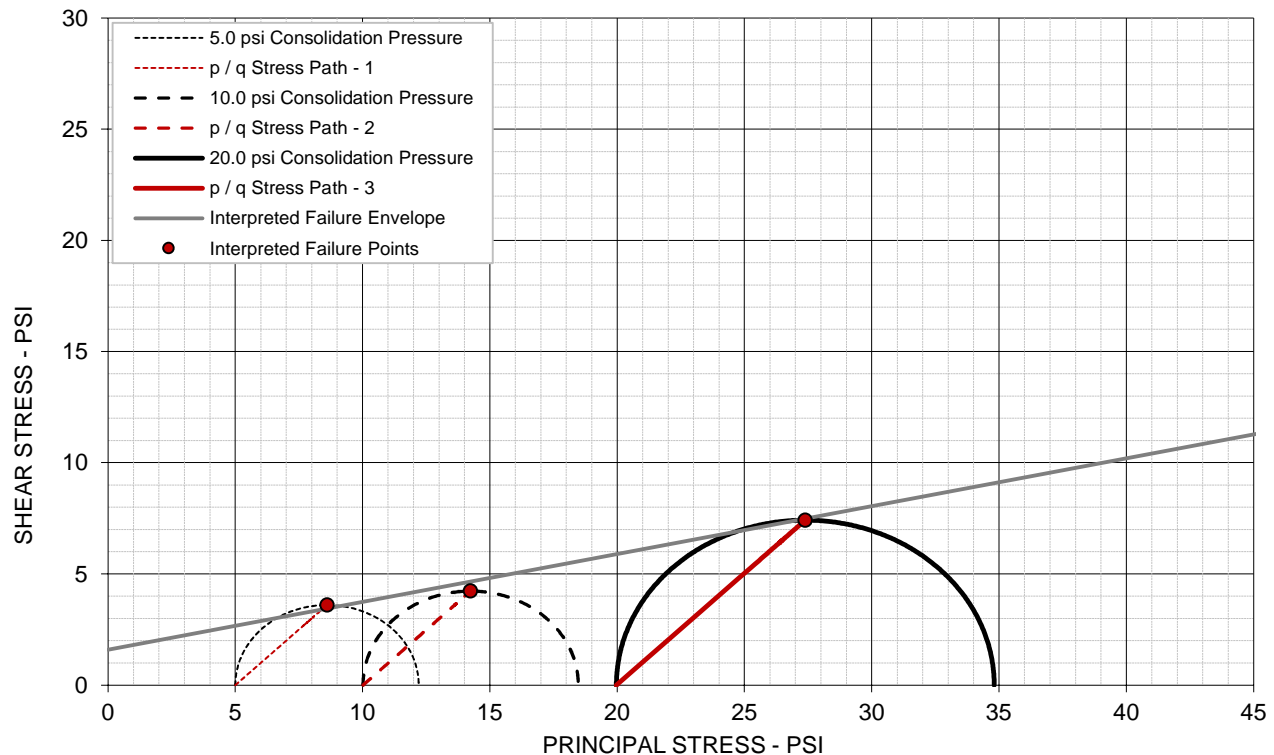


EFFECTIVE STRESS PARAMETERS		R ² = 1.00	α = 27.0 deg	a = 0.8 psi
PROJECT: S-46-998 BRO Wildcat Creek			ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION TEST	
LOCATION: York County, SC			CLIENT: HNTB	
SAMPLE ID: S-46-998-2 Bulk 0-5'			521 Clemson Road Columbia, SC	
DESCRIPTION: Sandy Lean Clay (CL) / A-6 (7)				
				

ICU TRIAXIAL COMPRESSION TEST

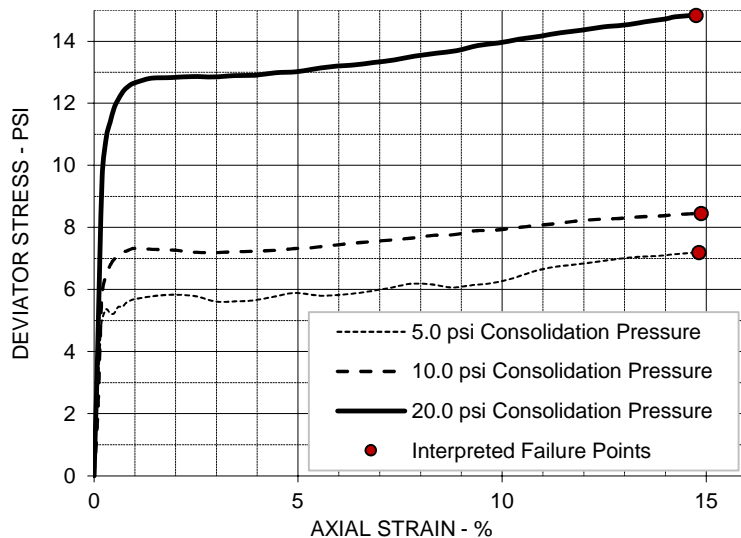
ASTM D4767 / AASHTO T297

Failure Criteria: Max Deviator Stress



TOTAL STRESS PARAMETERS

$\phi = 12.1$ deg $c = 1.6$ psi



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	17.0	17.0	17.0
Dry Density - pcf	100.6	100.6	100.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	5.99	6.00

AT TEST

Final Moisture - %	22.9	22.2	20.9
Dry Density - pcf	101.0	101.8	105.2
Calculated Diameter (in.)	2.83	2.83	2.81
Height - inches	5.94	5.91	5.88
Effect. Consol. Stress - psi	5.0	10.0	20.0
Failure Stress - psi	7.20	8.46	14.84
Total Pore Pressure - psi	83.3	87.4	94.5
Strain Rate - %/min.	0.0335	0.0336	0.0339
Failure Strain - %	14.8	14.9	14.8
σ_1 Failure - psi	12.20	18.47	34.81
σ_3 Failure - psi	5.00	10.01	19.97

TEST DESCRIPTION

ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION
 SAMPLE TYPE: Remolded
 DESCRIPTION: Sandy Lean Clay (CL) / A-6 (7)
 SAMPLE ID: S-46-998-2 Bulk 0-5'
 SPECIFIC GRAVITY: 2.65
 LL: 38 PL: 19 PI: 19 Percent -200: 53.9%
 Remarks: Remolded to 95% of the Standard Proctor

PROJECT INFORMATION

PROJECT: S-46-998 BRO Wildcat Creek
 LOCATION: York County, SC 7323P100
 PROJECT #: HNTB
 CLIENT: 07/12/23
 DATE:

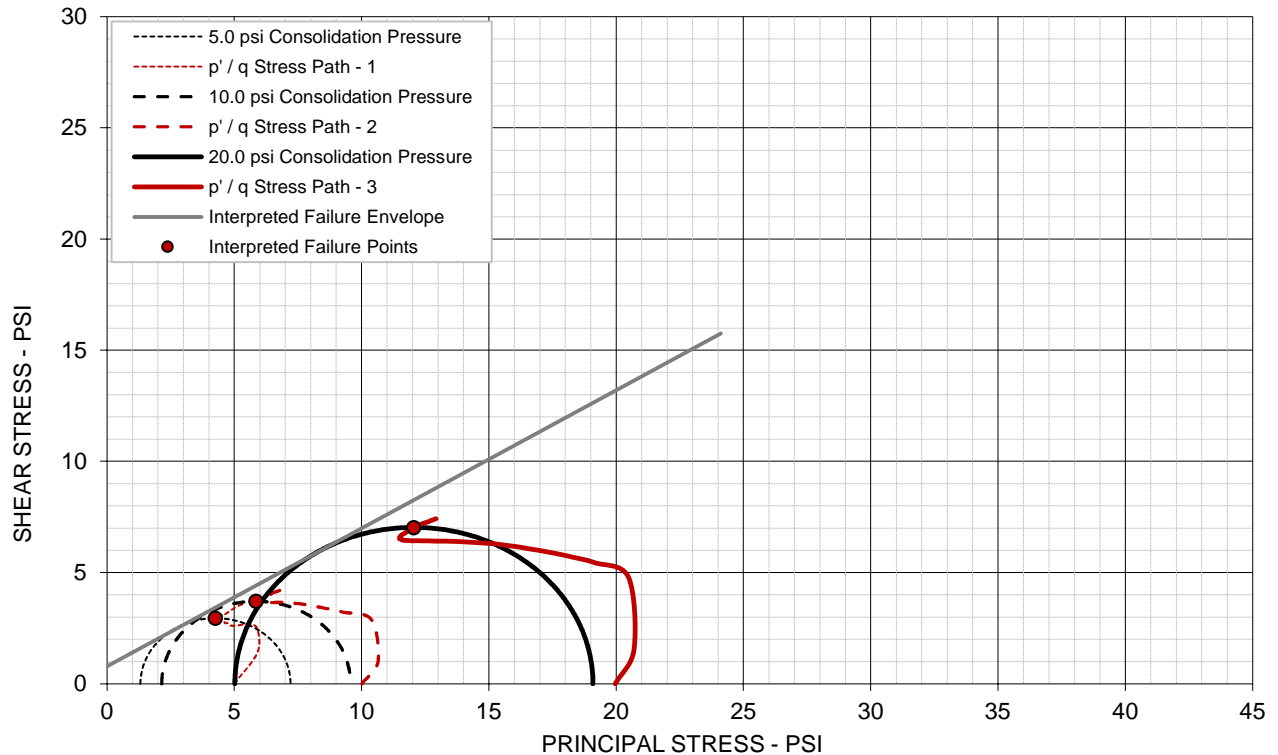
521 Clemson Road
 Columbia, SC



ICU TRIAXIAL COMPRESSION TEST

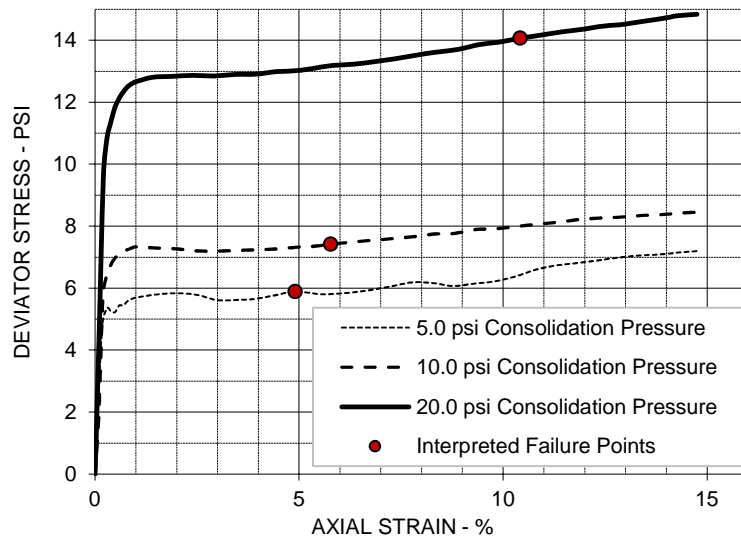
ASTM D4767 / AASHTO T297

Failure Criteria: Max Obliquity (s1': s3')



EFFECTIVE STRESS PARAMETERS

$\phi' = 31.8$ deg $c' = 0.8$ psi



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	17.0	17.0	17.0
Dry Density - pcf	100.6	100.6	100.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	5.99	6.00

AT TEST

Final Moisture - %	22.9	22.2	20.9
Dry Density - pcf	101.0	101.8	105.2
Calculated Diameter (in.)	2.83	2.83	2.81
Height - inches	5.94	5.91	5.88
Effect. Consol. Stress - psi	5.0	10.0	20.0
Failure Stress - psi	5.89	7.42	14.07
Total Pore Pressure - psi	83.7	87.9	95.0
Strain Rate - %/min.	0.0335	0.0336	0.0339
Failure Strain - %	4.9	5.8	10.4
σ_1' Failure - psi	7.20	9.57	19.09
σ_3' Failure - psi	1.31	2.15	5.02

TEST DESCRIPTION

ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION

SAMPLE TYPE: Remolded

DESCRIPTION: Sandy Lean Clay (CL) / A-6 (7)

SAMPLE ID: S-46-998-2 Bulk 0-5'

