

S-23-310 (Crestwood Drive) over Langston Tributary

Greenville County, SC

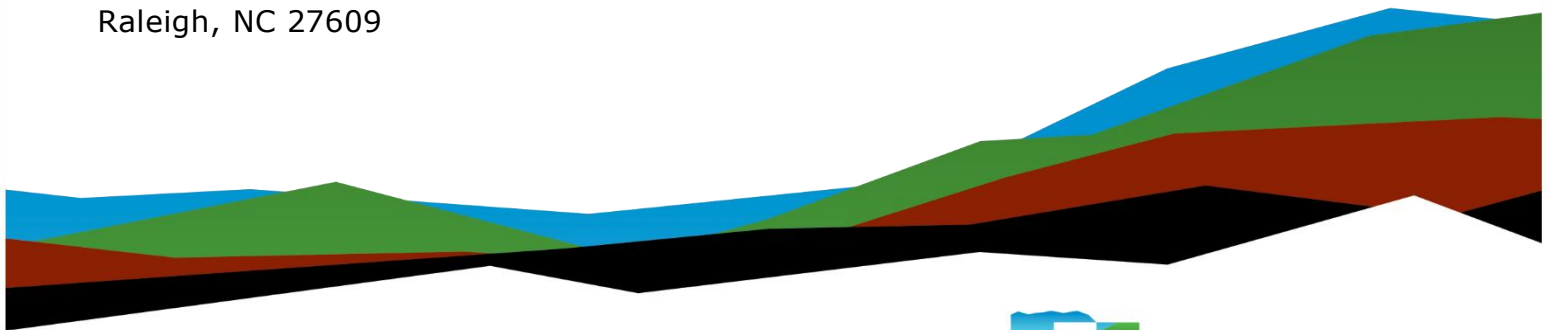
Geotechnical Subsurface Data Report

November 5, 2024 | SCDOT Project ID: P041162

Terracon Project No.: 8623P180 Revision 2

Prepared for:

HNTB Corporation
343 E. Six Forks Road, Suite 200
Raleigh, NC 27609



Nationwide
[Terracon.com](https://www.terracon.com)

- Facilities
- Environmental
- Geotechnical
- Materials



72 Pointe Circle
Greenville, SC 29615
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November 5, 2024

HNTB Corporation
343 E. Forks Road, Suite 200
Raleigh, NC 27609

Attn: Mr. Spencer Franklin, PE, Senior Vice President
P: 919-546-8997

Re: Geotechnical Subsurface Data Report
S-23-310 over Langston Tributary
Greenville County, South Carolina
SCDOT Project ID.: P041162
Terracon Project No.: 8623P180 Revision 2

Dear Mr. Franklin:

Terracon Consultants Inc. (Terracon) has completed the exploration, testing and limited engineering analysis services (contained in the geotechnical baseline report) for the above referenced project. The services were conducted in general accordance with our Task Order Number 001, dated May 25, 2023.

Introduction

HNTB Corporation (HNTB) has contracted Terracon to perform subsurface exploration, laboratory testing and limited preliminary geotechnical design considerations for a new box culvert along S-23-310 over Langston Tributary in Greenville County, South Carolina. The box culvert will replace the existing bridge. This GSDR was prepared in general accordance with the 2022 SCDOT Geotechnical Design Manual (GDM) and Preconstruction Design Memorandum (PCDM) 11 - Supplemental Design Criteria for Low Volume Bridge Replacement Projects.

Project Description

The project site is located at the S-23-310 (Crestwood Drive) crossing over Langston Tributary in Greenville County, South Carolina. Site location and exploration plans are presented in Appendix A of this report. Based on the conceptual plans by HNTB provided to Terracon on November 1, 2024, the culvert will be constructed on a new road alignment that is slightly shifted to the west of the current alignment. The current plan indicates the new culvert will

be a double-cell, 10 feet by 10 feet, cast-in-place box culvert that is 107 feet long. The overall height of the culvert is 11 feet 8 inches with a width of 22 feet.

Geotechnical Testing

The geotechnical exploration for this project was performed between June 5 and June 6, 2024. The results of our fieldwork and our associated laboratory testing are included in Appendices A and B.

Field Exploration

Our field exploration consisted of the following:

- Two (2) Standard Penetration Test (SPT) Borings (S-23-310-1 and S-23-310-2)
- One (1) offset boring near S-23-310-1 for bulk sample collection
- One (1) Downhole Shear Wave Velocity Test (DHT-1) located in Boring S-23-310-1
- One (1) Cone Penetration Test soundings (S-23-310-1C)

The tests were performed at the approximate locations as approved by SCDOT. A description of our testing methods and graphical logs outlining the soil conditions at each test location are presented in Appendix A. The test locations were established in the field by Terracon and surveyed by Thomas and Hutton, LLC after completion. Station and offset are based on the plans provided at the time the tests were performed.

Laboratory Testing

The following laboratory tests were performed on the soil samples collected at the site.

- Six (6) Natural Moisture Content Tests
- Six (6) Atterberg Limits Tests
- Four (4) Fines Content Tests
- Two (2) Grain Size Tests with Hydrometer
- One (1) Remolded, Consolidated-Undrained (CU) Triaxial Compression Test with Pore Pressure Readings
- One (1) Standard Proctor Test
- One (1) Corrosivity Suite (pH, chloride content, sulfate content, and resistivity tests)
- Five (5) Compressive Strength of Rock Cores

The general scope of the laboratory testing frequency was determined by the SCDOT. The laboratory testing assignment was performed by our engineers. The laboratory procedures and results of the laboratory tests are presented in Appendix B.

Closure

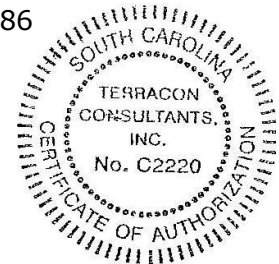
We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

Terracon Consultants, Inc.

Maggie McKenney, EIT
Senior Staff Engineer

Jonathan Ard, PE
Manager Regional Services
SC Registration No. 30886



Appendix A

Field Exploration

- Exhibit A-1 – Site Location Map
- Exhibit A-2 – Exploration Plans (2 Pages)
- Exhibit A-3 – Summary of Boring Data
- Exhibit A-4 – GeoScoping Form (2 Pages)
- Exhibit A-5 – Field Exploration Description (3 Pages)
- Exhibit A-6 – Soil/Rock Description Terms (2 Pages)
- Exhibit A-7 – Soil/Rock Symbols
- Exhibit A-8 – Boring Logs (3 Pages)
- Exhibit A-9 – Rock Core Photograph Logs (3 pages)
- Exhibit A-10 – Geophysical Testing Results
- Exhibit A-11 – CPT Sounding Logs

Note: All exhibits are one page unless noted above

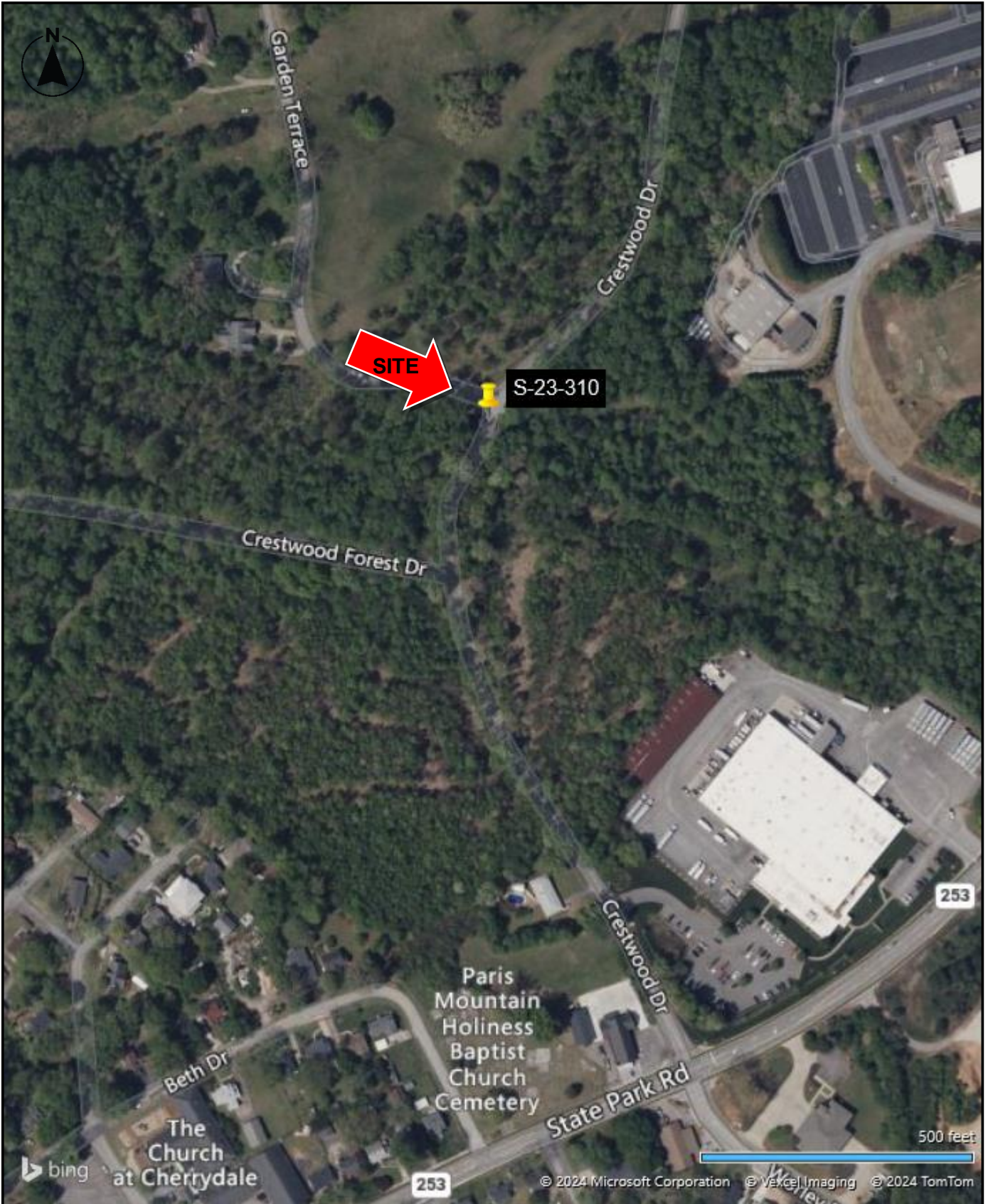


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

AERIAL PHOTOGRAPHY PROVIDED BY MICROSOFT BING MAPS

Project Number	8623P180	 72 Pointe Cir Greenville, South Carolina 29615	SITE LOCATION		Exhibit A-1
Scale	AS SHOWN		S-23-310 over Langston Tributary Crestwood Drive Greenville County, SC		
Client	HNTB				
Date	9/13/2024				



