

# S-37-168 (Little Choestoea Road) Bridge Replacement over Tributary to Choestoea Creek

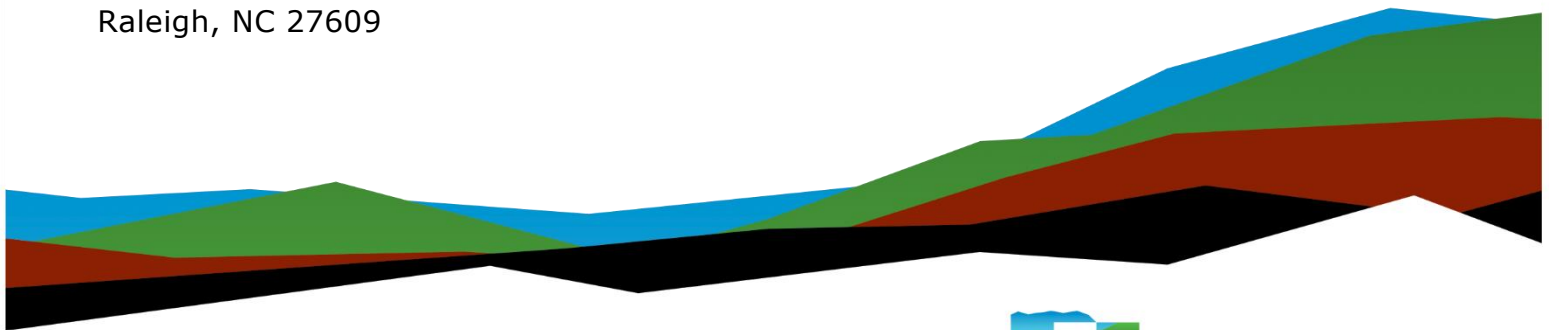
Oconee County, SC

## Geotechnical Baseline Report

March 22, 2025 | SCDOT Project ID: P042511  
Terracon Project No.: 8623P180

### Prepared for:

HNTB Corporation  
343 E. Six Forks Road, Suite 200  
Raleigh, NC 27609



Nationwide  
[Terracon.com](https://terracon.com)

- Facilities
- Environmental
- Geotechnical
- Materials



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March 22, 2025

HNTB Corporation  
343 E. Forks Road, Suite 200  
Raleigh, NC 27609

Attn: Mr. Spencer Franklin, PE, Senior Vice President  
P: 919-546-8997

Re: Geotechnical Baseline Report  
S-37-168 Bridge Replacement over Tributary to Choestoea Creek  
Oconee County, South Carolina  
SCDOT Project ID.: P042511  
Terracon Project No.: 8623P180

Dear Mr. Franklin:

Terracon Consultants Inc. (Terracon) has completed the exploration, testing and limited engineering analysis services for the above referenced project. The services were conducted in general accordance with our Task Order Number 001, dated May 25, 2023 and Supplement Number 001, dated October 31, 2023.

## Introduction

HNTB Corporation (HNTB) has contracted Terracon to perform subsurface exploration, laboratory testing and limited preliminary engineering recommendations for the replacement of the S-37-168 Bridge over Tributary to Choestoea Creek in Oconee County, South Carolina. The results of the subsurface exploration and laboratory testing have been separately presented in a Geotechnical Subsurface Data Report (GSDR). For convenience, the data is also provided here in this Geotechnical Baseline Report (GBR) along with a characterization of the subsurface conditions for the project. Limited preliminary geotechnical recommendations are associated with the requested scope of study and are included in this GBR. This GBR was prepared in general accordance with the 2022 SCDOT Geotechnical Design Manual (GDM) and Preconstruction Design Memorandum (PCDM) 11 - Supplemental Design Criteria for Low Volume Bridge Replacement Projects.

## **Project Description**

The project site is located at the S-37-168 (Little Choestoea Road) crossing over the tributary to Choestoea Creek in Oconee County, South Carolina. Site location and exploration plans are presented in Appendix A of this report. Based on the conceptual plans by HNTB dated 3/13/2025, the replacement bridge will be constructed on the same alignment as the current bridge and will be considered a low volume bridge. The current plans indicate the new bridge will be a 130-ft long multi-span bridge constructed with an adjacent cored slab for span A and adjacent box beam slab for span B.

## **Geotechnical Testing**

The geotechnical exploration for this project was performed between January 17 and 18, 2024. The results of our field work and our associated laboratory testing are included in Appendices A and B.

### **Field Exploration**

Our field exploration consisted of the following:

- Three (3) Standard Penetration Test (SPT) Borings (S-37-168-1T, S-37-168-2T, and S-37-168-3T)
- Two (2) offset auger probes near S-37-168-1T and S-37-168-3T for bulk sample collection

The tests were performed at the approximate locations as approved by SCDOT. A description of our testing methods and graphical logs outlining the soil conditions at each test location are presented in Appendix A. The test locations were established in the field by Terracon and surveyed by Thomas & Hutton after completion.

### **Laboratory Testing**

The following laboratory tests were performed on the soil samples collected at the site.

- Twenty-five (25) Natural Moisture Content Tests
- Eight (8) Atterberg Limits Tests
- Eight (8) Grain Size Tests
- Four (4) Grain Size Tests with Hydrometer
- One (1) Remolded, Consolidated-Undrained (CU) Triaxial Compression Test with Pore Pressure Readings
- One (1) Standard Proctor Test
- One (1) Corrosivity Suite (pH, chloride content, sulfate content, and resistivity tests)
- Eight (8) Compressive Strength of Rock Cores

The general scope of the laboratory testing frequency was determined by the SCDOT. The laboratory testing assignment was performed by our engineers. The laboratory procedures and results of the laboratory tests are presented in Appendix B.

## Subsurface Conditions

### Regional Geology

The bridge site is located on route S-37-168, north of the town of South Union in Oconee County, South Carolina. The site lies generally within the Piedmont Physiographic Province. More specifically, the site is located within the Sixmile Thrust Sheet. According to regional geologic mapping and published geologic reports, the project is in an area that contains muscovite-biotite schist, biotite schist, sillimanite-mica schist and gneiss, amphibolite, biotite gneisses including some that are porphyroblastic, felsic gneiss, and some manganiferous schist and metamorphosed manganese silicate. The rocks are likely metamorphosed marine sediments that also received some volcanic material. The bridge end bents and approach embankments contain existing fill above alluvial and/or residual soils, very dense residual soils classified as Intermediate Geomaterials (IGM) and bedrock.

### Soil and Rock Stratification

The soils encountered at this site consist of fill that is loose to medium dense silty sand and firm sandy silts in the upper 12 ½ to 23 ½ feet. Beneath the fill and creek are alluvial soils consisting of very loose to loose poorly graded sands, poorly graded sands with silt, silty sands to approximate depth of 18 ½ to 36 feet below the ground surface and/or bridge deck. Below the alluvium, residual soils consisting of poorly graded sands with silt and silty sands that were encountered to approximate depths of 52 ½ to 69 feet below ground and/or bridge surface, with some residual soils characterized as being intermediate geomaterials (IGM) exhibiting SPT N values of more than 100 blows per foot (bpf) and were followed by bedrock. Bedrock was present to the maximum depth explored of 84 feet below the ground surface.

Geology	Approximate Elevation of Layer Bottom (ft, NAVD88)	USCS Soil Type	Measured Field N Value	Plasticity Index	Fines Content	REC / RQD
Fill <sup>3</sup>	729 to 748	ML, SM	4 to 11	NP	36 to 47	--
Alluvium	721 to 743	SM, SP-SM, SP	0 to 9	NP to 8	2 to 47	--
Residuum	684 to 706	SM, SP-SM	5 to 100+	NP	10 to 20	--
Rock	PMDE <sup>1</sup>	--	--	--	--	75-100% / 18-100%

1. PMDE = Present to Maximum Depth Explored

2. NP=no-plastic

3. Only encountered in Borings S-37-168-1T and S-37-168-3T



## Design and Construction Considerations

### Foundations

Driven steel H-piles driven to practical refusal on rock or within IGM materials (i.e., >20 blows per inch with appropriately sized hammer) are expected to be feasible for the proposed bridge end bents.

The approximate elevation to the top of very dense residual soils (IGM) at End Bent 1 is 695 feet NAVD88 and is about 11 feet thick overlying bedrock with a RQD of 18% at the top of rock. At End Bent 3 IGM was encountered at an elevation 709 feet NAVD88 and is about 4 feet thick overlying bedrock with a RQD of 88% at the top of rock. Per section 16.3.1 of the GDM, reinforced pile tips will be needed to minimize potential pile damage while penetrating through IGM and the top of rock. Pile drivability using the wave equation should be performed along with estimating stresses during driving and, in general, verifying the ability of the Contractor's selected hammer to drive the piles to the desired penetration while preventing overstressing of the pile.

According to the conceptual bridge plans by HNTB dated 3/13/2025, approximately 4 feet of fill is expected at the beginning of the bridge and no fill at the end of the bridge. Foundations should typically be installed after the approach embankment construction to reduce potential downdrag settlement issues. The pile design should account for drag loads, should new fill be placed after installing foundation piles.

Drilled shafts are anticipated to be feasible for the proposed bridge interior bent. Assuming redundant drilled shafts, Table 9-4 GDM 2022 allows using a resistance factor of 0.60 (both side resistance and end bearing) for a single redundant drilled shaft in rock. It is assumed that the drilled shaft will be cased to the top of rock and the side resistance along the casing length will not be considered in estimating axial resistances. Appropriate group effects should be considered as necessary per GDM Chapter 16.

We have observed variability in the top of rock and thickness of IGM, as seen in **Soil and Rock Stratification**. Therefore, we expect variability in foundation tip elevations at each bent location. Resistance of piles driven to practical refusal in IGM or rock will be limited by their structural resistance.

## Corrosion and Deterioration

Corrosion testing was performed on a composite sample obtained from split spoons in the upper 2 to 25 feet. Corrosion testing included pH, resistivity, chlorides, and sulfates content are summarized in the table below, and the test results are included in Appendix B.

Corrosion Test	Results Bent 1, Boring S-37-168-3 Composite Sample from 2 to 25 feet	Indication of Corrosivity <sup>1</sup>
pH	5.1	Less than 5.5
Resistivity	3,149 ohm-cm	Less than 2,000 ohm-cm
Chloride	205 ppm	Greater than 500 ppm
Sulfate	118 ppm	Greater than 1,000 ppm

1. AASHTO LRFD bridge design specifications, Ninth Edition 2020, Section 10.7.5.

Based on the criteria for electro-chemical properties in the GDM Section 7.18, the electro-chemical classification of the project site is aggressive. Interpretation of these data should be communicated with the project's structural engineer.

## Embankment Construction

Based on the conceptual plans by HNTB, cut excavation is expected in front of the end bents. Bulk samples were obtained between End Bent 1 and Interior Bent 3 from the top 5 feet of existing embankment material. Per our scope, the bulk sample was tested for soil classification and was also remolded to 95% of the Standard Proctor prior to being tested under CU Triaxial Compression. Test results are presented in Appendix B and summarized in the table below.

Sample No.	Station	Offset (ft)	Sample Depth (ft)	USCS Soil Type	Compaction		Shear Strength <sup>1</sup>	
					Optimum Moisture (%)	Max Dry Density (pcf)	Total (psi / °)	Effective (psi / °)
S-37-168-1T & 3T Bulk	85+07 86+26	5.5R 4L	1 – 5	SM	18.4	105.6	c=1.86 ø=17	c'=0.35 ø'=33

1. Based on a maximum deviator stress failure criterion

## Geotechnical Baseline Report

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC

March 22, 2025 | Terracon Project No. 8623P180 | SCDOT Project ID: P042511



## Closure

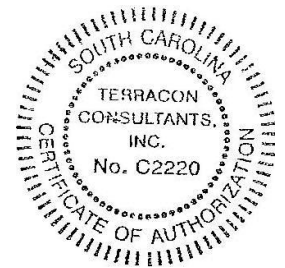
We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

**Terracon Consultants, Inc.**

Maggie McKenney, EIT  
Senior Staff Engineer

Jonathan Ard, PE  
Regional Service Manager  
SC Registration No. 30886



Reviewed by Terracon's Authorized Project Reviewer: Abdul Fekrat, P.E.

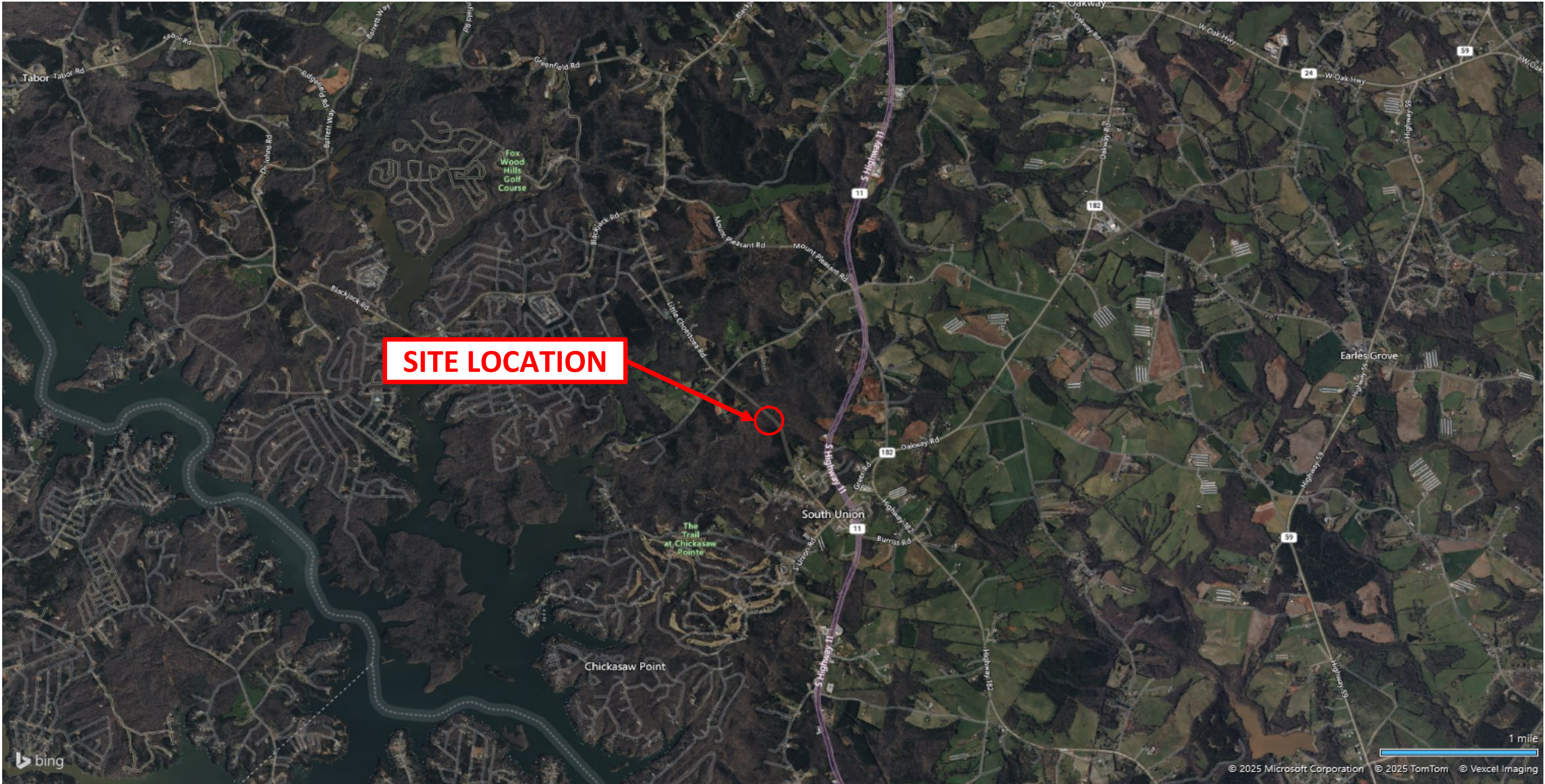
## **Appendix A**

### **Field Exploration**

- Exhibit A-1 – Site Location Map
- Exhibit A-2 – Aerial Exploration Plan
- Exhibit A-3 – Boring Location Diagram
- Exhibit A-4 – Field Testing Summary
- Exhibit A-5 – GeoScoping Form (2 Pages)
- Exhibit A-6 – Field Exploration Description (2 Pages)
- Exhibit A-7 – Soil/Rock Description Terms (2 Pages)
- Exhibit A-8 – Soil/Rock Symbols
- Exhibit A-9 – Boring Logs (7 Pages)
- Exhibit A-10 – Grout Logs (2 Pages)
- Exhibit A-11 – Rock Core Photograph Logs (3 Pages)

Note: All exhibits are one page unless noted above





AERIAL PHOTOGRAPHY PROVIDED BY BING  
DIAGRAM IS FOR GENERAL LOCATION ONLY,  
AND IS NOT INTENDED FOR CONSTRUCTION  
PURPOSES

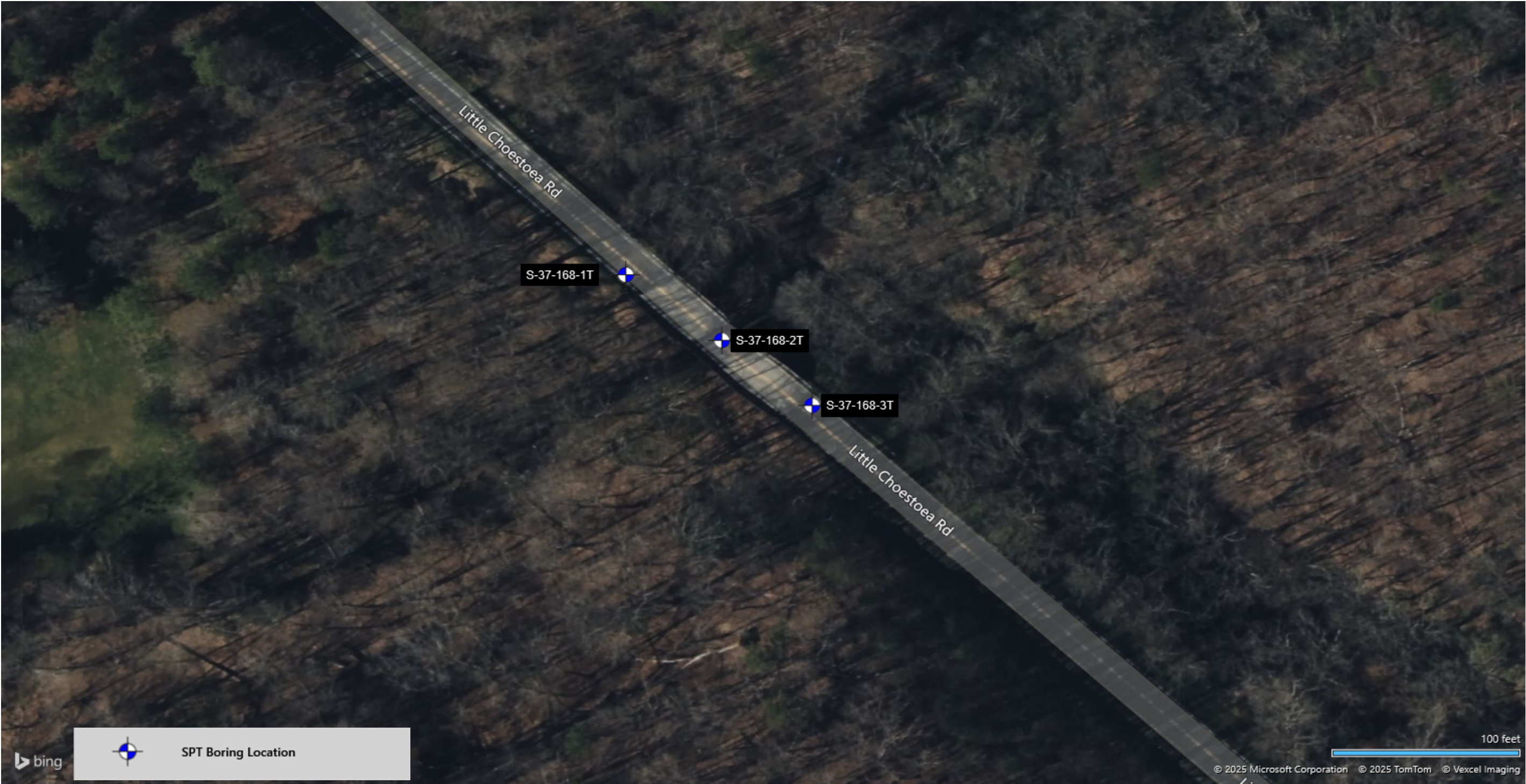
Project Mgr:	JA	Project No.	8623P180
Drawn by:	MM	Scale:	AS SHOWN
Checked by:	JA	Date:	3/21/2025
Approved by:	JA		



SITE LOCATION MAP	
<b>S-37-168 (Little Choestoea Road) Bridge Replacement over Tributary to Choestoea Creek</b>	
Oconee County, SC	P042511

**EXHIBIT**  
**A-1**





AERIAL PHOTOGRAPHY PROVIDED BY BING  
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PURPOSES


Project Mgr:	JA	Project No:	8623P180
Drawn by:	MM	Scale:	AS SHOWN
Checked by:	JA	Date:	3/21/2025
Approved by:	JA		

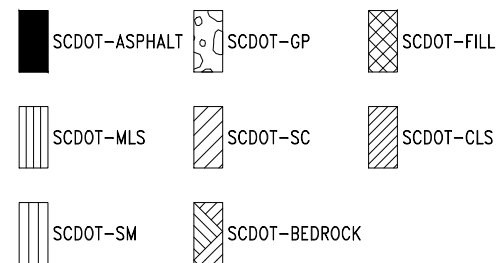




AERIAL EXPLORATION PLAN	
<b>S-37-168 (Little Choestoea Road) Bridge Replacement over Tributary to Choestoea Creek</b>	
Oconee County, SC	P042511

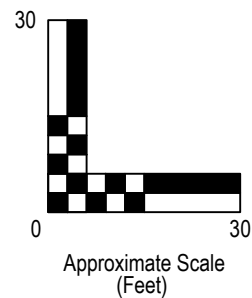
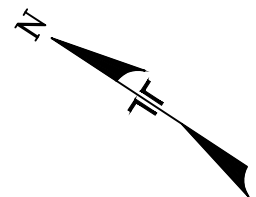
<b>EXHIBIT</b>
<b>A-2</b>

## LEGEND

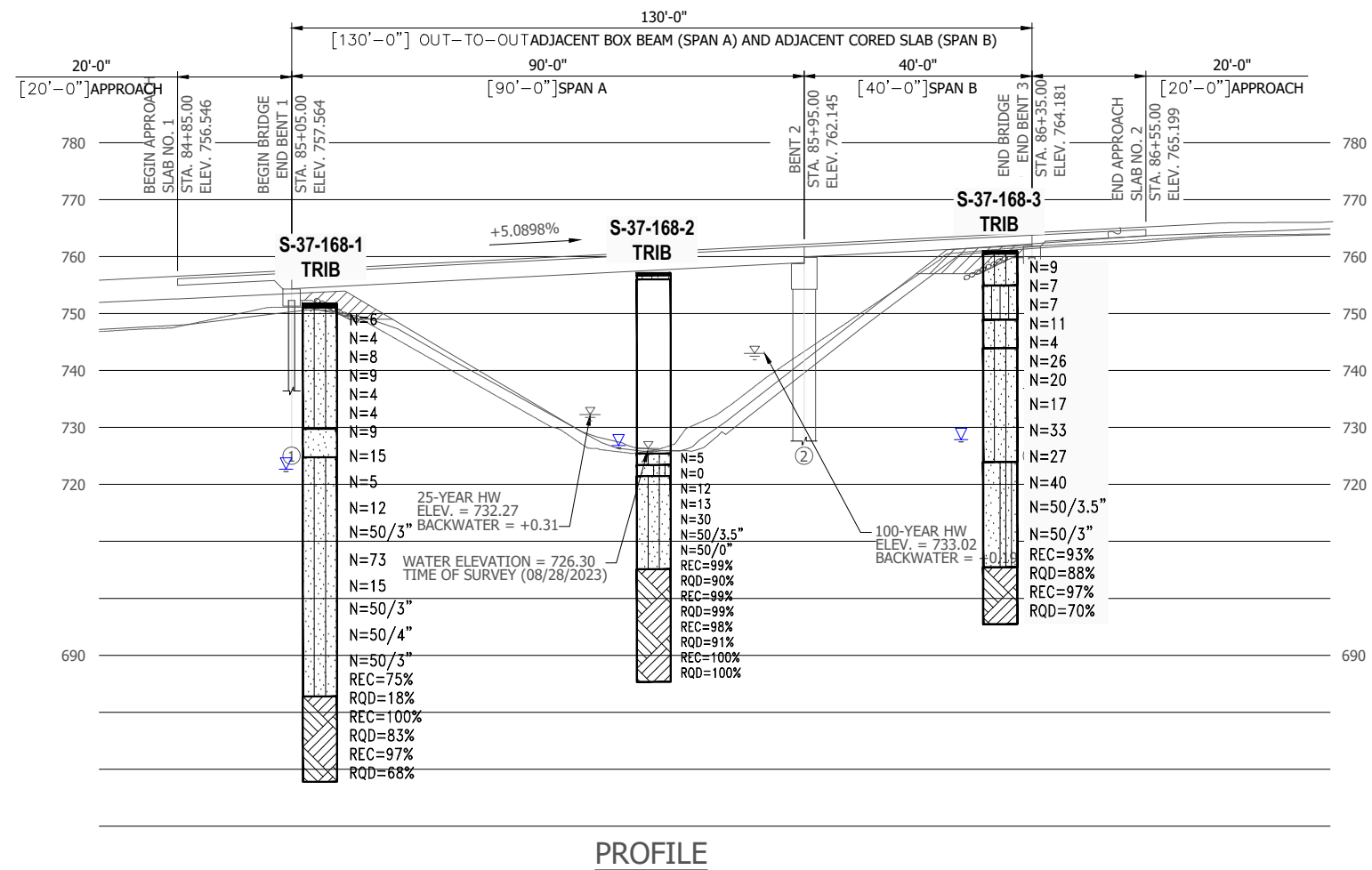
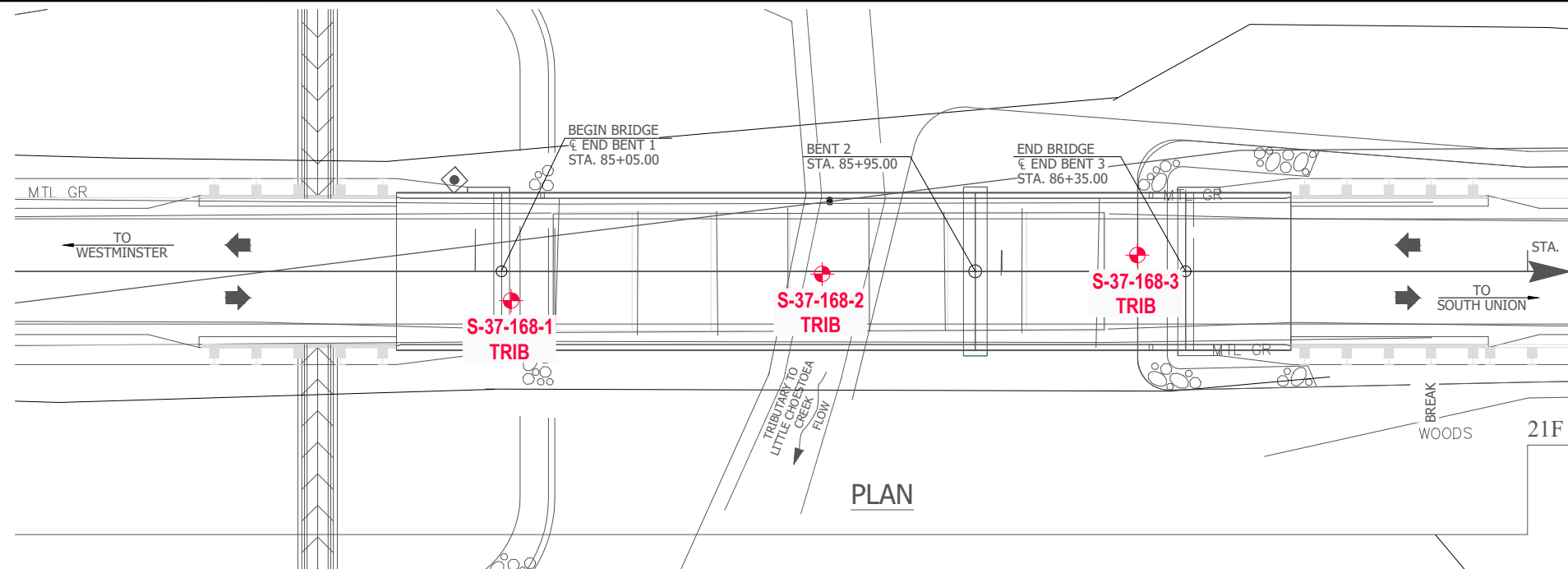
 APPROXIMATE BORING LOCATION



 Water Level Reading at time of drilling.  
 Water Level Reading after drilling.