SPECIFIC GRAVITY: 2.65

LL: 38 PL: 19 PI: 19 Percent -200: 53.9%

Remarks: Remolded to 95% of the Standard Proctor

PROJECT INFORMATION

PROJECT: S-46-998 BRO Wildcat Creek

LOCATION: York County, SC 7323P100

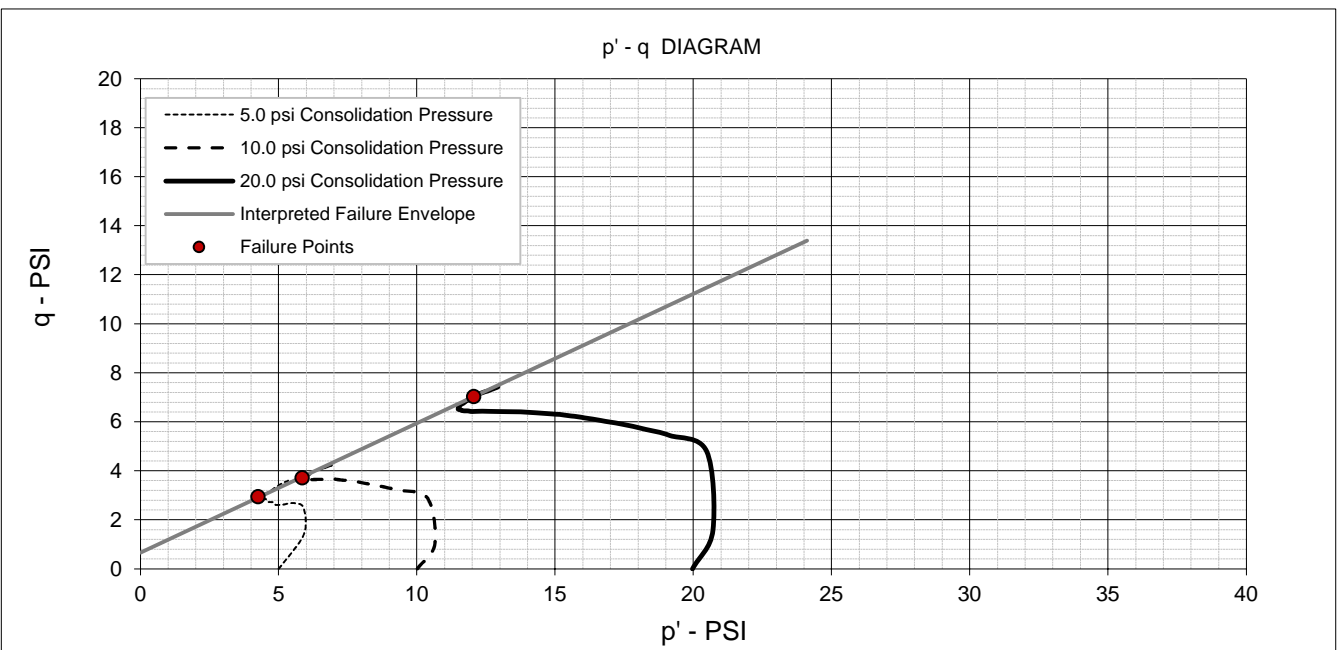
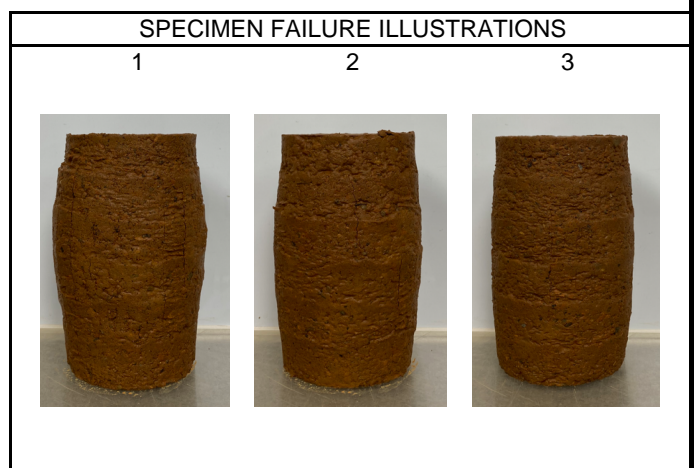
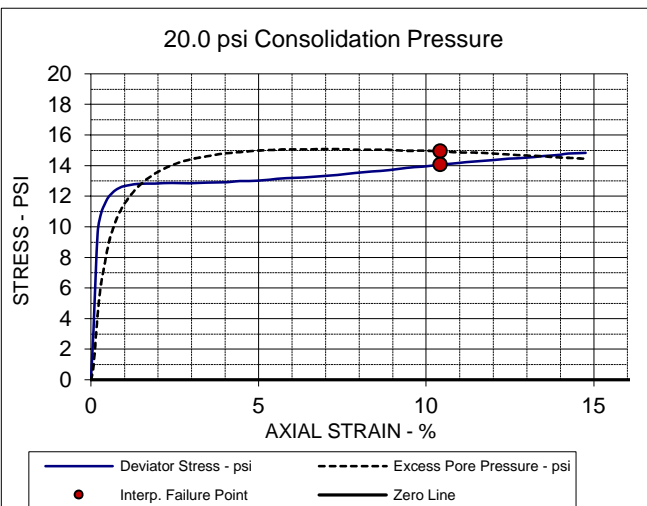
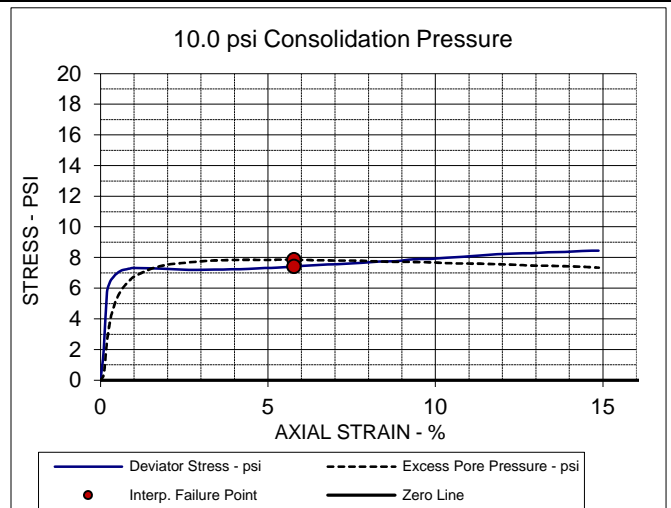
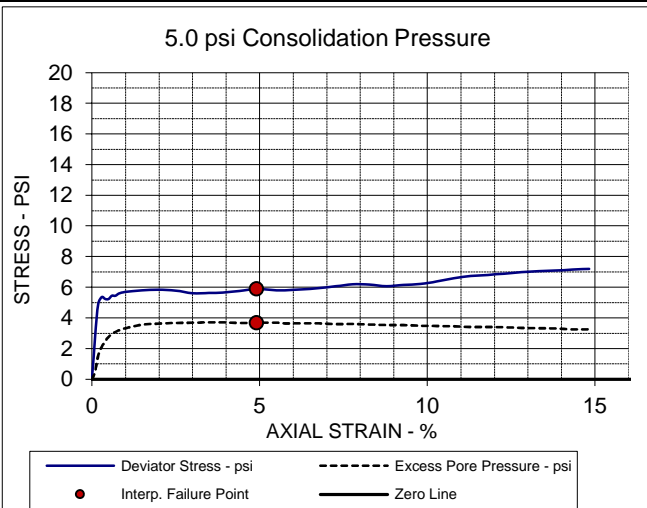
PROJECT #: HNTB


CLIENT: 07/12/23

DATE:

521 Clemson Road
Columbia, SC





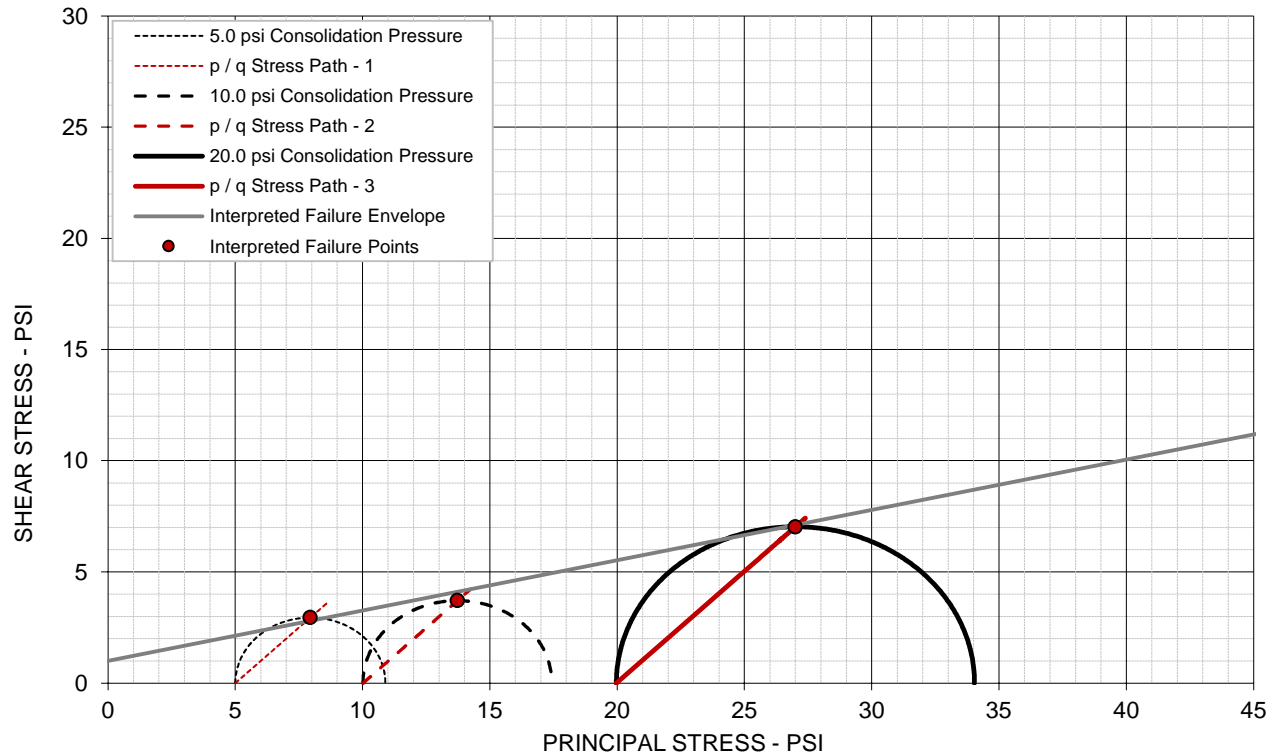
EFFECTIVE STRESS PARAMETERS		R ² = 1.00	α = 27.8 deg	a = 0.7 psi
PROJECT: S-46-998 BRO Wildcat Creek			ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION TEST	
LOCATION: York County, SC			CLIENT: HNTB	
SAMPLE ID: S-46-998-2 Bulk 0-5'			<div>521 Clemson Road Columbia, SC</div> <div></div>	
DESCRIPTION: Sandy Lean Clay (CL) / A-6 (7)				



ICU TRIAXIAL COMPRESSION TEST

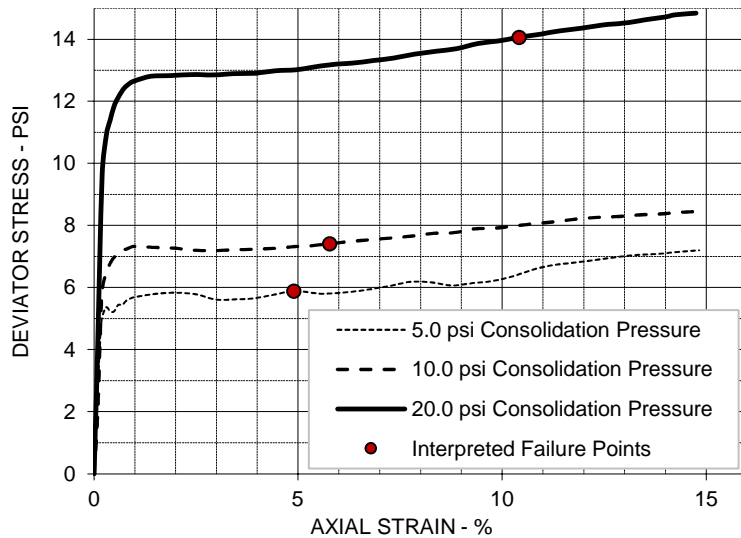
ASTM D4767 / AASHTO T297

Failure Criteria: Max Obliquity (s1': s3')



TOTAL STRESS PARAMETERS

$\phi = 12.8 \text{ deg}$ $c = 1.0 \text{ psi}$



SPECIMEN NO.

1 2 3

INITIAL

Moisture Content - %	17.0	17.0	17.0
Dry Density - pcf	100.6	100.6	100.6
Diameter - inches	2.86	2.86	2.86
Height - inches	6.00	5.99	6.00

AT TEST

Final Moisture - %	22.9	22.2	20.9
Dry Density - pcf	101.0	101.8	105.2
Calculated Diameter (in.)	2.83	2.83	2.81
Height - inches	5.94	5.91	5.88
Effect. Consol. Stress - psi	5.0	10.0	20.0
Failure Stress - psi	5.89	7.42	14.07
Total Pore Pressure - psi	83.7	87.9	95.0
Strain Rate - %/min.	0.0335	0.0336	0.0339
Failure Strain - %	4.9	5.8	10.4
σ_1 Failure - psi	10.89	17.43	34.03
σ_3 Failure - psi	5.00	10.01	19.97

TEST DESCRIPTION

ISOTROPICALLY CONSOLIDATED, UNDRAINED TRIAXIAL COMPRESSION
 SAMPLE TYPE: Remolded
 DESCRIPTION: Sandy Lean Clay (CL) / A-6 (7)
 SAMPLE ID: S-46-998-2 Bulk 0-5'
 SPECIFIC GRAVITY: 2.65
 LL: 38 PL: 19 PI: 19 Percent -200: 53.9%
 Remarks: Remolded to 95% of the Standard Proctor

PROJECT INFORMATION

PROJECT: S-46-998 BRO Wildcat Creek
 LOCATION: York County, SC 7323P100
 PROJECT #: HNTB
 CLIENT: 07/12/23
 DATE:

521 Clemson Road
 Columbia, SC



Client

HNTB North Carolina PC
Raleigh, NC

Project

S-46-998 BRO Wildcat Creek
7323P100

Date Received: 6/27/2023

Results from Corrosion Testing

Sample Location	S-46-998-2
Sample Depth (ft.)	0'-10'

pH Analysis, ASTM G 51	5.90
------------------------	------

Water Soluble Sulfate (SO ₄), ASTM D516-07 (mg/kg)	18
---	----

Chlorides, APHA 4500-Cl ⁻ E, (mg/kg)	96
---	----

Resistivity (Saturated), ASTM G 57, (ohm-cm)	2300
--	------

Analyzed By: Kyle Lemcke
Laboratory Manager

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.



PROJECT ID P041172 PROJECT NAME S-46-998 BRO Wildcat Creek
PROJECT COUNTY YORK

Borehole	Core Run Number	Core Run Top Depth	REC (%)	RQD (%)	q _u (psi)	Poisson's Ratio	Secant Modulus (ksi)	Unit Weight (pcf)	RMR	GSI
S-46-998-1	NQ-1	41.0	10	0						20
S-46-998-1	NQ-2	45.0	5	0						20
S-46-998-1	NQ-3	50.0	80	38	17860	0.06	1579	176	49	60
S-46-998-1	NQ-4	55.0	16	0						20
S-46-998-2	NQ-1	20.1	100	12						60
S-46-998-2	NQ-2	23.0	100	11	21775	0.21	1464	161	44	70
S-46-998-2	NQ-3	28.0	96	30	25106	0.18	1503	162	49	70

Report Number: 7323P100
Service Date: 06/02/23
Report Date: 07/10/23



Client

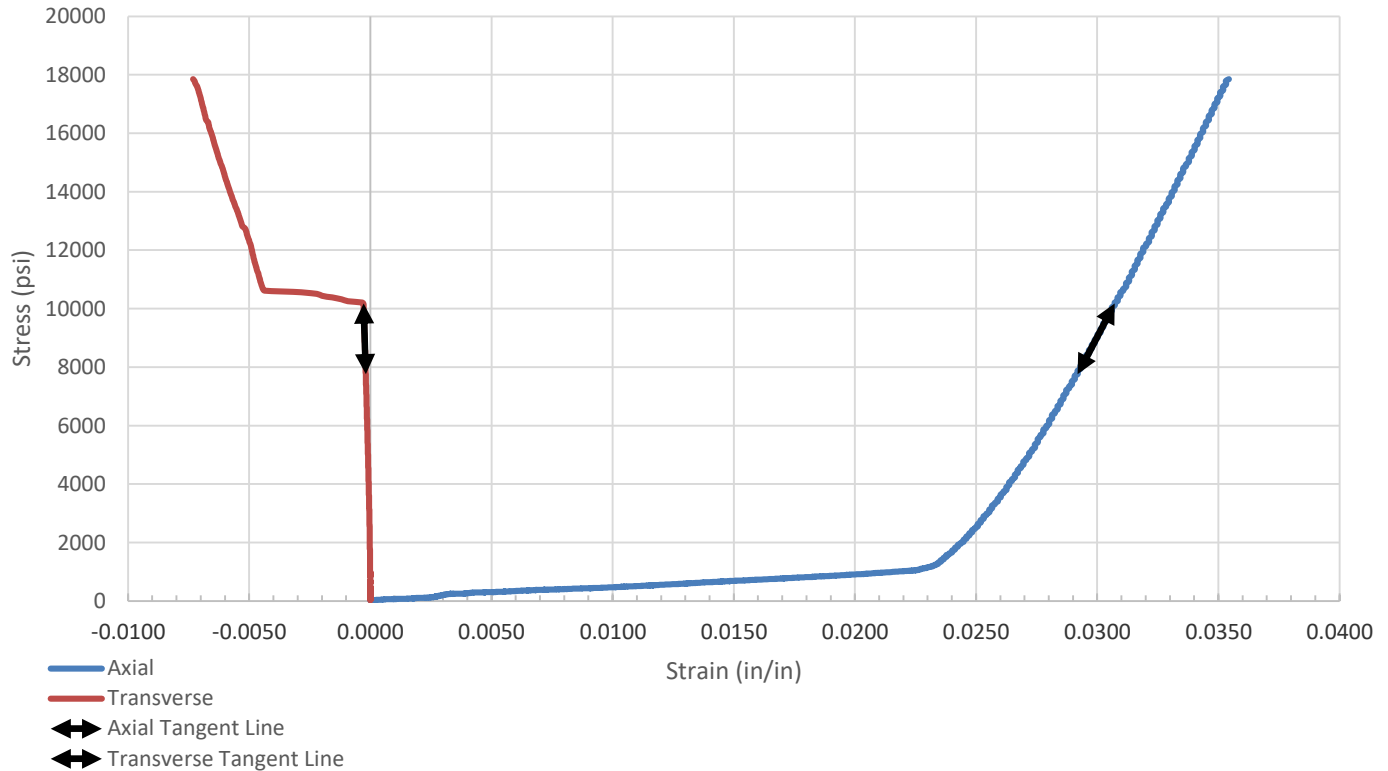
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	S-46-998		
Rock Type:	Diorite		
Boring:	S-46-998-1	Depth (feet):	50

SPECIMEN INFORMATION

Sample No.:	L-4088 (NQ -3)	Mass (g):	604.42
Length (in.):	4.27	Diameter (in.):	1.97
L/D Ratio:	2.2	Density (pcf):	176.110

TEST RESULTS

Failure Load (lbs):	54716
Failure Strain (%):	3.57
Unconfined Compressive Strength (psi):	17,860
Elastic Modulus, E, (ksi):	1579
Poisson's Ratio, ν :	0.061
Time of Failure (min):	03:58
Rate of Loading (psi/sec):	74.980
Moisture Content Post-break:	0.2%

Report Number: 7323P100

Service Date: 06/02/23

Report Date: 07/10/23

Client

HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D4543 Test Results:

<u>Parameter</u>	<u>Data</u>
Side Straightness:	0.0048
Perpendicularity Deviation:	
Diameter 1a:	0.0034
Diameter 1b:	0.0154
Diameter 2a:	0.0045
Diameter 2b:	0.0196
Max Deviation from Flatness:	0.0022
Parallelism Deviation:	
Diameter a:	0.04
Diameter b:	2.11

Equipment:

	TICCS ID:
Calipers:	W-44049
Scale:	B-71466
Dial Indicator:	C-70608
Compression (spherically seated):	C-48999

Samples were prepared and tested in accordance with ASTM D4543 and D7012.