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND
IS NOT INTENDED FOR CONSTRUCTION PURPOSES

AERIAL PHOTOGRAPHY PROVIDED
BY MICROSOFT BING MAPS

Project Number	8623P180
Scale	AS SHOWN
Client	HNTB
Date	9/13/2024

ierracon

72 Pointe Cir
Greenville, South Carolina 29615

EXPLORATION PLAN
S-23-310 over Langston Tributary Crestwood Drive Greenville County, SC

Exhibit
A-2

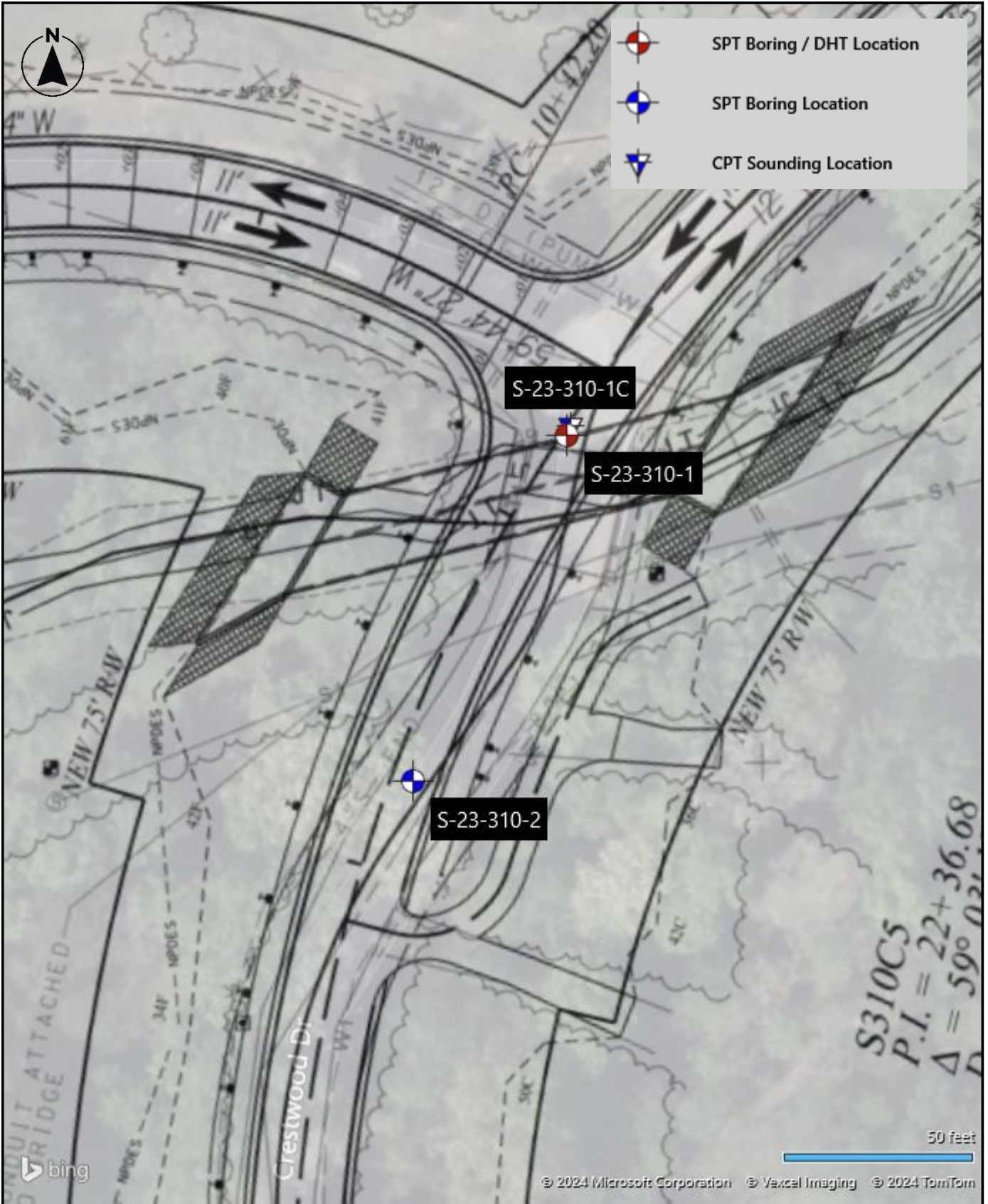



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND
IS NOT INTENDED FOR CONSTRUCTION PURPOSES

PRELIMINARY SITE PLAN
PROVIDED BY HNTB

Project Number	8623P180
Scale	AS SHOWN
Client	HNTB
Date	9/13/2024



72 Pointe Cir
Greenville, South Carolina 29615

EXPLORATION PLAN
S-23-310 over Langston Tributary Crestwood Drive Greenville County, SC

Exhibit
A-2

Summary of Boring Data – Exhibit A-3

S-23-310 over Langston Tributary | Greenville County, SC
Terracon Project No. 8623P180 | SCDOT Project ID: P041162



Summary of Boring Data

Boring No.	Ground Elevation (ft)	Test Depth (ft)	Northing (ft)	Easting (ft)	Latitude (°)	Longitude (°)	Station (ft) ¹	Offset (ft) ¹
S-23-310-1	1019.2	51	1117650.37	1580127.56	34.897212	-82.400164	23+24	1 R
S-23-310-2	1024.9	22	1117560.17	1580085.79	34.896963	-82.400299	22+24	6 R
S-23-310-1C	1019.2	11.4	1117652.86	1580128.60	34.897219	-82.400161	23+27	1 R

1. Plans were provided by HNTB after the field exploration and survey. Station and offset values are estimated based on overlay in Google Earth TM.
2. A bulk sample was collected about 10.5 feet and 13.5 feet east of S-23-310-1.

GeoScoping Form

PROJECT INFORMATION			
Project ID:	P041162	Date of Trip:	6/6/2024
County:	Greenville	Location:	Sans Souci
Rd/ Route:	S-23-310	Local Name:	Crestwood Drive
Attendees:	M. McKenney		

EXISTING BRIDGE INFORMATION			
Bridge Length:	30 ft	Bridge Width:	28 ft
Superstructure Type:	Concrete framing and decking	Substructure Type:	Timber piles
Begin Bridge Sta ¹ :	22+91	End Bridge Sta ¹ :	23+21
Begin Bridge Embankment Sta ¹ :	21+91	End Bridge Embankment Sta ¹ :	24+21
Structure Number:	04543	Posted Weight Limit:	14 tons
Crossing:	Langston Tributary	Skew:	N/A
Latitude:	34.897219°	Longitude:	-82.400161°
Existing Fill Height:	approx 6 ft	Approx Existing Slope Angle:	2H:1V
1. Begin & End Bridge Embankment 100 ft down Sta. or up Sta., respectively. Sta. estimated from overlay of bridge plan provided by HNTB.			

EXISTING ROADWAY EMBANKMENT INFORMATION			
Begin Project Sta:	21+50	Begin Bridge Embankment Sta:	21+91
Accessibility Issues:	None Observed		
Ground Cover:	Asphalt pavement and grassed shoulders		
Existing Fill Height:	N/A	Approx Existing Slope Angle:	2H:1V
Local Development:	developed - residential		
Topography:	slope to creek		
Traffic Control Necessary:	Yes, lane closure		
Surface Soils:	Silty sand w/ gravel	Muck:	No
Exposed Rock in Stream Bed:	Yes	Exposed Rock in banks:	No
Wetlands on Site:	Yes	Wetland Adjacent:	Yes
Depth FG to Water:	11 feet	Water Depth:	1 to 2 feet
Depth to Existing Ground:	approximately 13 feet at center of bridge		
Scour Condition at EB:	N/A	Scour Condition at IB:	N/A
End Bridge Embankment Sta:	24+21	End Project Sta:	25+50
Accessibility Issues:	None Observed		
Ground Cover:	Asphalt pavement and grassed shoulders		
Existing Fill Height:	6 feet, sloping	Approx Existing Slope Angle:	2H:1V
Local Development:	developed - residential		
Topography:	graded slope to creek		
Traffic Control Necessary:	Yes, lane closure		
Surface Soils:	Silty sand	Muck:	No
Exposed Rock in Stream Bed:	Yes	Exposed Rock in banks:	No
Wetlands on Site:	Yes	Wetland Adjacent:	Yes
Depth FG to Water:	11 feet	Water Depth:	1 to 2 feet
Depth to Existing Ground:	approximately 13 feet at center of bridge		
Scour Condition at EB:	N/A	Scour Condition at IB:	N/A

GeoScoping Form

UTILITIES INFORMATION	
Attached:	A fiber line was observed to be attached to the bridge deck on the west side of the bridge
Above Ground:	N/A
Underground:	An underground waterline was observed in the east shoulder An underground sewerline was observed south of the bridge running east to west

Comments:

Field Exploration Description Overview

The testing locations were proposed to and approved by SCDOT and located in the field by Terracon using measurements from existing structures shown on the provided drawings. The borings were surveyed by Thomas and Hutton after testing and drilling was complete. The locations, as shown in the Exploration Plans, are shown to the scale indicated.

A field log of each test location was prepared by our engineer. The final boring logs included with this report represent the engineer's description of the encountered conditions modified as necessary based on laboratory test results of the individual samples.

Soil Test Borings (STB)

All boring and sampling operations were conducted in general accordance with the following procedures:

- SCDOT Geotechnical Design Manual 2022
- Preconstruction Design Memorandum (PCDM) 11 - Supplemental Design Criteria for Low Volume Bridge Replacement Projects
- ASTM D5783, "Standard Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geo-environmental Exploration"
- ASTM D6151, "Standard Practice for Using Hollow-Stem Augers for Geotechnical Exploration and Soil Sampling"
- ASTM D1586 "Test Method for Penetration Test and Split-Barrel Sampling of Soils"
- ASTM D4220 "Standard Practices for Preserving and Transporting Soil"
- ASTM D2113 "Standard Practice for Rock Core Drilling and Sampling of Rock for Site Exploration"
- ASTM D5079 "Standard Practices for Preserving and Transporting Rock Core Samples"

Each soil test boring was advanced using rotary wash drilling techniques. The sampling program is summarized in the following table:

Test ID	Total Depth	Interval of Continuous Sampling
S-23-310-1	51 feet w/ 38 feet rock coring	2 to 10 feet
S-23-310-2	22 feet w/ 10 feet rock coring	0.5 to 12 feet
S-23-310-1 Offset	5 feet	Bulk Sample ¹
S-23-310-1C	11.4 feet (refusal)	CPT - No sampling

1. Bulk sample was obtained with 2 ¼-inch Hollow Stem Auger (HSA).

Soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-barrel sampler, also known as a standard split-spoon. The sampler is advanced into the soil a total of 18 to 24 inches by striking the drill rod using a 140-pound automatic hammer falling 30 inches.