THIS DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES



Project Mngr: MM  
Drawn By: RLW  
Checked By: MM/MRF  
Approved By: JNA

Project No. 8623P180  
Scale: AS SHOWN  
File No. 8623P180 PC  
Date: MARCH 2025

  
72 Pointe Circle Greenville, SC 29615  
864-292-2901 864-292-6361

BORING LOCATION DIAGRAM  
SCDOT PROJECT ID: P042511  
S-37-168 BRO TRIBUTARY TO CHOESTOE CREEK  
OCONEE COUNTY  
SOUTH CAROLINA

EXHIBIT  
A-3

Soil Testing Location Table - Exhibit A-4

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC

Terracon Project No.: 8623P180 | SCDOT Project ID: P042511



Test Number	Type	Test Hole Local	Northing	Easting	Latitude	Longitude	Station <sup>1</sup>	Offset <sup>1</sup>	Elevation <sup>2</sup> (ft)	Depth (ft)
S-37-168-1T	STB	Begin Bridge	998996.19	1379631.13	34.56202	-83.06072	85+07	5.5-R	753.4	84.0
S-37-168-2T	STB	Interior Bent	998960.48	1379683.12	34.56192	-83.06055	85+66	0	757.3	72.5
S-37-168-3T	STB	End Bridge	998922.58	1379730.75	34.56182	-83.06039	86+26	4-L	761.5	65.5

- 1. Stations and offsets were based on the state plane coordinates collected by Thomas & Hutton and measured from center line by Terracon in MicroStation.
- 2. Elevations are based on vertical datum NAVD88.
- 3. A composite bulk sample was collected approximately 5 feet feet from S-37-168-1T and S-37-168-3T.



## GeoScoping Form

PROJECT INFORMATION			
Project ID:	P042511	Date of Trip:	1/7/2025
County:	Oconee	Location:	Westminster, SC
Rd/ Route:	S-37-168	Local Name:	Little Choestoea Rd
Attendees:	M. McKenney		

EXISTING BRIDGE INFORMATION			
Bridge Length:	105 ft	Bridge Width:	25 ft
Superstructure Type:	NA	Substructure Type:	Timber and Steel H-Piles
Begin Bridge Sta <sup>1</sup> :	Unknown	End Bridge Sta <sup>1</sup> :	Unknown
Begin Bridge Embankment Sta <sup>1</sup> :	Unknown	End Bridge Embankment Sta <sup>1</sup> :	Unknown
Structure Number:	5594	Posted Weight Limit:	18 tons
Crossing:	Tributary to Choestoea Creek	Skew:	N/A
Latitude:	34.56202	Longitude:	-83.06072
Existing Fill Height:	approx 13-24 ft	Approx Existing Slope Angle:	2H:1V

1. Begin & End Bridge Embankment 100 ft down Sta. or up Sta., respectively. Sta. estimated from overlay of bridge plan provided by HNTB.

EXISTING ROADWAY EMBANKMENT INFORMATION			
Begin Project Sta:	85+20	Begin Bridge Embankment Sta:	Unknown
Accessibility Issues:	None Observed		
Ground Cover:	Asphalt pavement and grassed shoulders with trees		
Existing Fill Height:	23 feet, sloping	Approx Existing Slope Angle:	1.5H:1V
Local Development:	Rural		
Topography:	Steep		
Traffic Control Necessary:	No		
Surface Soils:	Silty sand	Muck:	No
Exposed Rock in Stream Bed:	Yes	Exposed Rock in banks:	No
Wetlands on Site:	No	Wetland Adjacent:	Yes
Depth FG to Water:	31 feet	Water Depth:	1 feet
Depth to Existing Ground:	approximately 32 feet at center of bridge		
Scour Condition at EB:	Critical	Scour Condition at IB:	Critical

End Bridge Embankment Sta:	86+00	End Project Sta:	38+00
Accessibility Issues:	None Observed		
Ground Cover:	Asphalt pavement, tree, grass		
Existing Fill Height:	13 feet	Approx Existing Slope Angle:	1.5H:1V
Local Development:	Rural		
Topography:	Steep		
Traffic Control Necessary:	NO		
Surface Soils:	Silty Sand	Muck:	No
Exposed Rock in Stream Bed:	Yes	Exposed Rock in banks:	No
Wetlands on Site:	No	Wetland Adjacent:	Yes
Depth FG to Water:	31 feet	Water Depth:	1.5 feet
Depth to Existing Ground:	approximately 32 feet at center of bridge		
Scour Condition at EB:	Critical	Scour Condition at IB:	Critical

**GeoScoping Form**

UTILITIES INFORMATION	
Attached:	None.
Above Ground:	Overhead power
Underground:	None

Comments:

## **Field Exploration Description Overview**

The testing locations were determined by Terracon and submitted to SCDOT for approval. Terracon located the test locations in the field using handheld GPS and measurements from existing structures shown on the provided drawings. The borings were surveyed by Thomas & Hutton after testing and drilling was complete. The locations, as shown in the Exploration Plans, are shown to the scale indicated.

A field log of each test location was prepared by our engineer. The final boring logs included with this report represent the engineer's description of the encountered conditions modified as necessary based on laboratory test results of the individual samples.

### **Soil Test Borings (STB)**

All boring and sampling operations were conducted in general accordance with the following procedures:

- SCDOT Geotechnical Design Manual 2022
- ASTM D5783, "Standard Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geo-environmental Exploration"
- ASTM D6151, "Standard Practice for Using Hollow-Stem Augers for Geotechnical Exploration and Soil Sampling"
- ASTM D1586 "Test Method for Penetration Test and Split-Barrel Sampling of Soils"
- ASTM D4220 "Standard Practices for Preserving and Transporting Soil"
- ASTM D2113 "Standard Practice for Rock Core Drilling and Sampling of Rock for Site Exploration"
- ASTM D5079 "Standard Practices for Preserving and Transporting Rock Core Samples"

Each soil test boring was advanced using rotary wash drilling techniques. Soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-barrel sampler, also known as a standard split-spoon. The sampler is advanced into the soil a total of 18 to 24 inches by striking the drill rod using a 140-pound automatic hammer falling 30 inches. The number of blows required to advance the sampler for each of three to four, 6-inch increments is recorded. The sum of the number of blows for the second and third increments is called the "Standard Penetration Value", or N-value ( $N_{meas}$ , blows per foot). The N-value, when properly evaluated, is an index to the soil strength.

Soil classification provides a general guide to the engineering properties of various soil types and enables the engineer to apply his experience to current situations. In our exploration, samples obtained during drilling operations are examined and visually classified by a geotechnical engineer using the procedures outlined in ASTM D2487 - Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System). Laboratory testing was also performed on select split-spoon samples to evaluate index properties for further classification. The soils are described according to color, texture, and relative density or

**Exhibit A-6 – Subsurface Exploration Description**

S-37-168 BRO Little Choestoea Creek | Oconee County, SC  
Terracon Project No. 8623P180 | SCDOT Project ID: P042512



consistency (based on standard penetration resistance). The designations shown on the logs are described in the 2022 SCDOT Geotechnical Design Manual, Chapter 6.

The borings were advanced either to the planned drilling depth at which they were terminated, or to refusal of the drilling equipment. Select borings were continued below this depth using diamond bit rock coring techniques. NQ2 sized cores were recovered from the borehole. The rock recovery ratios (REC, percentage of the total core run), Rock Quality Designation (RQD, percentage of the total core run of pieces greater than 4 inches) were recorded along with a description of the rock. An explanation of the rock descriptions shown on the logs is provided in the SCDOT GDM Chapter 6. Photos of the recovered rock core specimens are provided in the Rock Core Photograph Log.

Groundwater readings were collected from the soil test borings after 24 hours if site constraints allowed the borings to stay open. If collected, water levels are indicated on the boring logs. The borings were advanced using mud rotary drilling techniques, and time-of-drilling water levels may not be reliable.

At the conclusion of the work, the boreholes holes were backfilled with the drill cuttings and clean sand. The upper 20 feet of the tests in the existing roadways and embankments were grouted with a cement bentonite grout. Test locations performed in existing pavements were capped with cold-patch asphalt.

## SOIL DESCRIPTION TERMS

### Relative Density/Consistency Terms

<u>Relative Density</u> <sup>1</sup>			<u>Consistency</u> <sup>2</sup>		
Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Unconfined Compression Strength (q <sub>u</sub> ) (tsf)	SPT Blow Count
Very Loose	0 to 15%	4 and less	Very Soft	0.25 and less	2 and less
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	51 and more	Very Stiff	2.01 to 4.00	16 to 30
			Hard	4.01 and more	31 and more

### Moisture Condition

<u>Descriptive Term</u>	<u>Criteria</u>
Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually in coarse-grained soils below the water table

### Color

Describe the sample color while sample is still moist.

### Angularity<sup>1</sup>

<u>Descriptive Term</u>	<u>Criteria</u>
Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces.
Subangular	Particles are similar to angular description but have rounded edges.
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges.
Rounded	Particles have smoothly curved sides and no edges.

### HCl Reaction<sup>3</sup>

<u>Descriptive Term</u>	<u>Criteria</u>
None Reactive	No visible reaction
Weakly Reactive	Some reaction, with bubbles forming slowly
Strongly Reactive	Violent reaction, with bubbles forming immediately

### Cementation<sup>3</sup>

<u>Descriptive Term</u>	<u>Criteria</u>
Weakly Cemented	Crumbles or breaks with handling or little finger pressure
Cemented	Crumbles or breaks with considerable finger pressure
Strongly Cemented	Will not crumble or break with finger pressure

### Particle-Size Range<sup>1</sup>

<u>Gravel</u>	Diameter, mm	Sieve Size	<u>Sand</u>	Diameter, mm	Sieve Size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	¾ inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4.00 to 4.76	#10 to #4

### Primary Soil Type<sup>1, 2</sup>

The primary soil type will be shown in all capital letters.

### USCS Soil Designation

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

### AASHTO Soil Designation

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

<sup>1</sup>Applies to coarse-grained soils (major portion retained on No. 200 sieve)

<sup>2</sup>Applies to fine-grained soils (major portion passing No. 200 sieve)

<sup>3</sup>Use as required

## DESCRIPTION OF ROCK PROPERTIES

### WEATHERING

Fresh	Rock fresh, crystals bright, few joints may show slight staining. Rock rings under hammer if crystalline.
Very slight	Rock generally fresh, joints stained, some joints may show thin clay coatings, crystals in broken face show bright. Rock rings under hammer if crystalline.
Slight	Rock generally fresh, joints stained, and discoloration extends into rock up to 1 in. Joints may contain clay. In granitoid rocks some occasional feldspar crystals are dull and discolored. Crystalline rocks ring under hammer.
Moderate	Significant portions of rock show discoloration and weathering effects. In granitoid rocks, most feldspars are dull and discolored; some show clayey. Rock has dull sound under hammer and shows significant loss of strength as compared with fresh rock.
Moderately Severe	All rock except quartz discolored or stained. In granitoid rocks, all feldspars dull and discolored and majority show kaolinization. Rock shows severe loss of strength and can be excavated with geologist's pick.
Severe	All rock except quartz discolored or stained. Rock "fabric" clear and evident, but reduced in strength to strong soil. In granitoid rocks, all feldspars kaolinized to some extent. Some fragments of strong rock usually left.
Very severe	All rock except quartz discolored or stained. Rock "fabric" discernible, but mass effectively reduced to "soil" with only fragments of strong rock remaining.
Complete	Rock reduced to "soil". Rock "fabric" not discernible or discernible only in small, scattered locations. Quartz may be present as dikes or stringers.

### HARDNESS (for engineering description of rock – not to be confused with Moh's scale for minerals)

Very hard	Cannot be scratched with knife or sharp pick. Breaking of hand specimens requires several hard blows of geologist's pick.
Hard	Can be scratched with knife or pick only with difficulty. Hard blow of hammer required to detach hand specimen.
Moderately hard	Can be scratched with knife or pick. Gouges or grooves to ¼ in. deep can be excavated by hard blow of point of a geologist's pick. Hand specimens can be detached by moderate blow.
Medium	Can be grooved or gouged 1/16 in. deep by firm pressure on knife or pick point. Can be excavated in small chips to pieces about 1-in. maximum size by hard blows of the point of a geologist's pick.
Soft	Can be gouged or grooved readily with knife or pick point. Can be excavated in chips to pieces several inches in size by moderate blows of a pick point. Small thin pieces can be broken by finger pressure.
Very soft	Can be carved with knife. Can be excavated readily with point of pick. Pieces 1-in. or more in thickness can be broken with finger pressure. Can be scratched readily by fingernail.

### Joint, Bedding, and Foliation Spacing in Rock<sup>a</sup>

Spacing	Joints	Bedding/Foliation
Less than 2 in.	Very close	Very thin
2 in. – 1 ft.	Close	Thin
1 ft. – 3 ft.	Moderately close	Medium
3 ft. – 10 ft.	Wide	Thick
More than 10 ft.	Very wide	Very thick

<sup>a</sup>Spacing refers to the distance normal to the planes, of the described feature, which are parallel to each other or nearly so.

### Rock Quality Designation (RQD)<sup>a</sup>

RQD, as a percentage	Diagnostic Description
Exceeding 90	Excellent
90 – 75	Good
75 – 50	Fair
50 – 25	Poor
Less than 25	Very poor

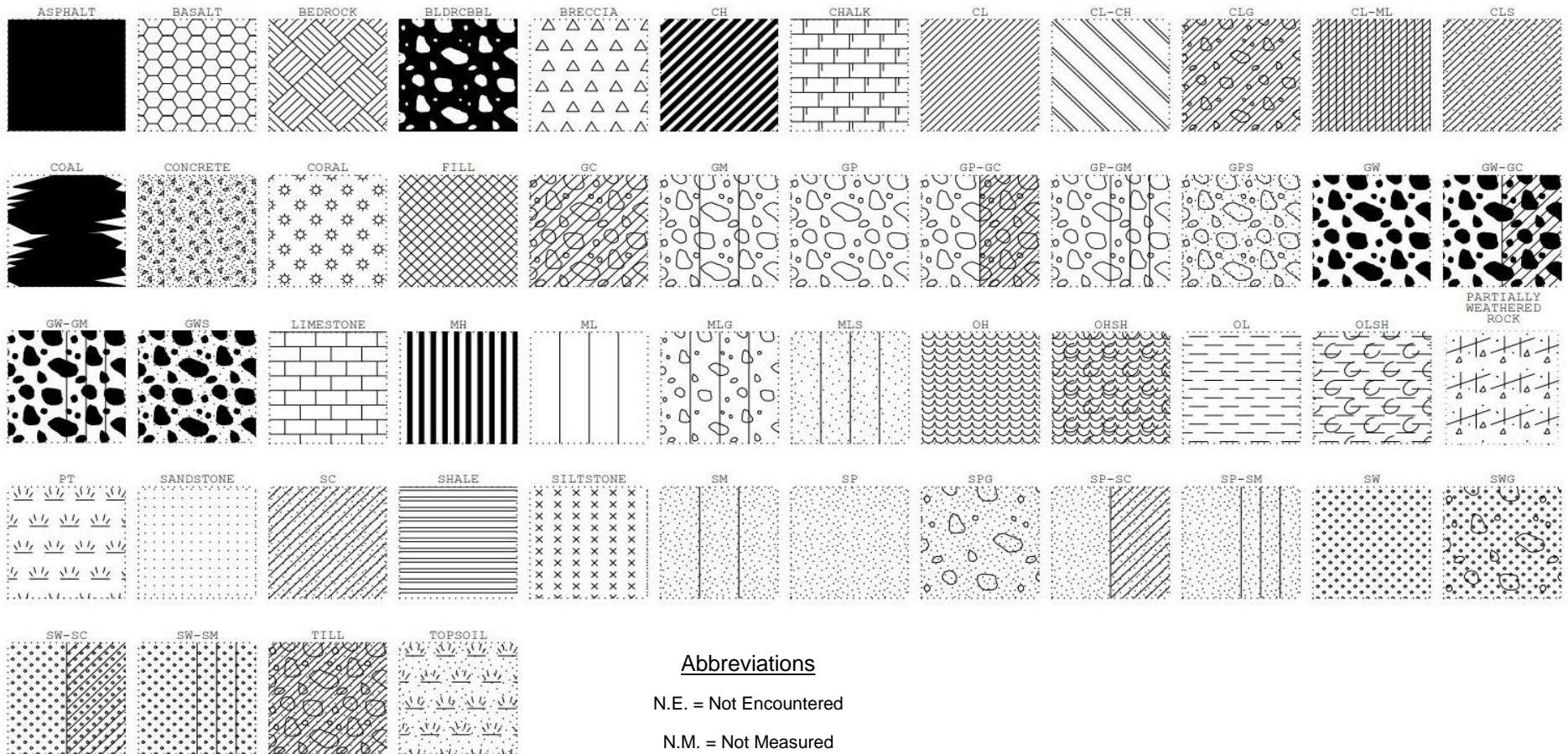
<sup>a</sup>RQD (given as a percentage) = length of core in pieces 4 in. and longer/length of run.

### Joint Openness Descriptors

Openness	Descriptor
No Visible Separation	Tight
Less than 1/32 in.	Slightly open
1/32 to 3/8 in.	Moderately open
1/8 to 3/8 in.	Open
3/8 in. to 0.1 ft.	Moderately wide
Greater than 0.1 ft.	Wide

References: American Society of Civil Engineers. Manuals and Reports on Engineering Practice - No. 56. Subsurface Investigation for Design and Construction of Foundations of Buildings. New York: American Society of Civil Engineers, 1976. U.S. Department of the Interior, Bureau of Reclamation, Engineering Geology Field Manual.





Project Manager:  
MEM

Drawn by:  
K.JZ

Checked by:  
SG

Approved by:  
DJC

Project No.  
8623P180

Scale:  
N.T.S.

File Name:  
Soil – Rock – Log

Date:  
Jul 2023

**Terracon**

72 Pointe Circle  
PH. (864) 292-2901

Greenville, SC 29615  
FAX. (864) 292-6361

## SOIL AND ROCK SYMBOLS

Exhibit A-8

# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511				<b>County:</b>	Oconee		<b>Boring No.:</b>	S-37-168-1T			
<b>Site Description:</b>	S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek							<b>Route:</b>	S-37-168			
<b>Eng./Geo.:</b>	M. Mc Kenney		<b>Boring Location:</b>	85+07		<b>Offset:</b>	5.5R		<b>Alignment:</b>	Existing		
<b>Elev.:</b>	753.39		<b>Latitude:</b>	34.56202		<b>Longitude:</b>	-83.06072		<b>Date Started:</b>	1/17/2024		
<b>Total Depth:</b>	84 ft		<b>Soil Depth:</b>	69 ft		<b>Core Depth:</b>	15 ft		<b>Date Completed:</b>	1/18/2024		
<b>Bore Hole Diameter (in):</b>	4		<b>Sampler Configuration</b>			<b>Liner Required:</b>	Y Ⓝ		<b>Liner Used:</b>	Y Ⓝ		
<b>Drill Machine:</b>	DR# 554		<b>Drill Method:</b>	RW/RC		<b>Hammer Type:</b>	Automatic		<b>Energy Ratio:</b>	88.5%		
<b>Core Size:</b>	NQ2		<b>Driller:</b>	B. Burnette		<b>Groundwater:</b>	TOB N.M.			<b>24HR</b>	29.5 ft	

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd 6"	3rd 6"	4th 6"	N Value	<div> ● SPT N VALUE ●  PL X MC X LL X  ▲ FINES CONTENT (%)  ⊕ RQD (%) ■ REC (%) </div>
	0.0	Existing Roadway									
	0.7	Asphalt (8.5 inches)									
		<b>FILL</b> - Loose, moist, reddish yellow, subangular, none reactive, weakly cemented, fine to medium, Silty SAND (SM) (A-4), with mica, 5YR6/6, LL=PL=PI=0, NMC=24.9, % <sub>#200</sub> =39.5		2.0							
					SS-1	2	3	3	4	6 X ●	○ ▲
748.4		Very loose, reddish brown, 5YR5/4, NMC=27.7		4.0							
					SS-2	1	2	2	3	4 ●	○
		Loose, LL=PL=PI=0, NMC=27.2, % <sub>#200</sub> =47.2		6.0							
					SS-3	4	4	4	5	8 X ●	○ ▲
743.4		NMC=22.7		8.0							
		reddish brown, 5YR5/4			SS-4	4	5	4	5	9 ●	○
		Very loose, NMC=25.1		13.5							
					SS-5	1	2	2		4 ●	○
738.4											
		NMC=29.5, % <sub>#200</sub> =36.7		18.5							
					SS-6	1	2	2		4 ●	○ ▲
733.4											
	23.5	<b>ALLUVIUM</b> - Loose, moist, reddish brown, subangular, none reactive, weakly cemented, fine to medium, Poorly graded SAND (SP) (A-3), with mica, trace organics, 5YR5/4, LL=PL=PI=0, NMC=16.7, % <sub>#200</sub> =2.3		23.5							
					SS-7	3	4	5		9 X ●	○
728.4											
	28.5	<b>RESIDUUM</b> - Medium dense, moist, dark reddish gray, subangular, none reactive, weakly cemented, fine to coarse, Silty SAND (SM) (A-2-4), with rock fragments, 5YR5/4, NMC=13.0		28.5							
					SS-8	12	9	6		15 ●	○
723.4											
		Loose, wet, light reddish brown, fine to medium, with mica 5YR6/3, NMC=36.0		33.5							
					SS-9	3	2	3		5 ●	○
718.4											

## LEGEND

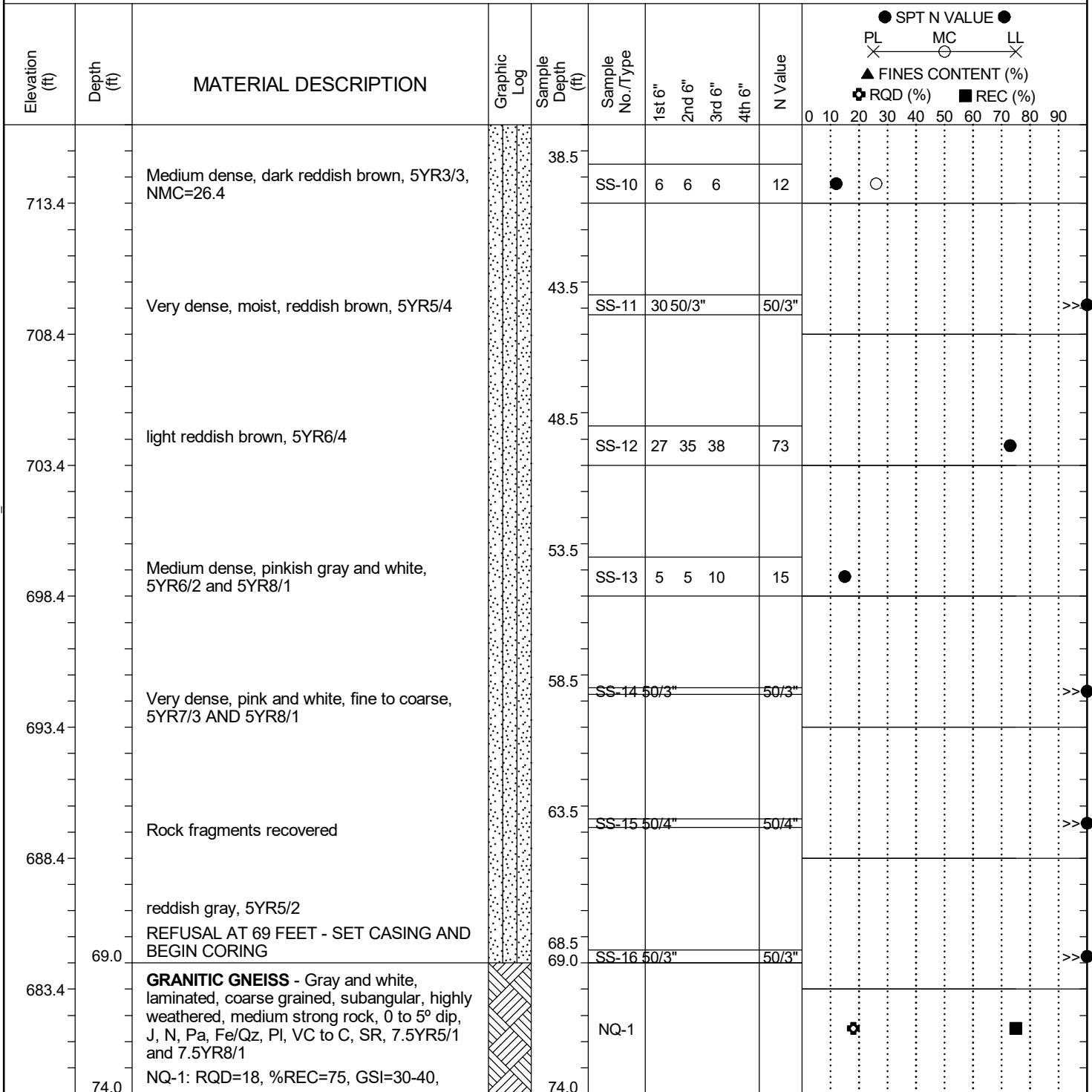
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	



# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511				<b>County:</b>	Oconee		<b>Boring No.:</b>	S-37-168-1T			
<b>Site Description:</b>	S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek							<b>Route:</b>	S-37-168			
<b>Eng./Geo.:</b>	M. McKenney		<b>Boring Location:</b>	85+07		<b>Offset:</b>	5.5R		<b>Alignment:</b>	Existing		
<b>Elev.:</b>	753.39		<b>Latitude:</b>	34.56202		<b>Longitude:</b>	-83.06072		<b>Date Started:</b>	1/17/2024		
<b>Total Depth:</b>	84 ft		<b>Soil Depth:</b>	69 ft		<b>Core Depth:</b>	15 ft		<b>Date Completed:</b>	1/18/2024		
<b>Bore Hole Diameter (in):</b>	4		<b>Sampler Configuration</b>			<b>Liner Required:</b>	Y Ⓝ		<b>Liner Used:</b>	Y Ⓝ		
<b>Drill Machine:</b>	DR# 554		<b>Drill Method:</b>	RW/RC		<b>Hammer Type:</b>	Automatic		<b>Energy Ratio:</b>	88.5%		
<b>Core Size:</b>	NQ2		<b>Driller:</b>	B. Burnette		<b>Groundwater:</b>	TOB N.M.			<b>24HR</b>	29.5 ft	