Report Number: 7323P100
Service Date: 06/02/23
Report Date: 07/10/23



Client

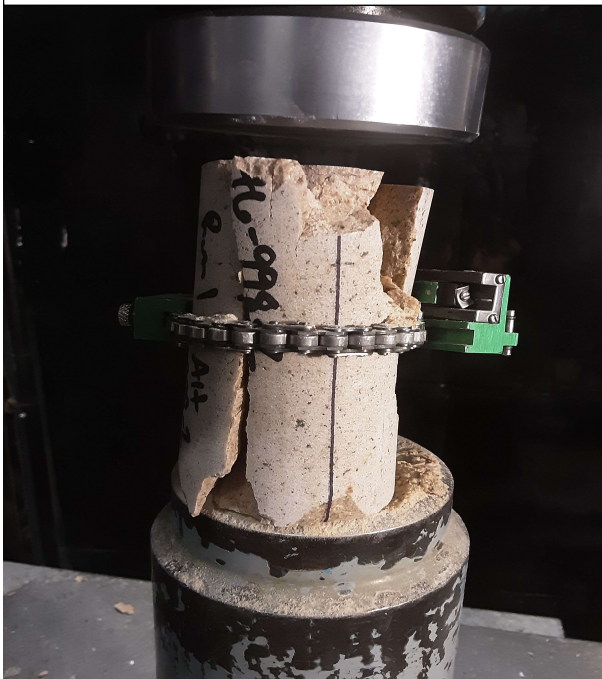
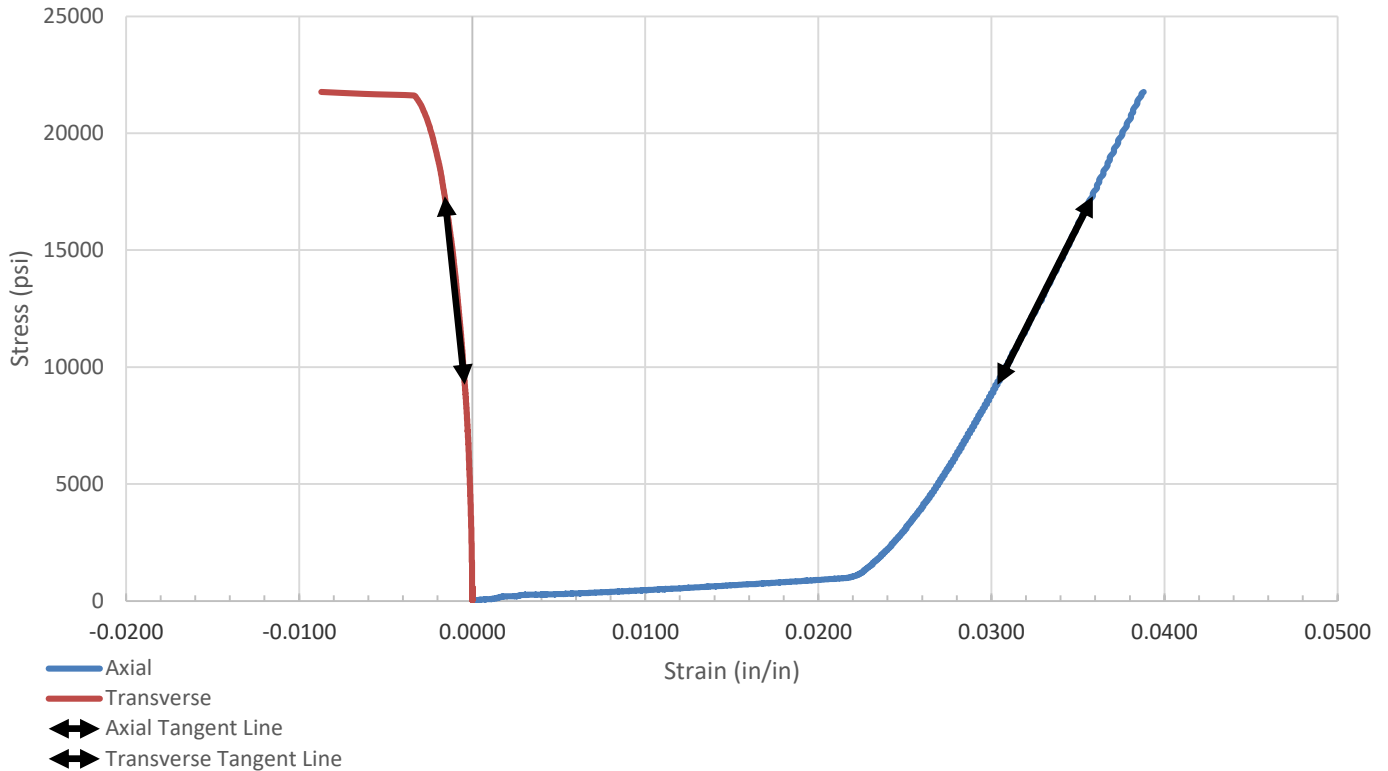
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	S-46-998		
Rock Type:	Diorite		
Boring:	S-46-998-2	Depth (feet):	20.1

SPECIMEN INFORMATION

Sample No.:	L-4089 (NQ -1)	Mass (g):	536.22
Length (in.):	4.107	Diameter (in.):	1.98
L/D Ratio:	2.1	Density (pcf):	161.212

TEST RESULTS

Failure Load (lbs):	67182
Failure Strain (%):	3.92
Unconfined Compressive Strength (psi):	21,775
Elastic Modulus, E, (ksi):	1464
Poisson's Ratio, u:	0.211
Time of Failure (min):	06:00
Rate of Loading (psi/sec):	60.419
Moisture Content Post-break:	0.2%

Report Number: 7323P100

Service Date: 06/02/23

Report Date: 07/10/23

Client

HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D4543 Test Results:

<u>Parameter</u>	<u>Data</u>
Side Straightness:	0.0101
Perpendicularity Deviation:	
Diameter 1a:	0.0116
Diameter 1b:	0.0225
Diameter 2a:	0.0116
Diameter 2b:	0.0204
Max Deviation from Flatness:	0.0044
Parallelism Deviation:	
Diameter a:	0.08
Diameter b:	2.75

Equipment:

	TICCS ID:
Calipers:	W-44049
Scale:	B-71466
Dial Indicator:	C-70608
Compression (spherically seated):	C-48999

Samples were prepared and tested in general accordance with ASTM D4543 and D7012.

Report Number: 7323P100
Service Date: 06/02/23
Report Date: 07/10/23



Client

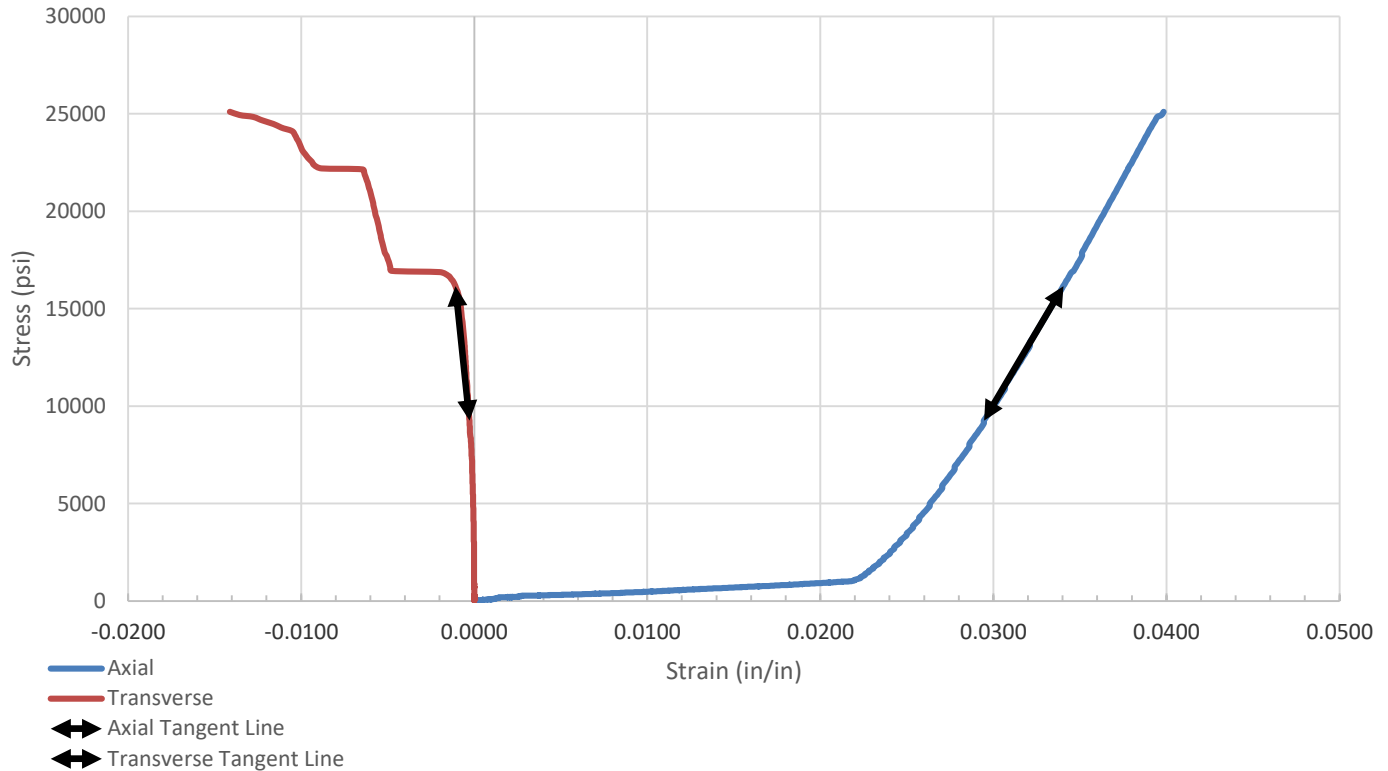
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	S-46-998		
Rock Type:	Diorite		
Boring:	S-46-998-2	Depth (feet):	23

SPECIMEN INFORMATION

Sample No.:	L-4090 (NQ-2)	Mass (g):	543.83
Length (in.):	4.164	Diameter (in.):	1.98
L/D Ratio:	2.1	Density (pcf):	161.752

TEST RESULTS

Failure Load (lbs):	77226
Failure Strain (%):	3.98
Unconfined Compressive Strength (psi):	25,106
Elastic Modulus, E, (ksi):	1503
Poisson's Ratio, ν :	0.177
Time of Failure (min):	03:01
Rate of Loading (psi/sec):	138.403
Moisture Content Post-break:	0.1%

Report Number: 7323P100

Service Date: 06/02/23

Report Date: 07/10/23

Client

HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D4543 Test Results:

<u>Parameter</u>	<u>Data</u>
Side Straightness:	0.0038
Perpendicularity Deviation:	
Diameter 1a:	0.0030
Diameter 1b:	0.0008
Diameter 2a:	0.0022
Diameter 2b:	0.0075
Max Deviation from Flatness:	0.0045
Parallelism Deviation:	
Diameter a:	0.08
Diameter b:	0.44

Equipment:

	TICCS ID:
Calipers:	W-44049
Scale:	B-71466
Dial Indicator:	C-70608
Compression (spherically seated):	C-48999

Samples were prepared and tested in general accordance with ASTM D4543 and D7012.