The number of blows required to advance the sampler for each of three to four, 6-inch increments is recorded. The sum of the number of blows for the second and third increments is called the “Standard Penetration Value”, or N-value (N_{meas} , blows per foot). The N-value, when properly evaluated, is an index to the soil strength.

Soil classification provides a general guide to the engineering properties of various soil types and enables the engineer to apply his experience to current situations. In our exploration, samples obtained during drilling operations are examined and visually classified by a geotechnical engineer using the procedures outlined in ASTM D2487 - Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System). Laboratory testing was also performed on select split-spoon samples to evaluate index properties for further classification. The soils are described according to color, texture, and relative density or consistency (based on standard penetration resistance). The designations shown on the logs are described in the 2022 SCDOT Geotechnical Design Manual, Chapter 6.

The borings were advanced either to the planned drilling depth at which they were terminated, or to refusal of the drilling equipment. Select borings were continued below this depth using diamond bit rock coring techniques. NQ2 sized cores were recovered from the borehole. The rock recovery ratios (REC, percentage of the total core run), Rock Quality Designation (RQD, percentage of the total core run of pieces greater than 4 inches) were recorded along with a description of the rock. An explanation of the rock descriptions shown on the logs is provided in the SCDOT GDM Chapter 6. Photos of the recovered rock core specimens are provided in the Rock Core Photograph Log.

As practical, groundwater readings were collected from each of the soil test borings after 24 hours. These water levels are indicated on the boring logs. The borings were advanced using mud rotary drilling techniques. As the drilling method introduces water into the borehole, time-of-drilling water levels may not be reliable.

At the conclusion of the work, the boreholes and sounding holes were backfilled with the drill cuttings and clean sand. The upper 20 feet of those in the embankments were grouted with a cement bentonite grout and capped with cold-patch asphalt.

Cone Penetration Test (CPT) Soundings

Cone Penetration Test soundings were conducted in accordance with ASTM D5778 *Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils*.

Downhole Shear Wave Velocity Test (DHT)

One downhole seismic test was performed in a cased borehole drilled for this project. After the test boring was completed, the boring was filled with a fluid water/cement/bentonite grout and then a threaded PVC pipe casing (capped at the bottom end) was inserted into the borehole, providing a uniform bond between the soil and pipe exterior.

The downhole seismic test consisted of placing two downhole triaxial geophones at selected depth intervals in the borehole casing. The geophone was connected to a recording device (Seismic Source Daq Link 5 Seismograph) at the surface and clamped to the side of the casing at the selected test depth. The geophones are equipped with a spring-arm that is released at the bottom of the boring. The spring expands and forces the geophone against the casing wall. The interval between each geophone and each test depth was 3 feet for the entire depth of the cased borehole. An instrumented hammer was then used to strike a steel plate with cleats at the bottom (often called a shear wave golf shoe) that penetrated the ground and prevented sliding when struck. The steel plate was oriented to generate horizontal shear waves (SH) at the surface. An additional plate was also struck to better produce compression waves. The horizontal distance was measured and the plate was set exactly 10 feet from the borehole. The recorder was set to record the arrival times of the shear waves at the geophone locations. At least 15 blows (5 in each direction on the golf shoe, and 5 on the steel plate) were struck for each test depth to electronically stack and polarize the observed data, and to increase the signal-to-noise ratio. The data was stored on computer disks for processing and computation. The geophone was raised to the next depth interval and the process was repeated.

Shear Wave Velocity Test Results shows the downhole shear wave velocity and compressive wave velocity test results. The data was evaluated using the Fixed Interval method. S-wave arrival times using the Interval method were picked based on the onset of the signal (first break) as observed in the software package TomTime by GeoTom.

SOIL DESCRIPTION TERMS

Relative Density/Consistency Terms

<u>Relative Density</u> ¹			<u>Consistency</u> ²		
Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Unconfined Compression Strength (q _u) (tsf)	SPT Blow Count
Very Loose	0 to 15%	4 and less	Very Soft	0.25 and less	2 and less
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	51 and more	Very Stiff	2.01 to 4.00	16 to 30
			Hard	4.01 and more	31 and more

Moisture Condition

<u>Descriptive Term</u>	<u>Criteria</u>
Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually in coarse-grained soils below the water table

Color

Describe the sample color while sample is still moist.

Angularity¹

<u>Descriptive Term</u>	<u>Criteria</u>
Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces.
Subangular	Particles are similar to angular description but have rounded edges.
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges.
Rounded	Particles have smoothly curved sides and no edges.

HCl Reaction³

<u>Descriptive Term</u>	<u>Criteria</u>
None Reactive	No visible reaction
Weakly Reactive	Some reaction, with bubbles forming slowly
Strongly Reactive	Violent reaction, with bubbles forming immediately

Cementation³

<u>Descriptive Term</u>	<u>Criteria</u>
Weakly Cemented	Crumbles or breaks with handling or little finger pressure
Cemented	Crumbles or breaks with considerable finger pressure
Strongly Cemented	Will not crumble or break with finger pressure

Particle-Size Range¹

<u>Gravel</u>	Diameter, mm	Sieve Size	<u>Sand</u>	Diameter, mm	Sieve Size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	¾ inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4.00 to 4.76	#10 to #4

Primary Soil Type^{1, 2}

The primary soil type will be shown in all capital letters.

USCS Soil Designation

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

AASHTO Soil Designation

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

¹Applies to coarse-grained soils (major portion retained on No. 200 sieve)

²Applies to fine-grained soils (major portion passing No. 200 sieve)

³Use as required

DESCRIPTION OF ROCK PROPERTIES

WEATHERING

Fresh	Rock fresh, crystals bright, few joints may show slight staining. Rock rings under hammer if crystalline.
Very slight	Rock generally fresh, joints stained, some joints may show thin clay coatings, crystals in broken face show bright. Rock rings under hammer if crystalline.
Slight	Rock generally fresh, joints stained, and discoloration extends into rock up to 1 in. Joints may contain clay. In granitoid rocks some occasional feldspar crystals are dull and discolored. Crystalline rocks ring under hammer.
Moderate	Significant portions of rock show discoloration and weathering effects. In granitoid rocks, most feldspars are dull and discolored; some show clayey. Rock has dull sound under hammer and shows significant loss of strength as compared with fresh rock.
Moderately Severe	All rock except quartz discolored or stained. In granitoid rocks, all feldspars dull and discolored and majority show kaolinization. Rock shows severe loss of strength and can be excavated with geologist's pick.
Severe	All rock except quartz discolored or stained. Rock "fabric" clear and evident, but reduced in strength to strong soil. In granitoid rocks, all feldspars kaolinized to some extent. Some fragments of strong rock usually left.
Very severe	All rock except quartz discolored or stained. Rock "fabric" discernible, but mass effectively reduced to "soil" with only fragments of strong rock remaining.
Complete	Rock reduced to "soil". Rock "fabric" not discernible or discernible only in small, scattered locations. Quartz may be present as dikes or stringers.

HARDNESS (for engineering description of rock – not to be confused with Moh's scale for minerals)

Very hard	Cannot be scratched with knife or sharp pick. Breaking of hand specimens requires several hard blows of geologist's pick.
Hard	Can be scratched with knife or pick only with difficulty. Hard blow of hammer required to detach hand specimen.
Moderately hard	Can be scratched with knife or pick. Gouges or grooves to ¼ in. deep can be excavated by hard blow of point of a geologist's pick. Hand specimens can be detached by moderate blow.
Medium	Can be grooved or gouged 1/16 in. deep by firm pressure on knife or pick point. Can be excavated in small chips to pieces about 1-in. maximum size by hard blows of the point of a geologist's pick.
Soft	Can be gouged or grooved readily with knife or pick point. Can be excavated in chips to pieces several inches in size by moderate blows of a pick point. Small thin pieces can be broken by finger pressure.
Very soft	Can be carved with knife. Can be excavated readily with point of pick. Pieces 1-in. or more in thickness can be broken with finger pressure. Can be scratched readily by fingernail.

Joint, Bedding, and Foliation Spacing in Rock^a

Spacing	Joints	Bedding/Foliation
Less than 2 in.	Very close	Very thin
2 in. – 1 ft.	Close	Thin
1 ft. – 3 ft.	Moderately close	Medium
3 ft. – 10 ft.	Wide	Thick
More than 10 ft.	Very wide	Very thick

^aSpacing refers to the distance normal to the planes, of the described feature, which are parallel to each other or nearly so.

Rock Quality Designation (RQD)^a

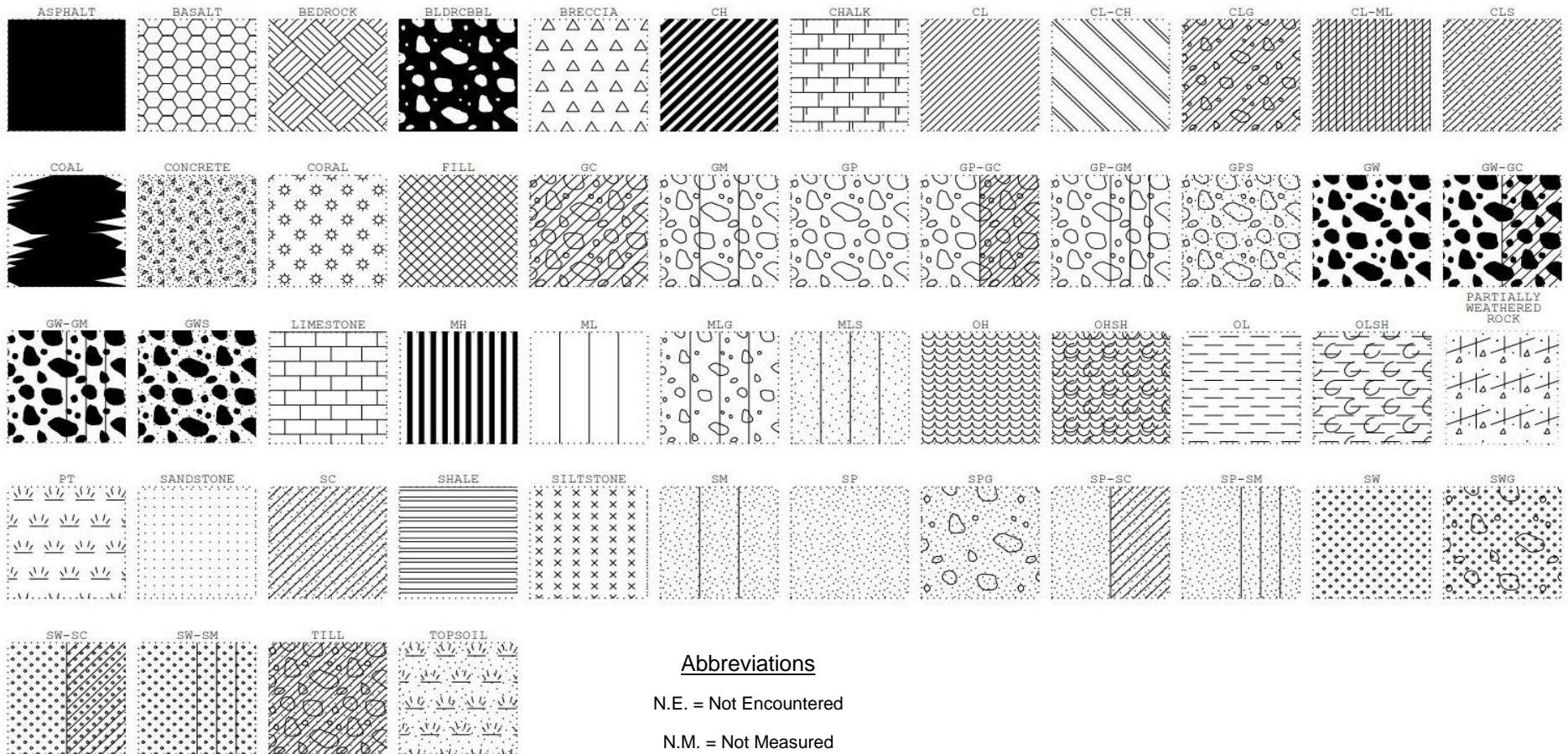
RQD, as a percentage	Diagnostic Description
Exceeding 90	Excellent
90 – 75	Good
75 – 50	Fair
50 – 25	Poor
Less than 25	Very poor

^aRQD (given as a percentage) = length of core in pieces 4 in. and longer/length of run.