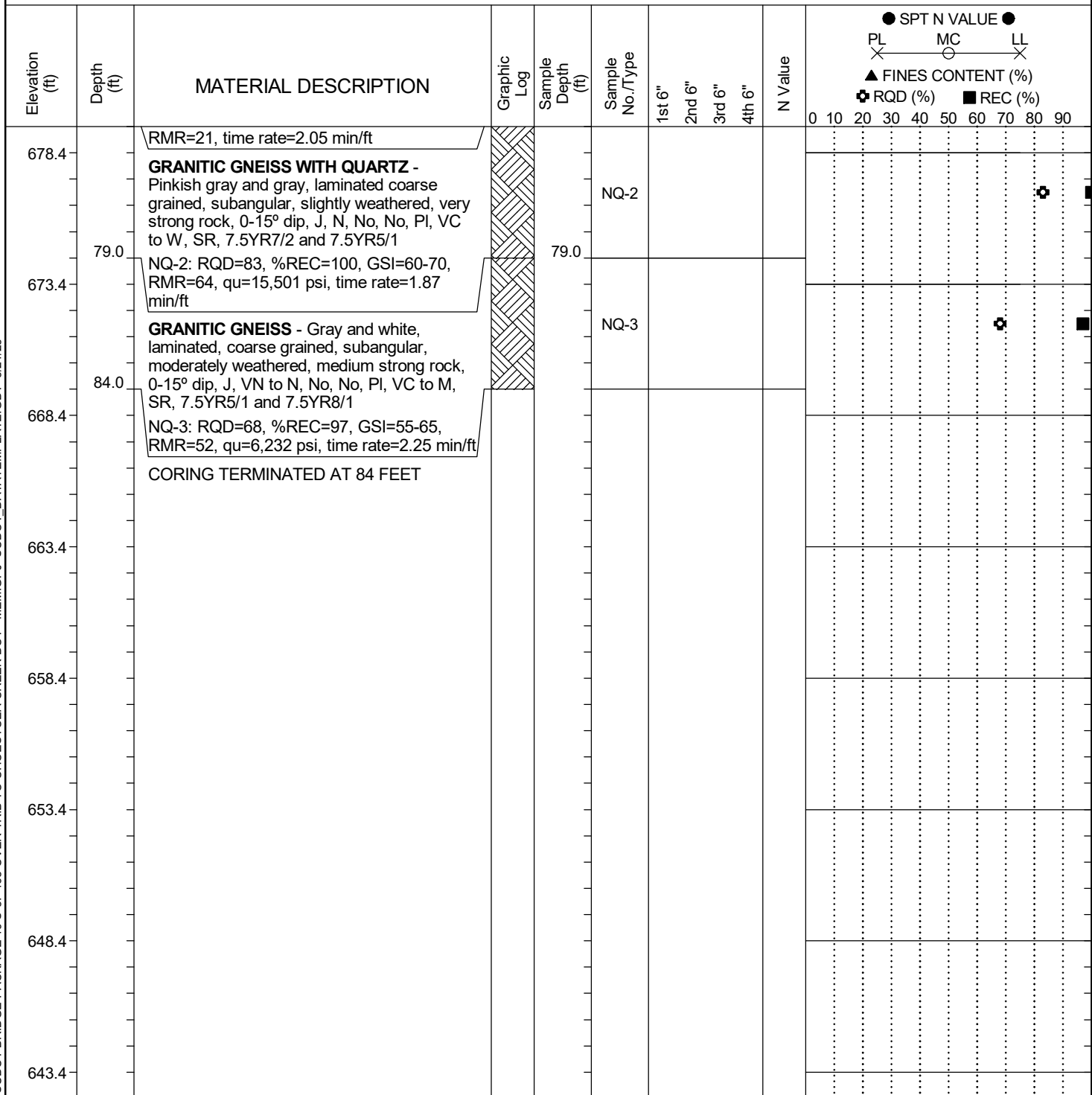
## LEGEND

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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511				<b>County:</b>	Oconee		<b>Boring No.:</b>	S-37-168-1T		
<b>Site Description:</b>		S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek						<b>Route:</b>	S-37-168		
<b>Eng./Geo.:</b>	M. McKenney		<b>Boring Location:</b>	85+07		<b>Offset:</b>	5.5R		<b>Alignment:</b>	Existing	
<b>Elev.:</b>	753.39		<b>Latitude:</b>	34.56202		<b>Longitude:</b>	-83.06072		<b>Date Started:</b>	1/17/2024	
<b>Total Depth:</b>	84 ft		<b>Soil Depth:</b>	69 ft		<b>Core Depth:</b>	15 ft		<b>Date Completed:</b>	1/18/2024	
<b>Bore Hole Diameter (in):</b> 4			<b>Sampler Configuration</b>			<b>Liner Required:</b> Y (N)		<b>Liner Used:</b> Y (N)			
<b>Drill Machine:</b>	DR# 554		<b>Drill Method:</b>	RW/RC		<b>Hammer Type:</b>	Automatic		<b>Energy Ratio:</b>	88.5%	
<b>Core Size:</b>	NQ2		<b>Driller:</b>	B. Burnette		<b>Groundwater:</b>	TOB N.M.			<b>24HR</b>	29.5 ft



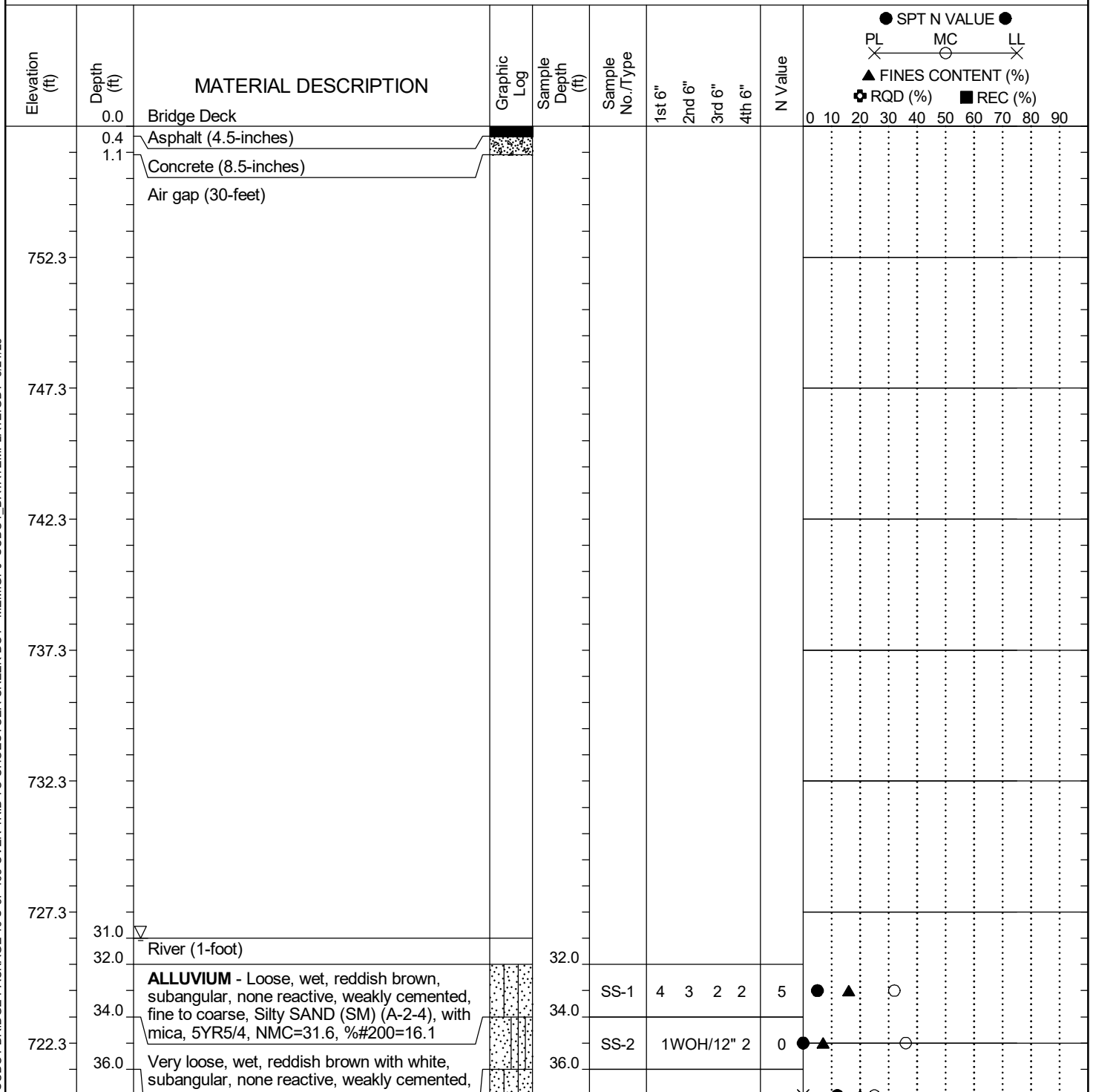
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT 8623180 SCDOT BRIDGE PACKAGE 19 S-37-168 OVER TRIB TO CHOESTOE CREEK DOT - MEM.GPJ SCDOT\_DATATEMPLATE.GDT 3/21/25

# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511				<b>County:</b>	Oconee		<b>Boring No.:</b>	S-37-168-2T			
<b>Site Description:</b>		S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek						<b>Route:</b>	S-37-168			
<b>Eng./Geo.:</b>		M. McKenney		<b>Boring Location:</b>		85+66		<b>Offset:</b>		0		
<b>Alignment:</b>		Existing		<b>Elev.:</b>		757.32		<b>Latitude:</b>		34.56192		
<b>Longitude:</b>		-83.06055		<b>Date Started:</b>		1/18/2024						
<b>Total Depth:</b>		72.5 ft		<b>Soil Depth:</b>		20.5 ft		<b>Core Depth:</b>		20 ft		
<b>Date Completed:</b>		1/19/2024										
<b>Bore Hole Diameter (in):</b>			4		<b>Sampler Configuration</b>			<b>Liner Required:</b>		Y (N)		
<b>Liner Used:</b>			Y (N)									
<b>Drill Machine:</b>			DR# 554		<b>Drill Method:</b>		RW/RC		<b>Hammer Type:</b>		Automatic	
<b>Energy Ratio:</b>			88.5%									
<b>Core Size:</b>		NQ2		<b>Driller:</b>		B. Burnette		<b>Groundwater:</b>		TOB 31 ft		
<b>24HR</b>		N.M.										



## LEGEND

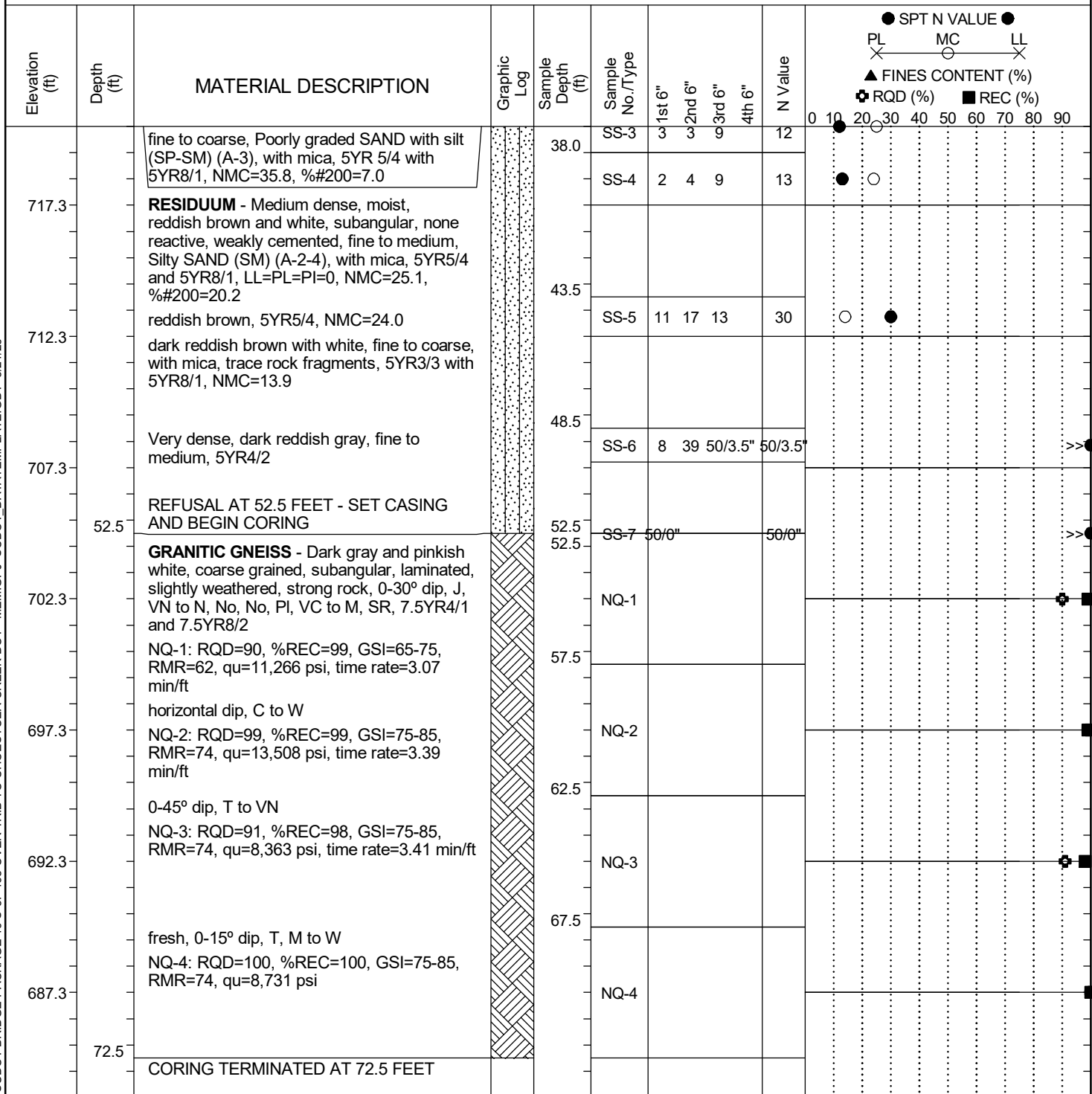
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT 8623180 SCDOT BRIDGE PACKAGE 19 S-37-168 OVER TRIB TO CHOESTOE CREEK DOT - MEM.GPJ SCDOT\_DATATEMPLATE.GDT 3/21/25

# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511				<b>County:</b>	Oconee		<b>Boring No.:</b>	S-37-168-2T				
<b>Site Description:</b>		S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek						<b>Route:</b>	S-37-168				
<b>Eng./Geo.:</b>	M. McKenney		<b>Boring Location:</b>	85+66		<b>Offset:</b>	0		<b>Alignment:</b>	Existing			
<b>Elev.:</b>	757.32		<b>Latitude:</b>	34.56192		<b>Longitude:</b>	-83.06055		<b>Date Started:</b>	1/18/2024			
<b>Total Depth:</b>		72.5 ft		<b>Soil Depth:</b>	20.5 ft		<b>Core Depth:</b>	20 ft		<b>Date Completed:</b>	1/19/2024		
<b>Bore Hole Diameter (in):</b>			4		<b>Sampler Configuration</b>			<b>Liner Required:</b>	Y (N)		<b>Liner Used:</b>	Y (N)	
<b>Drill Machine:</b>		DR# 554		<b>Drill Method:</b>	RW/RC		<b>Hammer Type:</b>	Automatic		<b>Energy Ratio:</b>	88.5%		
<b>Core Size:</b>		NQ2		<b>Driller:</b>	B. Burnette			<b>Groundwater:</b>	TOB 31 ft		<b>24HR</b>	N.M.	



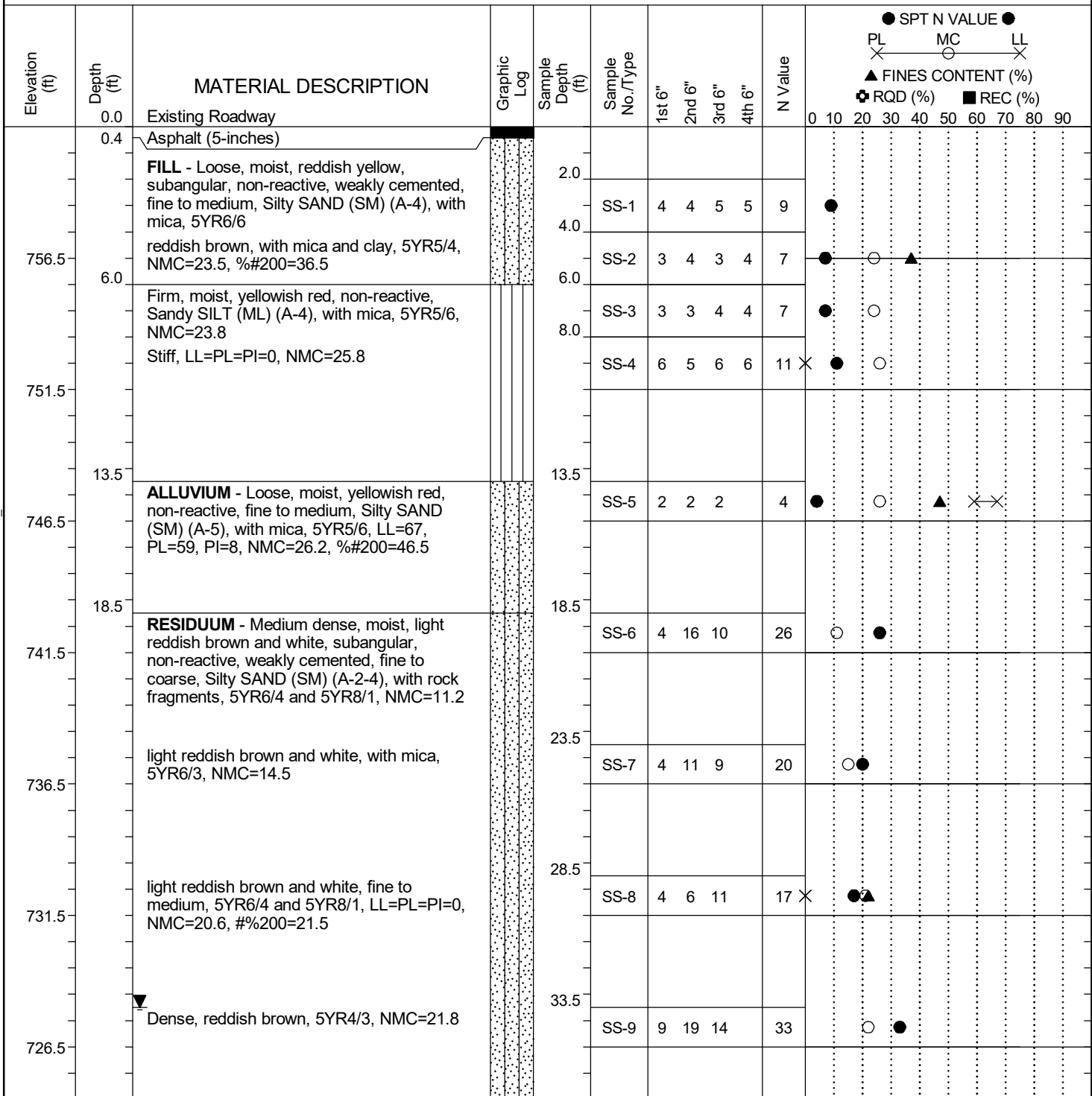
## LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS	- Split Spoon	HSA	- Hollow Stem Auger
UD	- Undisturbed Sample	RW	- Rotary Wash
AWG	- Rock Core, 1-1/8"	CFA	- Continuous Flight Augers
		RC	- Rock Core
		DC	- Driving Casing
NQ	- Rock Core, 1-7/8"		
CU	- Cuttings		
CT	- Continuous Tube		

SC DOT 8623180 SCDOT BRIDGE PACKAGE 19 S-37-168 OVER TRIB TO CHOESTOE CREEK DOT - MEM.GPJ SCDOT\_DATATEMPLATE.GDT 3/21/25

# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511	<b>County:</b>	Oconee	<b>Boring No.:</b>	S-37-168-3T
<b>Site Description:</b>	S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek			<b>Route:</b>	S-37-168
<b>Eng./Geo.:</b>	M. Mc Kenney	<b>Boring Location:</b>	86+26	<b>Offset:</b>	4L
<b>Elev.:</b>	761.53	<b>Latitude:</b>	34.56182	<b>Longitude:</b>	-83.06039
<b>Date Started:</b>	1/18/2024				
<b>Total Depth:</b>	65.5 ft	<b>Soil Depth:</b>	55.5 ft	<b>Core Depth:</b>	10 ft
<b>Date Completed:</b>	1/18/2024				
<b>Bore Hole Diameter (in):</b>	4	<b>Sampler Configuration</b>	<b>Liner Required:</b> Y (N)		<b>Liner Used:</b> Y (N)
<b>Drill Machine:</b>	DR# 554	<b>Drill Method:</b>	RW/RC	<b>Hammer Type:</b>	Automatic
<b>Energy Ratio:</b>	88.5%				
<b>Core Size:</b>	NQ2	<b>Driller:</b>	B. Burnette	<b>Groundwater:</b>	TOB N.M.
<b>24HR</b>	33.5 ft				



## LEGEND

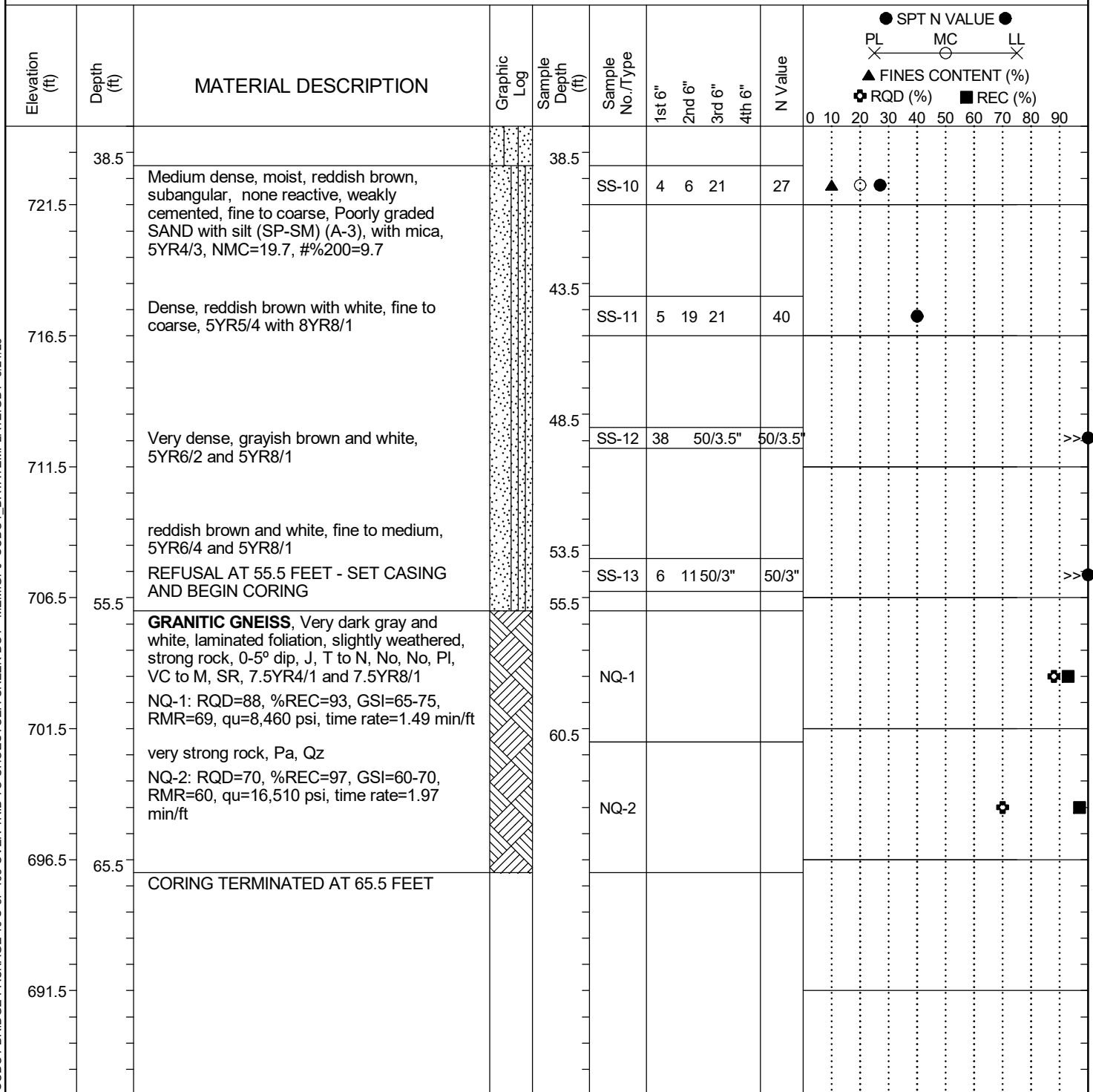
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT 8623180 SCDOT BRIDGE PACKAGE 19 S-37-168 OVER TRIB TO CHOESTOE CREEK DOT - MEM.GPJ SCDOT\_DATATEMPLATE.GDT 3/21/25

# SCDOT Soil Test Log

<b>Project ID:</b>	P0452511	<b>County:</b>	Oconee	<b>Boring No.:</b>	S-37-168-3T
<b>Site Description:</b>	S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek			<b>Route:</b>	S-37-168
<b>Eng./Geo.:</b>	M. Mc Kenney	<b>Boring Location:</b>	86+26	<b>Offset:</b>	4L
<b>Elev.:</b>	761.53	<b>Latitude:</b>	34.56182	<b>Longitude:</b>	-83.06039
<b>Total Depth:</b>	65.5 ft	<b>Soil Depth:</b>	55.5 ft	<b>Core Depth:</b>	10 ft
<b>Bore Hole Diameter (in):</b>	4	<b>Sampler Configuration</b>		<b>Liner Required:</b>	Y (N)
<b>Drill Machine:</b>	DR# 554	<b>Drill Method:</b>	RW/RC	<b>Hammer Type:</b>	Automatic
<b>Core Size:</b>	NQ2	<b>Driller:</b>	B. Burnette	<b>Groundwater:</b>	TOB N.M.
				<b>24HR</b>	33.5 ft