Report Number: 7323P100
Service Date: 06/02/23
Report Date: 07/10/23



Client

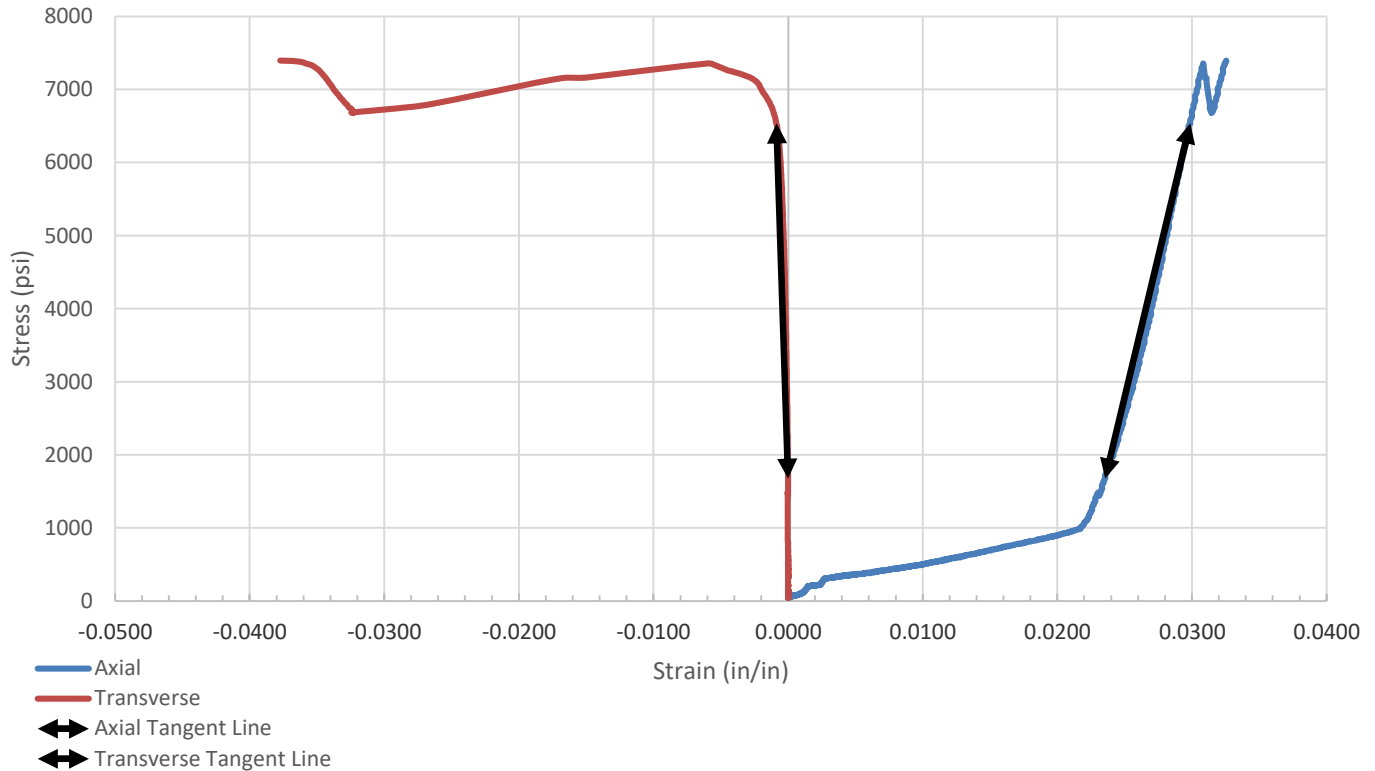
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	S-46-998		
Rock Type:	Diorite		
Boring:	S-46-998-2	Depth (feet):	28

SPECIMEN INFORMATION

Sample No.:	L-4091 (NQ -3)	Mass (g):	545.2
Length (in.):	4.14	Diameter (in.):	1.98
L/D Ratio:	2.1	Density (pcf):	162.196

TEST RESULTS

Failure Load (lbs):	22894
Failure Strain (%):	3.27
Unconfined Compressive Strength (psi):	7,398
Elastic Modulus, E, (ksi):	770
Poisson's Ratio, u:	0.136
Time of Failure (min):	03:48
Rate of Loading (psi/sec):	32.475
Moisture Content Post-break:	0.4%

Report Number: 7323P100

Service Date: 06/02/23

Report Date: 07/10/23

Client

HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 20

Project No. 7323P100

ASTM D4543 Test Results:

<u>Parameter</u>	<u>Data</u>
Side Straightness:	0.0032
Perpendicularity Deviation:	
Diameter 1a:	0.0068
Diameter 1b:	0.0114
Diameter 2a:	0.0077
Diameter 2b:	0.0113
Max Deviation from Flatness:	0.0144
Parallelism Deviation:	
Diameter a:	0.92
Diameter b:	0.88

Equipment:

	TICCS ID:
Calipers:	W-44049
Scale:	B-71466
Dial Indicator:	C-70608
Compression (spherically seated):	C-48999

Samples were prepared and tested in general accordance with ASTM D4543 and D7012.

Appendix C

Supporting Documents

3-Point Acceleration Design Response Spectrum by
SCDOT
Rig Calibration Report (14 Pages)

Note: All exhibits are one page unless noted above.

3-Point Acceleration Design Response Spectrum

SCDOT v3.2 - 06/01/2023

Project ID:	P041172	Latitude:	34.8895
Route:	S-46-998	County:	46 - York
Project:	Bridge Replacement over Wildcat Creek (Robertson Road)		
	Longitude:		

Designer:	N. Harman - Support
Date:	7/24/2023

Design EQ	PGA	S _{DS}	S _{D1}	M _W	R	PGV	D ₅₋₉₅	T _o
	g	g	g	-	km	inches/sec	sec	sec
FEE	0.06	0.15	0.01	7.30	178.37	0.48	45.74	0.17
SEE	0.12	0.28	0.03	7.39	186.47	1.06	48.30	0.16

Fundamental Period of Structure, T_0^*	Range of Interest		$V_{s,H}^*$	H	T_{NH}	
	sec				sec	
sec	0.5^*T_0	2.0^*T_0	ft/sec	ft	$(4^*H)/V_{s,H}^*$	$(6^*H)/V_{s,H}^*$
0.00	0.00	0.00	1380.72	52.00	0.10	0.23
0.00	0.00	0.00	H = B-C Boundary			

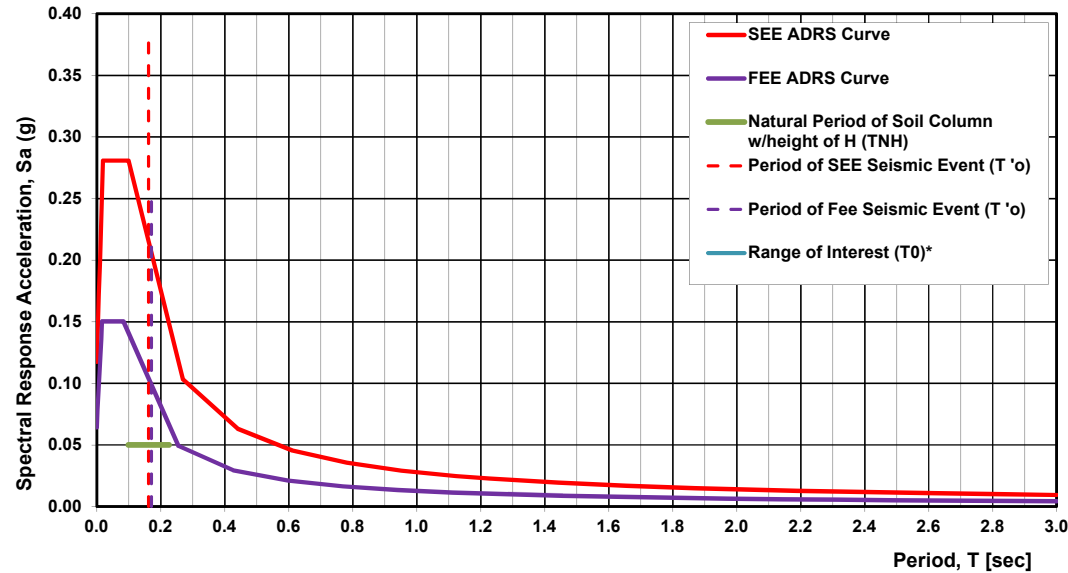
H = B-C Boundary

Damping:	5%
Geologic Condition:	Geologically Realistic (Q = 100)*
ADRS Location within Soil Column:	SCP
	At Ground Surface

South Carolina Piedmont

*Same Geologic Condition as used in SCENARIO_PC (2006)

SC Seismic ADRS Curve



FEE Data

T	S _a
0.00	0.064
0.00	0.078
0.01	0.093
0.01	0.107
0.01	0.121
0.01	0.136
0.02	0.150
0.02	0.150
0.03	0.150
0.03	0.150
0.04	0.150
0.04	0.150
0.05	0.150
0.06	0.150
0.06	0.150
0.07	0.150
0.07	0.150
0.08	0.150
0.08	0.150
0.25	0.049
0.43	0.029
0.60	0.021
0.77	0.016
0.94	0.013
1.11	0.011
1.28	0.010
1.46	0.009
1.63	0.008
1.80	0.007
1.97	0.006
2.14	0.006
2.31	0.005
2.49	0.005
2.66	0.005
2.83	0.004
3.00	0.004

SEE Data

T	S _a
0.00	0.117
0.00	0.145
0.01	0.172
0.01	0.199
0.01	0.226
0.02	0.254
0.02	0.281
0.03	0.281
0.03	0.281
0.04	0.281
0.05	0.281
0.06	0.281
0.07	0.281
0.07	0.281
0.08	0.281
0.09	0.281
0.09	0.281
0.10	0.281
0.27	0.103
0.44	0.063
0.61	0.046
0.78	0.036
0.95	0.029
1.12	0.025
1.29	0.021
1.46	0.019
1.63	0.017
1.81	0.015
1.98	0.014
2.15	0.013
2.32	0.012
2.49	0.011
2.66	0.010
2.83	0.010
3.00	0.009

SPT Automatic Hammer Energy Measurement Report

Drill Rig Model: Diedrich D-50
 Drill Rig Serial Number: D50-479
 Asset Number: DR#1109

July 3, 2023

July 03, 2023

Terracon Consultants Inc.
 521 Clemson Rd.
 Columbia, SC 29229

Attn: Chris Costner
 E: chris.costner@terracon.com

Re: SPT Automatic Hammer Energy Measurement Report
 Rig Serial Number: D50-479 Terracon Project Number: DUXX0500

Dear Mr. Costner:

This report provides the Energy Transfer Ratio (ETR) for the Standard Penetration Testing (SPT) automatic hammer as summarized below:

Table 1: Hammer Efficiency Summary

Drill Rig Make/Model	Drill Rig Serial Number	Drill Rig Year	Asset Number	Energy Transfer Ratio (ETR)	Hammer Efficiency Correction (Ce)
Diedrich D50	D50-479	2021	DR#1109	93.9% ± 2.3%	1.57

If you have any questions concerning this summary, or if we may be of further service, please contact us.

James P. Smith

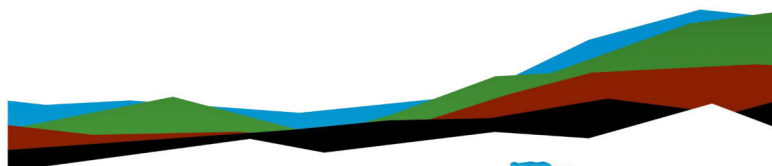
James P. Smith
 National Manager of Equipment & Training

Rob Kramer

Rob Kramer
 Geophysical Services Manager, COG

Attachments:

- Exhibit A: PDA SPT Analyzer Results
- Exhibit B: PDA Equipment Calibration



Prepared for:

Terracon Consultants, Inc.
 Columbia, South Carolina



Facilities | Environmental | **Geotechnical** | Materials |

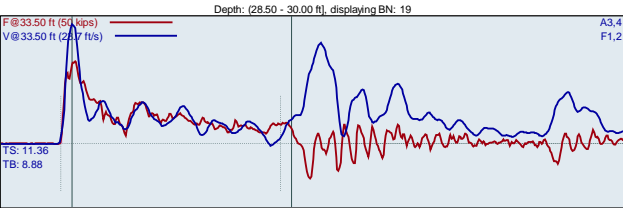


MEASUREMENT SUMMARY

ITEM	DESCRIPTION
Drill Rig Owner	Terracon Consultant, Inc. - Columbia, SC
Drill Rig Operator	Shiver Truesdale; Terracon Exploration
Testing Date	07/03/2023
Testing Location	Columbia, SC
Boring Identification	B-1
Hammer Type	140 pounds (automatic)
Boring Method	Rotary Wash
Drill Rods	<ul style="list-style-type: none"> ■ AWJ ■ 1-3/4" outside diameter ■ 3/16" wall thickness
Calibration Testing Equipment	<ul style="list-style-type: none"> ■ 2-foot AWJ rod instrumented w/ two strain gauges and two accelerometers ■ Model SPT Analyzer™ (PDA)
ASTM Methods Used	<p>ASTM D1586, Standard Test Method for Standard Penetration Test and Split-Barrel Sampling of Soils</p> <p>ASTM D4633-16, Standard Method for Energy Measurement for Dynamic Penetrometers</p>
SPT Calibration Personnel	Micah Hatch- Department Manager, Terracon Consultants, Inc.

Exhibit A

PDA SPT Analyzer Results



F1 : [512AWJ] 207.75 PDICAL (1) FF1
F2 : [512AWJ2] 208.76 PDICAL (1) FF1

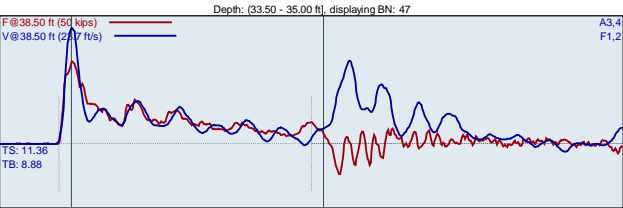
A3 (PR): [K5998] 403.535 mw/6.4v5000g (1) VF1
A4 (PR): [K10453] 411.89 mw/6.4v5000g (1) VF1

FMX: Maximum Force
VMX: Maximum Velocity
BPM: Blows/Minute

EFV: Maximum Energy
ETR: Energy Transfer Ratio - Rated

BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
3	6	32	23.8	48.1	315	89.9
4	6	34	23.8	47.8	319	91.3
5	6	32	23.5	48.4	316	90.4
6	6	32	23.3	47.8	324	92.5
7	6	32	23.0	48.3	325	92.8
8	6	32	22.2	48.2	316	90.4
9	6	32	22.0	47.7	324	92.7
10	6	32	22.7	48.5	324	92.7
11	6	32	21.6	47.9	326	93.3
12	6	31	21.3	48.4	326	93.1
13	9	32	21.7	47.7	329	94.1
14	9	32	22.1	48.0	326	93.3
15	9	32	21.8	47.9	331	94.7
16	9	31	21.7	48.2	329	93.9
17	9	32	21.8	48.1	334	95.4
18	9	33	22.2	47.5	338	96.5
19	9	32	22.1	48.4	340	97.1
20	9	32	22.2	48.1	326	93.2
21	9	32	22.7	47.8	332	94.8
Average		32	22.1	48.1	329	93.9
Std Dev		0	0.4	0.3	6	1.6
Maximum		33	23.0	48.5	340	97.1
Minimum		31	21.3	47.5	316	90.4
N-value: 15						

Sample Interval Time: 22.48 seconds.



F1 : [512AWJ] 207.75 PDICAL (1) FF1
F2 : [512AWJ2] 208.76 PDICAL (1) FF1

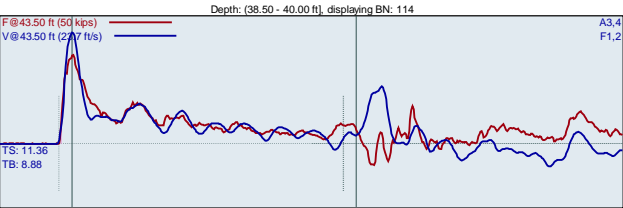
A3 (PR): [K5998] 403.535 mw/6.4v5000g (1) VF1
A4 (PR): [K10453] 411.89 mw/6.4v5000g (1) VF1

FMX: Maximum Force
VMX: Maximum Velocity
BPM: Blows/Minute

EFV: Maximum Energy
ETR: Energy Transfer Ratio - Rated

BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
23	7	32	19.5	48.3	314	89.7
24	7	31	19.4	48.4	305	87.0
25	7	31	19.9	48.1	311	88.9
26	7	31	19.4	48.2	310	88.6
27	7	31	19.5	48.0	311	88.8
28	7	31	19.9	48.1	310	88.7
29	9	31	19.6	48.0	314	89.8
30	9	32	19.8	48.3	314	89.7
31	9	30	19.2	48.0	309	88.3
32	9	31	19.7	48.2	313	89.5
33	9	31	19.6	47.9	313	89.5
34	9	30	19.3	48.0	310	88.7
35	9	32	21.2	48.3	319	91.2
36	9	32	21.3	48.3	321	91.7
37	9	33	22.0	48.1	327	93.5
38	12	32	20.4	48.2	302	86.2
39	12	32	21.6	47.9	319	91.2
40	12	32	21.1	47.9	321	91.7
41	12	31	20.4	48.0	311	88.9
42	12	31	20.9	48.1	324	92.6
43	12	32	21.8	48.0	322	92.1
44	12	32	22.3	48.1	323	92.3
45	12	32	21.4	48.1	317	90.6
46	12	32	21.8	48.2	323	92.4
47	12	32	21.5	48.2	322	92.0
48	12	33	22.0	48.2	320	91.4
49	12	32	21.8	47.7	322	92.1

Sample Interval Time: 32.40 seconds.