Joint Openness Descriptors

Openness	Descriptor
No Visible Separation	Tight
Less than 1/32 in.	Slightly open
1/32 to 3/8 in.	Moderately open
1/8 to 3/8 in.	Open
3/8 in. to 0.1 ft.	Moderately wide
Greater than 0.1 ft.	Wide

References: American Society of Civil Engineers. Manuals and Reports on Engineering Practice - No. 56. Subsurface Investigation for Design and Construction of Foundations of Buildings. New York: American Society of Civil Engineers, 1976. U.S. Department of the Interior, Bureau of Reclamation, Engineering Geology Field Manual.



Project Manager:	MEM
Drawn by:	KJZ
Checked by:	SG
Approved by:	DJC

Project No.	8623P180
Scale:	N.T.S.
File Name:	Soil – Rock – Log
Date:	Jul 2023



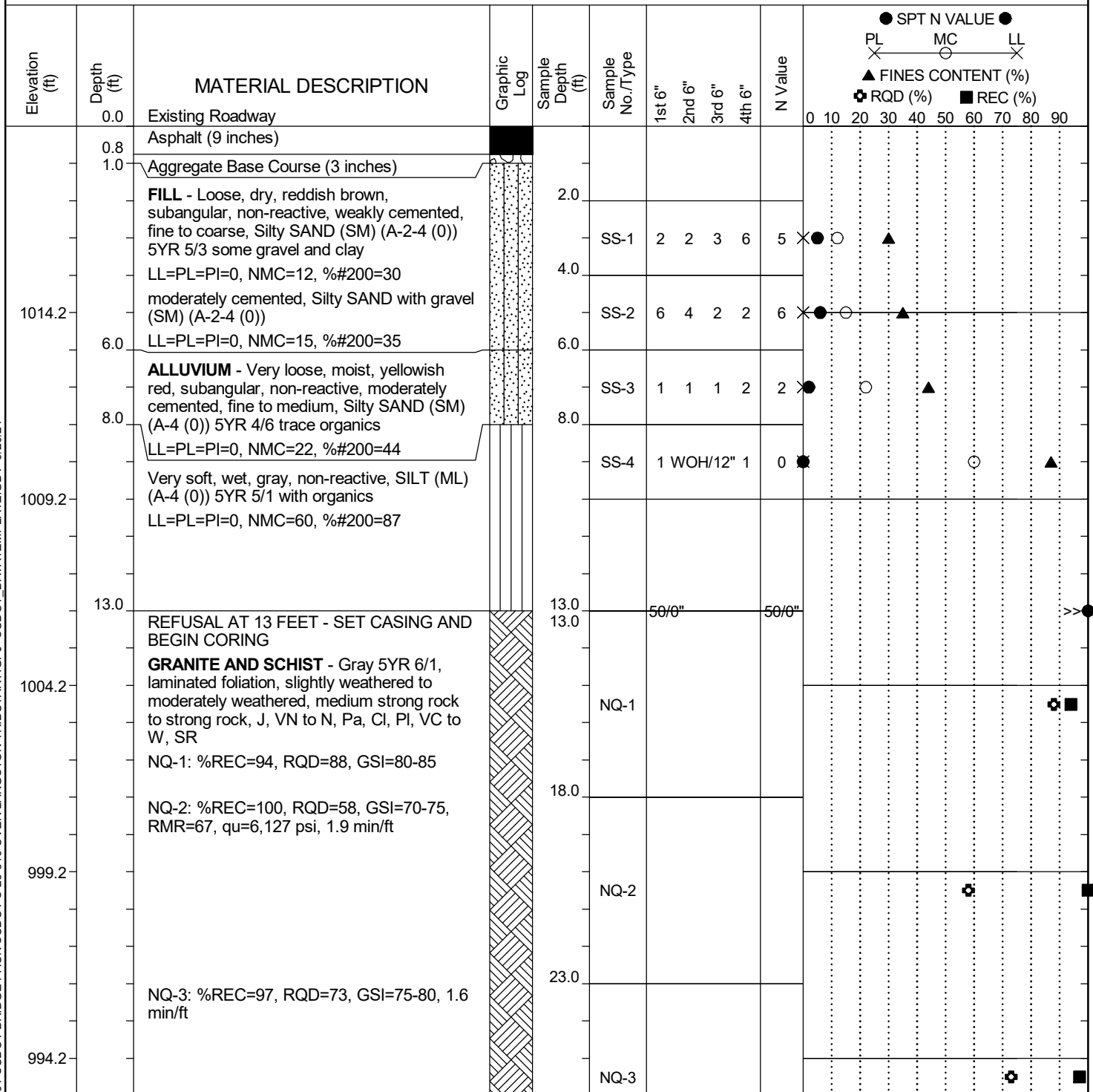
72 Pointe Circle
PH. (864) 292-2901
Greenville, SC 29615
FAX. (864) 292-6361

SOIL AND ROCK SYMBOLS

Exhibit A-7

SCDOT Soil Test Log

Project ID:	P041162				County:	Greenville		Boring No.:	S-23-310-1	
Site Description:	S-23-310 over Langston Tributary							Route:	S-23-310	
Eng./Geo.:	S. Greaber		Boring Location: 23+24			Offset:	1 R		Alignment:	Proposed
Elev.:	1019.2 ft		Latitude:	34.8972121		Longitude:	-82.4001641		Date Started:	6/5/2024
Total Depth:	51 ft		Soil Depth:	13 ft		Core Depth:	38 ft		Date Completed:	6/6/2024
Bore Hole Diameter (in):			4		Sampler Configuration		Liner Required:	Y (N)		Liner Used: Y (N)
Drill Machine:	DR#554		Drill Method:	RW/RC		Hammer Type:	Automatic		Energy Ratio:	88.5%
Core Size:	NQ2		Driller:	B. Burnette		Groundwater:	TOB	N.M.		24HR N.M.



LEGEND

Continued Next Page

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SC.DOT 8623P180T SCDOT BRIDGE PACK SCDOT S-23-310 OVER LANGSTON TRIBUTARY.GPJ SCDOT_DATATEMPLATE.GDT 9/20/24

SCDOT Soil Test Log

Project ID:		P041162			County:		Greenville		Boring No.:		S-23-310-1				
Site Description:			S-23-310 over Langston Tributary							Route:		S-23-310			
Eng./Geo.:		S. Greaber		Boring Location:			23+24		Offset:		1 R				
Alignment:										Proposed					
Elev.:		1019.2 ft		Latitude:		34.8972121		Longitude:		-82.4001641		Date Started:		6/5/2024	
Total Depth:		51 ft		Soil Depth:		13 ft		Core Depth:		38 ft		Date Completed:		6/6/2024	
Bore Hole Diameter (in):			4		Sampler Configuration			Liner Required:		Y (N)		Liner Used:		Y (N)	
Drill Machine:		DR#554		Drill Method:		RW/RC		Hammer Type:		Automatic		Energy Ratio:		88.5%	
Core Size:		NQ2		Driller:		B. Burnette		Groundwater:		TOB N.M.		24HR		N.M.	

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd 6"	3rd 6"	4th 6"	N Value	<div> ● SPT N VALUE ● PL X MC LL X ▲ FINES CONTENT (%) ⊕ RQD (%) ■ REC (%) </div>
989.2		NQ-4: %REC=95, RQD=65, GSI=70-75, RMR=67, qu=5,190 psi, 1.4 min/ft		28.0	NQ-4						
984.2		NQ-5: %REC=100, RQD=78, GSI=75-80, 1.9 min/ft		33.0	NQ-5						
979.2		NQ-6: %REC=100, RQD=86, GSI=80-85, RMR=74, qu=10,054 psi, 2.1 min/ft		38.0	NQ-6						
974.2		NQ-7: %REC=98, RQD=58, GSI=70-75, 2.1 min/ft		43.0	NQ-7						
969.2		NQ-8: %REC=94, RQD=82, GSI=75-80, RMR=71, qu=6,048 psi, 1.9 min/ft		48.0	NQ-8						
51.0		CORING TERMINATED AT 51 FEET AND SET DHT CASING									