## LEGEND

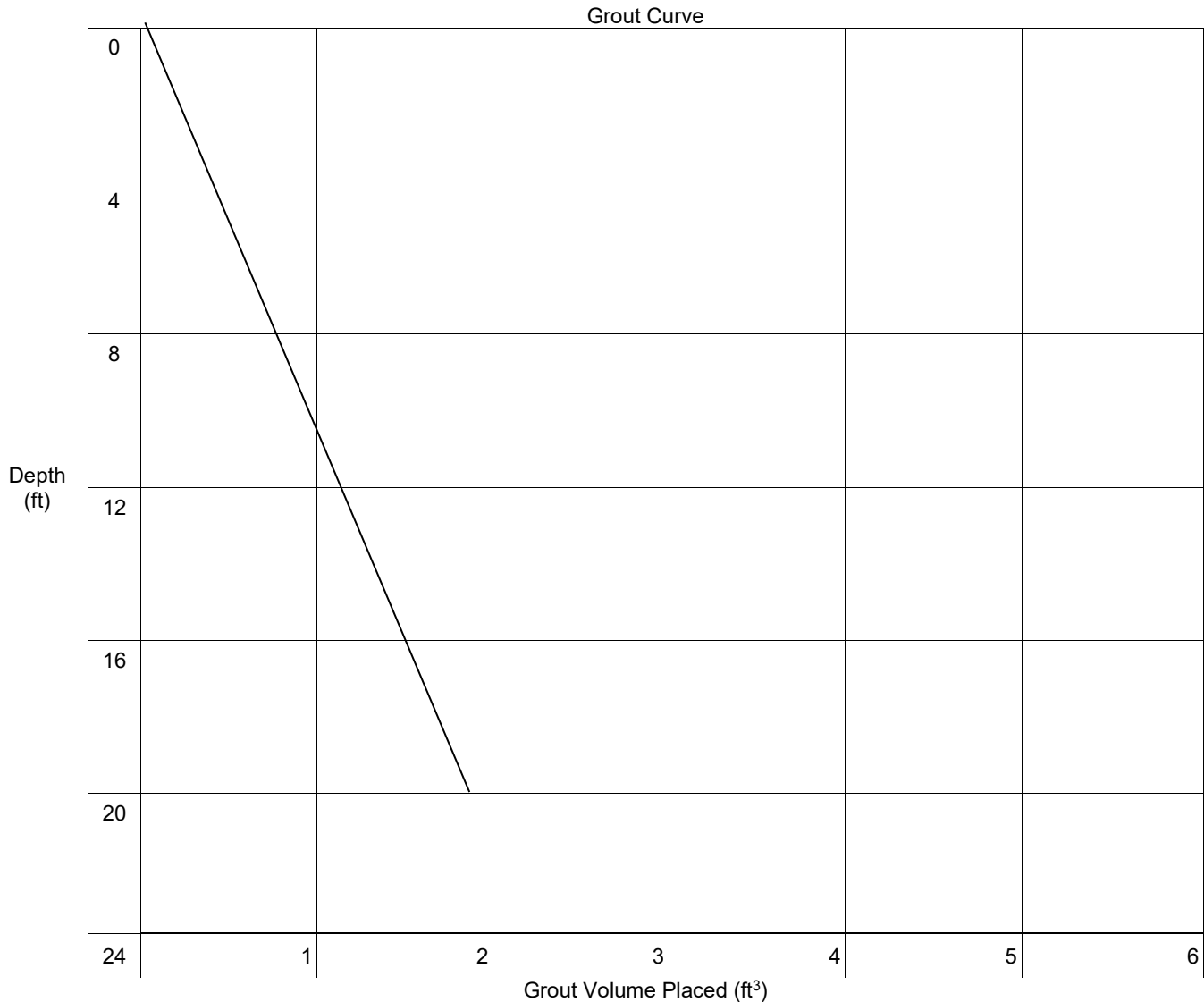
SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT 8623180 SCDOT BRIDGE PACKAGE 19 S-37-168 OVER TRIB TO CHOESTOE CREEK DOT - MEM.GPJ SCDOT\_DATATEMPLATE.GDT 3/21/25



## Exhibit A-10: GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-37-168 BRO Tributary to Choestoea Creek		Test Hole No.:	S-37-168-1T
Project ID:	P042511		Station:	85+07
Consultant Firm:	Terracon Consultants, Inc.		Offset:	5.5R
Grouted By:	Burnette	Date	1/18/24	
Notes:	Mix design: 1 pound cement mix, 1 pound bentonite, 6 pounds water			

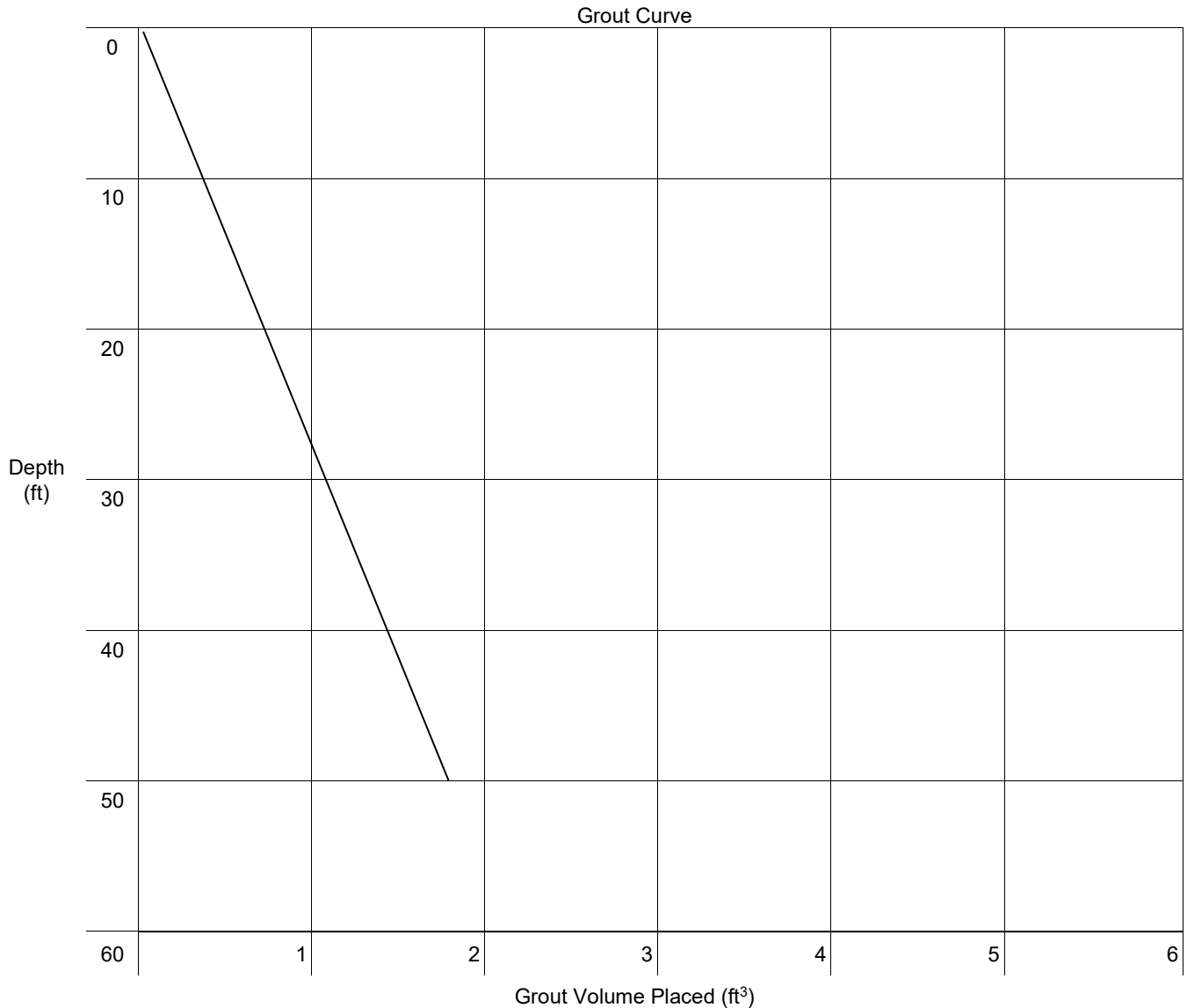


Number of Bags On-Site	15	ea.
Depth of Test Hole Grouted	20	ft.
Diameter of Test Hole	0.33	ft.
Area of Test Hole	0.09	ft²
Volume of Test Hole	1.8	ft³
Volume of Casing (If applicable)	-	ft³
Theoretical Volume of Test Hole	1.8	ft³
Number of Bags Used	3	ea.
Volume Placed	1.7	ft³



## Exhibit A-10: GROUT LOG OF TEST HOLES FOR GEOTECHNICAL ON-CALL

Project Name:	S-37-168 BRO Tributary to Choestoea Creek		Test Hole No.:	S-37-168-3T	
Project ID:	P042511		Station:	86+26	
Consultant Firm:	Terracon Consultants, Inc.	Date	1/19/2024	Offset:	4L
Grouted By:	Burnette				
Notes:	Mix design: 1 pound cement mix, 1 pound bentonite, 6 pounds water				



Number of Bags On-Site	15	ea.
Depth of Test Hole Grouted	20	ft.
Diameter of Test Hole	0.33	ft.
Area of Test Hole	0.09	ft²
Volume of Test Hole	1.8	ft³
Volume of Casing (If applicable)	--	ft³
Theoretical Volume of Test Hole	1.8	ft³
Number of Bags Used	2.9	ea.
Volume Placed	1.7	ft³



**Rock Core Photograph Logs – Exhibit A-11**

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC

Terracon Project No. 8623P180 | SCDOT Project ID: P0452511



**NQ-1**



S-37-168-1 Trib, NQ-1 and NQ-2 (69 to 79 feet)

**NQ-3**



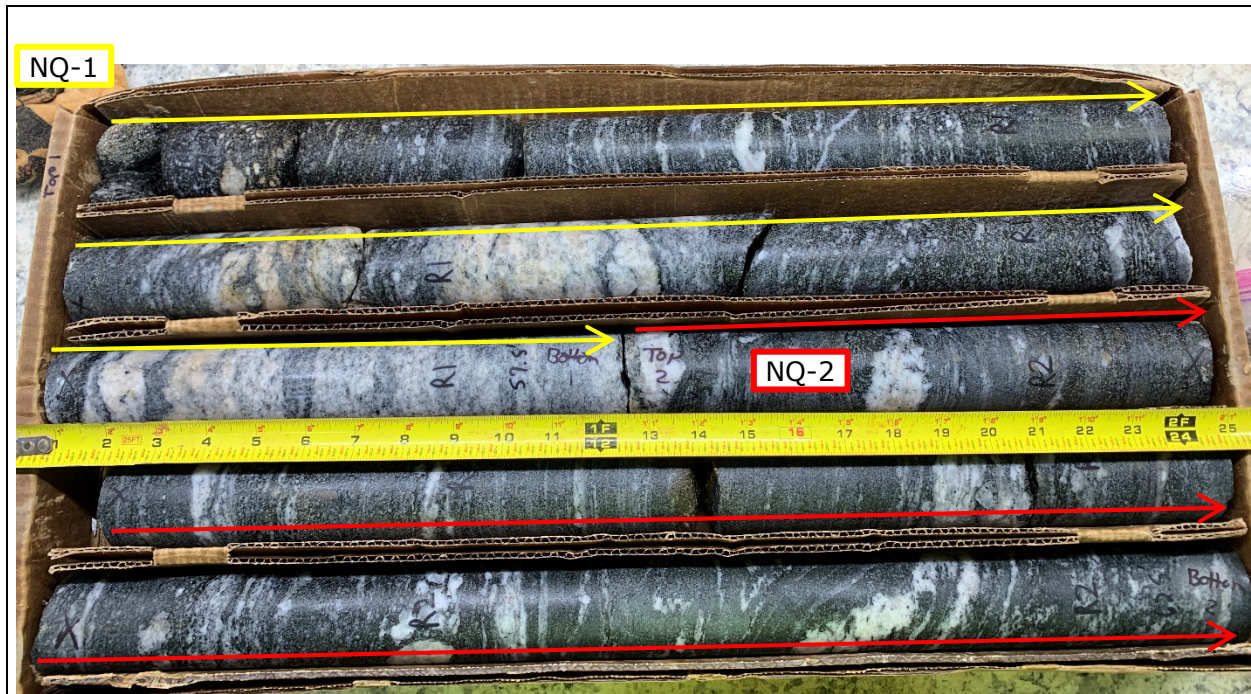
S-37-168-1 Trib, NQ-3 (79 to 84 feet)



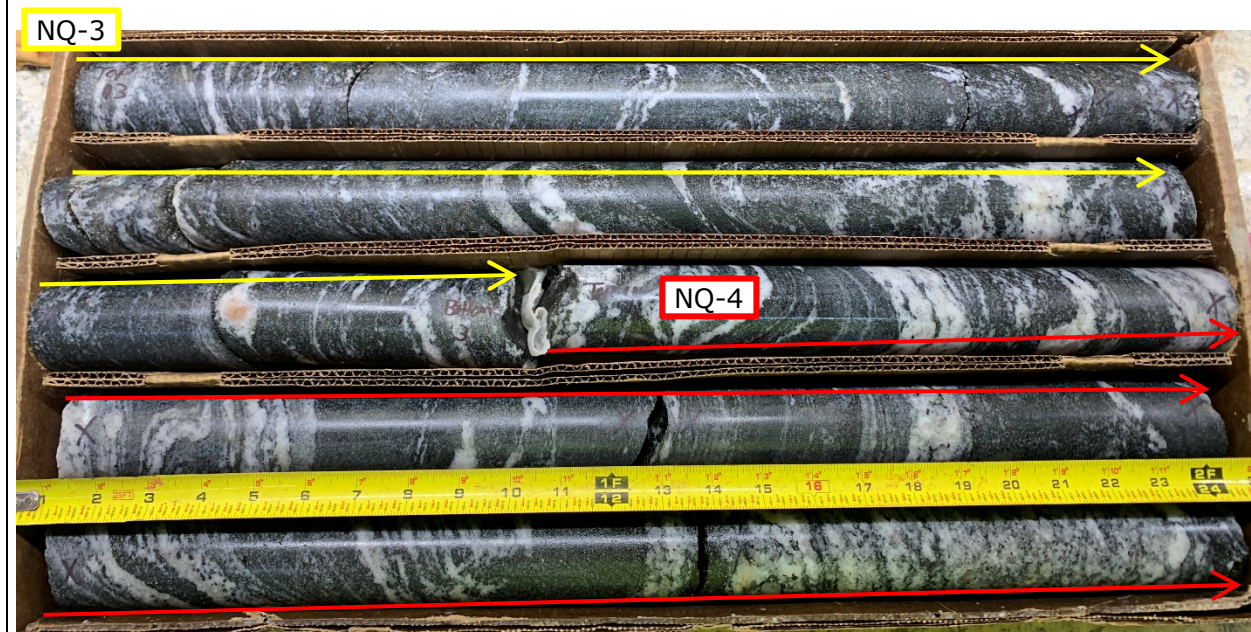
**Rock Core Photograph Logs – Exhibit A-11**

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC

Terracon Project No. 8623P180 | SCDOT Project ID: P0452511



S-37-168-2 Trib, NQ-1 and NQ-2 (52.5 to 62.5 feet)



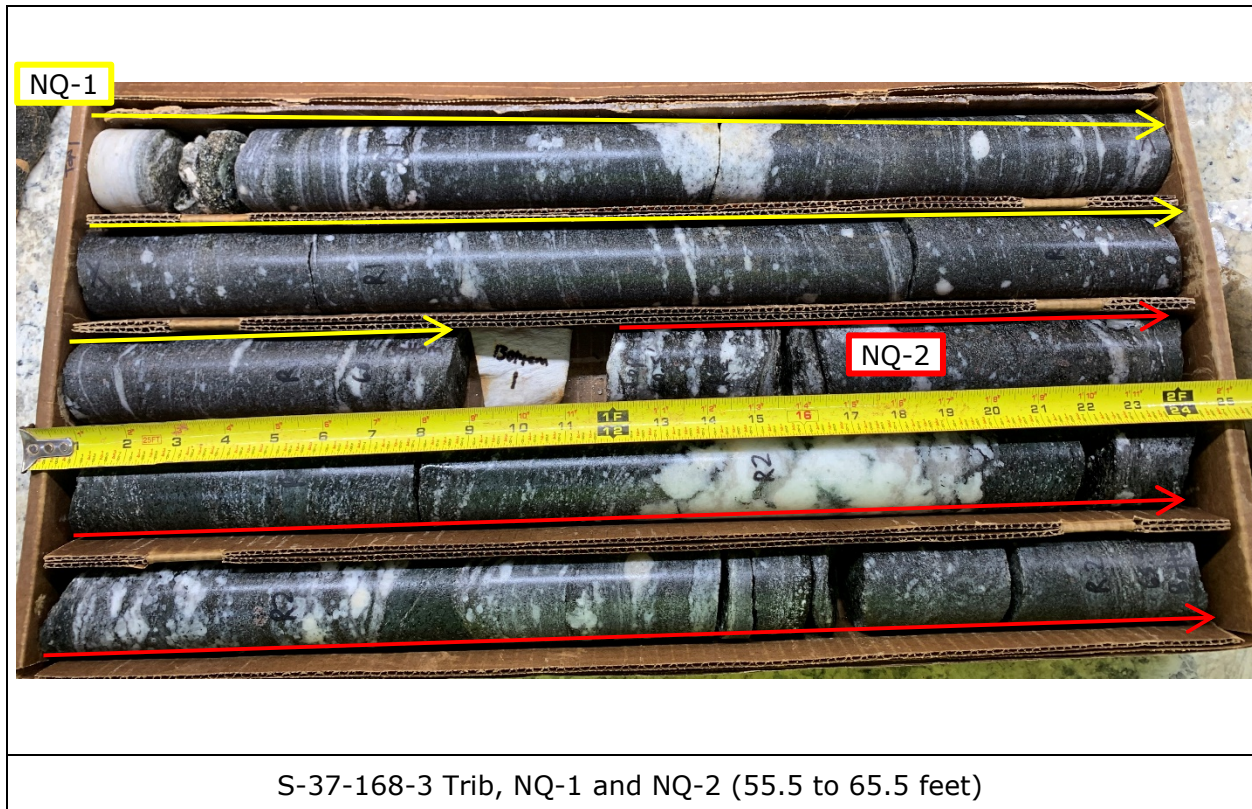
S-37-168-2 Trib, NQ-3 and NQ-4 (62.5 to 72.5 feet)



**Rock Core Photograph Logs – Exhibit A-11**

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC

Terracon Project No. 8623P180 | SCDOT Project ID: P0452511



S-37-168-3 Trib, NQ-1 and NQ-2 (55.5 to 65.5 feet)

**Appendix B – Laboratory Testing**

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC  
Terracon Project No. 8623P180 | SCDOT Project ID: P042511



## **Appendix B**

### **Laboratory Testing**

Exhibit B-1 – Laboratory Testing Description  
Summary of Laboratory Data  
Laboratory Data Sheets (28 Pages)

Note: All exhibits are one page unless noted above.

## Exhibit B-1 – Laboratory Testing Description

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC  
Terracon Project No. 8623P180 | SCDOT Project ID: P042511



### Laboratory Testing Description

The samples collected during the field exploration were taken to our laboratory for additional testing. The laboratory testing scope was developed by the SCDOT and laboratory assignment was performed by Terracon. The laboratory tests were conducted on selected soil samples from the borings and the bulk sample locations. The test results are presented in this appendix.

The laboratory test results were used to confirm the soil descriptions presented on the boring logs in Appendix A. Laboratory tests were performed in general accordance with the applicable ASTM, AASHTO, SCDOT or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

■ Moisture Content	AASHTO T265/(ASTM D2216)
■ Atterberg Limits	AASHTO T89/T90(ASTM D4318)
■ Proctor (Standard effort)	AASHTO T99/ (ASTM D698)
■ Triaxial Shear CU w/ PP	AASHTO T297/(ASTM D4767)
■ Grain Size Distribution	ASTM D6913
■ Hydrometer	ASTM D7928
■ Compressive Strength of Rock Cores	ASTM D7012
■ Corrosion Series	AASHTO D422
	AASHTO T289/ASTM G51
	AASHTO T290/ASTM C1580
	AASHTO T291