F1 : [512AWJ] 207.75 PDICAL (1) FF1
F2 : [512AWJ2] 208.76 PDICAL (1) FF1

A3 (PR): [K5998] 403.535 mw/6.4v5000g (1) VF1
A4 (PR): [K10453] 411.89 mw/6.4v5000g (1) VF1

FMX: Maximum Force
VMX: Maximum Velocity
BPM: Blows/Minute

EFV: Maximum Energy
ETR: Energy Transfer Ratio - Rated

BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
51	17	35	21.7	47.4	339	96.8
52	17	35	21.7	48.4	336	96.0
53	17	35	21.7	48.2	336	96.1
54	17	34	21.7	48.2	337	96.3
55	17	34	21.7	48.1	339	96.9
56	17	34	21.2	48.1	337	96.2
57	17	34	21.2	48.3	336	95.9
58	17	33	21.2	48.4	322	92.1
59	17	35	22.2	48.1	343	98.0
60	17	33	21.2	48.0	324	92.6
61	17	34	21.8	48.3	337	96.3
62	17	34	21.7	48.0	331	94.5
63	17	33	21.3	48.2	335	95.6
64	17	34	21.8	48.2	336	95.9
65	17	34	22.1	48.0	329	94.0
66	17	33	21.4	48.3	336	96.1
67	19	34	21.7	48.5	331	94.7
68	19	33	21.3	47.8	336	95.9
69	19	33	21.2	48.2	344	98.4
70	19	35	22.9	47.7	344	98.4
71	19	34	22.1	48.1	339	96.9
72	19	33	20.9	48.3	333	95.0
73	19	33	20.8	48.1	331	94.4
74	19	34	20.7	47.9	329	94.1
75	19	34	20.9	48.2	331	94.6
76	19	34	21.1	47.7	343	97.9
77	19	35	20.9	48.3	332	94.9
78	19	35	21.1	48.3	336	96.1
79	19	34	20.5	48.1	338	96.5
80	19	34	20.6	48.4	338	96.7
81	19	34	20.4	48.3	326	93.2

82	19	33	20.2	48.0	336	96.0
83	19	34	20.3	48.3	333	95.1
84	19	33	20.1	47.7	325	92.8
85	19	33	19.9	48.4	330	94.4
86	31	35	20.5	47.9	332	94.9
87	31	34	20.5	48.1	330	94.3
88	31	33	20.3	47.8	324	92.5
89	31	33	20.8	48.3	336	95.9
90	31	33	20.6	48.0	331	94.6
91	31	34	20.4	48.4	338	96.7
92	31	33	20.2	48.5	329	94.0
93	31	34	20.6	48.0	336	96.1
94	31	34	20.7	48.6	334	95.3
95	31	34	20.5	48.6	334	95.4
96	31	34	20.3	48.5	331	94.5
97	31	33	20.3	48.4	331	94.6
98	31	34	20.3	48.5	332	94.9
99	31	33	20.5	48.4	333	95.2
100	31	33	20.0	48.0	338	96.5
101	31	34	21.1	48.3	332	94.8
102	31	33	20.0	48.1	334	95.4
103	31	34	20.2	48.8	329	93.9
104	31	33	20.0	48.3	331	94.4
105	31	33	20.1	48.1	330	94.3
106	31	35	20.4	47.9	334	95.5
107	31	34	20.2	48.3	331	94.5
108	31	34	20.1	48.0	335	95.7
109	31	34	20.0	48.3	327	93.3
110	31	34	20.0	47.9	330	94.2
111	31	34	20.3	47.6	331	94.6
112	31	34	20.5	47.2	333	95.1
113	31	35	20.4	47.6	336	95.9
114	31	35	20.6	47.7	335	95.7
115	31	34	20.6	47.2	335	95.7
116	31	34	20.6	47.2	339	96.7
Average			34	20.6	48.1	333
Std Dev			1	0.6	0.4	4
Maximum			35	22.9	48.8	344
Minimum			33	19.9	47.2	324
N-value: 50						

Sample Interval Time: 81.06 seconds.

Summary of SPT Test Results

Project: Dietrich D50 (SN 479), Test Date: 7/3/2023						EFV: Maximum Energy ETR: Energy Transfer Ratio - Rated				
FMX: Maximum Force VMX: Maximum Velocity BPM: Blows/Minute										
Test Length ft		Blows Applied /ft	N Value	N50 Value	Average FMX kips	Average VMX ft/s	Average BPM bpm	Average EFV ft-lb	Average ETR %	
33.50		6-6-9	15	23	32	22.1	48.1	329	93.9	
38.50		7-9-12	21	32	32	20.9	48.1	318	90.7	
43.50		17-19-31	50	78	34	20.6	48.1	333	95.2	
Overall Average Values:						33	20.9	48.1	328	93.9
Standard Deviation:						1	0.9	0.3	8	2.3
Overall Maximum Value:						35	23.0	48.8	344	98.4
Overall Minimum Value:						30	19.2	47.2	302	86.2



Exhibit B

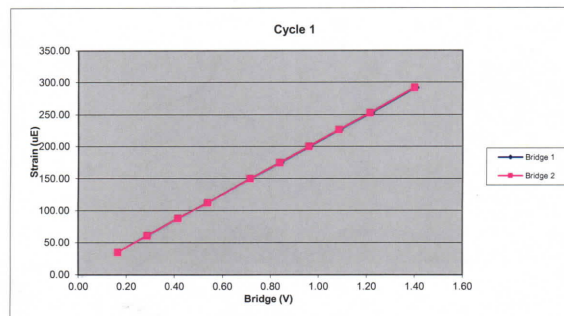
PDA Equipment Calibration



512AWJ		Cycle 1		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1203.06	34.31	0.16	0.16
3	2126.16	60.40	0.29	0.29
4	3077.44	86.97	0.42	0.42
5	3982.41	111.72	0.54	0.54
6	5285.39	149.30	0.72	0.72
7	6200.50	174.57	0.84	0.84
8	7099.62	199.59	0.96	0.96
9	8021.85	226.03	1.09	1.08
10	8981.90	252.42	1.22	1.21
11	10350.08	291.62	1.40	1.40

Bridge 1		Bridge 2	
Force Calibration (lb/V)	7371.63	Force Calibration (lb/V)	7404.01
Offset	-2.95	Offset	-5.32
Correlation	1.000000	Correlation	0.999999
Strain Calibration (µE/V)	207.13	Strain Calibration (µE/V)	208.04
Offset	0.34	Offset	0.27
Correlation	0.999991	Correlation	0.999992

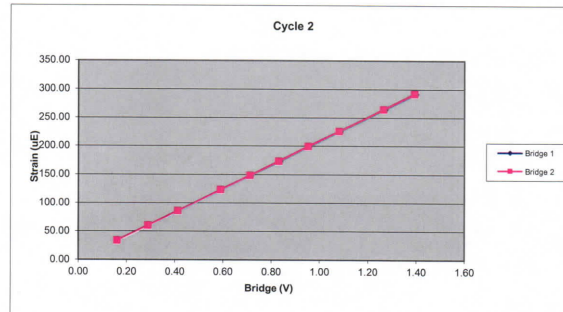
Force Strain Calibration	
EA (Kips)	35589.20
Offset	-14.99
Correlation	0.999992



512AWJ		Cycle 2		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1195.16	33.02	0.16	0.16
3	2140.49	59.36	0.29	0.29
4	3060.77	84.68	0.41	0.41
5	4361.31	122.48	0.59	0.59
6	5276.03	147.78	0.71	0.71
7	6152.73	172.65	0.83	0.83
8	7048.15	198.82	0.96	0.95
9	8008.49	225.14	1.08	1.08
10	9364.20	264.06	1.27	1.26
11	10320.35	291.14	1.40	1.39

Bridge 1		Bridge 2	
Force Calibration (lb/V)	7383.19	Force Calibration (lb/V)	7408.85
Offset	1.99	Offset	1.61
Correlation	0.999999	Correlation	1.000000
Strain Calibration (µE/V)	209.13	Strain Calibration (µE/V)	209.86
Offset	-1.28	Offset	-1.29
Correlation	0.999988	Correlation	0.999991

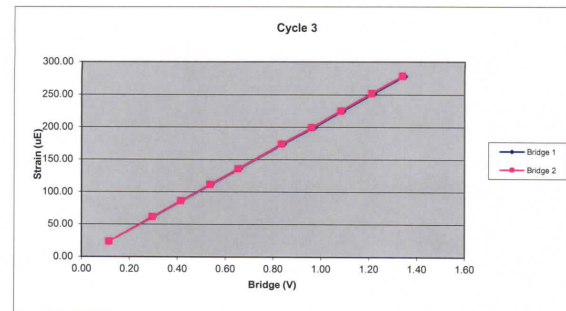
Force Strain Calibration	
EA (Kips)	35302.85
Offset	47.38
Correlation	0.999989



512AWJ		Cycle 3		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	843.37	22.72	0.11	0.11
3	2199.17	60.67	0.30	0.29
4	3069.54	85.62	0.42	0.41
5	3979.10	110.64	0.54	0.54
6	4849.18	135.11	0.66	0.65
7	6197.28	173.33	0.84	0.84
8	7134.13	198.98	0.97	0.96
9	8033.64	224.83	1.09	1.08
10	8976.83	251.64	1.22	1.21
11	9937.94	277.86	1.35	1.34

Bridge 1		Bridge 2	
Force Calibration (lb/V)	7369.64	Force Calibration (lb/V)	7419.12
Offset	-2.56	Offset	0.17
Correlation	0.999999	Correlation	0.999999
Strain Calibration (µE/V)	206.99	Strain Calibration (µE/V)	208.38
Offset	-1.03	Offset	-0.95
Correlation	0.999995	Correlation	0.999995

Force Strain Calibration	
EA (Kips)	35602.66
Offset	34.21
Correlation	0.999994



Bridge Excitation (V) 5
Shunt Resistor (ohm) 60.4k

Calibration Factors		512AWJ	
Bridge 1 (µE/V)	207.75	Bridge 2 (µE/V)	208.76
EA Factor (Kips)	35498.24	Area (in ²)	1.18

Calibrated by: Sam Davis
Calibrated Date: 8/31/2022

Pile Dynamics Inc
30725 Aurora Rd
Solon, OH 44139

Traceable to N.I.S.T.

Accelerometer Calibration Certificate
Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.
Calibration performed on 03Aug2022

Serial No: K5998 Temperature: 74.7 °F
Model: PR Humidity: 53%
Calibrated on: Channel 3 on 8G 5161 LE

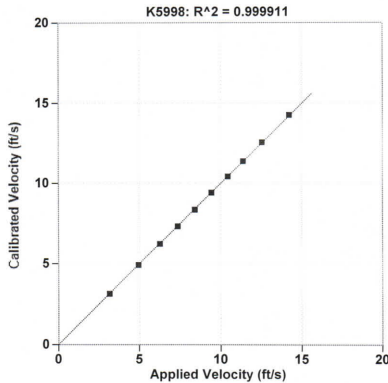
PDA CALIBRATION FACTOR
403.5 mv/5000g
(80.7 μ v/g)
R²: 0.999911 [Chip programmed]

Operator: William Johnson

Signed

Ref Acc 1: 72505! Cal on: 24Mar2022
1035 g/s/volt
Ref Acc 2: 72517! Cal on: 24Mar2022
1049 g/s/volt

Reference accelerometer calibrations are traceable to
the United States National Institute of Standards and
Technology (NIST).



Date printed: 03Aug2022, version: 2020.30.170 -1.89

Accelerometer Calibration Certificate
Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.
Calibration performed on 03Aug2022

Serial No: K10493 Temperature: 74.7 °F
Model: PR Humidity: 53%
Calibrated on: Channel 3 on 8G 5161 LE

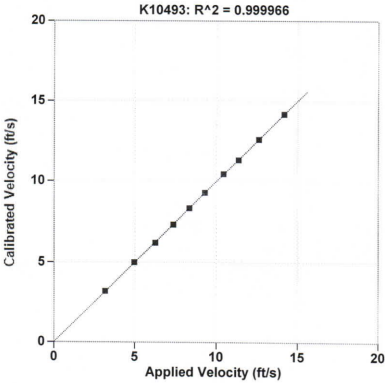
PDA CALIBRATION FACTOR
411.9 mv/5000g
(82.4 μ v/g)
R²: 0.999966 [Chip programmed]

Operator: William Johnson

Signed

Ref Acc 1: 72505! Cal on: 24Mar2022
1035 g/s/volt
Ref Acc 2: 72517! Cal on: 24Mar2022
1049 g/s/volt

Reference accelerometer calibrations are traceable to
the United States National Institute of Standards and
Technology (NIST).



Date printed: 03Aug2022, version: 2020.30.170 -1.65



Report of SPT Energy Measurements HPC CME-550X ATV (Serial No. 308513) Charlotte, North Carolina S&ME Project No. 22350319



April 22, 2022

North Carolina Department of Transportation
Geotechnical Engineering Unit
1589 Mail Service Center
Raleigh, North Carolina 27699

c/o HPC Land Services
490 South First Street
PO Box 155
Albemarle, North Carolina 28002

Attention: Shunyi (Chris) Chen, Ph.D., P.E.

Cc: Ms. Cheryl A. Youngblood, L.G.
Ms. Christina M. Bruinsma, L.G.

Reference: **Report of SPT Energy Measurements
HPC CME-550X ATV (Serial No. 308513)**
Charlotte, North Carolina
S&ME Project No. 22350319
NC PE Firm License No. F-0176

Dear Dr. Chen:

We have completed the Standard Penetration Test (SPT) energy measurements on the automatic hammer used with HPC Land Services' (HPC) CME-550X ATV-mounted drill rig (Serial No. 308513). This service was performed by Mr. Alex Rodriguez, E.I. of our firm on April 19, 2022, in general accordance with ASTM D4633 and the most recent revision of the North Carolina Department of Transportation (NCDOT) Geotechnical Engineering Unit's requirements. Review of the data quality and analyses was performed by Mr. Joseph Williamson, P.E. Copies of the Certificates of Proficiency issued by Pile Dynamics based on the Dynamic Measurement and Analysis Proficiency Test for Mr. Rodriguez and Mr. Williamson are included in Appendix I. The testing procedures, equipment used during testing, and detailed results are presented in this report.

1.0 Dynamic Testing Methodology

Testing was performed using a model PAX (Serial No. 3733L) Pile Driving Analyzer™ (PDA) manufactured by Pile Dynamics, Inc. The PDA was used to record and interpret data from two piezoresistive accelerometers (Serial Nos. K4644 and K4665) bolted to a 2.0-foot-long AWJ drill rod (Serial No. 203) internally instrumented with two strain transducers. Calibration sheets for the accelerometers and the instrumented rod are included in Appendix II. The instrumented AWJ drill rod has a cross-sectional area of 1.19 square inches and an outside diameter of approximately 1.75 inches. Therefore, we calculate the inside diameter to be approximately 1.25 inches at the gauge location. The accelerometers and strain gauges, which are diametrically opposed near the middle of the instrumented rod, monitor acceleration and strain for each hammer blow. The analyzer converts the data to velocities and forces and computes the maximum transferred hammer energies with the "EFV" method described in ASTM D4633. Preliminary results are recorded and displayed in real time for each blow.