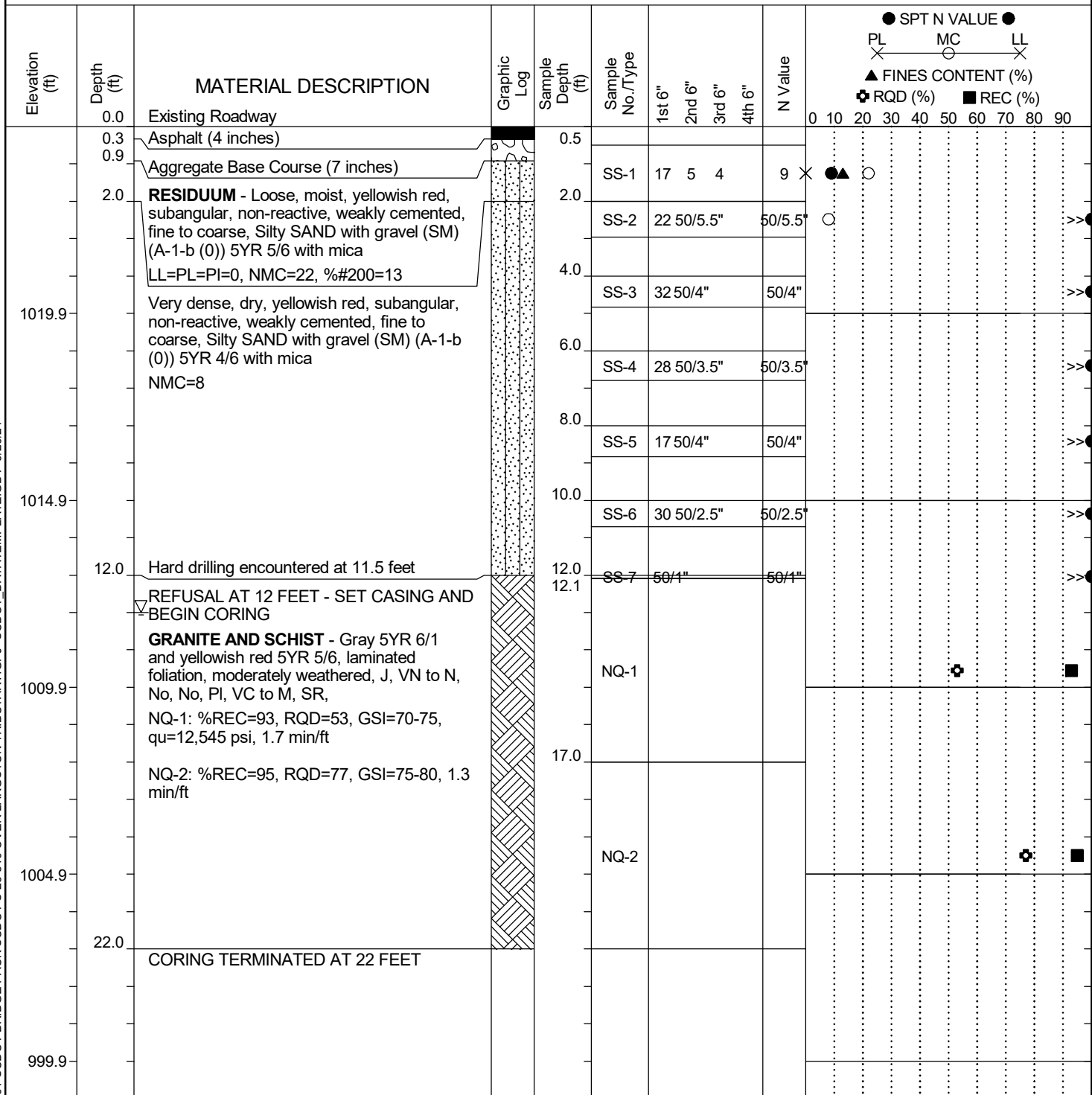
LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS	- Split Spoon	HSA	- Hollow Stem Auger
UD	- Undisturbed Sample	RW	- Rotary Wash
AWG	- Rock Core, 1-1/8"	CFA	- Continuous Flight Augers
		RC	- Rock Core
		DC	- Driving Casing
NQ	- Rock Core, 1-7/8"		
CU	- Cuttings		
CT	- Continuous Tube		

SC.DOT 8623P180T SCDOT BRIDGE PACK SCDOT S-23-310 OVER LANGSTON TRIBUTARY.GPJ SCDOT_DATATEMPLATE.GDT 9/20/24

SCDOT Soil Test Log

Project ID:	P041162	County:	Greenville	Boring No.:	S-23-310-2
Site Description:	S-23-310 over Langston Tributary			Route:	S-23-310
Eng./Geo.:	S. Greaber	Boring Location:	22+24	Offset:	6 R
Elev.:	1024.9 ft	Latitude:	34.8969627	Longitude:	-82.4002993
Date Started:	6/6/2024				
Total Depth:	22 ft	Soil Depth:	12 ft	Core Depth:	10 ft
Date Completed:	6/6/2024				
Bore Hole Diameter (in):	4	Sampler Configuration	Liner Required: Y (N)		Liner Used: Y (N)
Drill Machine:	DR#554	Drill Method:	RW/RC	Hammer Type:	Automatic
Energy Ratio:	88.5%				
Core Size:	NQ2	Driller:	B. Burnette	Groundwater:	TOB 13 ft
24HR:	N.M.				



LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS	- Split Spoon	NQ	- Rock Core, 1-7/8"
UD	- Undisturbed Sample	HSA	- Hollow Stem Auger
AWG	- Rock Core, 1-1/8"	CFA	- Continuous Flight Augers
		DC	- Driving Casing
		RW	- Rotary Wash
		RC	- Rock Core
		CU	- Cuttings
		CT	- Continuous Tube

SC.DOT 8623P180T SCDOT BRIDGE PACK SCDOT S-23-310 OVER LANGSTON TRIBUTARY.GPJ SCDOT_DATATEMPLATE.GDT 9/20/24

Rock Core Photograph Logs – Exhibit A-9

S-23-310 over Langston Tributary | Greenville County, SC

Terracon Project No. 8623P180 | SCDOT Project ID: P041162



S-23-310-1, NQ-1 and NQ-2 (13 to 23 feet)



S-23-310-1, NQ-3 and NQ-4 (23 to 33 feet)

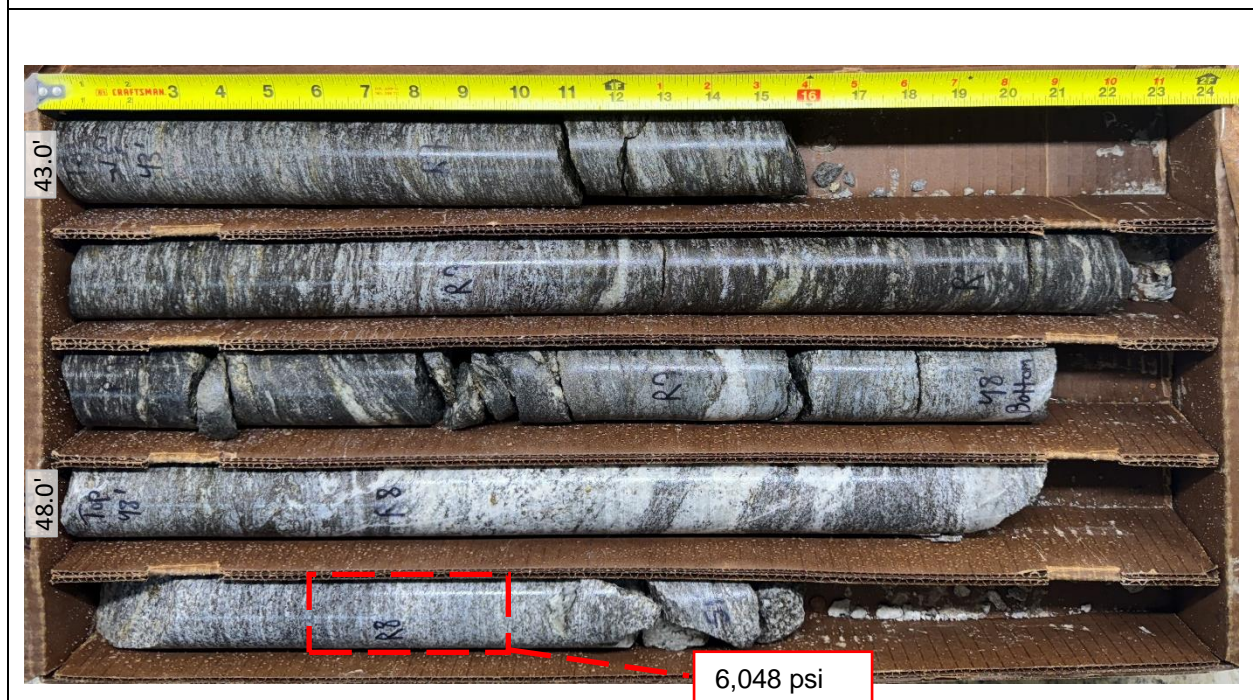
Rock Core Photograph Logs – Exhibit A-9

S-23-310 over Langston Tributary | Greenville County, SC

Terracon Project No. 8623P180 | SCDOT Project ID: P041162



S-23-310-1, NQ-5 and NQ-6 (33 to 43 feet)

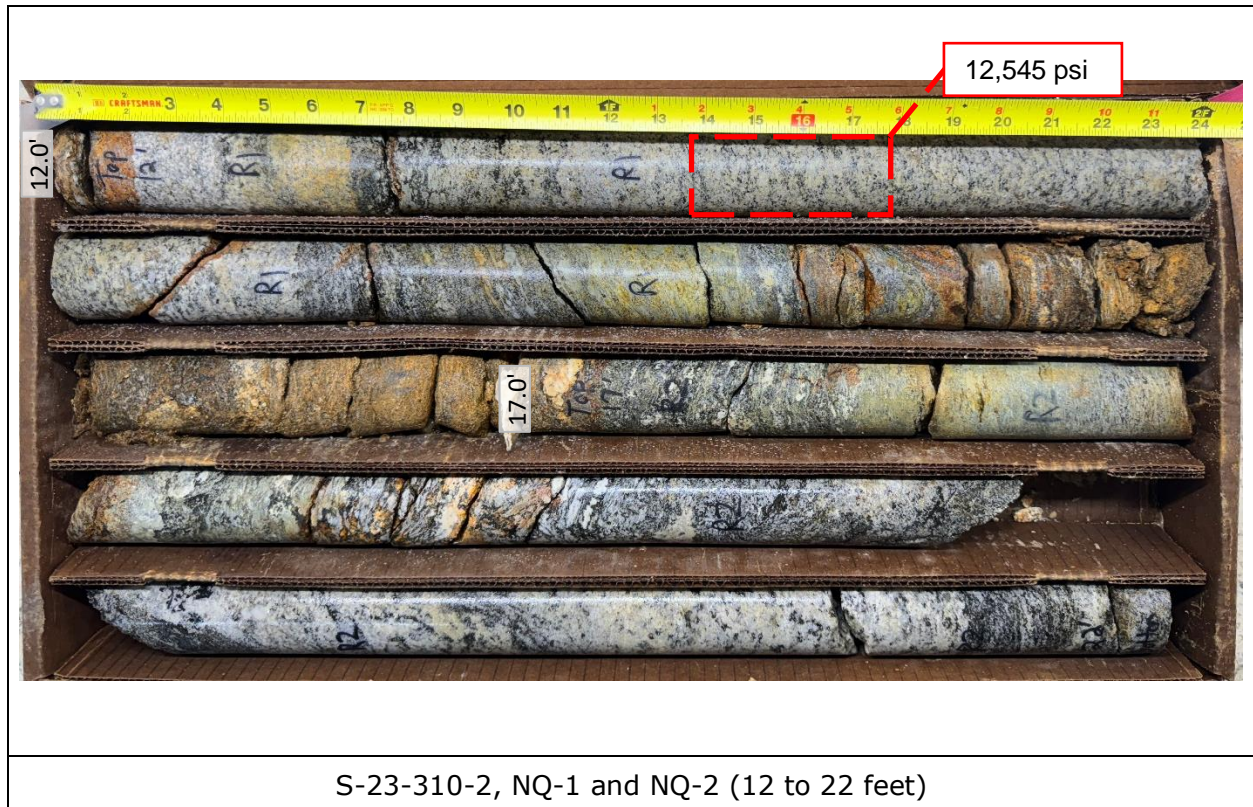


S-23-310-1, NQ-7 and NQ-8 (43 to 51 feet)