# INDEX PROPERTIES VERSUS DEPTH

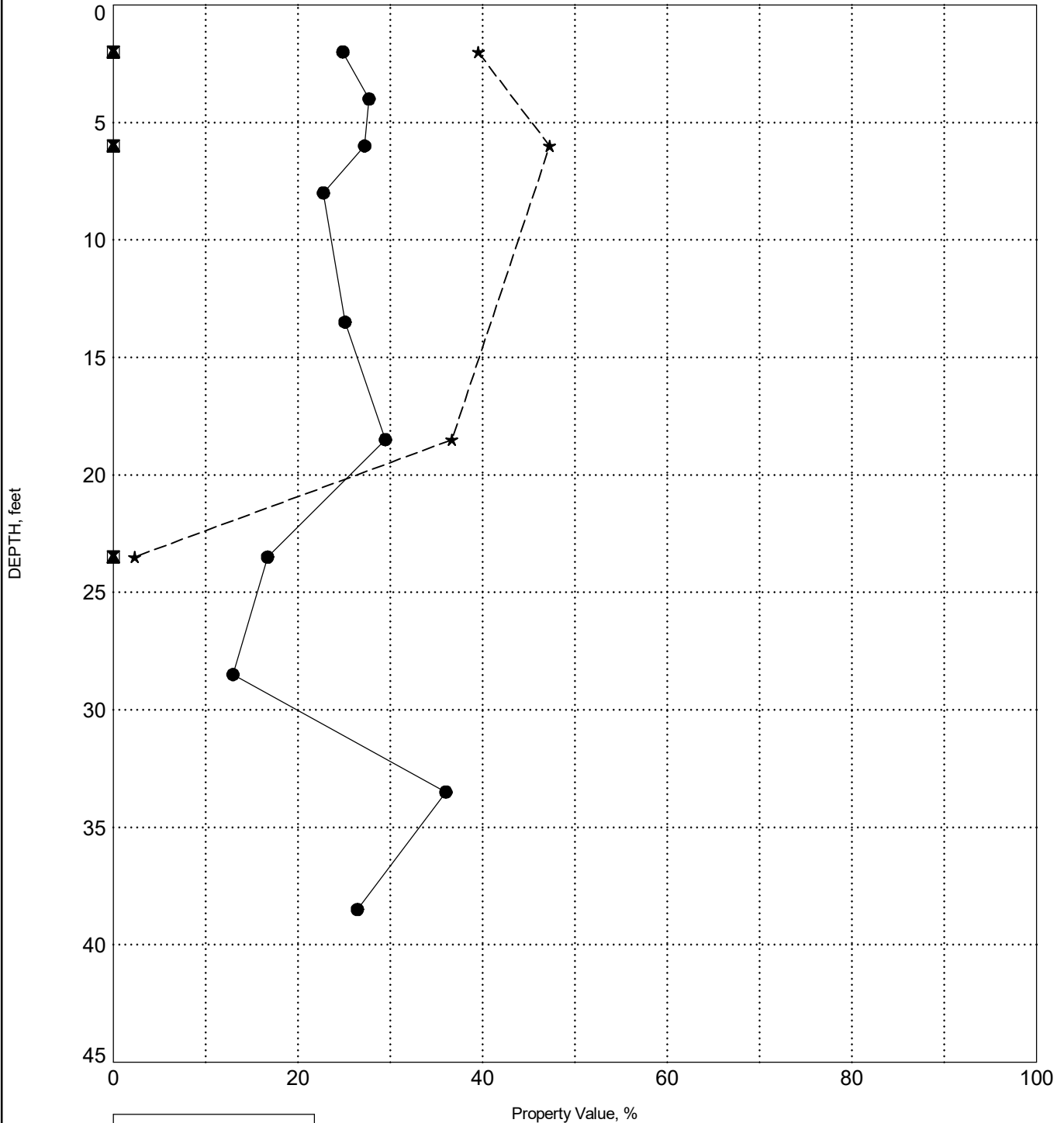
PROJECT ID P0452511

PROJECT NAME S-37-168T (Little Choestoea Road) BRO Tributary to Choestoea Cree

PROJECT COUNTY Oconee

SURFACE ELEVATION: 753.4

## BORING S-37-168-1T



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines



# INDEX PROPERTIES VERSUS DEPTH

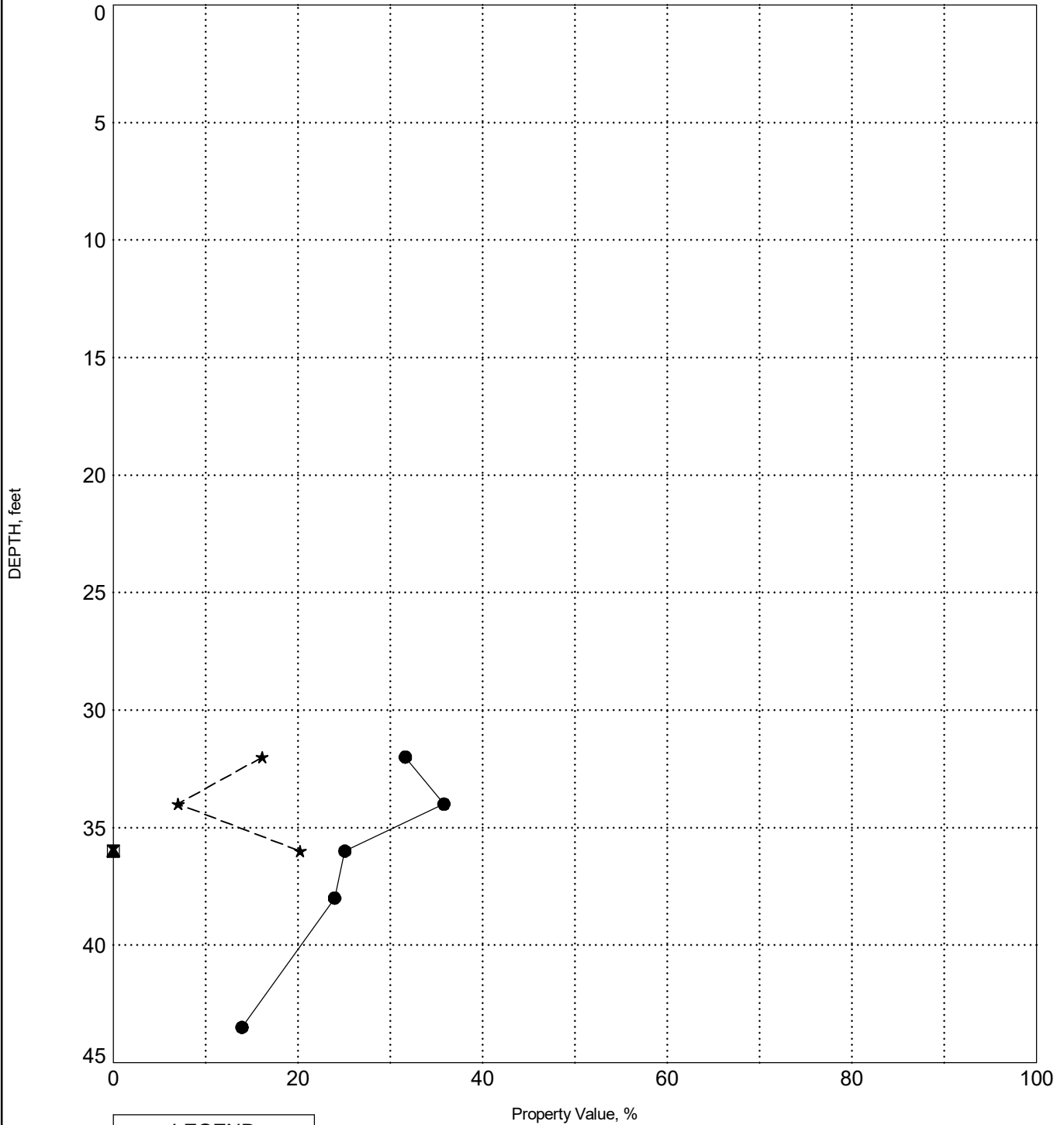
PROJECT ID P0452511

PROJECT NAME S-37-168T (Little Choestoea Road) BRO Tributary to Choestoea Cree

PROJECT COUNTY Oconee

SURFACE ELEVATION: 757.3

## BORING S-37-168-2T



LEGEND	
●	Water Content
■	Plastic Limit
▲	Liquid Limit
★	Fines



# INDEX PROPERTIES VERSUS DEPTH

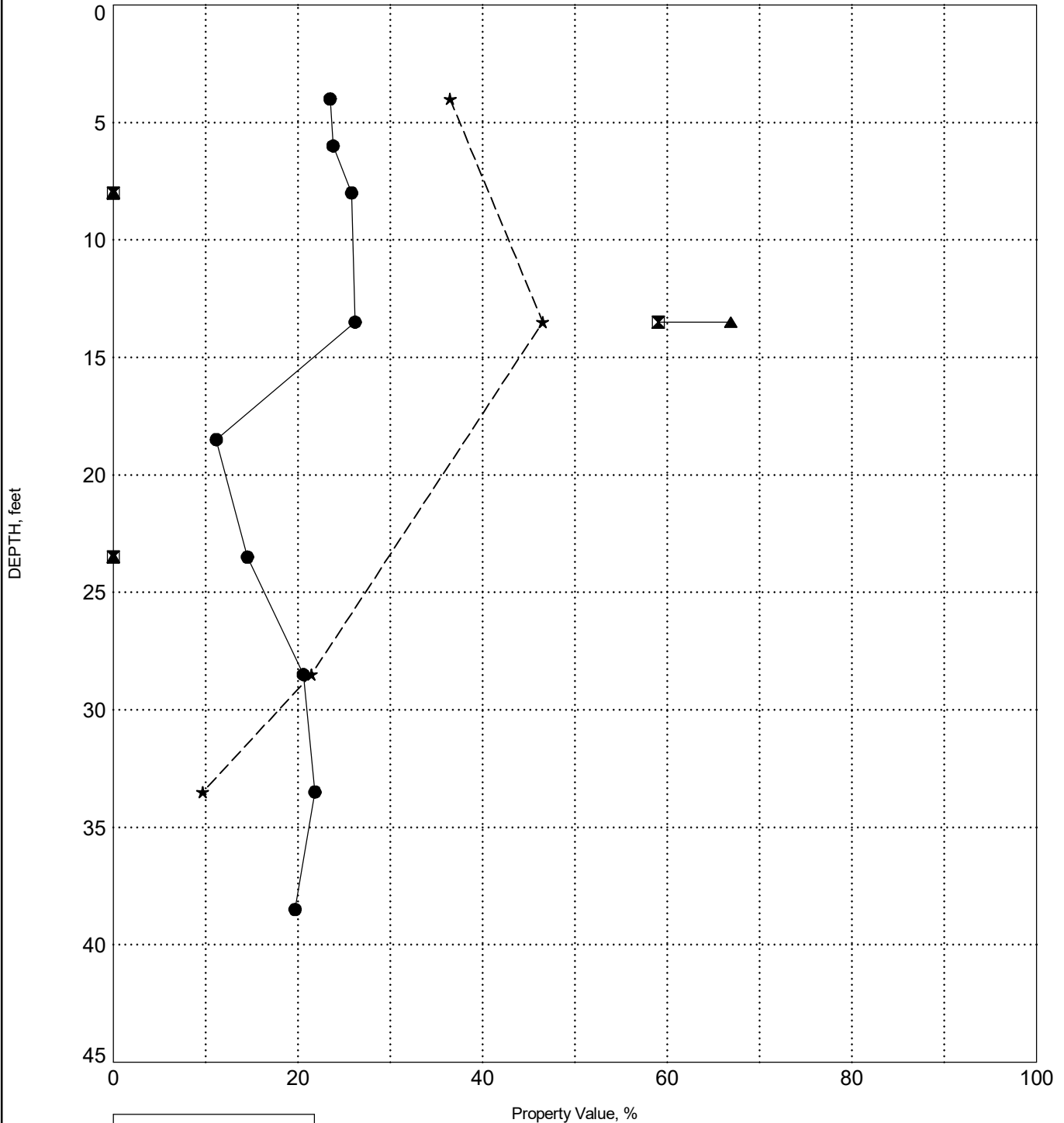
PROJECT ID P0452511

PROJECT NAME S-37-168T (Little Choestoea Road) BRO Tributary to Choestoea Cree

PROJECT COUNTY Oconee

SURFACE ELEVATION: 761.5

## BORING S-37-168-3T



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines



Summary of Laboratory Results

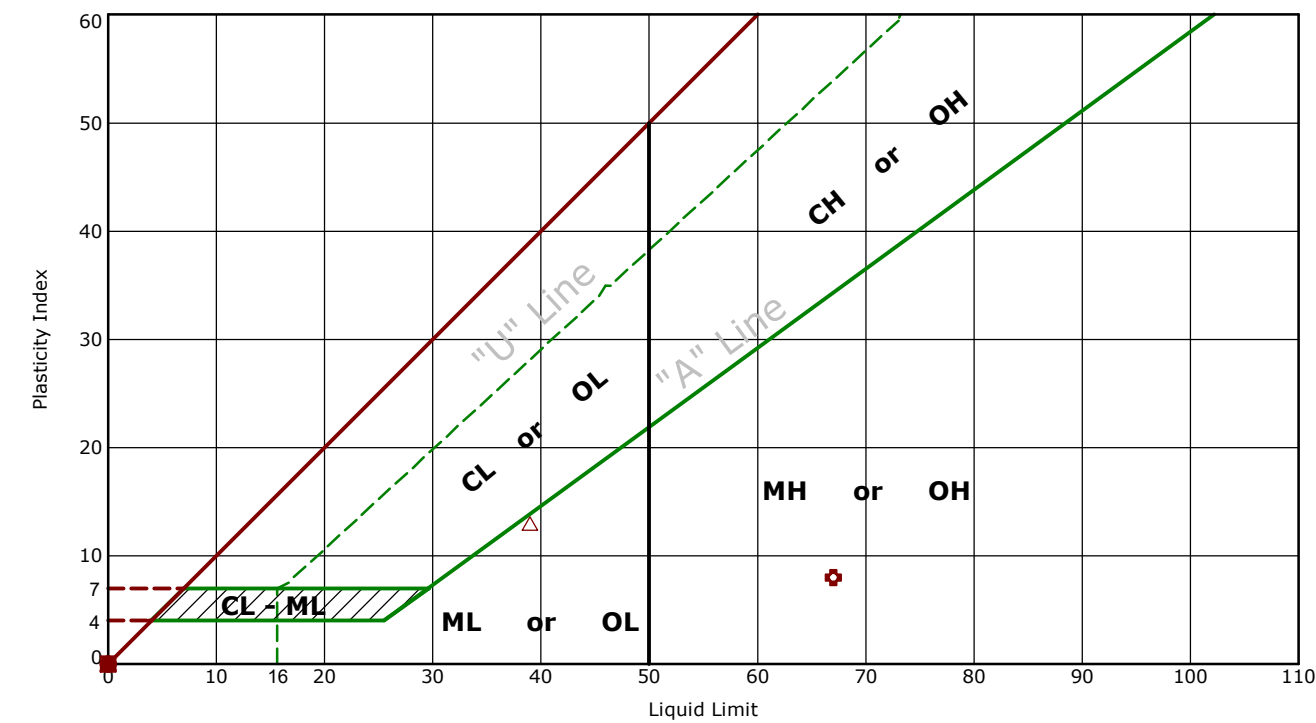
Boring ID	Depth (Ft.)	Soil Classification USCS & AASHTO	Liquid Limit	Plastic Limit	Plasticity Index	% Gravel	% Sand	% Fines	% Silt	% Clay	Water Content (%)	Proctor Dry Density (pcf)/Opt. Moisture (%)
S-37-168-1T	2-4	SILTY SAND(SM) / A-4 (0)	NP	NP	NP	1.4	59.0	39.5			24.9	
S-37-168-1T	4-6	SILTY SAND(SM) / A-4 (0)									27.7	
S-37-168-1T	6-8	SILTY SAND(SM) / A-4 (0)	NP	NP	NP	1.4	51.4	47.2			27.2	
S-37-168-1T	8-10	SILTY SAND(SM) / A-4 (0)									22.7	
S-37-168-1T	13.5-15	SILTY SAND(SM) / A-4 (0)									25.1	
S-37-168-1T	18.5-20	SILTY SAND(SM) / A-4 (0)				0.4	63.0	36.7	18.8	17.8	29.5	
S-37-168-1T	23.5-25	POORLY GRADED SAND(SP) / A-3 (0)	NP	NP	NP	0.9	94.2	2.3	0.6	1.7	16.7	
S-37-168-1T	28.5-30	SILTY SAND(SM) / A-2-4 (0)									13.0	
S-37-168-1T	33.5-35	SILTY SAND(SM) / A-2-4 (0)									36.0	
S-37-168-1T	38.5-40	SILTY SAND(SM) / A-2-4 (0)									26.4	
S-37-168-2T	32-34	SILTY SAND(SM) / A-2-4 (0)				0.0	82.9	16.1			31.6	
S-37-168-2T	34-36	POORLY GRADED SAND WITH SILT(SP-SM) / A-3 (0)				1.7	91.2	7.0	4.7	2.3	35.8	
S-37-168-2T	36-38	SILTY SAND(SM) / A-2-4 (0)	NP	NP	NP	0.0	78.9	20.2			25.1	
S-37-168-2T	38-40	SILTY SAND(SM) / A-2-4 (0)									24.0	
S-37-168-2T	43.5-45	SILTY SAND(SM) / A-2-4 (0)									13.9	
S-37-168-3T	4-6	SILTY SAND(SM) / A-4 (0)				1.2	62.4	36.5			23.5	
S-37-168-3T	6-8	SANDY SILT(ML) / A-4									23.8	
S-37-168-3T	8-10	SANDY SILT(ML) / A-4	NP	NP	NP						25.8	
S-37-168-3T	13.5-15	SILTY SAND(SM) / A-5 (3)	67	59	8	0.7	52.8	46.5	14.1	32.4	26.2	

Summary of Laboratory Results

Boring ID	Depth (Ft.)	Soil Classification USCS & AASHTO	Liquid Limit	Plastic Limit	Plasticity Index	% Gravel	% Sand	% Fines	% Silt	% Clay	Water Content (%)	Proctor Dry Density (pcf)/Opt. Moisture (%)
S-37-168-3T	18.5-20	SILTY SAND(SM) / A-2-4 (0)									11.2	
S-37-168-3T	23.5-25	SILTY SAND(SM) / A-2-4 (0)									14.5	
S-37-168-3T	28.5-30	SILTY SAND(SM) / A-2-4 (0)	NP	NP	NP	0.0	78.5	21.5			20.6	
S-37-168-3T	33.5-35	SILTY SAND(SM) / A-2-4 (0)									21.8	
S-37-168-3T	38.5-40	POORLY GRADED SAND WITH SILT(SP-SM) / A-3 (0)				0.7	89.7	9.7			19.7	
S-37-168-1&3T Bulk	1-5	SILTY SAND(SM) / A-6 (3)	39	26	13	0.7	54.2	45.1			46.3	105.6 / 18.4

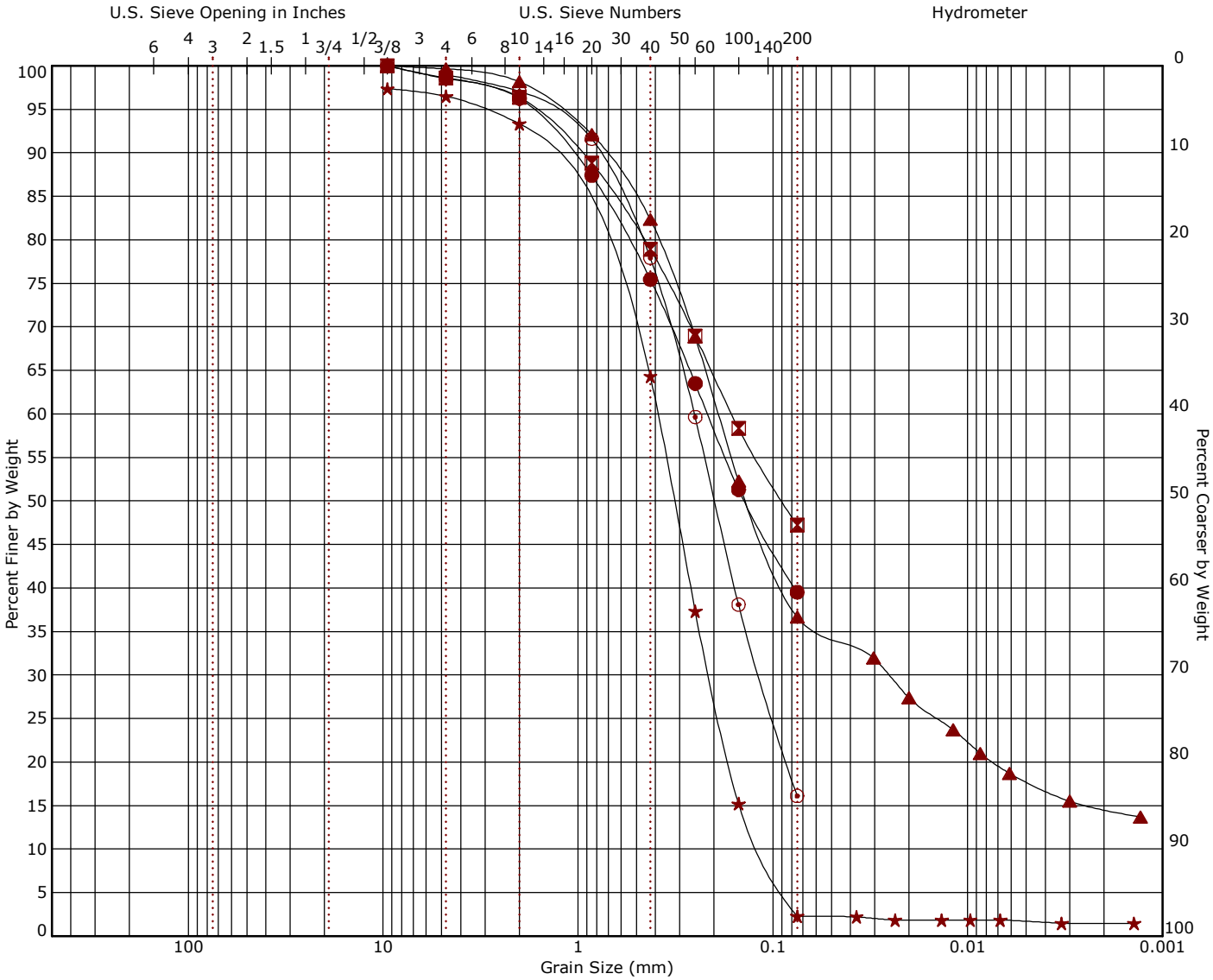
# Atterberg Limit Results

ASTM D4318



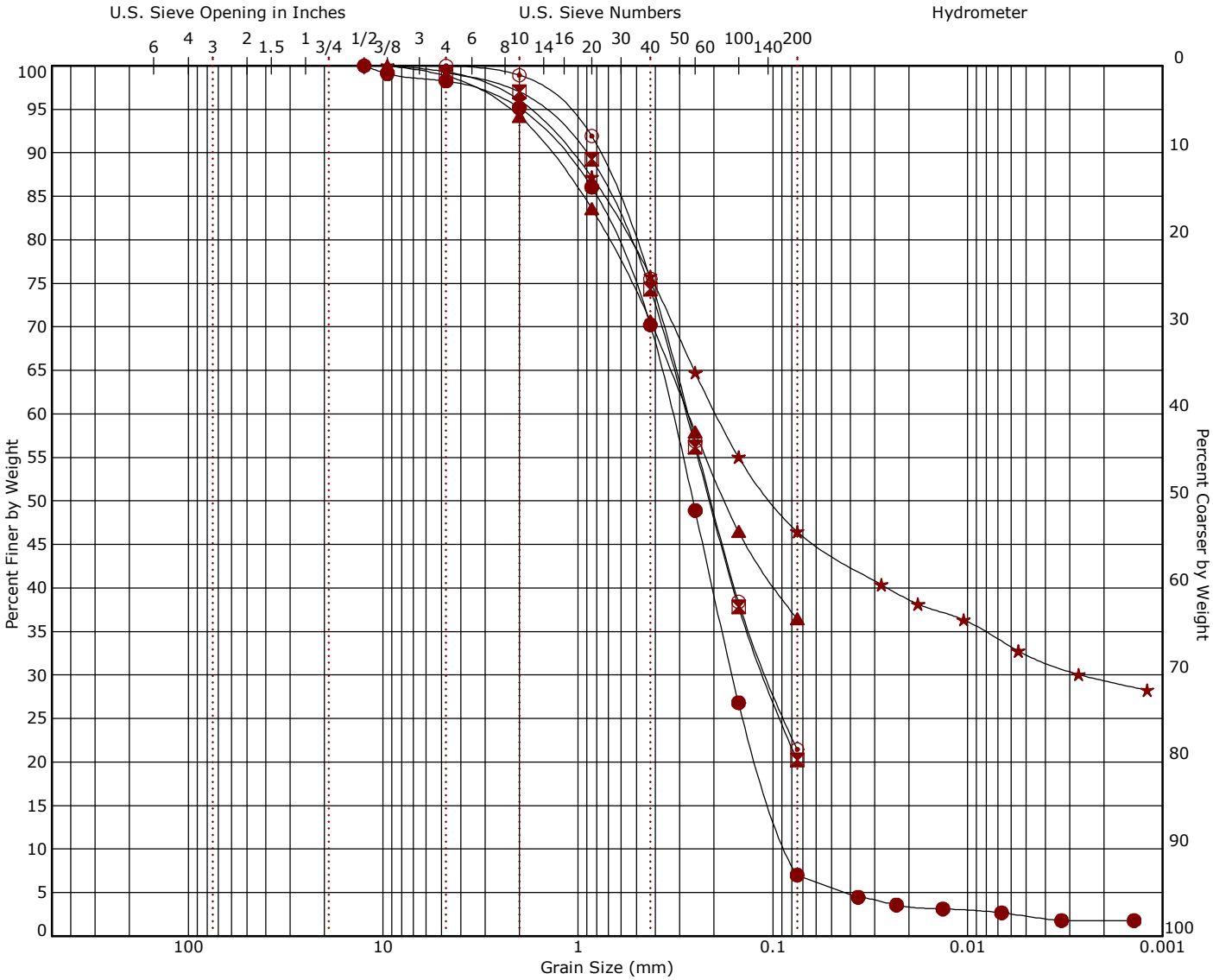
	Boring ID	Depth (Ft)	LL	PL	PI	Fines	AASHTO	Description
●	S-37-168-1T	2 - 4	NP	NP	NP	39.5	A-4 (0)	SILTY SAND
⊠	S-37-168-1T	6 - 8	NP	NP	NP	47.2	A-4 (0)	SILTY SAND
▲	S-37-168-1T	23.5 - 25	NP	NP	NP	2.3	A-3 (0)	POORLY GRADED SAND
★	S-37-168-2T	36 - 38	NP	NP	NP	20.2	A-2-4 (0)	SILTY SAND
⊙	S-37-168-3T	8 - 10	NP	NP	NP		A-4	SANDY SILT
⊕	S-37-168-3T	13.5 - 15	67	59	8	46.5	A-5 (3)	SILTY SAND
○	S-37-168-3T	28.5 - 30	NP	NP	NP	21.5	A-2-4 (0)	SILTY SAND
△	S-37-168-1&3T Bulk	1 - 5	39	26	13	45.1	A-6 (3)	SILTY SAND

**Grain Size Distribution**  
**ASTM D422 / ASTM C136**

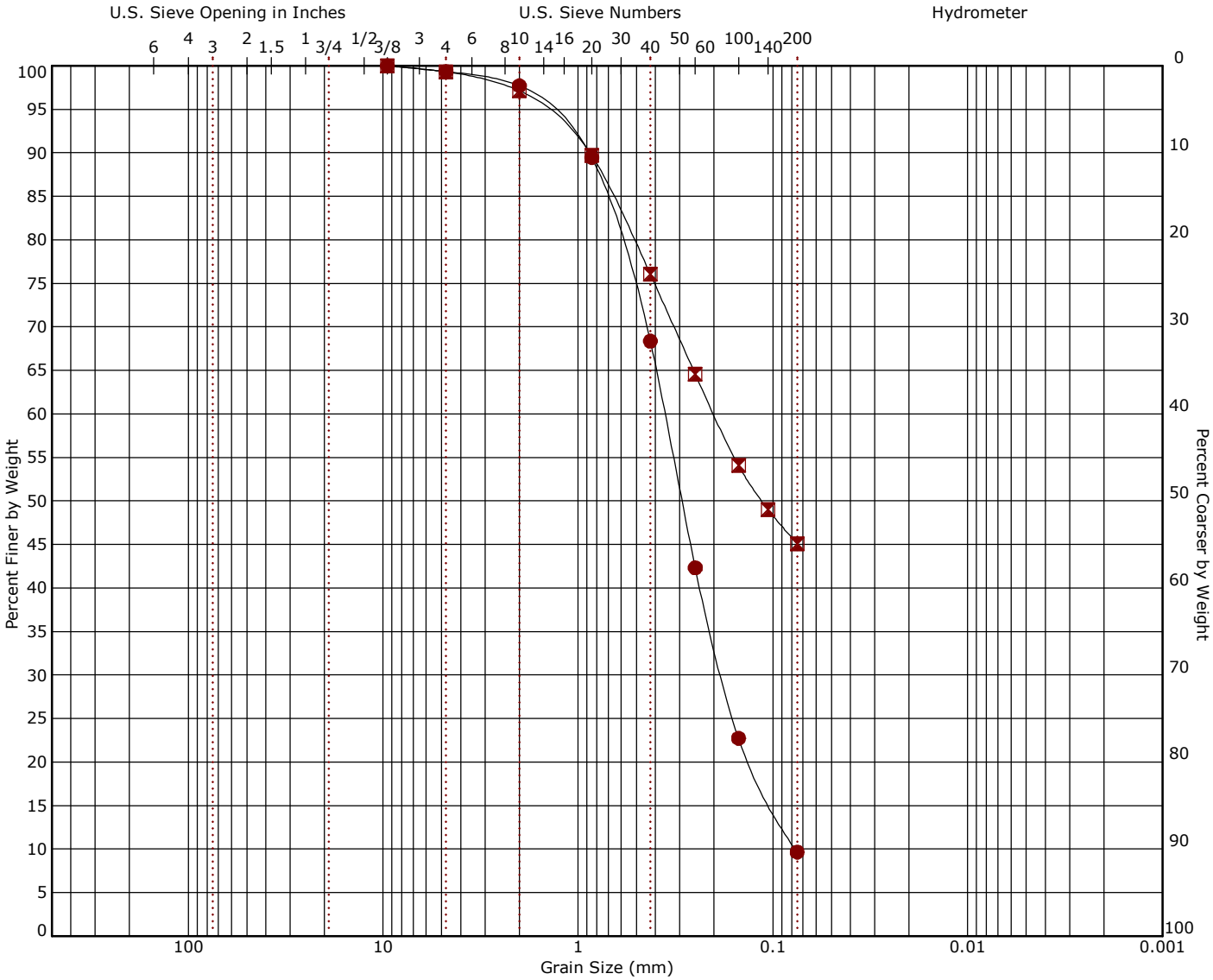


Cobbles		Gravel		Sand			Silt or Clay				
		coarse	fine	coarse	medium	fine					
Boring ID	Depth (Ft)	USCS Classification			USCS	AASHTO	LL	PL	PI	Cc	Cu
● S-37-168-1T	2 - 4	SILTY SAND			SM	A-4 (0)	NP	NP	NP		
■ S-37-168-1T	6 - 8	SILTY SAND			SM	A-4 (0)	NP	NP	NP		
▲ S-37-168-1T	18.5 - 20	SILTY SAND			SM	A-4 (0)					
★ S-37-168-1T	23.5 - 25	POORLY GRADED SAND			SP	A-3 (0)	NP	NP	NP	1.01	3.44
⊙ S-37-168-2T	32 - 34	SILTY SAND			SM	A-2-4 (0)					
Boring ID	Depth (Ft)	D <sub>100</sub>	D <sub>60</sub>	D <sub>30</sub>	D <sub>10</sub>	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
● S-37-168-1T	2 - 4	9.5	0.216			0.0	1.4	59.0	39.5		
■ S-37-168-1T	6 - 8	9.5	0.163			0.0	1.4	51.4	47.2		
▲ S-37-168-1T	18.5 - 20	9.5	0.19	0.025		0.0	0.4	63.0		18.8	17.8
★ S-37-168-1T	23.5 - 25	9.5	0.39	0.211	0.113		0.9	94.2		0.6	1.7
⊙ S-37-168-2T	32 - 34	4.75	0.253	0.116				82.9	16.1		

**Grain Size Distribution**  
**ASTM D422 / ASTM C136**

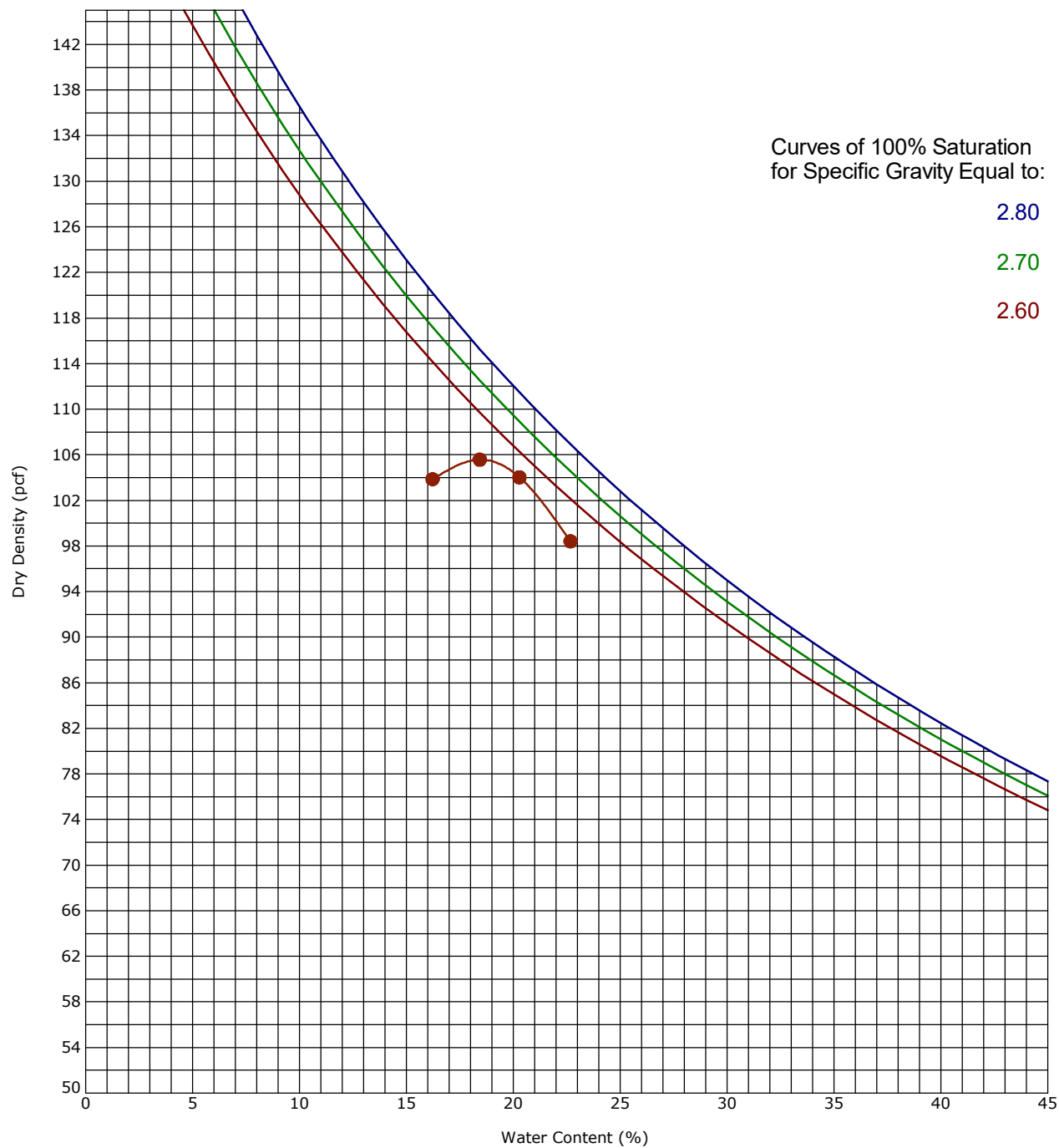


Grain Size Distribution  
ASTM D422 / ASTM C136

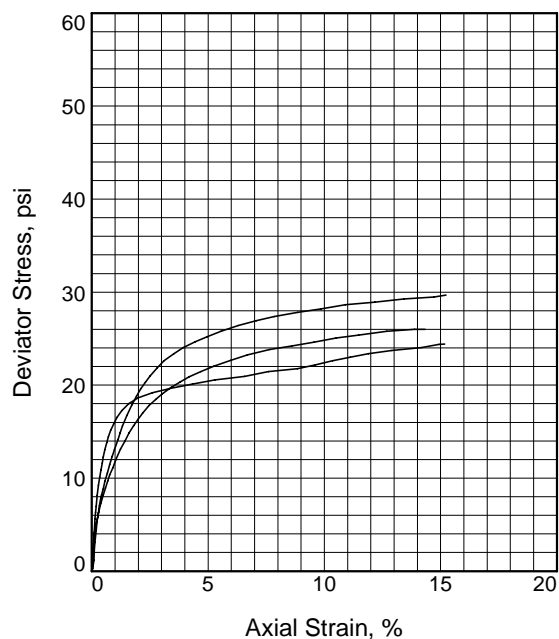
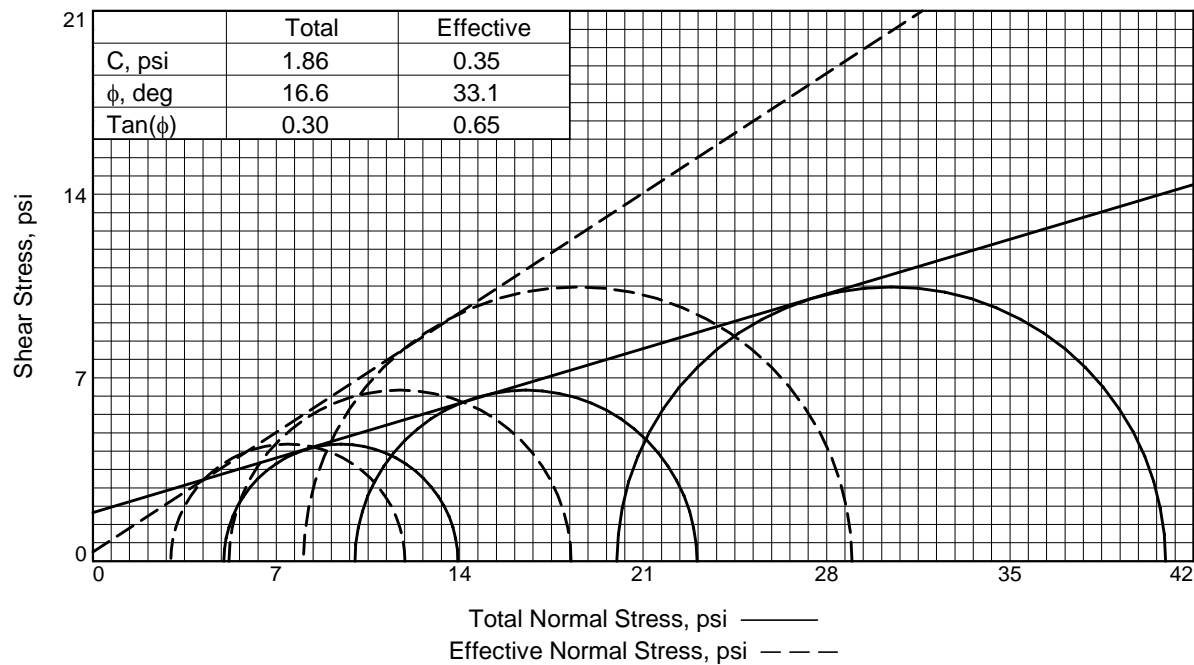