S&ME, Inc. | 9751 Southern Pine Boulevard | Charlotte, NC 28273 | p 704.523.4726 | www.smeinc.com



Report of SPT Energy Measurements
HPC CME-550X ATV (Serial No. 308513)
Charlotte, North Carolina
S&ME Project No. 22350319

2.0 Testing and Observations

S&ME personnel were on site April 19, 2022, to observe and perform high-strain dynamic testing during SPT sampling on the CME-550X ATV-mounted drill rig operated by Zack Kiker of HPC. The measurements were taken during drilling and sampling of test hole at an HPC project site in Charlotte, North Carolina. SPT energy measurements were recorded during five sampling intervals at depths of approximately 33.5, 38.5, 43.5, 48.5, and 53.5 ft below the ground surface. The 38.5 ft sample interval did not meet the NCDOT required blow count criteria and was not considered in the analysis. The information presented in the tables below summarizes the equipment and tooling used during the SPT energy measurements.

Table 2-1: Drill Rig Information

Manufacturer	CME
Model	550X
Serial Number	308513
Operator	Z. Kiker
Carrier	ATV

Table 2-2: Hammer Information

Model / Type	CME / Auto
Serial Number	308513
Typical Drop Height (inches)	30
Typical Ram Weight (pounds)	140

Table 2-3: Drilling and Instrumented Rod Information

Drill Rod Type	AWJ
OD (inches)	1.75
ID (inches)	1.25
Cross-Sectional Area (in ²)	1.19
Typical Lengths (feet)	5
Instrumented Rod Type	AWJ (Serial No. 203)
OD (inches)	1.75
ID (inches)	1.25
Cross-Sectional Area (in ²)	1.19
Total Instrumented Rod Length (feet)	2.0
Length Below Gages (feet)	0.8
Split-Spoon Length (feet)	2.85



Report of SPT Energy Measurements
HPC CME-550X ATV (Serial No. 308513)
Charlotte, North Carolina
S&ME Project No. 22350319

3.0 Dynamic Testing Results

The total rod length from the instrumentation to the tip of the split-spoon sampler was determined by adding 3.65 ft to the drill rod length at each sample depth. The SPT Energy Measurement Data Summary tables in Appendix III present the test data from every hammer blow at each sampling interval, along with representative force and velocity traces for each test interval. Per ASTM D4633, only the blows from the final foot of each sample interval (i.e. the blows that determine the N-value) are considered when computing the average measurement values of each test interval.

The reported blow counts obtained by the drill rig personnel, a summary of the test data, and average computed hammer energy and transfer ratio values are provided in Table 3-1. Based on the test data, the automatic hammer on the CME-550X operated at an average rate of about 55 blows per minute (bpm) during dynamic testing. The measured average transferred hammer energy (EFV) of the four sample intervals ranged from 292 to 297 ft-lbs, which corresponds to Energy Transfer Ratio (ETR) values of 83.5 to 85.0%, respectively. Plots and tables of the following are also included in Appendix III and present the test data with depth for each test interval:

- Penetration vs. BLC¹
- Penetration vs. CSX⁴
- ETR vs. Rod Length
- Penetration vs. FMX²
- Penetration vs. VMX³
- Average ETR vs. Rod Length
- Penetration vs. EFV³
- Penetration vs. ETR⁶

Table 3-1: Summary of Dynamic Testing Results

Data Set ID	Sample Depth (ft)	Drill Rod Length (ft)	Instrumentation to Sampler Tip Length (ft)	Blows per 6" Increment / N-value	Soil Sample Description (Piedmont Residual)	Avg. BPM	Avg. EFV (ft-lbs)	Avg. ETR (%)
1	33.5 - 35	35	38.65	3-5-8 / 13	Sandy Silt	55	297	84.8
2	43.5 - 45	45	48.65	12-20-24 / 44	Sandy Silt	55	297	85.0
3	48.5 - 50	50	53.65	10-22-26 / 48	Sandy Silt	55	295	84.4
4	53.5 - 55	55	58.65	7-14-20 / 34	Sandy Silt	55	292	83.5
Overall Average						55	295	84.4

The overall average transferred hammer energy for the automatic hammer on the CME-550X ATV-mounted drill rig was 295 foot-pounds, with an average ETR of 84.4%.

¹ BLC - Blow Count per 6-in. increment

² FMX - Maximum Compressive Force
³ EFV - Maximum Transferred Energy

⁴ CSX - Maximum Compressive Stress
⁵ VMX - Maximum Velocity

⁶ ETR - Energy Transfer Ratio - Ratio of Calculated Energy to Theoretical Energy of 140 lb hammer falling 30 inches



4.0 Limitations of Report

This report has been prepared in accordance with generally accepted geotechnical engineering practice for specific application to this project. The conclusions contained in this report were based on the applicable standards of our profession in this geographic area at the time this report was prepared. No other warranty, express or implied, is made.

5.0 Closing

S&ME appreciates the opportunity to provide this report to the North Carolina Department of Transportation, Geotechnical Engineering Unit. Please let us know if you have any questions concerning this report.

Sincerely,

S&ME, Inc.

Alex M. Rodriguez, E.I.
Staff Professional



Joseph R. Williamson, P.E.
Apr 22 2022

Joseph R. Williamson, P.E.
Project Manager
N.C. Registration No. 42168

Senior Reviewed by: Gregory J. Canivan, P.E.

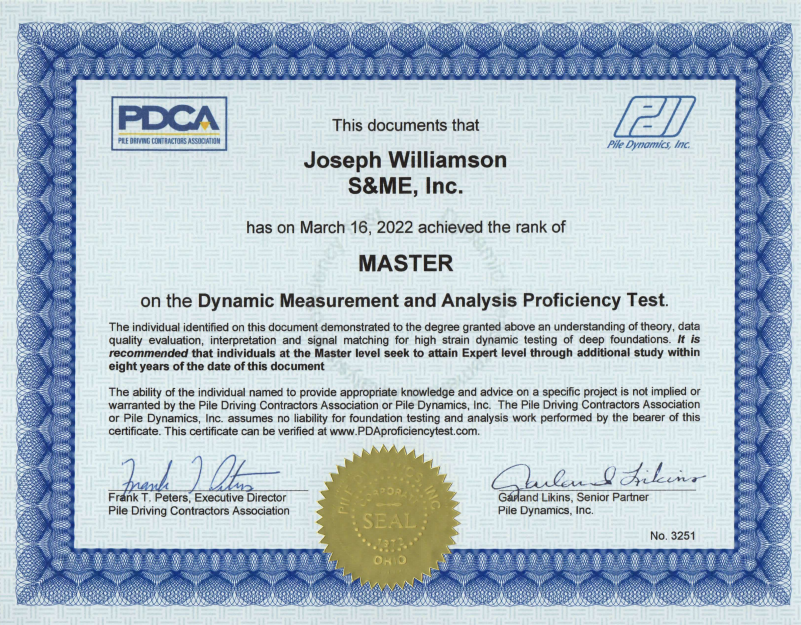
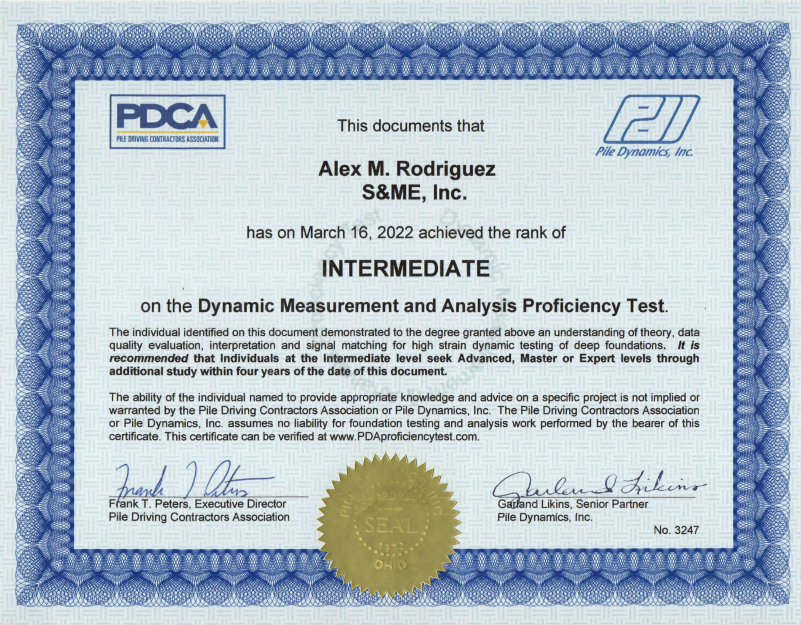
Appendices:

- Appendix I – Certificates of Proficiency
- Appendix II – Instrumented Rod and Accelerometer Calibration Sheets
- Appendix III – SPT Energy Evaluation Form
- Appendix IV – CME-550X ATV (SN 308513) SPT Energy Measurements Summary Plots and Tables

Appendix I – Certificates of Proficiency

April 22, 2022

4





Quality Assurance for Deep Foundations

PDI Certificate of Calibration

PDI Accelerometer Calibration

Model PR-K Serial # K4664Cal Date: OCT 16, 2021Cal Due: OCT 15, 2023Temperature: 72 deg. FHumidity: 65 %Calibrated at
Pile Dynamics, Inc.
30725 Aurora Road
Cleveland, OH 44139

Manufactured by Pile Dynamics, Inc.

Procedure used: PDA Accelerometer Calibration Procedure 2016-6, Revision 20160422

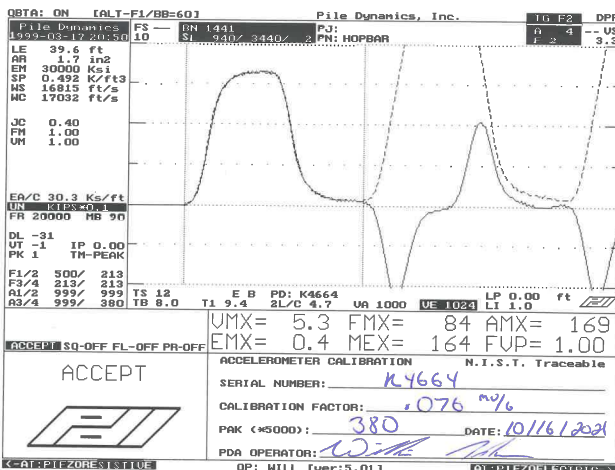
Equipment was found to be

X in tolerance As Received _____ out of tolerance As ReceivedX in tolerance As Returned _____ out of tolerance As ReturnedPDI Calibration: 380 mv/5000gCalibration Standards Utilized
Hopkinson Bar Force Calibration F2, verified on July 28, 2021

PDI HopBar DOS PAK, serial number 1273K, verified on July 19, 2021

Calibration performed by: William Johnson
William Johnson, TechnicianReviewed by: Robert Sprenger
Robert Sprenger, Production Manager

Accelerometer CC-5 Issued 20160426

30725 Aurora Road • Cleveland, Ohio 44139 USA • +1-216-831-6131 • Fax +1-216-831-0916
E-mail: info@pile.com • www.pile.com

Smart Sensor

Programmed By: WJCRC Value 4898

Quality Assurance for Deep Foundations

PDI Certificate of Calibration

PDI Accelerometer Calibration

Model PR-K Serial # K4665Cal Date: AUG 10, 2021Cal Due: AUG 9, 2023Temperature: 72 deg. FHumidity: 70 %Calibrated at
Pile Dynamics, Inc.
30725 Aurora Road
Cleveland, OH 44139

Manufactured by Pile Dynamics, Inc.

Procedure used: PDA Accelerometer Calibration Procedure 2016-6, Revision 20160422

Equipment was found to be

X in tolerance As Received _____ out of tolerance As ReceivedX in tolerance As Returned _____ out of tolerance As ReturnedPDI Calibration: 365 mv/5000gCalibration Standards Utilized
Hopkinson Bar Force Calibration F2, verified on July 28, 2021

PDI HopBar DOS PAK, serial number 1273K, verified on July 19, 2021

Calibration performed by: William Johnson
William Johnson, TechnicianReviewed by: Bob Sprenger
Bob Sprenger, production Manager

Accelerometer CC-5 Issued 20160426

30725 Aurora Road • Cleveland, Ohio 44139 USA • +1-216-831-6131 • Fax +1-216-831-0916
E-mail: info@pile.com • www.pile.com

DATA: ON [ALT-F1/BB=60] Pile Dynamics, Inc. 10 1.2 3.3
F1: 10 F2: 10 F3: 10 F4: 10 F5: 10 F6: 10 F7: 10 F8: 10 F9: 10 F10: 10
LE 39.6 ft BR 1.7 in2 EM 30000 Ksi SP 0.492 K/ft3 MS 16815 ft/s MC 16851 ft/s
JC 0.40 FM 1.00 UM 1.00
EA/C 30.3 Ks/ft FR 20000 RB 90
DL -44 UT -1 IP 0.00 PK 1 TH-PEAK
F1/2 500/ 213 F3/4 213/ 213 A1/2 999/ 999 TS 12 E 8 PD: K4665 UA 1000 U: 1021 LP 0.00 ft LI 1.0
A3/4 999/ 363 TB 8.0 T1 9.6 2L/C 4.7
UMX= 4.9 FMX= 75 AMX= 169
EMX= 0.3 MEX= 147 FUP= 0.98
ACCEPT
ACCELEROMETER CALIBRATION N.I.S.T. Traceable
SERIAL NUMBER: K4665
CALIBRATION FACTOR: 0.073 %/g
PAK (#5000): 365 DATE: 8/10/2021
PDA OPERATOR: [Signature]
OP: WILL [Signature] 5.013

Smart Sensor

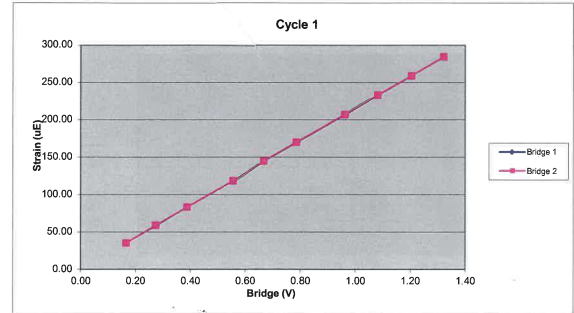
Programmed By: WJS

CRC Value CS9F

203AWJ		Cycle 1		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1246.03	34.91	0.17	0.17
3	2081.11	58.65	0.28	0.27
4	2941.63	83.02	0.39	0.39
5	4214.76	118.53	0.56	0.56
6	5056.27	144.80	0.67	0.67
7	5965.04	170.09	0.79	0.79
8	7294.42	207.16	0.96	0.96
9	8206.79	233.22	1.08	1.08
10	9138.76	259.03	1.21	1.20
11	10040.70	284.32	1.32	1.32

Bridge 1	Bridge 2
Force Calibration (lb/V)	7599.69
Force Calibration (lb/V)	7601.86
Offset	-24.15
Offset	-17.41
Correlation	0.999998
Correlation	0.999998
Strain Calibration (µE/V)	215.94
Strain Calibration (µE/V)	216.00
Offset	-0.96
Offset	-0.77
Correlation	0.999977
Correlation	0.999970