Rock Core Photograph Logs – Exhibit A-9

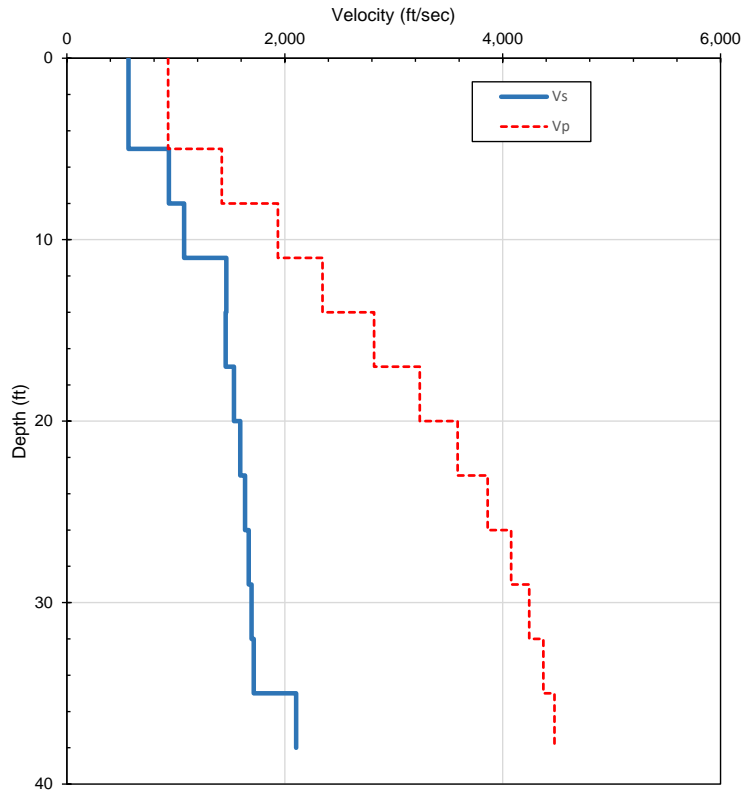
S-23-310 over Langston Tributary | Greenville County, SC

Terracon Project No. 8623P180 | SCDOT Project ID: P041162




S-23-310-2, NQ-1 and NQ-2 (12 to 22 feet)

Downhole Seismic Velocity Fixed Interval Method



Depth (ft)	Vp (ft/sec)	Vs (ft/sec)	Δi (ft)	Δt (sec)	Est. In-Situ Unit Wt (pcf)
5	929	566	5	0.00884	100
8	1422	937	3	0.00320	
11	1936	1077	3	0.00279	
14	2347	1463	3	0.00205	
17	2820	1458	3	0.00206	173
20	3240	1533	3	0.00196	
23	3587	1591	3	0.00189	
26	3862	1635	3	0.00183	
29	4078	1669	3	0.00180	158
32	4245	1696	3	0.00177	
35	4374	1716	3	0.00175	
38	4476	2104	3	0.00143	182
100					2000
					62
					0.03100
Unit Weight of Soil estimated from SPT results					
Unit Weight of Rock based on average results from compression tests					
Sum of Data Over Profile					38
					0.03135
Weighted Average Shear Wave Velocity Over Profile					1,212 ft/sec
Est. Weighted Average Shear Wave Velocity Over 100-Ft ¹					1,604 ft/sec

1. Assuming shear wave of 2,000 f/s for rock below 38 feet.

Project Mgr: MM	Project No. 8623P180	 <p>72 Pointe Circle Ph: (864) 292-2901</p> <p>Greenville, South Carolina Fax: (864) 292-6361</p>	GEOPHYSICAL TESTING RESULTS DOWNHOLE SEISMIC TEST S-23-310 (Crestwood Drive) over Langston Tributary GREENVILLE COUNTY, SOUTH CAROLINA P041162	TEST NO. S-23-310-1 EXHIBIT A-10
Prepared by: MM	Scale: NA			
Checked by: SG	Date: 9/12/2024			
Approved by:				

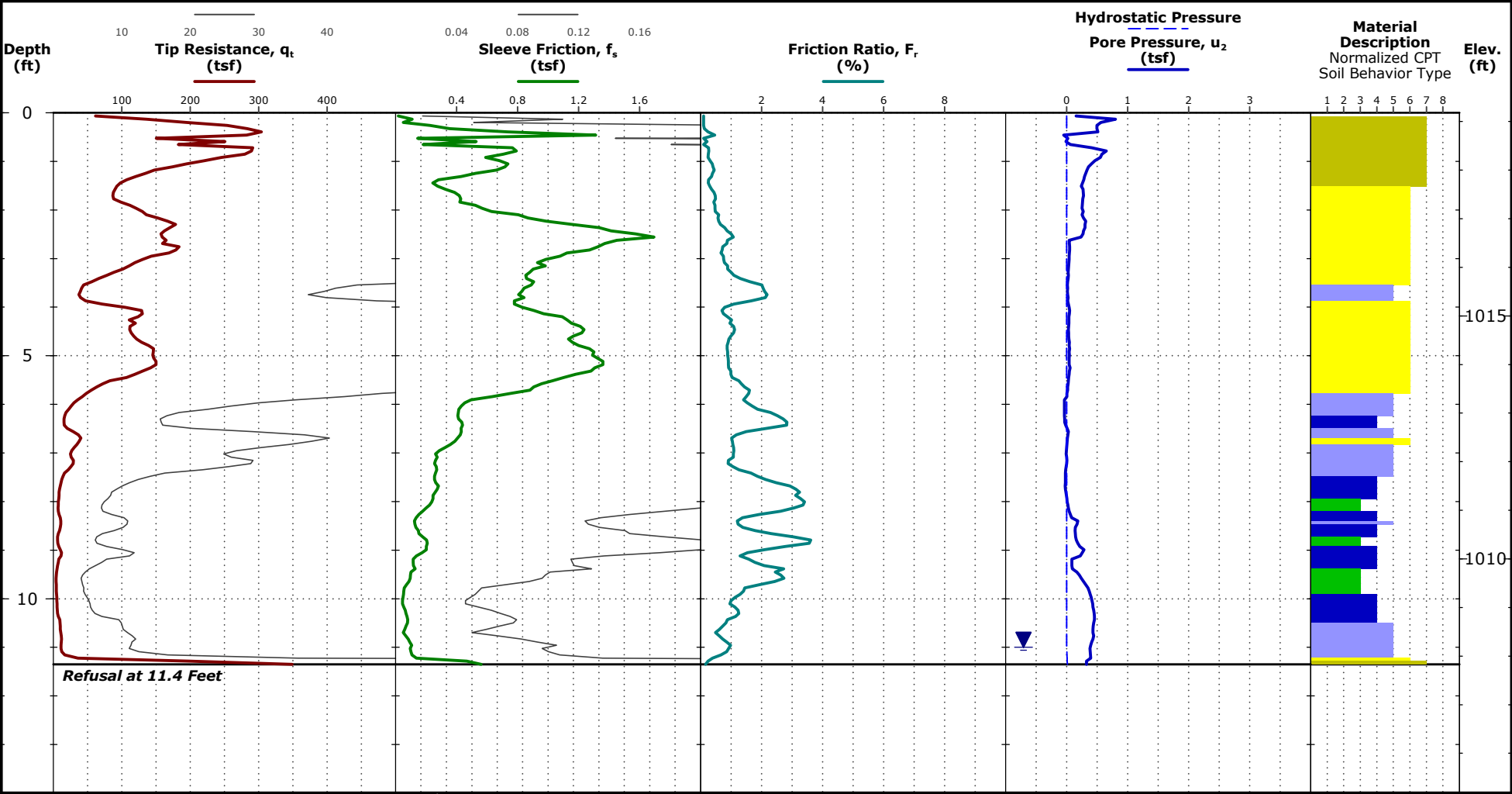
CPT Sounding ID S-23-310-1C



72 Pointe Cir
Greenville, SC
CPT Started: 8/21/2024
CPT Completed: 8/21/2024

Elevation: 1019.18 (ft)
Elevation Reference: Elevations were provided by others.

Latitude: 34.897219° Longitude: -82.400161°
Station: 23+27 Offset: 1 R



See [Exploration and Testing Procedures](#) for a description of field and laboratory procedures used and additional data, if any.
See [Supporting Information](#) for explanation of symbols and abbreviations.

Notes
Test Location: See [Exploration Plan](#)

CPT Equipment
CPT Rig: CR#CPT03
Operator: AM/BR
CPT sensor calibration reports available upon request
Probe No. 6025 with net area ratio of .84
Manufactured by Geoprobe Systems- Calibrated 4/17/2024
Tip and sleeve areas of 10 cm² and 150 cm²

Water Level Observation
11 ft estimated water depth
(used in normalizations and correlations)

- Normalized Soil Behavior Type (Robertson 1990)**
- 1 Sensitive, fine grained
 - 2 Organic soils - clay
 - 3 Clay - silty clay to clay
 - 4 Silt mixtures - clayey silt to silty clay
 - 5 Sand mixtures - silty sand to sandy silt
 - 6 Sands - clean sand to silty sand
 - 7 Gravelly sand to dense sand
 - 8 Very stiff sand to clayey sand
 - 9 Very stiff fine grained

Appendix B – Laboratory Testing

S-23-310 over Langston Tributary | Greenville County, SC Terracon
Project No. 8623P180 | SCDOT Project ID: P041162



Appendix B

Laboratory Testing

Exhibit B-1 – Laboratory Testing Description
Summary of Laboratory Data
Laboratory Data Sheets (21 Pages)

Note: All exhibits are one page unless noted above.

Exhibit B-1 – Laboratory Testing Description

S-23-310 over Langston Tributary | Greenville County, SC Terracon
Project No. 8623P180 | SCDOT Project ID: P041162



Laboratory Testing Description

The samples collected during the field exploration were taken to our laboratory for additional testing. The laboratory testing scope was developed by the SCDOT and laboratory assignment was performed by Terracon. The laboratory tests were conducted on selected soil samples from the borings and the bulk sample locations. The test results are presented in this appendix.