# Moisture-Density Relationship

## ASTM D698-Method B



Boring ID		Depth (Ft)		Description of Materials			
S-37-168-1&3T Bulk		1 - 5		SILTY SAND(SM)			
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
45	0.0	39	26	13	ASTM D698-Method B	105.6	18.4



Sample No.		1	2	3
Initial	Water Content, %	18.5	18.5	18.5
	Dry Density, pcf	100.2	100.2	100.2
	Saturation, %	73.2	73.2	73.2
	Void Ratio	0.6822	0.6822	0.6822
	Diameter, in.	2.85	2.85	2.85
	Height, in.	6.00	6.00	6.00
At Test	Water Content, %	24.9	22.8	22.3
	Dry Density, pcf	100.2	103.9	104.2
	Saturation, %	98.6	98.8	97.5
	Void Ratio	0.6822	0.6229	0.6181
	Diameter, in.	2.85	2.81	2.82
	Height, in.	6.00	5.96	5.91
Strain rate, in./min.		0.001	0.001	0.001
Back Pressure, psi		50.0	50.0	50.0
Cell Pressure, psi		55.0	60.0	70.0
Fail. Stress, psi		8.9	13.1	20.9
Excess Pore Pr., psi		2.0	4.8	12.0
Ult. Stress, psi				
Excess Pore Pr., psi				
$\bar{\sigma}_1$ Failure, psi		11.9	18.2	29.0
$\bar{\sigma}_3$ Failure, psi		3.0	5.2	8.0

#### Type of Test:

CU with Pore Pressures

#### Sample Type:

Description: Red Silty Sand (SM)

LL= 39

PL= 26

PI= 13

Assumed Specific Gravity= 2.7

Remarks: Three Specimen Series

Client: HNTB North Carolina PC

Project: SCDOT Bridge Package 19

Source of Sample: Bulks

Depth: 1.0-5.0 ft

Sample Number: S-37-168-1&3Trib-OS

Proj. No.: 8623P180

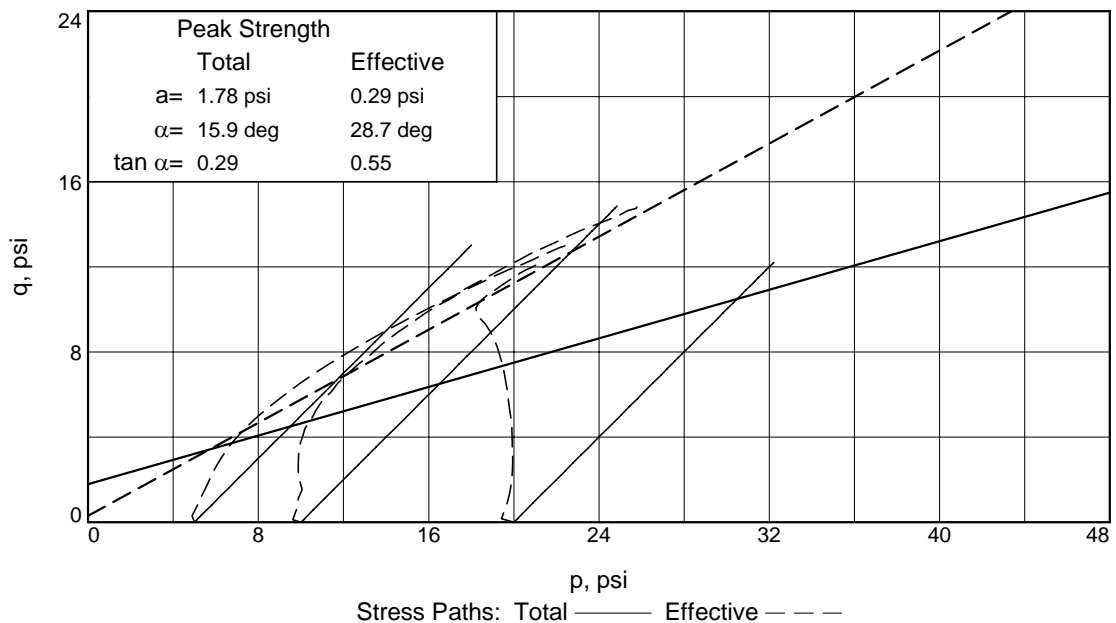
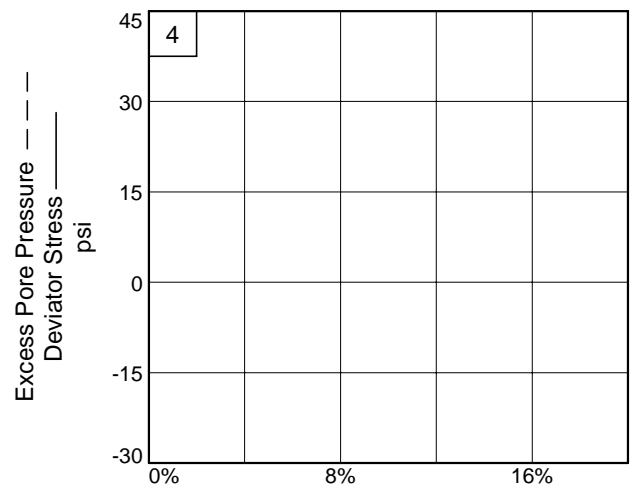
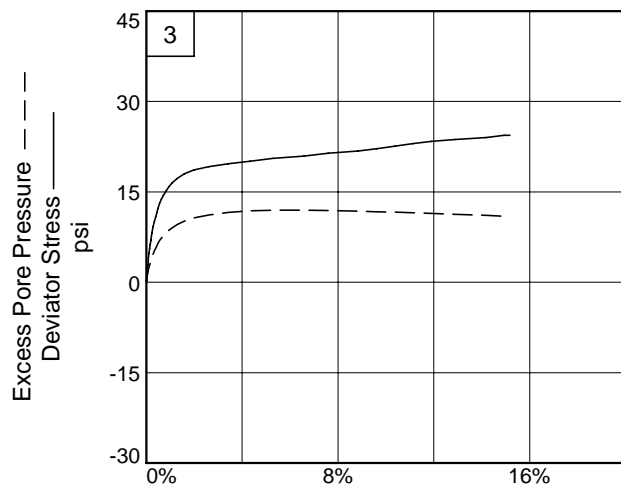
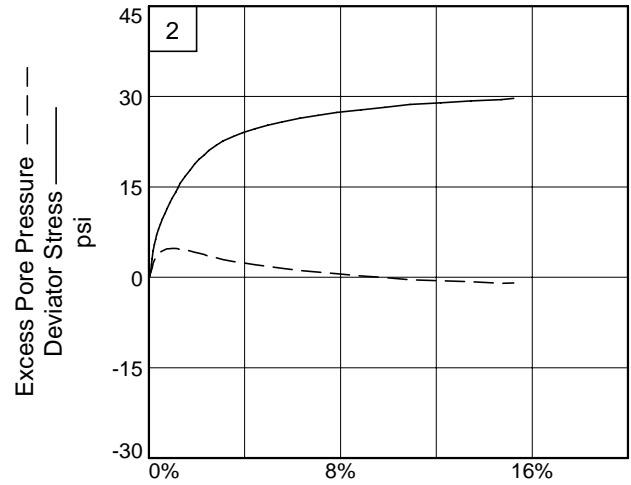
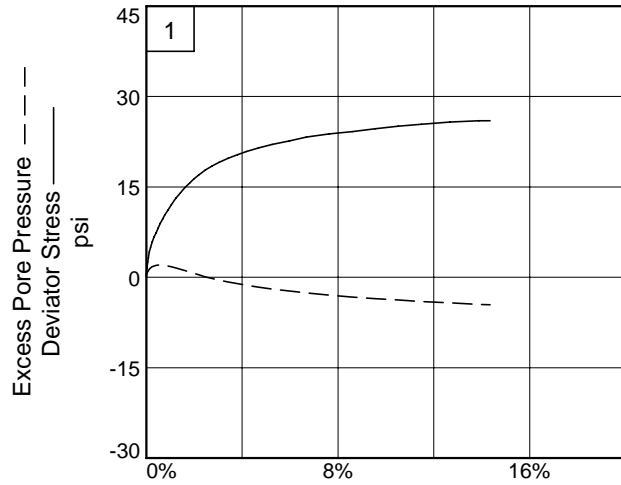
Date Sampled: 1/13/25

TRIAXIAL SHEAR TEST REPORT

Terracon Consultants, Inc.

North Charleston, South Carolina





**Client:** HNTB North Carolina PC

**Project:** SCDOT Bridge Package 19

**Source of Sample:** Bulks

**Depth:** 1.0-5.0 ft

**Sample Number:** S-37-168-1&3Trib-OS

**Project No.:** 8623P180

**Terracon Consultants, Inc.**

750 Pilot Road, Suite F  
Las Vegas, Nevada 89119  
(702) 597-9393



## Client

HNTB North Carolina PC

## Project

SCDOT Bridge Package 19 - Tributary to Choestoea Creek

**Sample Submitted By:** Terracon (86)

**Date Received:** 2/7/2025

**Lab No.:** 25-0056

## Results of Corrosion Analysis

<b>Sample Number</b>	--
<b>Sample Location</b>	S-37-168-3
<b>Sample Depth (ft.)</b>	2.0-25.0
pH Analysis, AASHTO T289	5.11
Water Soluble Sulfate (SO <sub>4</sub> ), AASHTO T290 (mg/kg)	118
Sulfides, ASTM D4658, (ppm)	Nil
Red-Ox, ASTM G200, (mV)	+729
Chlorides, AASHTO T291, (mg/kg)	205
Saturated Minimum Resistivity, ASTM G-57, (ohm cm)	3149

**Analyzed By**

A handwritten signature in black ink, appearing to read 'N. Campo'.

Nathan Campo  
Laboratory Coordinator

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.



# Rock Coring Summary

PAGE 1 OF 1

PROJECT ID P0452511

PROJECT NAME S-37-168 (Little Choestoea Road) BRO Tributary to Choestoea Creek

PROJECT COUNTY Oconee

Borehole	Core Run Number	Core Run Top Depth	REC (%)	RQD (%)	q <sub>u</sub> (psi)	Poisson's Ratio	Secant Modulus (ksi)	Unit Weight (pcf)	RMR	GSI
S-37-168-1T	NQ-1	69.0	75	18					29	30
S-37-168-1T	NQ-2	74.0	100	83	15501	0.13	733	172	64	65
S-37-168-1T	NQ-3	79.0	97	68	6232	0.16	264	163	52	60
S-37-168-2T	NQ-1	52.5	99	90	11266	0.069	113	181	62	70
S-37-168-2T	NQ-2	57.5	99	99	13508	0.16	669	171	72	80
S-37-168-2T	NQ-3	62.5	98	91	8363	0.18	362	168	72	80
S-37-168-2T	NQ-4	67.5	100	100	8731	0.17	405	167	72	80
S-37-168-3T	NQ-1	55.5	93	88	8460	0.18	346	167	69	85
S-37-168-3T	NQ-2	60.5	97	70	16510	0.16	842	173	60	65



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures

ASTM D 7012

Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/14/2025

Sample No. S-37-168-1T Run/Sam No.: NQ-2  
Depth (ft): 75.9'-77.8'  
Description: Very Pale Brown / Light Gray Marble Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness	Maximum Gap $\leq 0.020$ in.					Tolerance Met	
End Flatness: Max.	Diameter 1a	in	Diameter 1b	in	$\leq 0.0010$	Tolerance Met	
End Flatness: Max.	Diameter 2a	in	Diameter 2b	in	$\leq 0.0010$	Tolerance Met	
Perpendicularity Slope	Diameter 1a		Diameter 1b		$\leq 0.0043$	Tolerance Met	
Perpendicularity Slope	Diameter 2a		Diameter 2b		$\leq 0.0043$	Tolerance Met	

Length (in): 1) 4.179 2) 4.180 3) 4.180 Avg. 4.179 in

Diameter (in): 1) 1.980 2) 1.980 3) 1.959 Avg. 1.973 in

Uniaxial Compressive Strength: 15,501 psi Mass: 575.3 g

Load: 47,377 lbs. Wet Unit Weight: 171.6 pcf

L/D: 2.1 Dry Unit Weight: 171.0 pcf

Water Content: 0.3 %

Time to Failure: 3.20 min

Load Rate: 247 lbs/sec

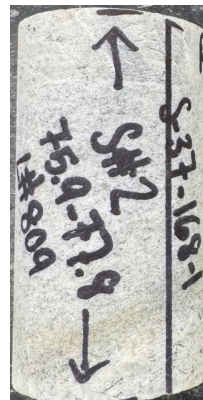
Young's Modulus	
Et (50% Co)	7.33E+05

Poisson's Ratio	
ut (50% Co)	0.128

REMARKS:

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- Prepared in general accordance to ASTM D4543
- 
- 
- 

Before



After



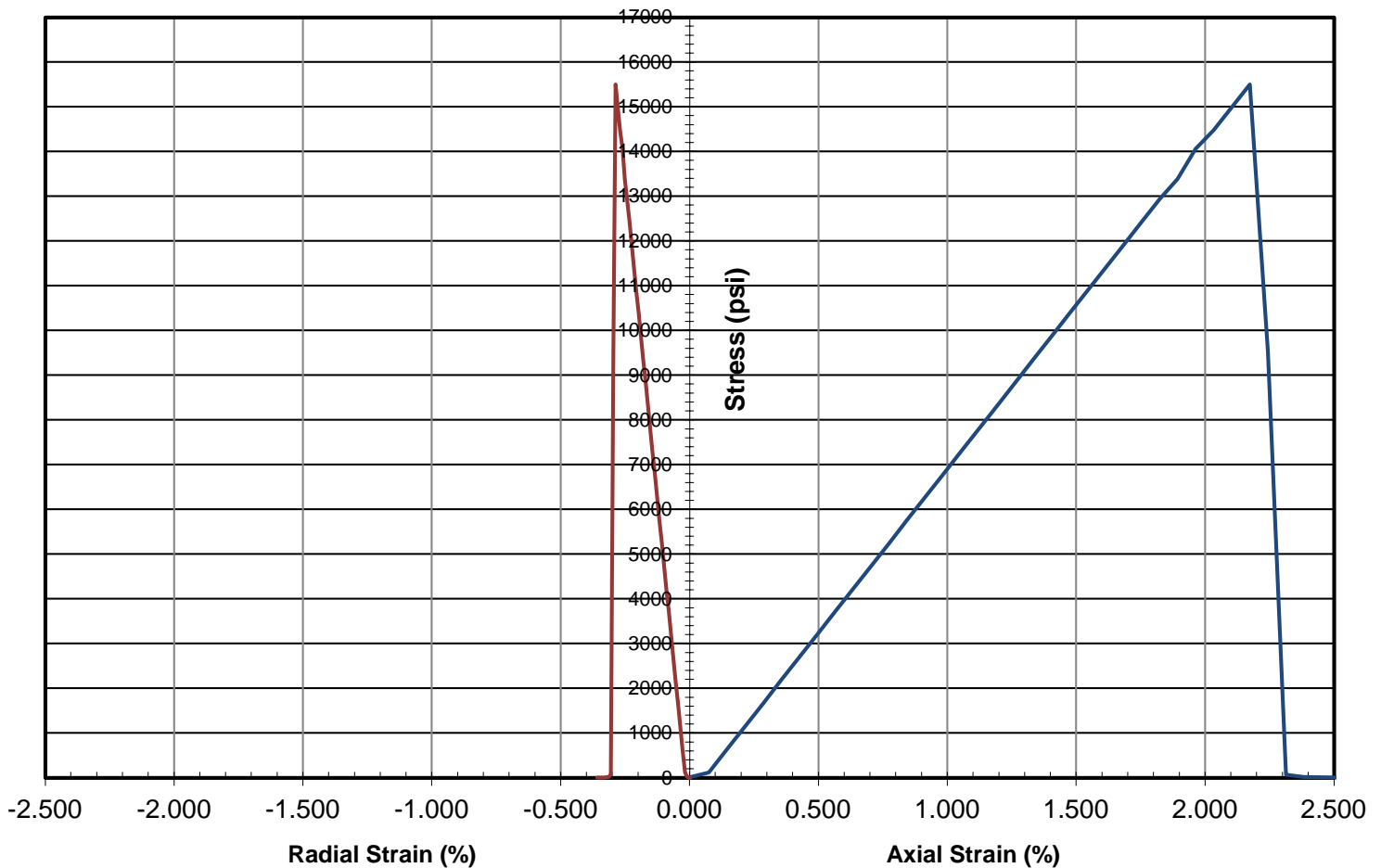


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-1T	Run No.:	NQ-2	Date: 03/14/25
Depth (ft):	75.9'-77.8'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



Young's Modulus

$C_o$  Max = 15,501 psi  
50%  $C_o$  Max 7,751 psi  
10%  $C_o$  Max 1,550 psi

$E_t$  (50%)  $C_o$  = 7.33E+05 psi

Poisson's Ratio

$C_o$  Max = 15,501 psi  
50%  $C_o$  Max 7,751 psi  
10%  $C_o$  Max 1,550 psi

$\nu_t$  (50%)  $C_o$  = 0.128



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures

ASTM D 7012

Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/13/2025

Sample No. S-37-168-1T Run/Sam No.: NQ-3  
Depth (ft): 81.5'-82.3'  
Description: Light Brownish Gray / Light Yellowish Brown Gneiss Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness		Maxumum Gap ≤ 0.020 in.						Tolerance Met	Yes
End Flatness: Max.	Diameter 1a	0.0007	in	Diameter 1b	0.0008	in	≤ 0.0010	Tolerance Met	Yes
End Flatness: Max.	Diameter 2a	0.0008	in	Diameter 2b	0.0008	in	≤ 0.0010	Tolerance Met	Yes
Perpendicularity Slope	Diameter 1a	0.00035		Diameter 1b	0.00040		≤ 0.0043	Tolerance Met	Yes
Perpendicularity Slope	Diameter 2a	0.00040		Diameter 2b	0.00040		≤ 0.0043	Tolerance Met	Yes

Length (in): 1) 4.245 2) 4.245 3) 4.244 Avg. 4.244 in

Diameter (in): 1) 1.981 2) 1.980 3) 1.980 Avg. 1.980 in

Uniaxial Compressive Strength: 6,232 psi Mass: 557.6 g

Load: 19,191 lbs. Wet Unit Weight: 162.5 pcf

L/D: 2.1 Dry Unit Weight: 161.2 pcf

Water Content: 0.8 %

Time to Failure: 3.30 min

Load Rate: 97 lbs/sec

Young's Modulus

Et (50% Co) 2.64E+05

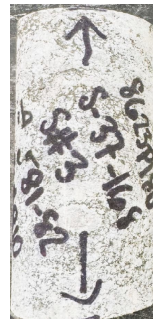
Poisson's Ratio

ut (50% Co) 0.156

REMARKS:

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- 
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- 
- 
- 
- 

Before



After



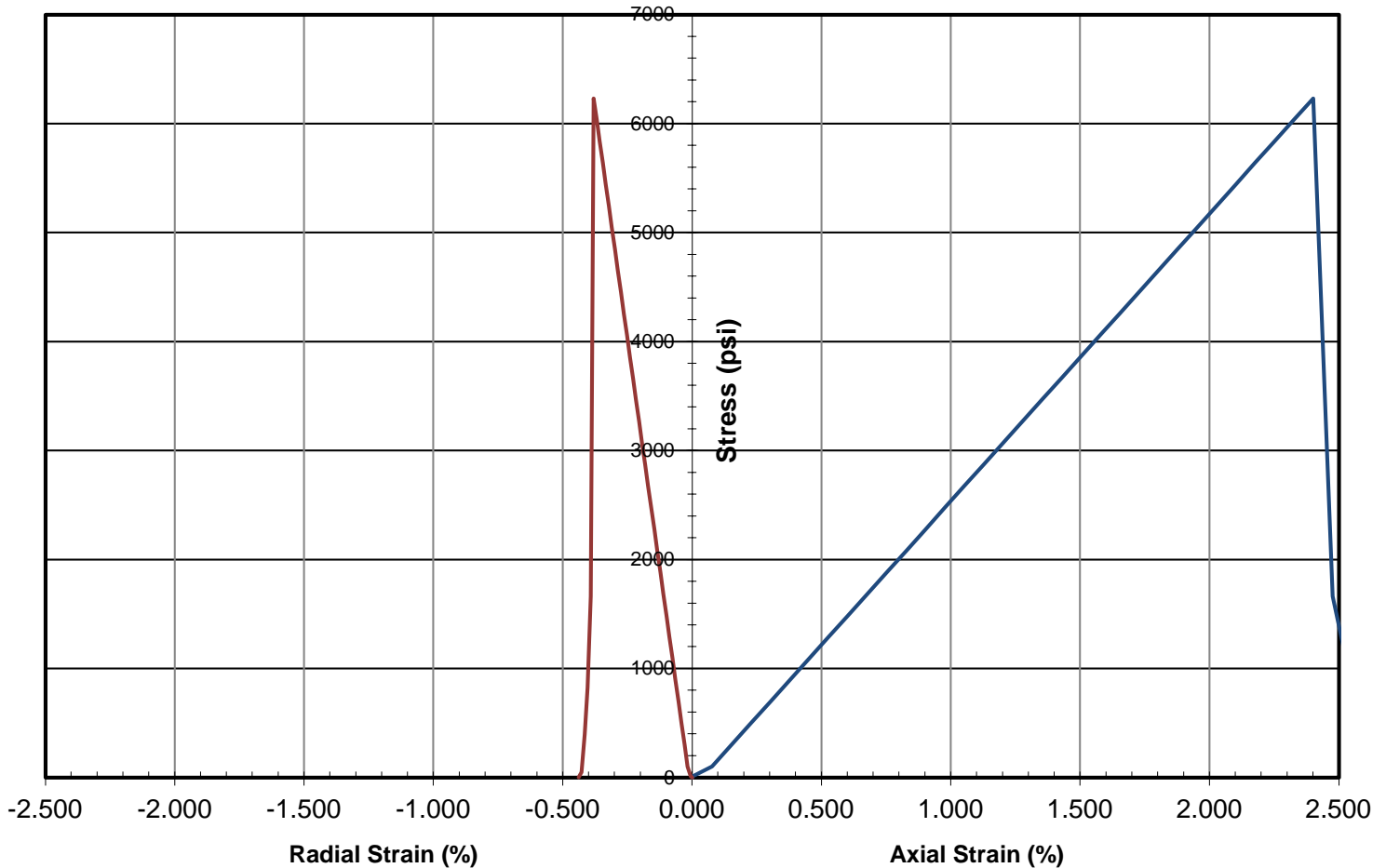


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-1T	Run No.:	NQ-3	Date: 03/14/25
Depth (ft):	81.5'-82.3'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