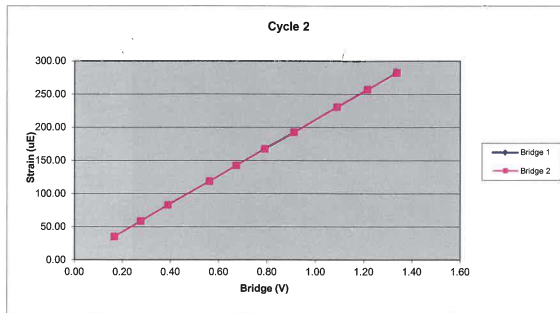
Force Strain Calibration	
EA (Kips)	35191.11
Offset	10.07
Correlation	0.999966



203AWJ		Cycle 2		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1258.51	34.91	0.17	0.17
3	2095.24	58.34	0.28	0.28
4	2959.17	82.77	0.39	0.39
5	4259.41	118.34	0.56	0.56
6	5099.16	142.25	0.67	0.67
7	6003.44	167.19	0.79	0.79
8	6914.94	192.10	0.91	0.91
9	8280.29	230.44	1.09	1.09
10	9224.63	256.64	1.22	1.21
11	10143.64	281.95	1.33	1.34

Bridge 1	Bridge 2
Force Calibration (lb/V)	7599.73
Force Calibration (lb/V)	7596.59
Offset	-6.06
Offset	-5.65
Correlation	0.999998
Correlation	0.999998
Strain Calibration (µE/V)	211.24
Strain Calibration (µE/V)	211.15
Offset	0.01
Offset	0.02
Correlation	0.999992
Correlation	0.999991

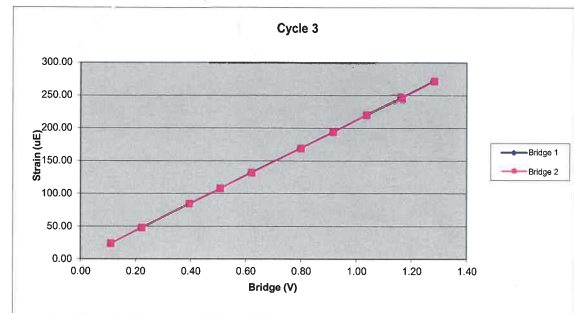
Force Strain Calibration	
EA (Kips)	35976.99
Offset	-6.36
Correlation	0.999996



203AWJ		Cycle 3		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	846.19	23.53	0.11	0.11
3	1695.30	47.55	0.22	0.22
4	3007.23	84.21	0.40	0.40
5	3855.56	107.72	0.51	0.51
6	4714.72	131.93	0.62	0.62
7	6082.40	169.34	0.80	0.80
8	6969.72	194.06	0.92	0.92
9	7881.90	220.26	1.04	1.04
10	8834.83	246.28	1.16	1.17
11	9750.91	271.69	1.28	1.28

Bridge 1	Bridge 2
Force Calibration (lb/V)	7615.81
Force Calibration (lb/V)	7596.50
Offset	-9.63
Offset	-6.69
Correlation	0.999996
Correlation	0.999998
Strain Calibration (µE/V)	212.11
Strain Calibration (µE/V)	211.57
Offset	0.00
Offset	0.08
Correlation	0.999995
Correlation	0.999995

Force Strain Calibration	
EA (Kips)	35905.21
Offset	-9.52
Correlation	0.999994



Bridge Excitation (V) 5
Shunt Resistor (ohm) 60.4k

Calibration Factors	203AWJ		
Bridge 1 ($\mu\text{E/V}$)	213.10	Bridge 2 ($\mu\text{E/V}$)	212.91
EA Factor (Kips)	35691.10	Area (in^2)	1.19

Calibrated by: *Alex Rodriguez*
Calibrated Date: 3/25/2021

Pile Dynamics Inc
30725 Aurora Rd
Solon, OH 44139

Traceable to N.I.S.T.

Appendix III - SPT Energy Evaluation Form



SPT Energy Evaluation Form

Project: HPC SPT Energy Testing
Project No.: 22350319
Boring No.: Test Hole
Date: 4/19/2022
Weather: Sunny 60°
Drill Rod Type: AWJ S' LONG

On-site Personnel

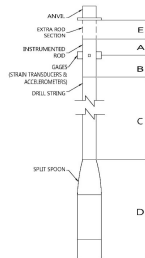
Drilling Company: HPC
Rig Operator: Zack Kiker
Engr/Geologist: -
Client Rep.: -
Analyzer Oper.: Alex Rodriguez

Rig/Hammer Info

Drill Rig Make/Model: CME 550X
Carrier Type: ATV
Rig Serial No.: 308513
Hammer Type/Model: CME 550X
Hammer Serial No.: N/A
Hammer Drop System: Automatic
Lubrication Condition: Per Manufacturer
Operation Rate (bpm): 50 - 55
Typical Drop Height (in.): 30
Typical Hammer Weight (lbs): 140
Anvil Dimension (in.): 12
Drilling Method: 2 1/4" HSA
Drop Height in Tolerance (y/n): Y

Rod Info

(A) + (B) Instr. Rod Length: 2.0 ft
(B) Instr. Rod Length below Gages: 0.8 ft
(D) Spoon Length: 2.85 ft
(E) Rod Length Above Instr. Rod (if applicable): - ft
(B)+(C)+(D) (LE) - Length Below Gages (Q + 3.65 ft)
Instr. Rod S/N: 203AWJ
Instr. Rod Outside Dia.: 1.75 in.
Instr. Rod Area: 1.19 in²
PDA Make/Model: PDI/PAX
PDA Serial No.: 3733L
Calib. Pulse Test (y/n): Y



Gage Info

Gage	Serial No.	Calibration No.
Accel. A3	K4664	380
A4	K4665	365
Strain F3	AWJ203-1	213.1
F4	AWJ203-2	212.91

Appendix IV - SPT Energy Measurements Plots and Tables

Date of Test	Test Depth Increment	Test Time Start / Stop	Length of Drill String (ft)	(LE) Length below Gages (ft)	Avg. Meas. Hammer Rate (BPM)	SPT Blow Counts						AASHTO / USCS Classification
	(ft to ft)	(military)	(C)	(B) + (C) + (D)	(BPM)	Total Blows by PDA	6"	12"	18"	N-Value	Extra Blows	
4/19/2022	33.5 - 35.0	8:50	35	38.65	55	16	3	5	8	13	0	A-4/ML
	38.5 - 40.0	8:56	40	43.65	55	78	20	28	30	58	0	A-4/ML
	43.5 - 45.0	9:02	45	48.65	55	56	12	20	24	44	0	A-4/ML
	48.5 - 50.0	9:10	50	53.65	55	58	10	22	26	48	0	A-4/ML
	53.5 - 55.0	9:20	55	58.65	55	41	7	14	20	34	0	A-4/ML

Notes:

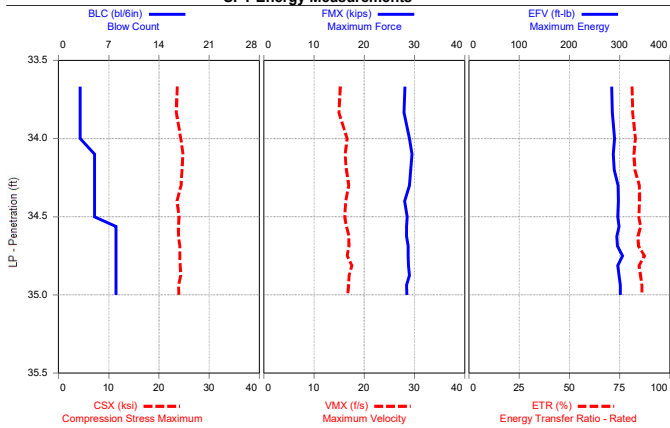
NOTE: (1) Note any unusual hammer operating conditions that affect the hammer performance, or changes in operating conditions (e.g. verticality, weather, or lubrication between trials). (2) Note any changes in rod diameter along drill string and record locations of short rod sections.

Alex Rodriguez
Prepared By (print/signature)

4/19/2022
Date



CME 550X (SN308513) - 33.5 - 35.0 Feet
SPT Energy Measurements



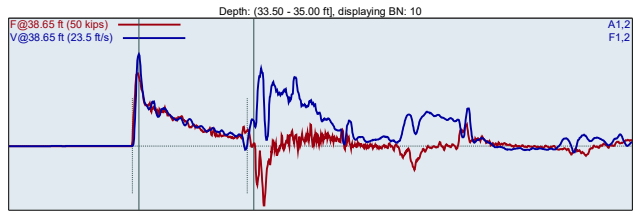
CME 550X (SN308513)
AMR

SPT Energy Measurements
Test date: 4/19/2022

TEST HOLE

AR: 1.19 in²
LE: 38.65 ft
WS: 16807.9 fts

SP: 0.492 k/ft³
EM: 30000 ksi



F1 : [203 AWJ-1] 213.1 PDICAL (1) FF1
F2 : [203 AWJ-2] 212.91 PDICAL (1) FF1

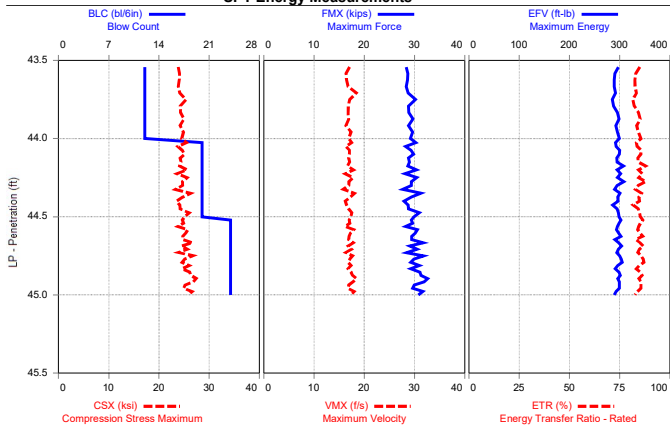
A1 (PR): [K4664] 380 mm/6.4v/5000g (1) VF1
A2 (PR): [K4665] 365 mm/6.4v/5000g (1) VF1

BPM: Blows/Minute							CSX: Compression Stress Maximum			
FMX: Maximum Force							DFN: Final Displacement			
VMX: Maximum Velocity							EFV: Maximum Energy			
DMX: Maximum Displacement							ETR: Energy Transfer Ratio - Rated			
BL#	LP	BC	BPM	FMX	VMX	DMX	CSX	DFN	EFV	ETR
	ft	/6"	bpm	kips	ft/s	in	ksi	in	ft-lb	%
1	33.67	3	1.9	28	15.2	2.10	23.6	2.00	284	81.2
2	33.83	3	55.3	28	15.0	2.00	23.4	2.00	285	81.5
3	34.00	3	55.1	29	16.6	2.00	24.3	2.00	290	82.8
4	34.10	5	55.1	29	16.2	1.68	24.8	1.20	287	82.1
5	34.20	5	54.9	29	16.4	1.48	24.6	1.20	289	82.6
6	34.30	5	55.1	29	16.9	1.32	24.4	1.20	297	84.8
7	34.40	5	54.8	28	16.3	1.26	23.6	1.20	297	84.9
8	34.50	5	54.9	29	16.1	1.20	24.0	1.20	296	84.6
9	34.56	8	55.0	28	16.4	0.94	23.9	0.75	299	85.4
10	34.63	8	54.9	28	16.9	0.89	23.9	0.75	294	84.1
11	34.69	8	54.9	29	17.0	0.91	24.1	0.75	295	84.4
12	34.75	8	54.9	29	16.6	0.91	24.1	0.75	306	87.3
13	34.81	8	54.9	29	17.5	0.75	24.2	0.75	296	84.7
14	34.88	8	54.9	29	17.0	0.78	24.3	0.75	299	85.4
15	34.94	8	55.0	28	16.9	0.75	23.9	0.75	301	86.1
16	35.00	8	55.1	29	16.7	0.82	24.0	0.75	301	86.0
Average			55.0	29	16.7	1.05	24.1	0.92	297	84.8
Std Dev			0.1	0	0.4	0.29	0.3	0.22	5	1.3
Maximum			55.1	29	17.5	1.68	24.8	1.20	306	87.3
Minimum			54.8	28	16.1	0.75	23.6	0.75	287	82.1
N-value: 13										

Sample Interval Time: 16.34 seconds.



CME 550X (SN308513) - 43.5 - 45.0 Feet
SPT Energy Measurements



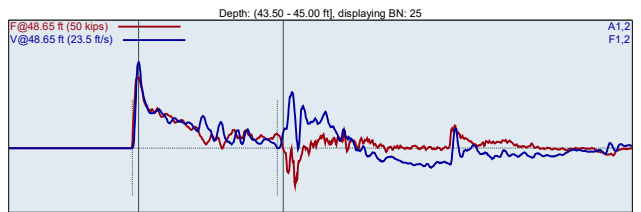
CME 550X (SN308513)
AMR

SPT Energy Measurements
Test date: 4/19/2022

TEST HOLE

AR: 1.19 in²
LE: 48.65 ft
WS: 16807.9 fts

SP: 0.492 k/ft³
EM: 30000 ksi

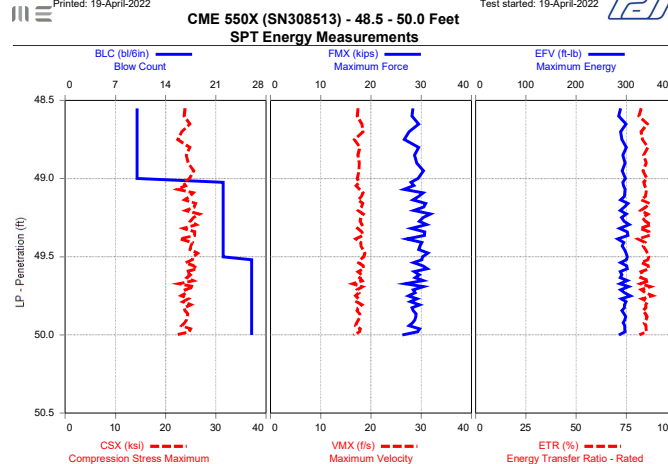


F1 : [203 AWJ-1] 213.1 PDICAL (1) FF1
F2 : [203 AWJ-2] 212.91 PDICAL (1) FF1

A1 (PR): [K4664] 380 mm/6.4v/5000g (1) VF1
A2 (PR): [K4665] 365 mm/6.4v/5000g (1) VF1