The laboratory test results were used to confirm the soil descriptions presented on the boring logs in Appendix A. Laboratory tests were performed in general accordance with the applicable ASTM, AASHTO, SCDOT or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

■ Rock Compressive Strength	ASTM D7012
■ Moisture Content	AASHTO T265/(ASTM D2216)
■ Atterberg Limits	AASHTO T89/T90(ASTM D4318)
■ Wash 200	AASHTO T11/(ASTM D1140)
■ Proctor (Standard effort)	AASHTO T99/ (ASTM D698)
■ Triaxial Shear CU w/ PP	AASHTO T297/(ASTM D4767)
■ Grain Size Distribution	ASTM D6913
■ Hydrometer	ASTM D7928
■ Corrosion Series	AASHTO D422
	AASHTO T289/ASTM G51
	AASHTO T290/ASTM C1580
	AASHTO T291

Summary of Laboratory Results

Boring ID	Depth (Ft.)	Soil Classification USCS & AASHTO	Liquid Limit	Plastic Limit	Plasticity Index	% Gravel	% Sand	% Fines	% Silt	% Clay	Water Content (%)	Proctor Dry Density (pcf)/Opt. Moisture (%)
S-23-310-1	2-4	SILTY SAND(SM) / A-2-4 (0)	NP	NP	NP	8.5	61.6	29.8			11.9	
S-23-310-1	4-6	SILTY SAND WITH GRAVEL(SM) / A-2-4 (0)	NP	NP	NP	17.5	47.3	35.2			15.2	
S-23-310-1	6-8	SILTY SAND(SM) / A-4 (0)	NP	NP	NP	0.0	56.0	44.0	23.4	20.7	22.4	
S-23-310-1	8-10	SILT(ML) / A-4 (0)	NP	NP	NP	0.0	13.2	86.8	49.2	37.6	59.7	
S-23-310-1 Offset	0-5	SILTY SAND(SM) / A-2-4 (0)	NP	NP	NP	5.8	69.6	24.6				121.6 / 10.4
S-23-310-2	0.5-2	SILTY SAND WITH GRAVEL(SM) / A-1-B (0)	NP	NP	NP	17.5	69.5	13.0			22.2	
S-23-310-2	2-2.96	SILTY SAND WITH GRAVEL(SM) / A-1-B									8.0	