Young's Modulus

$C_0$  Max = 6,232 psi    50%  $C_0$  Max 3,116 psi  
10%  $C_0$  Max 623 psi

$E_t$  (50%)  $C_0$  = 2.64E+05 psi

Poisson's Ratio

$C_0$  Max = 6,232 psi    50%  $C_0$  Max 3,116 psi  
10%  $C_0$  Max 623 psi

$\nu_t$  (50%)  $C_0$  = 0.156

## Client

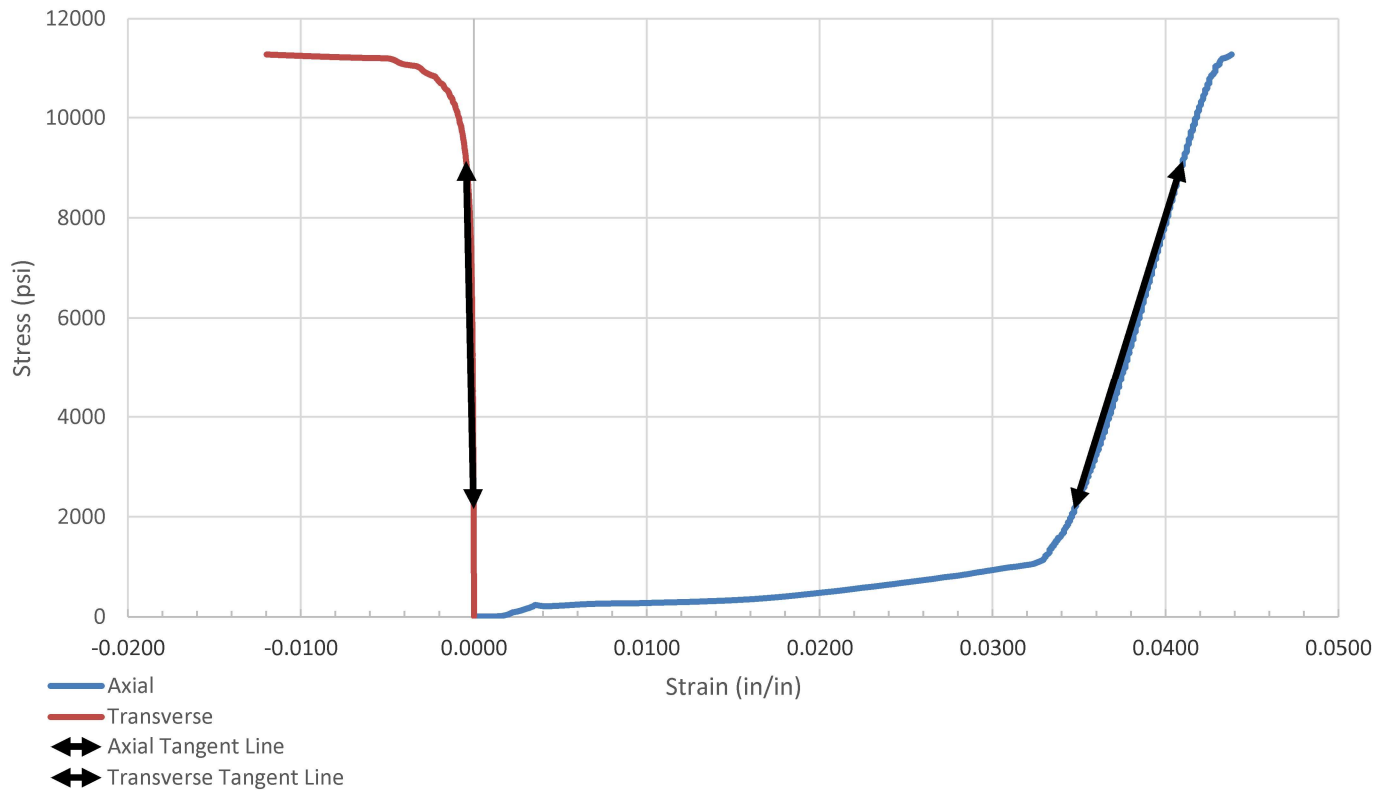
HNTB North Carolina PC  
4922 O'Hear Avenue Suite 203  
North Charleston, SC 29402

## Project

SCDOT Bridge Package 19  
Multiple Sites

Project No. 8623P180

### ASTM D7012 Stress/ Strain Curve



## SAMPLE LOCATION

Site:	SCDOT Bridge Package 19 - Over Tributary to Choestoea		
Rock Type:	Gneiss		
Boring:	S-37-168-2T	Depth (feet):	53.3-54.5

## SPECIMEN INFORMATION

Sample No.:	NQ-1	Mass (g):	617.15
Length (in.):	4.23	Diameter (in.):	1.98
L/D Ratio:	2.1	Density (pcf):	180.51

## TEST RESULTS

Failure Load (lbs):	34671
Failure Strain (%):	4.38
Unconfined Compressive Strength (psi):	11,266
Elastic Modulus, E, (ksi):	1113
Poisson's Ratio, u:	0.069
Time of Failure (min):	01:18
Rate of Loading (psi/sec):	144.807
Moisture Content Post-break:	0.1%



---

**Client**

HNTB North Carolina PC  
4922 O'Hear Avenue Suite 203  
North Charleston, SC 29402

**Project**

SCDOT Bridge Package 19  
Multiple Sites

Project No. 8623P180

---

**ASTM D4543 Test Results:**

<u>Parameter</u>	<u>Data</u>
Side Straightness:	0.0046
Perpendicularity Deviation:	
Diameter 1a:	0.0074
Diameter 1b:	0.0015
Diameter 2a:	0.0080
Diameter 2b:	0.0028
Max Deviation from Flatness:	0.0022
Parallelism Deviation:	
Diameter a:	0.04
Diameter b:	0.11

---

**Equipment:**

	TICCS ID:
Calipers:	W-54522
Scale:	B-71466
Dial Indicator:	C-70608
Compression (spherically seated):	C-48999

---

Samples were prepared and tested in accordance with ASTM D4543 and D7012. Deviations, if any, are noted below:  
Notes:

Per ASTM D4543, this specimen has not met the requirements for perpendicularity, by exceeding 0.250°.  
Per ASTM D4543, this specimen has not met the requirements for flatness, by exceeding 0.001 inches.  
According to ASTM D7012 Section 8.2.1, this specimen, although not meeting all requirements of ASTM D4543 is acceptable for testing. However, the results reported may differ from results obtained from a test specimen that meets the requirements of D4543.



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures

ASTM D 7012

Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/14/2025

Sample No. S-37-168-2T Run/Sam No.: NQ-2  
Depth (ft): 58.5'-59.5'  
Description: Light Brownish Gray / Light Gray Gneiss Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness		Maxumum Gap ≤ 0.020 in.						Tolerance Met	Yes
End Flatness: Max.	Diameter 1a	0.0007	in	Diameter 1b	0.0008	in	≤ 0.0010	Tolerance Met	Yes
End Flatness: Max.	Diameter 2a	0.0008	in	Diameter 2b	0.0008	in	≤ 0.0010	Tolerance Met	Yes
Perpendicularity Slope	Diameter 1a	0.00035		Diameter 1b	0.00040		≤ 0.0043	Tolerance Met	Yes
Perpendicularity Slope	Diameter 2a	0.00040		Diameter 2b	0.00040		≤ 0.0043	Tolerance Met	Yes

Length (in): 1) 4.315 2) 4.316 3) 4.316 Avg. 4.315 in

Diameter (in): 1) 1.979 2) 1.979 3) 1.980 Avg. 1.979 in

Uniaxial Compressive Strength: 13,508 psi Mass: 595.7 g

Load: 41,556 lbs. Wet Unit Weight: 170.9 pcf

L/D: 2.2 Dry Unit Weight: 170.4 pcf

Water Content: 0.3 %

Time to Failure: 2.90 min

Load Rate: 239 lbs/sec

Young's Modulus

Et (50% Co) 6.69E+05

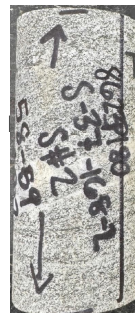
Poisson's Ratio

ut (50% Co) 0.158

REMARKS:

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Before



After



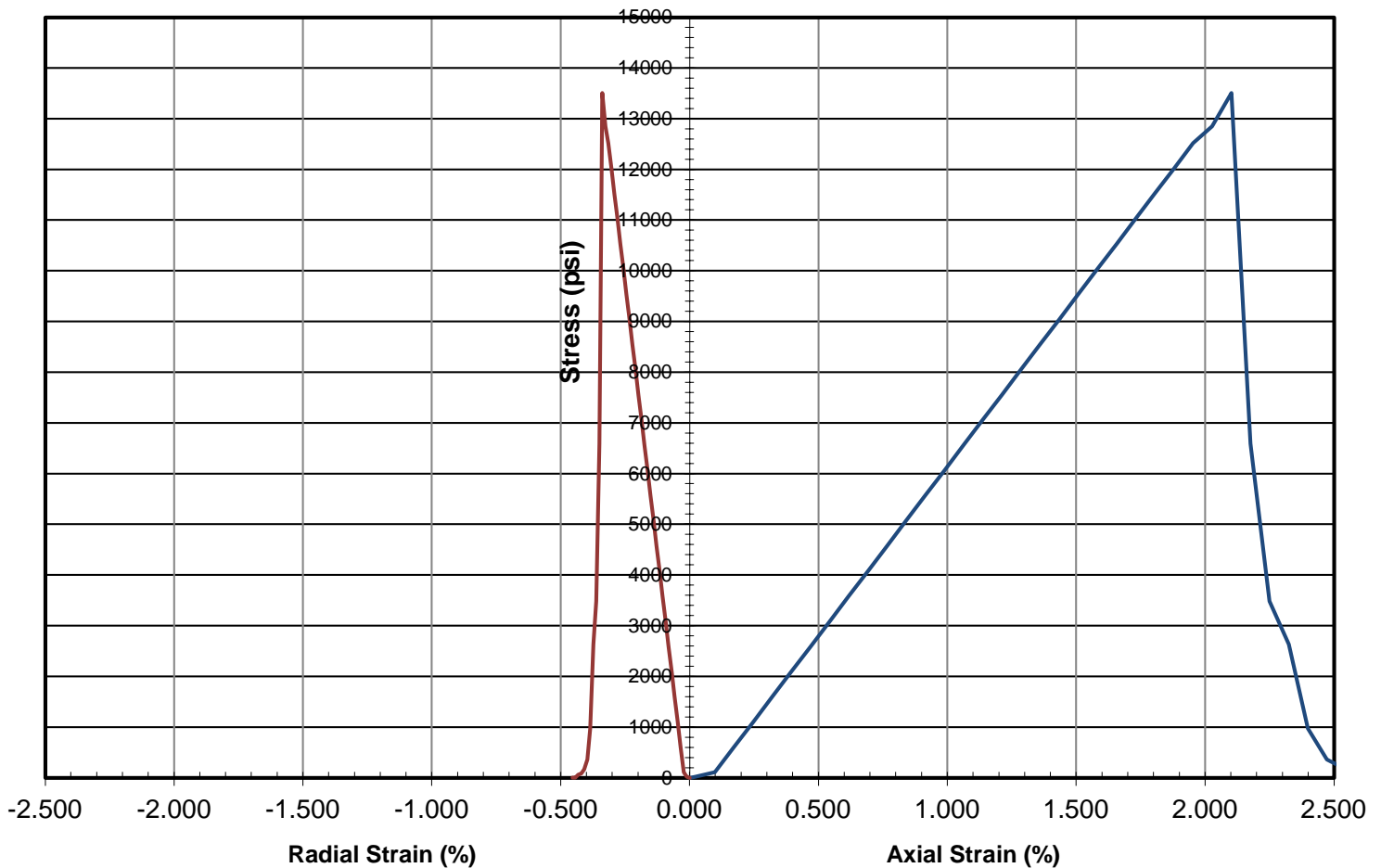


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-2T	Run No.:	NQ-2	Date: 03/14/25
Depth (ft):	58.5'-59.5'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



**Young's Modulus**

$C_o$  Max = 13,508 psi    50%  $C_o$  Max 6,754 psi  
10%  $C_o$  Max 1,351 psi

$E_t$  (50%)  $C_o$  = 6.69E+05 psi

**Poisson's Ratio**

$C_o$  Max = 13,508 psi    50%  $C_o$  Max 6,754 psi  
10%  $C_o$  Max 1,351 psi

$\nu_t$  (50%)  $C_o$  = 0.158



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures

ASTM D 7012

Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/14/2025

Sample No. S-37-168-2T Run/Sam No.: NQ-3  
Depth (ft): 63.0'-64.0'  
Description: Dark Gray Gneiss Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness	Maximum Gap $\leq 0.020$ in.					Tolerance Met	
End Flatness: Max.	Diameter 1a	in	Diameter 1b	in	$\leq 0.0010$	Tolerance Met	
End Flatness: Max.	Diameter 2a	in	Diameter 2b	in	$\leq 0.0010$	Tolerance Met	
Perpendicularity Slope	Diameter 1a		Diameter 1b		$\leq 0.0043$	Tolerance Met	
Perpendicularity Slope	Diameter 2a		Diameter 2b		$\leq 0.0043$	Tolerance Met	

Length (in): 1) 4.249 2) 4.249 3) 4.249 Avg. 4.249 in

Diameter (in): 1) 1.987 2) 1.986 3) 1.986 Avg. 1.986 in

Uniaxial Compressive Strength: 8,363 psi Mass: 579.5 g

Load: 25,911 lbs. Wet Unit Weight: 167.7 pcf

L/D: 2.1 Dry Unit Weight: 166.7 pcf

Water Content: 0.6 %

Time to Failure: 3.20 min

Load Rate: 135 lbs/sec

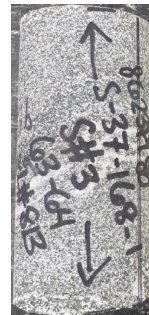
Young's Modulus	
Et (50% Co)	3.62E+05

Poisson's Ratio	
ut (50% Co)	0.184

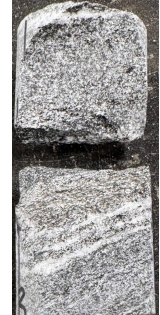
REMARKS:

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- Prepared in general accordance to ASTM D4543
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Before



After



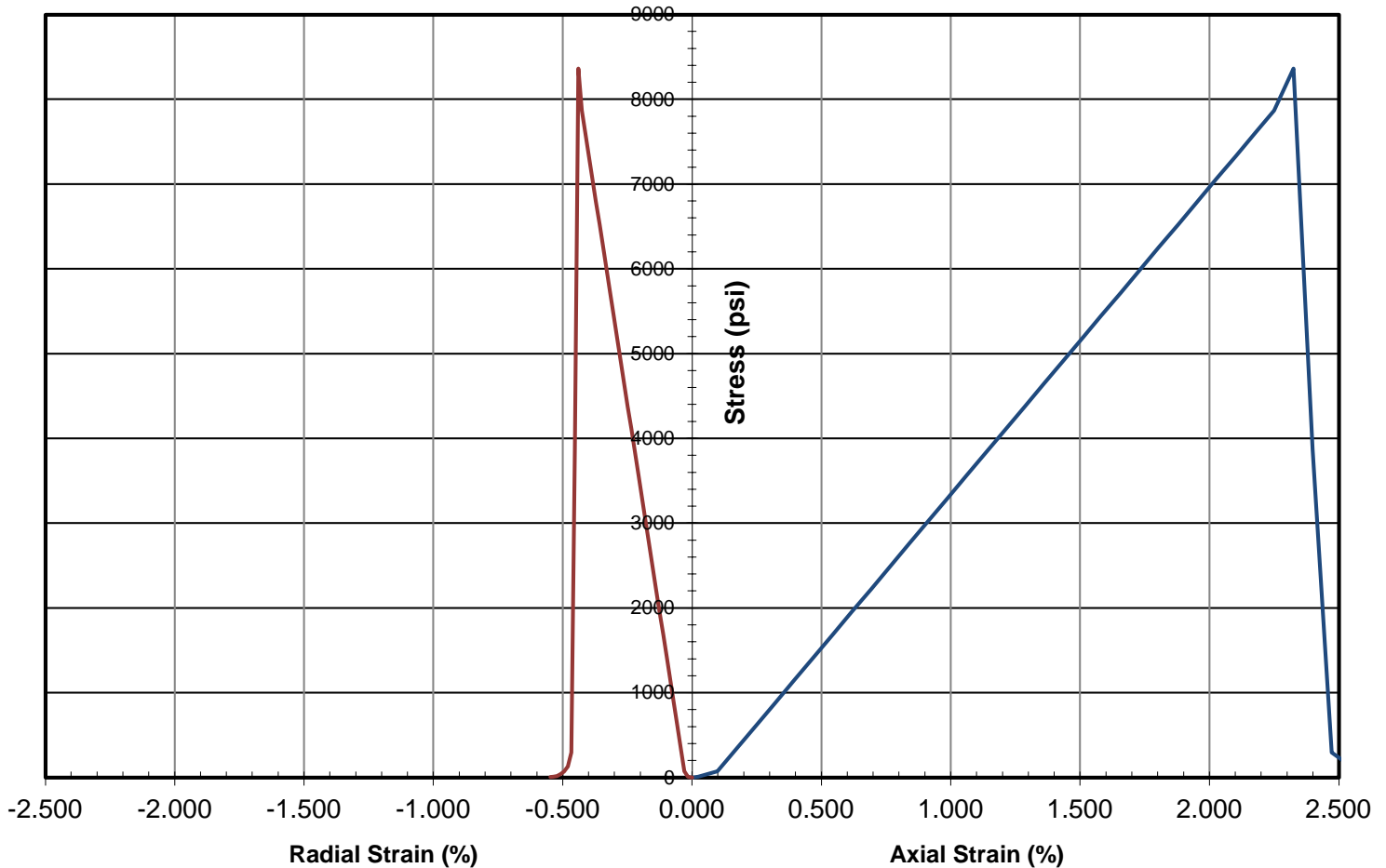


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-2T	Run No.:	NQ-3	Date: 03/14/25
Depth (ft):	63.0'-64.0'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



<b>Young's Modulus</b>			
$C_o$ Max =	8,363	psi	50% $C_o$ Max
			4,182
			10% $C_o$ Max
			836
			psi
$E_t$ (50%) $C_o$ =		3.62E+05	psi
<b>Poisson's Ratio</b>			
$C_o$ Max =	8,363	psi	50% $C_o$ Max
			4,182
			10% $C_o$ Max
			836
			psi
$\nu_t$ (50%) $C_o$ =		0.184	



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/14/2025

Sample No. S-37-168-2T Run/Sam No.: NQ-4  
Depth (ft): 69.7'-70.7'  
Description: Light Gray / Dark Gray Gneiss Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness		Maxumum Gap ≤ 0.020 in.						Tolerance Met	Yes
End Flatness: Max.	Diameter 1a	0.0008	in	Diameter 1b	0.0008	in	≤ 0.0010	Tolerance Met	Yes
End Flatness: Max.	Diameter 2a	0.0007	in	Diameter 2b	0.0007	in	≤ 0.0010	Tolerance Met	Yes
Perpendicularity Slope	Diameter 1a	0.00040		Diameter 1b	0.00035		≤ 0.0043	Tolerance Met	Yes
Perpendicularity Slope	Diameter 2a	0.00040		Diameter 2b	0.00035		≤ 0.0043	Tolerance Met	Yes

Length (in): 1) 4.265 2) 4.265 3) 4.265 Avg. 4.265 in

Diameter (in): 1) 1.978 2) 1.978 3) 1.978 Avg. 1.978 in

Uniaxial Compressive Strength: 8,731 psi Mass: 573.7 g

Load: 26,819 lbs. Wet Unit Weight: 166.8 pcf

L/D: 2.2 Dry Unit Weight: 165.8 pcf

Water Content: 0.6 %

Time to Failure: 2.90 min

Load Rate: 154 lbs/sec

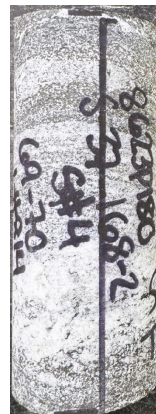
Young's Modulus	
Et (50% Co)	4.05E+05

Poisson's Ratio	
ut (50% Co)	0.170

REMARKS:

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Before



After





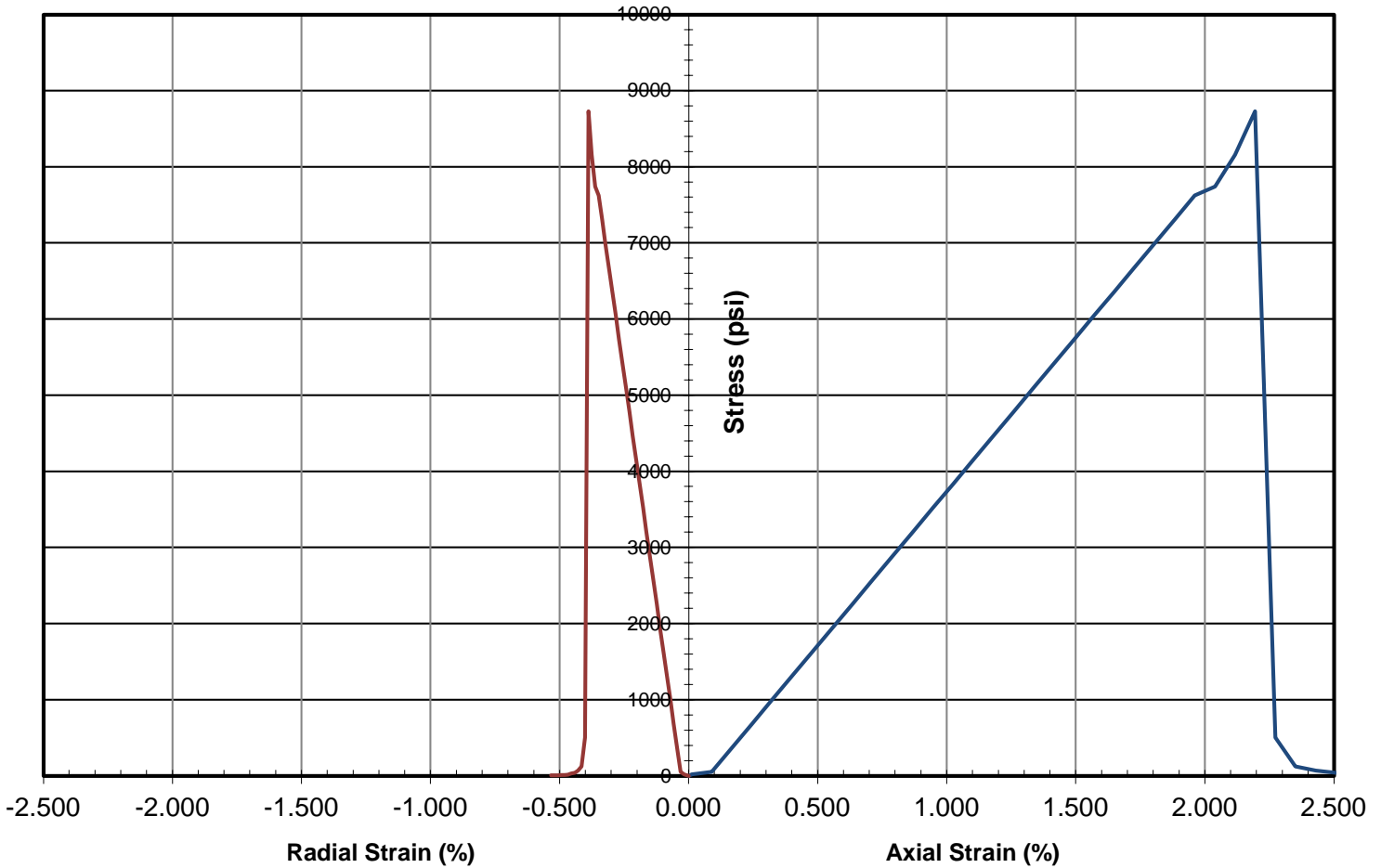


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-2T	Run No.:	NQ-4	Date: 03/14/25
Depth (ft):	69.7'-70.7'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



Young's Modulus			
$C_o$ Max =	8,731	psi	50% $C_o$ Max 4,365 psi
			10% $C_o$ Max 873 psi
			$E_t$ (50%) $C_o$ = 4.05E+05 psi
Poisson's Ratio			
$C_o$ Max =	8,731	psi	50% $C_o$ Max 4,365 psi
			10% $C_o$ Max 873 psi
			$\nu_t$ (50%) $C_o$ = 0.170



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures

ASTM D 7012

Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/14/2025

Sample No. S-37-168-3T Run/Sam No.: NQ-1  
Depth (ft): 56.7'-57.5'  
Description: Dark Gray Gneiss Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness	Maximum Gap $\leq 0.020$ in.					Tolerance Met	
End Flatness: Max.	Diameter 1a	in	Diameter 1b	in	$\leq 0.0010$	Tolerance Met	
End Flatness: Max.	Diameter 2a	in	Diameter 2b	in	$\leq 0.0010$	Tolerance Met	
Perpendicularity Slope	Diameter 1a		Diameter 1b		$\leq 0.0043$	Tolerance Met	
Perpendicularity Slope	Diameter 2a		Diameter 2b		$\leq 0.0043$	Tolerance Met	

Length (in): 1) 4.191 2) 4.191 3) 4.191 Avg. 4.191 in

Diameter (in): 1) 1.983 2) 1.983 3) 1.983 Avg. 1.983 in

Uniaxial Compressive Strength: 8,460 psi Mass: 567.5 g

Load: 26,118 lbs. Wet Unit Weight: 167.1 pcf

L/D: 2.1 Dry Unit Weight: 166.5 pcf

Water Content: 0.4 %

Time to Failure: 2.50 min

Load Rate: 174 lbs/sec

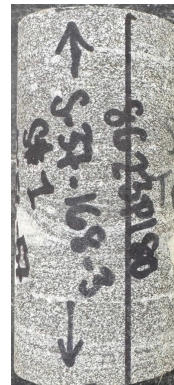
Young's Modulus	
Et (50% Co)	3.46E+05

Poisson's Ratio	
ut (50% Co)	0.176

REMARKS:

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- Prepared in general accordance to ASTM D4543
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Before



After



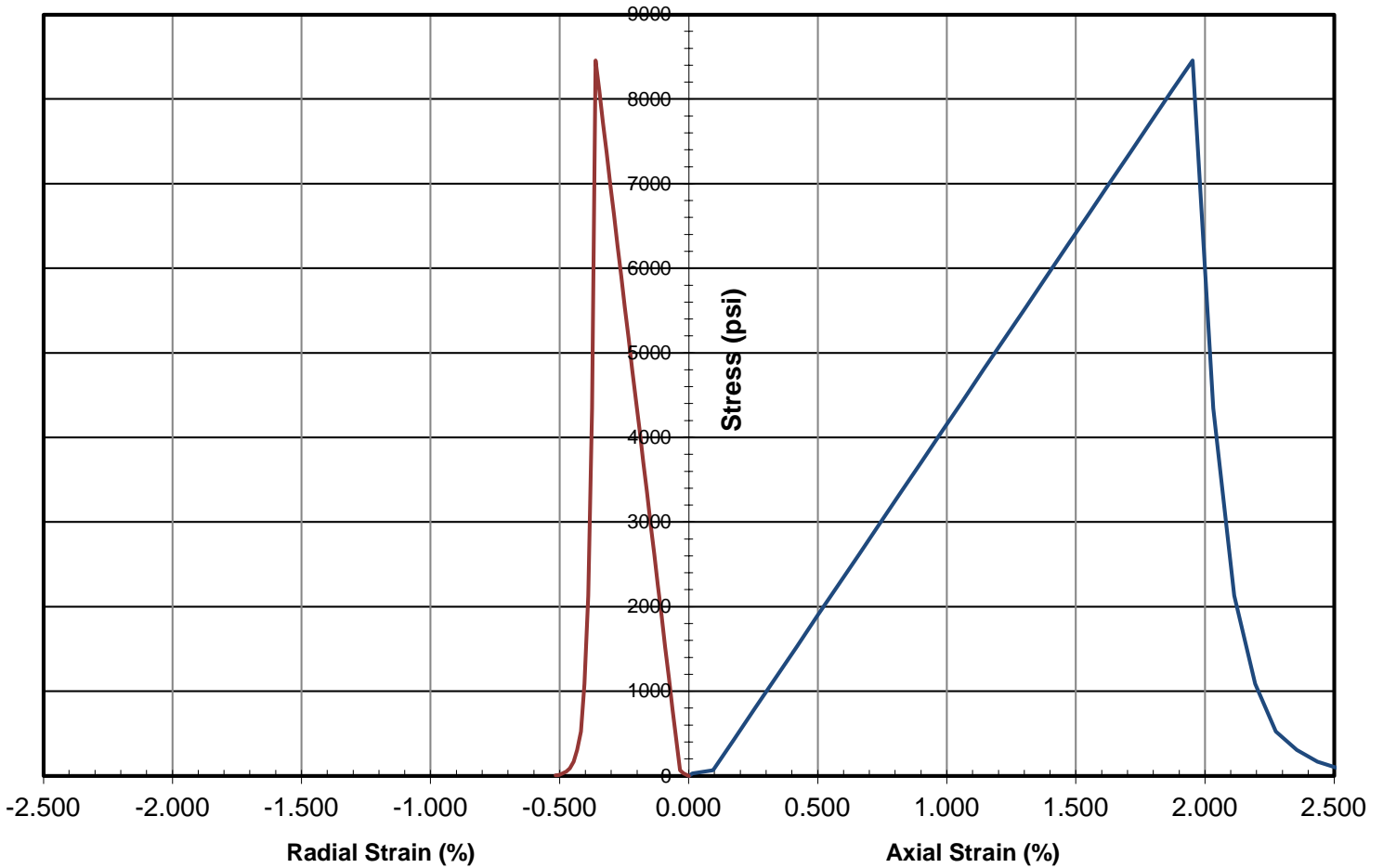


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-3T	Run No.:	NQ-1	Date: 03/14/25
Depth (ft):	56.7'-57.5'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