BL#	LP	BC	BPM	FMX	VMX	DMX	CSX	DFN	EFV	ETR			
	ft	/6"	bpm	kips	ft/s	in	ksi	in	ft-lb	%			
1	43.54	12	1.9	28	17.1	0.85	23.8	0.50	298	85.1			
2	43.58	12	57.5	29	16.4	0.71	24.1	0.50	291	83.0			
3	43.63	12	55.0	29	16.5	0.88	24.0	0.50	289	82.6			
4	43.67	12	55.2	28	16.8	0.61	23.8	0.50	289	82.7			
5	43.71	12	55.1	29	18.4	0.56	24.1	0.50	292	83.4			
6	43.75	12	54.8	30	17.1	0.51	25.4	0.50	285	81.5			
7	43.79	12	55.1	29	16.8	0.53	24.2	0.50	288	82.3			
8	43.83	12	55.2	29	16.8	0.55	24.3	0.50	295	84.4			
9	43.88	12	55.0	30	16.9	0.54	25.0	0.50	299	85.3			
10	43.92	12	54.9	29	16.4	0.53	24.2	0.50	292	83.5			
11	43.96	12	55.0	30	17.4	0.51	24.9	0.50	295	84.4			
12	44.00	12	55.1	29	16.9	0.55	24.5	0.50	299	85.5			
13	44.03	20	55.1	30	17.5	0.49	25.5	0.30	292	83.3			
14	44.05	20	55.1	28	16.3	0.50	23.8	0.30	293	83.7			
15	44.08	20	54.9	29	17.1	0.49	24.7	0.30	300	85.7			
16	44.10	20	55.2	30	17.0	0.49	25.1	0.30	299	85.5			
17	44.13	20	54.9	29	16.9	0.47	24.2	0.30	294	84.1			
18	44.15	20	54.8	29	17.1	0.47	24.4	0.30	295	84.3			
19	44.18	20	54.9	29	16.7	0.50	24.1	0.30	308	87.9			
20	44.20	20	54.8	30	17.9	0.44	25.5	0.30	295	84.3			
21	44.23	20	55.0	28	16.2	0.48	23.6	0.30	302	86.2			
22	44.25	20	54.8	30	17.8	0.43	25.6	0.30	296	84.4			
23	44.28	20	54.8	29	17.0	0.48	24.7	0.30	308	88.1			
24	44.30	20	54.8	29	16.9	0.44	24.7	0.30	295	84.2			
25	44.33	20	55.6	28	15.8	0.44	23.3	0.30	290	82.8			
26	44.35	20	53.9	31	18.1	0.43	26.3	0.30	301	85.9			
27	44.38	20	55.3	29	17.2	0.43	24.7	0.30	296	84.6			
28	44.40	20	54.7	28	16.2	0.45	23.5	0.30	297	84.9			
29	44.43	20	55.0	29	16.5	0.41	24.1	0.30	286	81.7			
30	44.45	20	54.8	29	16.9	0.41	24.2	0.30	295	84.3			
31	44.48	20	54.9	31	17.6	0.42	26.0	0.30	297	84.9			

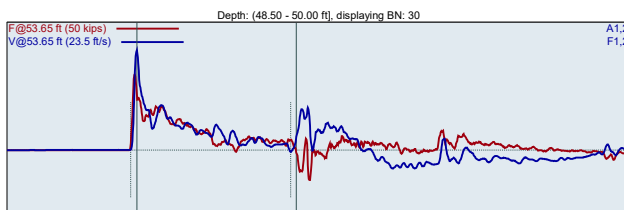
Sample Interval Time: 59.93 seconds



SPT Energy Measurements

Test date: 4/19/2022

EM: 30000 ksi

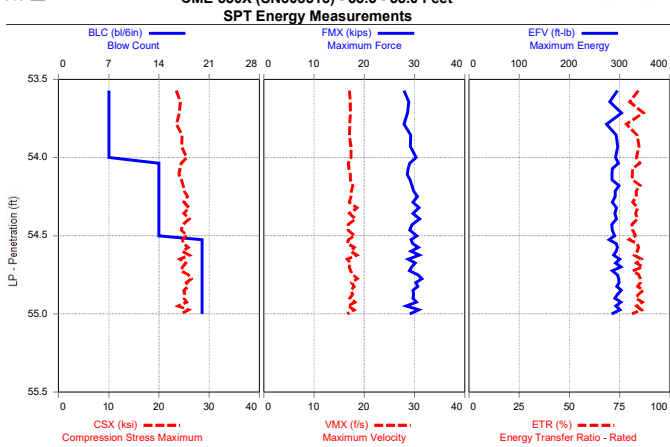


A1 (PR): [K4664] 380 mv/6.4v/5000g (1) VF1
A2 (PR): [K4665] 365 mv/6.4v/5000g (1) VF1

BL#	LP r/s	BC r/s	BPM bpm	FMX kbit	VMX r/s	DMX n	CSX ksi	DFN	EFV ft/lb	ET%
1	48.55	10	1.9	28	17.4	1.01	23.9	0.60	289	82.5
2	48.60	10	57.4	28	17.3	0.96	23.7	0.60	285	81.3
3	48.65	10	55.2	30	18.2	0.78	24.8	0.60	299	85.5
4	48.70	10	56.0	28	18.4	0.62	23.2	0.60	288	82.4
5	48.75	10	55.2	29	16.7	0.60	22.4	0.60	291	83.1
6	48.80	10	56.0	30	17.8	0.63	24.8	0.60	300	85.7
7	48.85	10	55.1	29	17.4	0.60	24.1	0.60	293	83.5
8	48.90	10	55.6	29	17.7	0.60	24.5	0.60	297	84.9
9	48.95	10	55.7	30	17.6	0.62	25.6	0.60	292	83.3
10	49.00	10	55.7	29	17.3	0.64	24.7	0.60	298	85.2
11	49.02	22	55.4	28	17.6	0.49	23.5	0.27	293	83.6
12	49.05	22	55.4	31	17.1	0.54	24.0	0.27	295	84.2
13	49.07	22	55.5	27	17.7	0.49	22.3	0.27	297	85.0
14	49.09	22	55.4	30	18.4	0.47	25.5	0.27	297	84.8
15	49.11	22	55.3	30	18.1	0.47	24.8	0.27	296	84.6
16	49.14	22	55.3	28	17.4	0.47	23.8	0.27	288	82.4
17	49.16	22	55.4	31	18.4	0.49	26.0	0.27	303	86.6
18	49.18	22	55.5	18	16.1	0.48	25.7	0.27	286	81.4
19	49.20	22	55.7	29	17.5	0.44	24.0	0.27	288	82.1
20	49.23	22	55.2	32	18.5	0.46	26.8	0.27	300	85.8
21	49.25	22	55.6	30	18.0	0.44	25.5	0.27	291	83.0
22	49.27	22	55.2	31	17.8	0.43	24.9	0.27	296	84.4
23	49.30	22	55.5	31	18.2	0.47	26.1	0.27	306	87.3
24	49.32	22	55.8	18	17.3	0.43	23.6	0.27	286	81.7
25	49.34	22	55.3	31	18.2	0.44	25.8	0.27	302	86.3
26	49.36	22	55.5	31	18.4	0.43	25.8	0.27	303	86.7
27	49.39	22	55.8	27	17.0	0.40	22.8	0.27	283	80.8
28	49.41	22	55.9	30	18.0	0.41	25.3	0.27	295	84.3
29	49.43	22	55.0	30	18.0	0.4	25.0	0.27	292	83.2
30	49.45	22	55.2	30	18.4	0.42	24.8	0.27	296	84.5
31	49.48	22	55.1	31	18.8	0.41	26.4	0.27	299	85.6

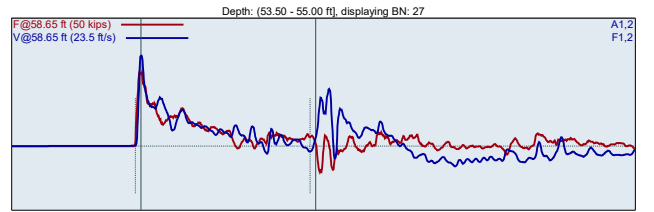
32	49.50	22	55.5	30	18.6	0.42	25.5	0.27	302	86.2
33	49.52	26	55.5	30	17.9	0.42	25.3	0.23	300	85.7
34	49.54	26	55.8	29	17.5	0.41	24.0	0.23	289	82.4
35	49.56	26	55.2	30	18.5	0.42	25.5	0.23	300	85.9
36	49.58	26	55.4	31	18.6	0.41	26.2	0.23	302	86.2
37	49.60	29	55.8	29	17.4	0.40	24.1	0.23	288	82.3
38	49.62	26	55.0	30	18.2	0.38	24.9	0.23	291	83.2
39	49.63	26	55.7	29	17.7	0.38	24.2	0.23	288	82.1
40	49.65	26	55.1	30	18.3	0.41	25.5	0.23	302	86.3
41	49.67	26	55.7	27	16.4	0.40	22.6	0.23	287	82.1
42	49.69	26	55.4	30	18.2	0.40	25.6	0.23	304	87.0
43	49.71	26	55.4	28	17.2	0.40	23.9	0.23	288	82.4
44	49.73	26	55.7	29	17.6	0.44	24.2	0.23	296	84.5
45	49.75	26	55.2	27	16.9	0.44	23.1	0.23	308	88.1
46	49.77	26	55.7	29	17.6	0.39	24.5	0.23	290	82.7
47	49.79	26	55.0	30	18.2	0.41	23.4	0.23	299	85.5
48	49.81	26	55.9	30	18.2	0.40	25.0	0.23	296	84.6
49	49.83	26	55.5	28	17.4	0.40	23.7	0.23	296	84.7
50	49.85	26	55.5	28	17.2	0.37	23.9	0.23	291	83.2
51	49.87	26	55.1	29	17.8	0.38	24.4	0.23	296	84.5
52	49.88	26	55.5	28	17.8	0.38	24.3	0.23	299	85.3
53	49.90	26	55.5	29	17.5	0.38	24.2	0.23	297	84.8
54	49.92	26	55.2	28	17.2	0.36	23.8	0.23	294	83.9
55	49.94	26	55.4	28	16.9	0.37	23.2	0.23	297	84.6
56	49.96	26	55.4	30	17.9	0.37	25.0	0.23	297	84.8
57	49.98	29	55.5	25	16.7	0.37	24.6	0.23	298	85.0
58	50.00	26	55.4	26	16.5	0.35	22.1	0.23	285	81.1
Average			55.4	29	17.8	0.42	24.6	0.25	295	84.4
Std Dev			0.2	1	0.6	0.04	1.1	0.02	6	1.7
Maximum			55.9	32	18.8	0.51	26.8	0.27	308	88.1
Minimum			55.0	28	16.4	0.35	22.1	0.23	283	80.8

Sample Interval Time: 61.53 seconds



PDA-S Ver. 2018.30 - Printed: 4/19/2022										
PDA-S Ver. 2018.30 - Printed: 4/19/2022										
32	54.78	20	55.2	31	18.7	0.39	26.4	0.30	298	85.2
33	54.80	20	55.1	30	17.5	0.41	25.4	0.30	299	85.3
34	54.83	20	55.5	31	18.0	0.40	25.8	0.30	294	84.0
35	54.85	20	55.1	30	17.3	0.42	25.0	0.30	303	86.7
36	54.88	20	55.4	30	17.7	0.40	25.1	0.30	295	84.3
37	54.90	20	55.1	30	17.1	0.39	25.0	0.30	291	83.1
38	54.93	20	55.4	30	18.1	0.38	25.5	0.30	302	86.4
39	54.95	20	55.0	28	16.7	0.38	23.8	0.30	292	83.4
40	54.98	20	55.7	31	18.0	0.38	25.9	0.30	301	85.9
41	55.00	20	55.2	29	16.6	0.36	24.4	0.30	284	81.1
Average										83.5
Std Dev										1.8
Maximum										86.7
Minimum										79.7
N-value: 34										

Sample Interval Time: 43.27 seconds.



BL#	LP	BC	BPM	FMX	VMX	DMX	CSX	DFN	EFV	ETR
	ft	/6"	bpm	kips	ft/s	in	ksi	in	ft-lb	%
1	53.57	7	1.9	28	17.1	1.38	23.5	0.86	295	84.3
2	53.64	7	58.2	29	17.2	1.01	24.2	0.86	280	80.0
3	53.71	7	54.6	29	17.2	0.95	24.1	0.86	304	86.8
4	53.79	7	56.2	28	17.1	0.86	23.5	0.86	274	78.3
5	53.86	7	55.5	29	17.1	0.86	24.6	0.86	293	83.8
6	53.93	7	55.0	29	17.3	0.86	24.5	0.86	296	84.7
7	54.00	7	55.8	30	17.4	0.86	25.5	0.86	292	83.3
8	54.04	14	55.2	29	16.8	0.56	24.5	0.43	298	85.1
9	54.07	14	55.4	29	17.0	0.53	24.2	0.43	285	81.5
10	54.11	14	55.4	29	17.2	0.52	24.0	0.43	285	81.3
11	54.14	14	55.7	29	17.2	0.50	24.5	0.43	285	81.4
12	54.18	14	55.2	30	17.7	0.50	24.8	0.43	298	85.3
13	54.21	14	55.1	30	17.4	0.50	25.1	0.43	291	83.2
14	54.25	14	55.6	31	17.4	0.49	25.7	0.43	291	83.3
15	54.29	14	55.4	30	17.0	0.49	25.0	0.43	285	81.5
16	54.32	14	55.0	31	18.6	0.45	26.0	0.43	294	83.9
17	54.36	14	55.7	30	17.0	0.51	25.0	0.43	290	83.0
18	54.39	14	55.4	31	18.3	0.45	26.1	0.43	294	83.9
19	54.43	14	55.1	29	16.8	0.47	24.8	0.43	284	81.1
20	54.46	14	55.9	29	16.8	0.43	24.4	0.43	285	81.4
21	54.50	14	54.9	30	18.1	0.43	25.6	0.43	290	82.8
22	54.53	20	55.7	29	16.8	0.43	24.6	0.30	279	79.7
23	54.55	20	55.1	30	16.6	0.45	24.9	0.30	293	83.8
24	54.58	20	55.3	31	18.0	0.43	25.9	0.30	295	84.4
25	54.60	20	55.0	29	17.3	0.45	24.7	0.30	293	83.7
26	54.63	20	55.4	31	18.6	0.38	26.0	0.30	288	82.3
27	54.65	20	55.1	29	16.6	0.44	24.1	0.30	300	85.7
28	54.68	20	55.1	30	17.2	0.45	25.3	0.30	292	83.4
29	54.70	20	55.8	29	17.0	0.46	24.7	0.30	303	86.4
30	54.73	20	55.0	29	17.3	0.39	24.4	0.30	285	81.5
31	54.75	20	55.2	31	17.7	0.41	25.8	0.30	295	84.4

Summary of SPT Test Results													
Project: CME 550X (SN308513), Test Date: 4/19/2022													
BPM: Blows/Minute													
FMX: Maximum Force													
VMX: Maximum Velocity													
DMX: Maximum Displacement													
CSX: Compression Stress Maximum													
DFN: Final Displacement													
EFV: Maximum Energy													
ETR: Energy Transfer Ratio - Rated													
Inst.	Start	Final	Blows	N	N60	Average	Average	Average	Average	Average	Average	Average	Average
Length	Depth	Depth	Applied	Value	Value	BPM	FMX	VMX	DMX	CSX	DFN	EFV	ETR
ft	ft	ft	bl			bpm	kips	ft/s	in	ksi	in	ft-lb	%
38.85	33.50	35.00	3-5-8	13	18	55.0	29	16.7	1.05	24.1	0.92	297	84.8
48.65	43.50	45.00	12-20-24	44	61	54.9	30	17.1	0.42	25.1	0.27	297	85.0
53.85	48.50	50.00	10-22-26	48	67	55.4	29	17.8	0.42	24.6	0.25	295	84.4
58.65	53.50	55.00	7-14-20	34	47	55.3	30	17.4	0.44	25.1	0.35	292	83.5
Overall Average Values:						55.2	30	17.4	0.49	24.8	0.35	295	84.4
Standard Deviation:						0.3	1	0.7	0.21	1.0	0.20	6	1.7
Overall Maximum Value:						55.9	33	18.8	1.68	27.4	1.20	308	88.1
Overall Minimum Value:						53.9	26	15.8	0.34	22.1	0.23	279	79.7