Young's Modulus

C<sub>0</sub> Max = 8,460 psi    50% C<sub>0</sub> Max 4,230 psi  
10% C<sub>0</sub> Max 846 psi

E<sub>t</sub> (50%) C<sub>0</sub> = 3.46E+05 psi

Poisson's Ratio

C<sub>0</sub> Max = 8,460 psi    50% C<sub>0</sub> Max 4,230 psi  
10% C<sub>0</sub> Max 846 psi

ν<sub>t</sub> (50%) C<sub>0</sub> = 0.176



Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures

ASTM D 7012

Method D

Laboratory Services Group

192 Exchange Boulevard

Glendale Heights, IL 60139

Phone: (630) 717-4263

Fax: (630) 357-9489

Project No.: 8623P180  
Project Name: SCDOT Bridge Package 19

Tested By: EB  
Calculated By: EB  
Checked By: WPQ

Date: 3/12/2025  
Date: 3/12/2025  
Date: 3/14/2025

Sample No. S-37-168-3T Run/Sam No.: NQ-2  
Depth (ft): 62.2'-63.3'  
Description: Very Pale Brown / Light Gray Marble Core

Rock Sample Moisture Condition at Time of Test: As Received

ASTM D4543 TOLERANCE CHECK

Side Straightness	Maximum Gap $\leq$ 0.020 in.					Tolerance Met	
End Flatness: Max.	Diameter 1a	in	Diameter 1b	in	$\leq$ 0.0010	Tolerance Met	
End Flatness: Max.	Diameter 2a	in	Diameter 2b	in	$\leq$ 0.010	Tolerance Met	
Perpendicularity Slope	Diameter 1a		Diameter 1b		$\leq$ 0.0043	Tolerance Met	
Perpendicularity Slope	Diameter 2a		Diameter 2b		$\leq$ 0.0043	Tolerance Met	

Length (in): 1) 4.235 2) 4.236 3) 4.235 Avg. 4.235 in

Diameter (in): 1) 1.980 2) 1.980 3) 1.980 Avg. 1.980 in

Uniaxial Compressive Strength: 16,510 psi Mass: 590.3 g

Load: 50,818 lbs. Wet Unit Weight: 172.5 pcf

L/D: 2.1 Dry Unit Weight: 172.0 pcf

Water Content: 0.3 %

Time to Failure: 2.70 min

Load Rate: 314 lbs/sec

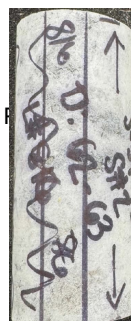
Young's Modulus	
Et (50% Co)	8.42E+05

Poisson's Ratio	
ut (50% Co)	0.156

REMARKS:

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- Prepared in general accordance to ASTM D4543
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Before



After



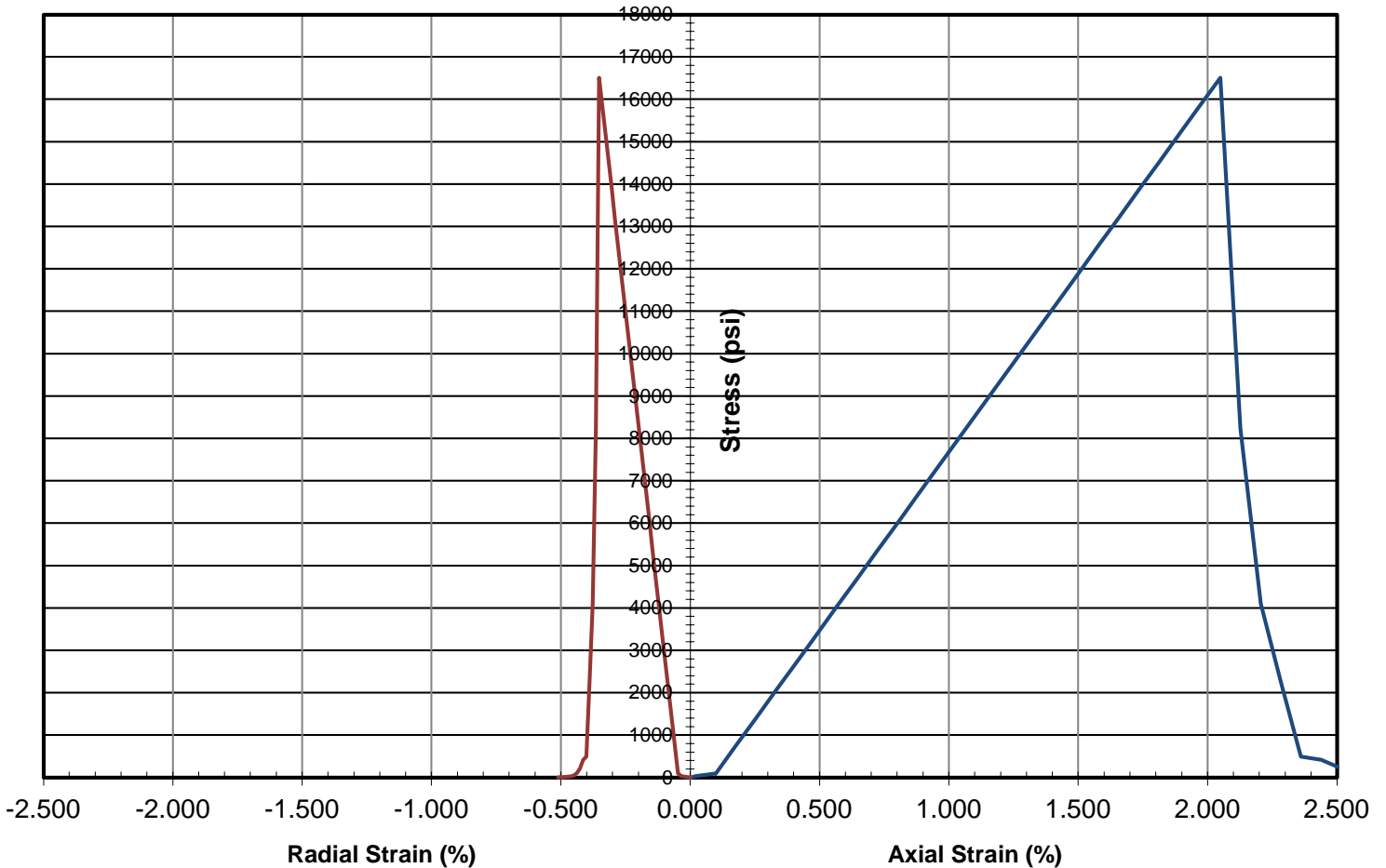


Compressive Strength and Elastic Moduli of Intact Rock Core  
Specimens under Varying Stress and Temperatures  
ASTM D 7012  
Method D

Laboratory Services Group	192 Exchange Boulevard	Glendale Heights, IL 60139	Phone: (630) 717-4263	Fax: (630) 357-9489
Project No.:	8623P180	Tested By:	EB	Date: 03/12/25
Project Name:	SCDOT Bridge Package 19	Calculated By:	EB	Date: 03/12/25
Boring No.:	S-37-168-3T	Run No.:	NQ-2	Date: 03/14/25
Depth (ft):	62.2'-63.3'	Checked By:	WPQ	

Stress vs Radial Strain

Stress vs Axial Strain



<b>Young's Modulus</b>			
$C_o$ Max =	16,510	psi	50% $C_o$ Max
			8,255
			10% $C_o$ Max
			1,651
			psi
		$E_t$ (50%) $C_o$ =	8.42E+05
			psi
<b>Poisson's Ratio</b>			
$C_o$ Max =	16,510	psi	50% $C_o$ Max
			8,255
			10% $C_o$ Max
			1,651
			psi
		$\nu_t$ (50%) $C_o$ =	0.156

## **Appendix C – Supporting Documents**

S-37-168 BRO Tributary to Choestoea Creek | Oconee County, SC  
Terracon Project No. 8623P180 | SCDOT Project ID: P042511



# **Appendix C**

## **Supporting Documents**

Rig Calibration Report – DR#554 (5 Pages)

Note: All exhibits are one page unless noted above.



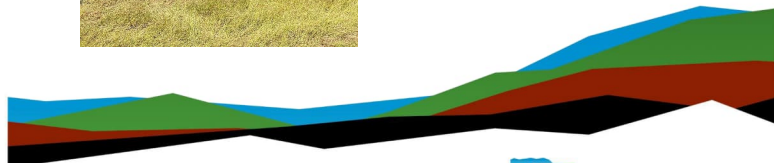
# SPT Automatic Hammer Energy Measurement Report

Drill Rig Model: GeoProbe 3126

Drill Rig Serial Number: 3126TTS52010006

Asset Number: DR#554

August 21, 2023



## Prepared for:

Terracon  
Greenville-Spartanburg, South Carolina



July 19, 2023

Terracon  
72 Pointe Circle  
Greenville, South Carolina 29607

Attn: Maggie McKenney  
E: m.mckenney@terracon.com

**Re:** SPT Automatic Hammer Energy Measurement Report  
Rig Serial Number: 3126TTS52010006  
Terracon Project Number: DYXX0500

Dear Ms. McKenney:

This report provides the Energy Transfer Ratio (ETR) for the Standard Penetration Testing (SPT) automatic hammer as summarized below:

**Table 1: Hammer Efficiency Summary**

Drill Rig Make/Model	Drill Rig Serial Number	Drill Rig Year	Asset Number	Energy Transfer Ratio (ETR)	Hammer Efficiency Correction (Ce)
GeoProbe 3126	3126TTS52010006	2021	GP#554	88.5% ± 4.2%	1.48

If you have any questions concerning this summary, or if we may be of further service, please contact us.

*Jim Smith*

James P. Smith  
National Manager of Equipment & Training

*Rob Kramer*

Rob Kramer  
Group Manager Geophysics

## Attachments:

Exhibit A: PDA SPT Analyzer Results  
Exhibit B: PDA Equipment Calibration

Facilities | Environmental | **Geotechnical** | Materials |



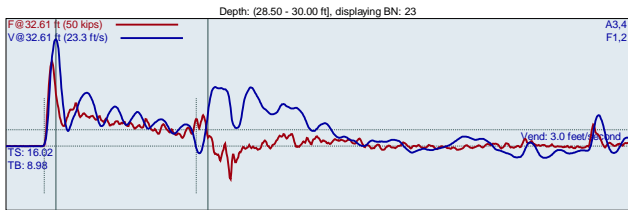
## MEASUREMENT SUMMARY

ITEM	DESCRIPTION
Drill Rig Owner	Terracon Greenville-Spartanburg - Greenville, SC
Drill Rig Operator	Brett Burnett; Terracon Exploration Services
Testing Date	08/21/2023
Testing Location	Spartanburg, SC
Boring Identification	B-1
Hammer Type	140 pounds (automatic)
Boring Method	Hollow Stem Auger
Drill Rods	<ul style="list-style-type: none"> <li>AWJ</li> <li>1-3/4" outside diameter</li> <li>3/16" wall thickness</li> </ul>
Calibration Testing Equipment	<ul style="list-style-type: none"> <li>2-foot AWJ rod instrumented w/ two strain gauges and two accelerometers</li> <li>Model SPT Analyzer™ (PDA)</li> </ul>
ASTM Methods Used	<p><b>ASTM D1586</b>, Standard Test Method for Standard Penetration Test and Split-Barrel Sampling of Soils</p> <p><b>ASTM D4633-16</b>, Standard Method for Energy Measurement for Dynamic Penetrometers</p>
SPT Calibration Personnel	Jim Smith, National Manager of Equipment and Training

## Exhibit A

### PDA SPT Analyzer Results

GP554-3126  
JIM SMITH  
TB-1  
AR: 1.20 in/2  
LE: 32.61 ft  
WS: 16807.9 fts  
28.5-30  
Interval start: 8/21/2023  
SP: 0.492 k/ft3  
EM: 30000 ksi



F1 : [648AWJ1] 226.21 PDICAL (1) FF1  
F2 : [648AWJ2] 225.58 PDICAL (1) FF1  
A3 (PR): [K4483] 410.187 mv/6.4v/5000g (1) VF1  
A4 (PR): [K10491] 421.907 mv/6.4v/5000g (1) VF1

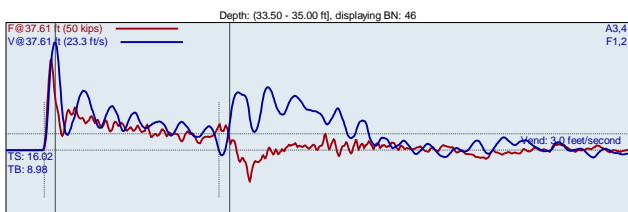
FMX: Maximum Force  
VMX: Maximum Velocity  
BPM: Blows/Minute  
EFV: Maximum Energy  
ETR: Energy Transfer Ratio - Rated

BL#	BC /ft	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
1	6	40	19.4	1.9	234	84.1
2	6	39	19.2	51.9	292	83.4
3	6	25	16.9	52.7	274	78.2
4	6	28	17.9	52.4	273	77.9
5	6	32	19.6	52.6	294	83.9
6	6	27	17.3	53.1	268	79.5
7	8	38	19.0	52.7	289	82.5
8	8	39	19.6	52.4	305	87.2
9	8	36	19.2	52.7	290	82.8
10	8	28	18.2	52.5	292	83.4
11	8	38	19.0	53.0	293	83.8
12	8	35	19.4	52.6	282	80.4
13	8	36	19.1	52.9	299	85.3
14	8	34	19.8	52.8	307	87.7
15	11	34	19.5	52.7	307	87.6
16	11	33	19.5	52.9	299	85.6
17	11	36	19.4	52.7	308	88.1
18	11	37	18.5	52.8	320	91.4
19	11	32	19.6	52.9	301	86.1
20	11	39	18.7	52.9	301	85.9
21	11	26	17.5	52.8	277	79.1
22	11	30	19.1	52.6	306	87.4
23	11	33	19.5	52.7	298	85.1
24	11	35	19.9	52.4	303	86.5
25	11	36	19.4	53.1	313	89.6

Average	34	19.2	52.8	299	85.6
Std Dev	3	0.6	0.2	10	3.0
Maximum	39	19.9	53.1	320	91.4
Minimum	26	17.5	52.4	277	79.1
N-value: 19					

Sample Interval Time: 27.36 seconds.

GP554-3126  
JIM SMITH  
TB-1  
AR: 1.20 in/2  
LE: 37.61 ft  
WS: 16807.9 fts  
28.5-30  
Interval start: 8/21/2023  
SP: 0.492 k/ft3  
EM: 30000 ksi

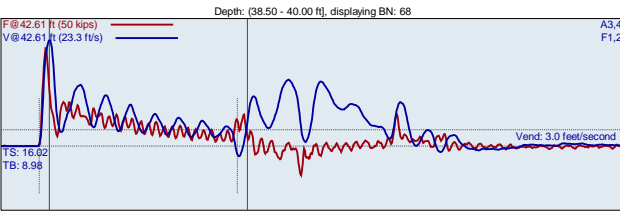


F1 : [648AWJ1] 226.21 PDICAL (1) FF1  
F2 : [648AWJ2] 225.58 PDICAL (1) FF1  
A3 (PR): [K4483] 410.187 mv/6.4v/5000g (1) VF1  
A4 (PR): [K10491] 421.907 mv/6.4v/5000g (1) VF1

BL#	BC /ft	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
26	5	38	19.1	1.9	302	86.4
27	5	35	18.9	52.0	301	86.1
28	5	29	18.8	52.0	299	85.5
29	5	35	19.2	52.7	299	85.5
30	5	37	19.4	52.5	297	84.8
31	8	37	19.5	52.4	307	87.7
32	8	26	16.4	52.7	282	80.5
33	8	34	19.5	52.4	307	87.6
34	8	40	19.1	52.2	307	87.6
35	8	37	19.4	52.6	299	85.5
36	8	40	20.6	52.4	321	91.7
37	8	41	19.6	52.8	308	87.9
38	8	40	19.8	52.7	313	89.5
39	10	34	20.2	52.2	323	92.2
40	10	32	19.4	52.8	297	84.9
41	10	36	19.8	52.6	311	88.8
42	10	37	19.7	52.5	317	90.7
43	10	35	20.0	52.6	324	92.6
44	10	38	19.5	52.7	308	88.1
45	10	34	20.1	52.4	322	92.0
46	10	35	19.7	52.4	322	92.0
47	10	37	19.9	52.6	314	89.7
48	10	37	19.8	52.7	332	94.8
Average						
		36	19.6	52.6	312	89.1
		3	0.8	0.2	12	3.3
		41	20.6	52.8	332	94.8
		26	16.4	52.2	282	80.5
N-value: 18						

Sample Interval Time: 25.16 seconds.

GP554-3126  
JIM SMITH  
TB-1  
AR: 1.20 in/2  
LE: 42.61 ft  
WS: 16807.9 fts  
28.5-30  
Interval start: 8/21/2023  
SP: 0.492 kftG  
EM: 30000 ksi



F1 : [648AWJ1] 226.21 PDICAL (1) FF1			A3 (PR): [K4483] 410.187 mm/6.4w/5000g (1) VF1			
F2 : [648AWJ2] 225.58 PDICAL (1) FF1			A4 (PR): [K10491] 421.907 mm/6.4w/5000g (1) VF1			
BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
49	5	34	19.6	1.9	307	87.6
50	5	34	19.3	52.0	301	86.1
51	5	27	16.5	52.7	278	79.4
52	5	33	19.9	52.5	310	88.6
53	5	29	17.7	52.7	288	82.2
54	8	29	18.6	52.5	295	84.2
55	8	23	15.6	52.9	287	82.0
56	8	34	20.1	52.6	323	92.2
57	8	28	18.1	52.8	295	84.3
58	8	38	18.8	53.1	312	89.1
59	8	35	19.2	52.6	329	94.0
60	8	36	19.3	52.9	327	93.3
61	8	40	19.7	52.8	323	92.4
62	9	35	18.8	53.0	320	91.3
63	9	37	19.1	52.7	320	91.3
64	9	35	19.9	52.9	327	93.4
65	9	29	18.8	52.7	314	89.7
66	9	35	19.7	53.0	342	97.8
67	9	36	19.9	52.8	331	94.5
68	9	38	19.3	52.8	335	95.8
69	9	36	19.9	52.5	325	92.9
70	9	39	19.5	52.9	329	94.0
Average		34	19.1	52.8	320	91.3
Std Dev		4	1.0	0.2	15	4.1
Maximum		40	20.1	53.1	342	97.8
Minimum		23	15.6	52.5	287	82.0
N-value: 17						

Sample Interval Time: 23.91 seconds.

Summary of SPT Test Results

Project: GP554-3126, Test Date: 8/21/2023				EFV: Maximum Energy				
FMX: Maximum Force				ETR: Energy Transfer Ratio - Rated				
VMX: Maximum Velocity								
BPM: Blows/Minute								
Test Length ft	Blows Applied /6"	N Value	N60 Value	Average FMX kips	Average VMX ft/s	Average BPM bpm	Average EFV ft-lb	Average ETR %
32.61	6-8-11	19	28	34	19.2	52.8	299	85.6
37.61	5-8-10	18	26	36	19.6	52.6	312	89.1
42.61	5-8-9	17	25	34	19.1	52.8	320	91.3
Overall Average Values:				35	19.3	52.7	310	88.5
Standard Deviation:				4	0.8	0.2	15	4.2
Overall Maximum Value:				41	20.6	53.1	342	97.8
Overall Minimum Value:				23	15.6	52.2	277	79.1



Exhibit B

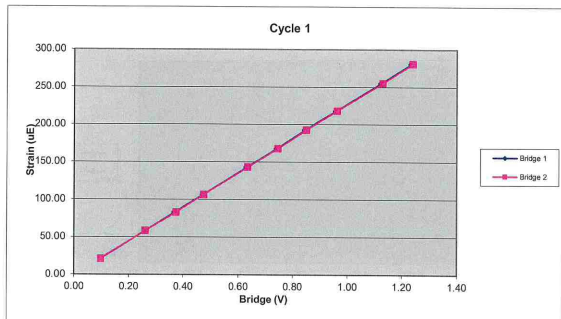
PDA Equipment Calibration



648AWJ		Cycle 1		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	799.99	21.12	0.10	0.10
3	2111.63	58.22	0.26	0.26
4	2997.39	82.70	0.37	0.37
5	3848.07	106.26	0.47	0.47
6	5131.83	143.07	0.63	0.63
7	6017.79	167.81	0.74	0.75
8	6872.07	192.74	0.85	0.85
9	7783.57	218.15	0.96	0.96
10	9136.93	255.02	1.12	1.13
11	10026.70	280.73	1.24	1.24

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8120.30	Force Calibration (lb/V)	8089.75
Offset	-4.24	Offset	-2.24
Correlation	0.999998	Correlation	0.999995
Strain Calibration (µE/V)	228.56	Strain Calibration (µE/V)	227.70
Offset	-1.57	Offset	-1.51
Correlation	0.999991	Correlation	0.999983

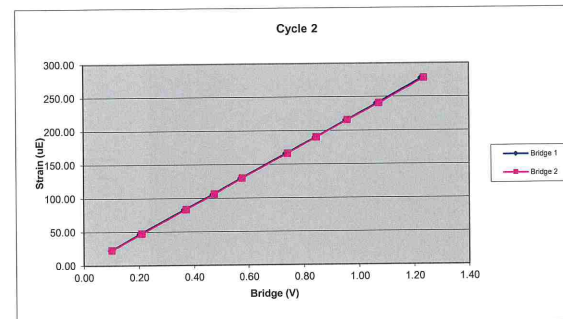
Force Strain Calibration	
EA (Kips)	35527.98
Offset	51.69
Correlation	0.999986



648AWJ		Cycle 2		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	805.54	22.23	0.10	0.10
3	1679.81	47.04	0.20	0.21
4	2989.11	83.03	0.37	0.37
5	3830.62	105.81	0.47	0.47
6	4658.00	129.50	0.57	0.58
7	5984.74	165.81	0.74	0.74
8	6848.87	189.76	0.84	0.84
9	7747.90	215.15	0.95	0.96
10	8674.21	240.08	1.07	1.07
11	9994.82	277.48	1.23	1.24

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8127.14	Force Calibration (lb/V)	8103.79
Offset	10.37	Offset	-14.59
Correlation	0.999997	Correlation	0.999997
Strain Calibration (µE/V)	225.29	Strain Calibration (µE/V)	224.64
Offset	0.36	Offset	-0.33
Correlation	0.999990	Correlation	0.999992

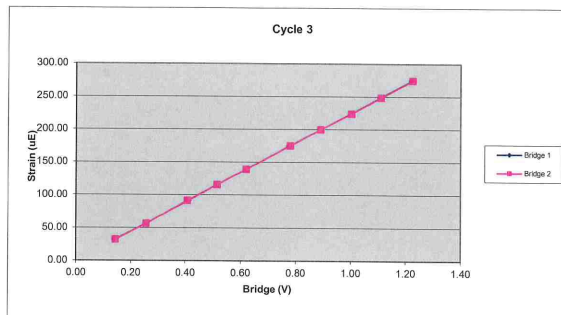
Force Strain Calibration	
EA (Kips)	36073.41
Offset	-2.66
Correlation	0.999993



648AWJ		Cycle 3		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1153.24	31.90	0.14	0.14
3	2056.55	56.28	0.26	0.26
4	3310.19	91.18	0.41	0.41
5	4155.51	115.51	0.51	0.51
6	5035.81	139.16	0.62	0.62
7	6303.78	175.10	0.78	0.78
8	7221.91	199.87	0.89	0.89
9	8120.94	223.92	1.00	1.00
10	9001.15	248.68	1.11	1.11
11	9931.66	274.33	1.22	1.23

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8132.32	Force Calibration (lb/V)	8118.57
Offset	-20.37	Offset	-15.36
Correlation	0.999998	Correlation	0.999997
Strain Calibration (µE/V)	224.79	Strain Calibration (µE/V)	224.41
Offset	-0.57	Offset	-0.43
Correlation	0.999984	Correlation	0.999985

Force Strain Calibration	
EA (Kips)	36175.62
Offset	0.42
Correlation	0.999984



Bridge Excitation (V) 5  
Shunt Resistor (ohm) 60.4k

Calibration Factors		648AWJ	
Bridge 1 (µE/V)	226.21	Bridge 2 (µE/V)	225.58
EA Factor (Kips)	35925.67	Area (in <sup>2</sup> )	1.20

Calibrated by: *Aht*  
Calibrated Date: 3/3/2022

Pile Dynamics Inc  
30725 Aurora Rd  
Solon, OH 44139

Traceable to N.I.S.T.

Accelerometer Calibration Certificate  
Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.  
Calibration performed on 26Oct2021

Serial No: K4483 Temperature: 22.1 °C  
Model: PR Humidity: 45%  
Calibrated on: Channel 3 on 8G 5161 LE

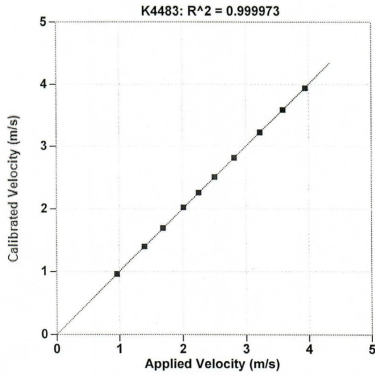
PDA CALIBRATION FACTOR  
410.2 mv/5000g  
(62.0  $\mu$ v/g)  
R<sup>2</sup>: 0.999973 [Chip programmed]

Operator: William Johnson

Signed

Ref Acc 1: 690961 Cal on: 27Jan2021  
978 g's/volt  
Ref Acc 2: 691321 Cal on: 09Feb2021  
960 g's/volt

Reference accelerometer calibrations are traceable to  
the United States National Institute of Standards and  
Technology (NIST).



Date printed: 26Oct2021, version: 2020.30.170 0.57

Accelerometer Calibration Certificate  
Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.  
Calibration performed on 25Jan2022

Serial No: K10491 Temperature: 19.3 °C  
Model: PR Humidity: 30%  
Calibrated on: Channel 3 on 8G 5161 LE

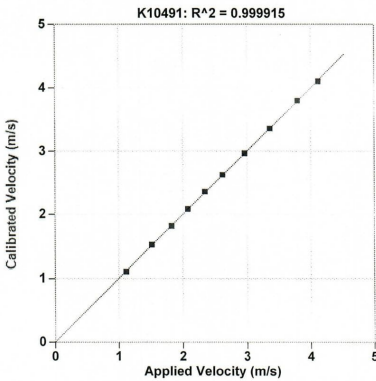
PDA CALIBRATION FACTOR  
421.9 mv/5000g  
(84.4  $\mu$ v/g)  
R<sup>2</sup>: 0.999915 [Chip programmed]

Operator: William Johnson

Signed

Ref Acc 1: 691321 Cal on: 09Feb2021  
960 g's/volt  
Ref Acc 2: 690961 Cal on: 27Jan2021  
978 g's/volt

Reference accelerometer calibrations are traceable to  
the United States National Institute of Standards and  
Technology (NIST).



Date printed: 25Jan2022, version: 2020.30.170 0.05