INDEX PROPERTIES VERSUS DEPTH

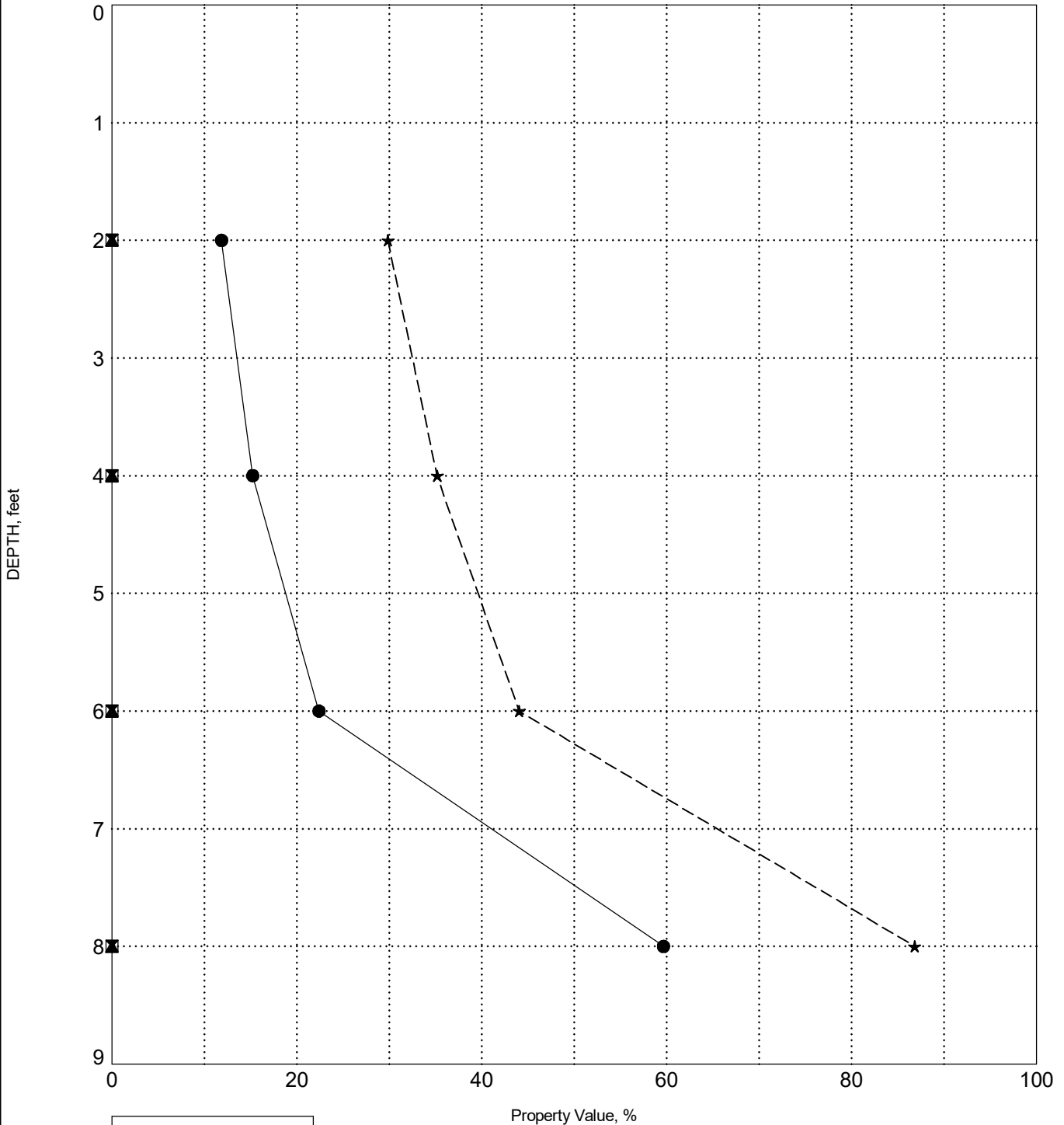
PROJECT ID P041162

PROJECT NAME S-23-310 over Langston Tributary

PROJECT COUNTY Greenville

SURFACE ELEVATION: 1019.2

BORING S-23-310-1



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines



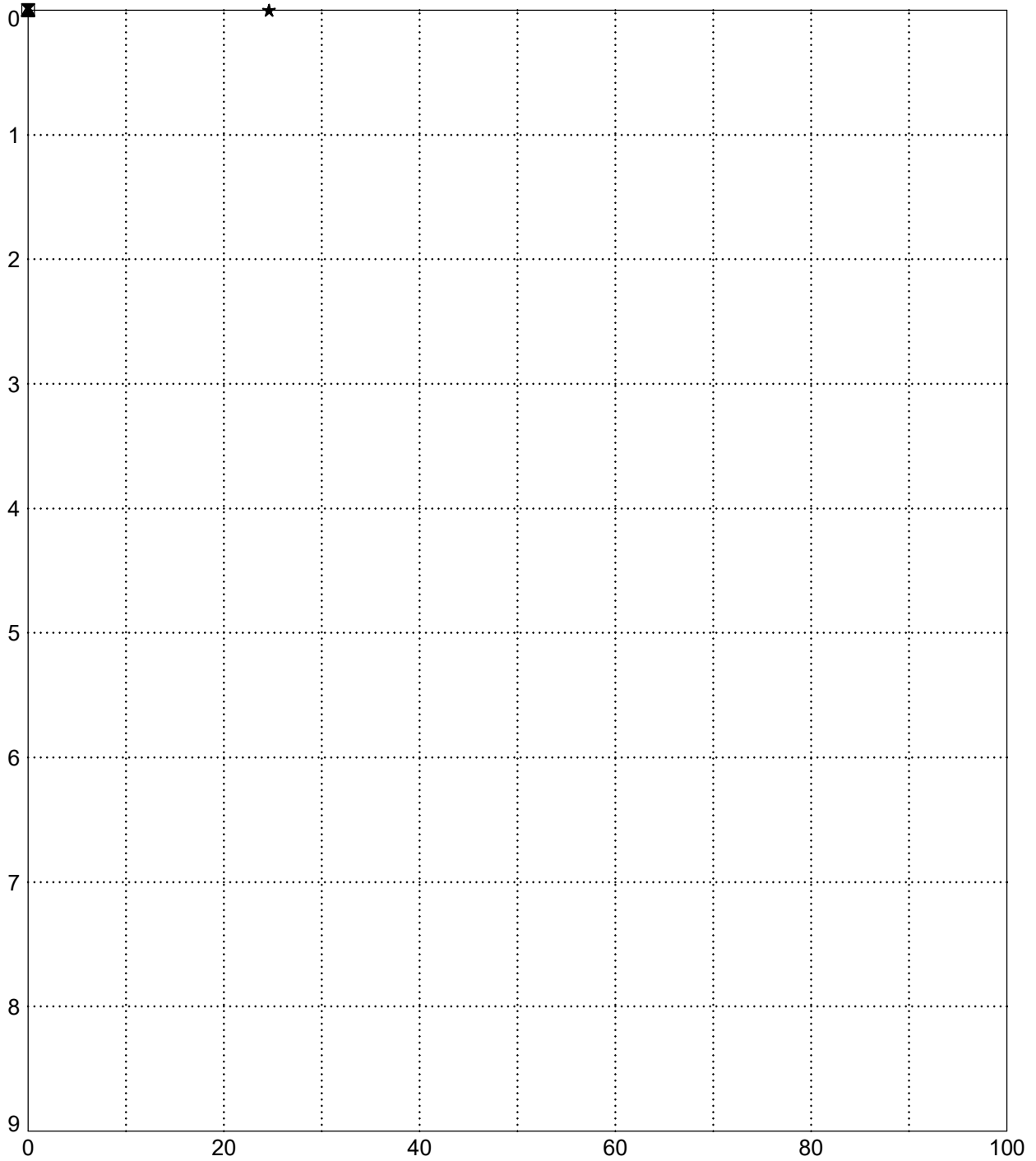
INDEX PROPERTIES VERSUS DEPTH

PROJECT ID P041162

PROJECT NAME S-23-310 over Langston Tributary

PROJECT COUNTY Greenville

BORING S-23-310-1 Offset



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines



INDEX PROPERTIES VERSUS DEPTH

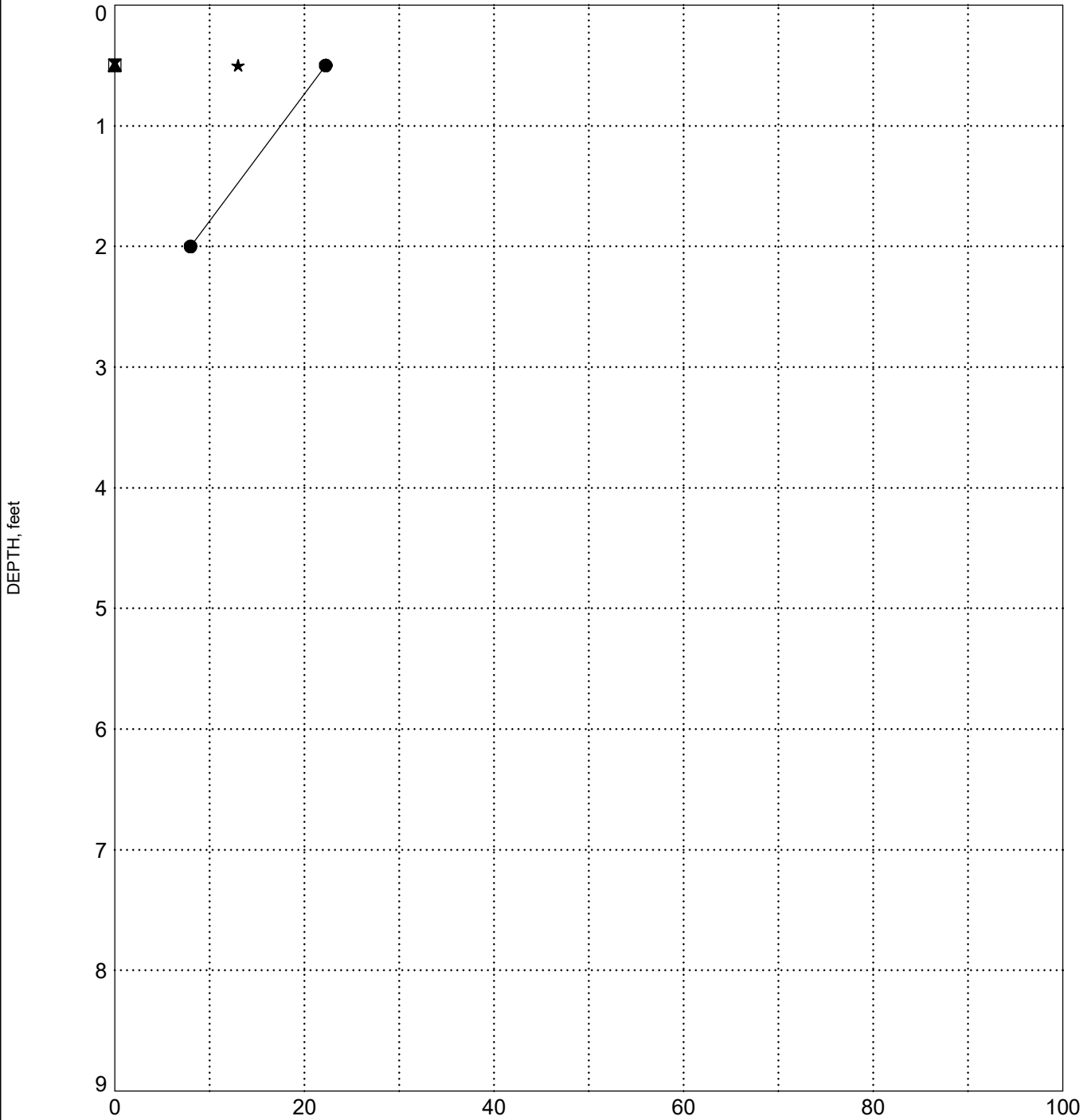
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PROJECT NAME S-23-310 over Langston Tributary

PROJECT COUNTY Greenville

SURFACE ELEVATION: 1024.9

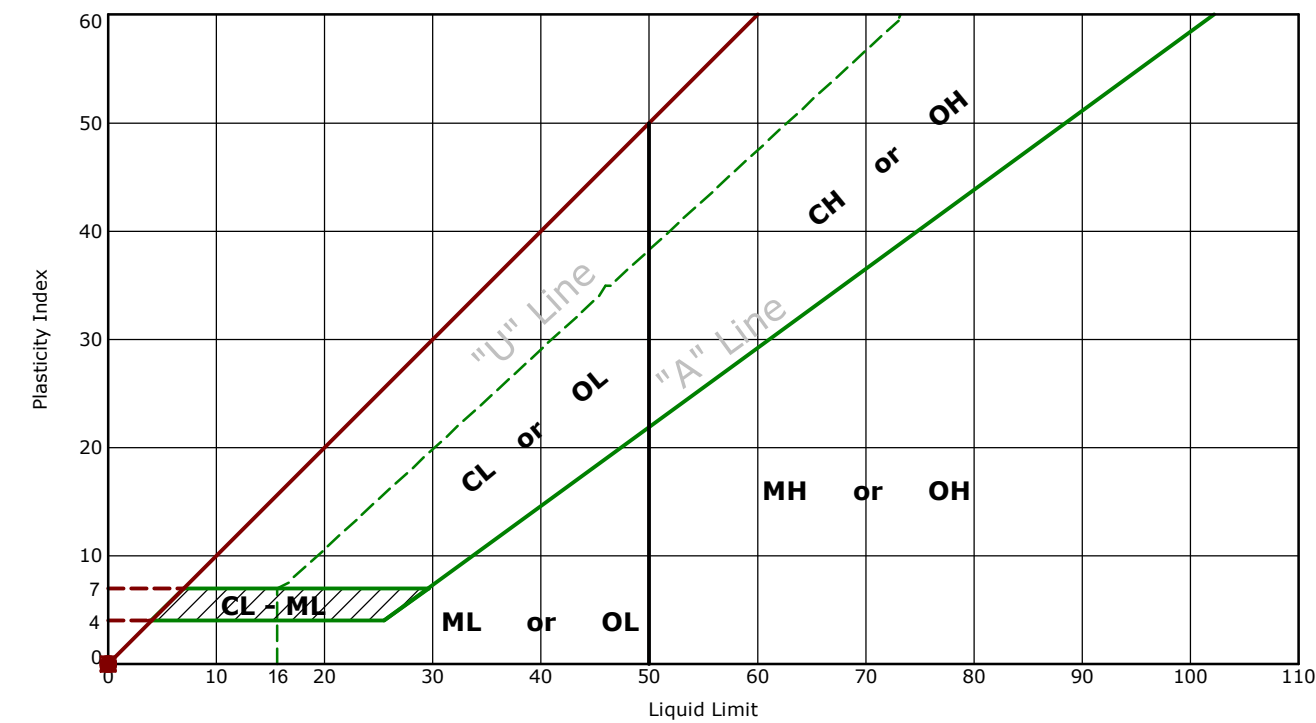
BORING S-23-310-2



LEGEND	
●	Water Content
⊠	Plastic Limit
▲	Liquid Limit
★	Fines

Atterberg Limit Results

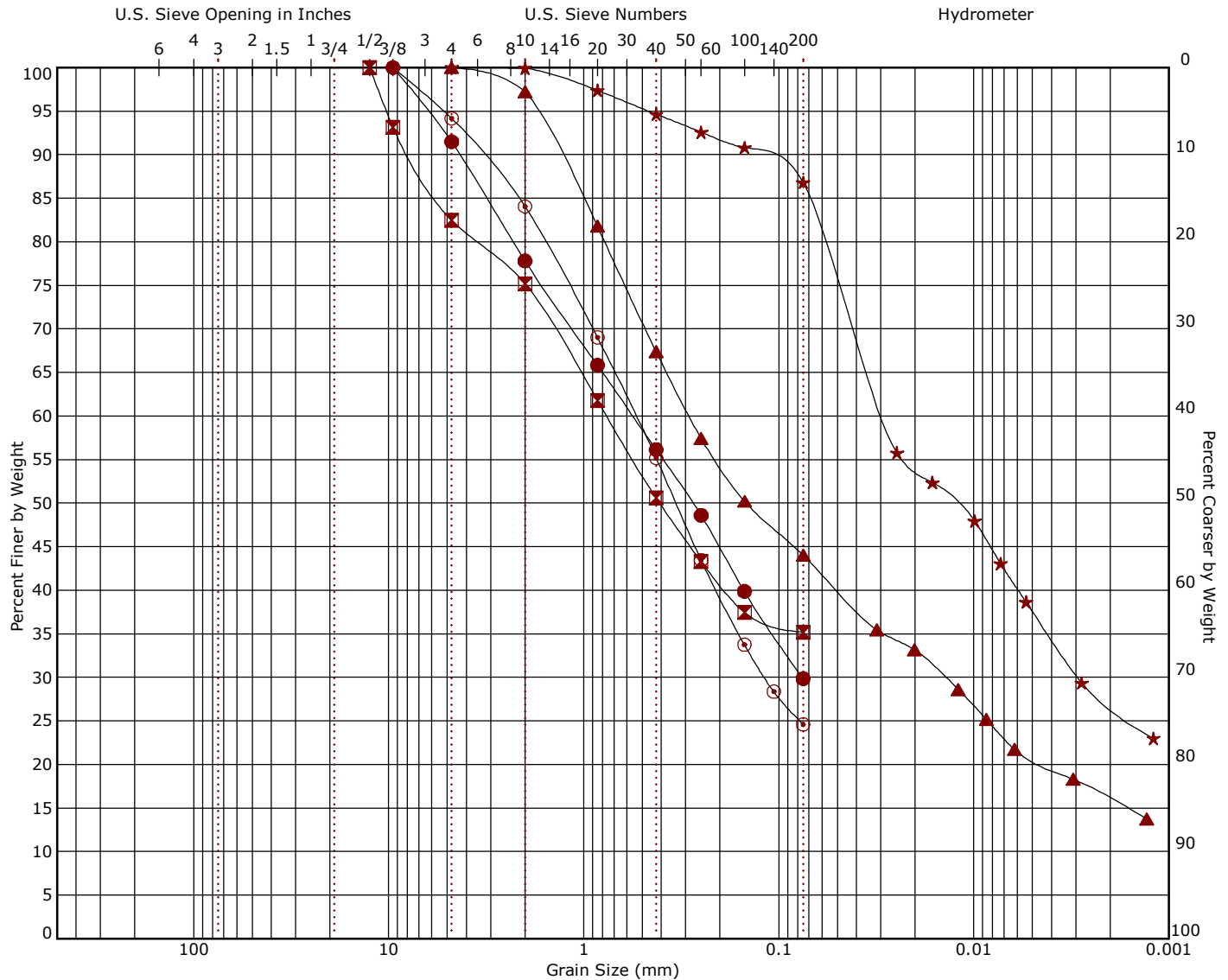
ASTM D4318



	Boring ID	Depth (Ft)	LL	PL	PI	Fines	AASHTO	Description
●	S-23-310-1	2 - 4	NP	NP	NP	29.8	A-2-4 (0)	SILTY SAND
⊠	S-23-310-1	4 - 6	NP	NP	NP	35.2	A-2-4 (0)	SILTY SAND with GRAVEL
▲	S-23-310-1	6 - 8	NP	NP	NP	44.0	A-4 (0)	SILTY SAND
★	S-23-310-1	8 - 10	NP	NP	NP	86.8	A-4 (0)	SILT
⊙	S-23-310-1 Offset	0 - 5	NP	NP	NP	24.6	A-2-4 (0)	SILTY SAND
⊕	S-23-310-2	0.5 - 2	NP	NP	NP	13.0	A-1-b (0)	SILTY SAND with GRAVEL

Grain Size Distribution

ASTM D422 / ASTM C136



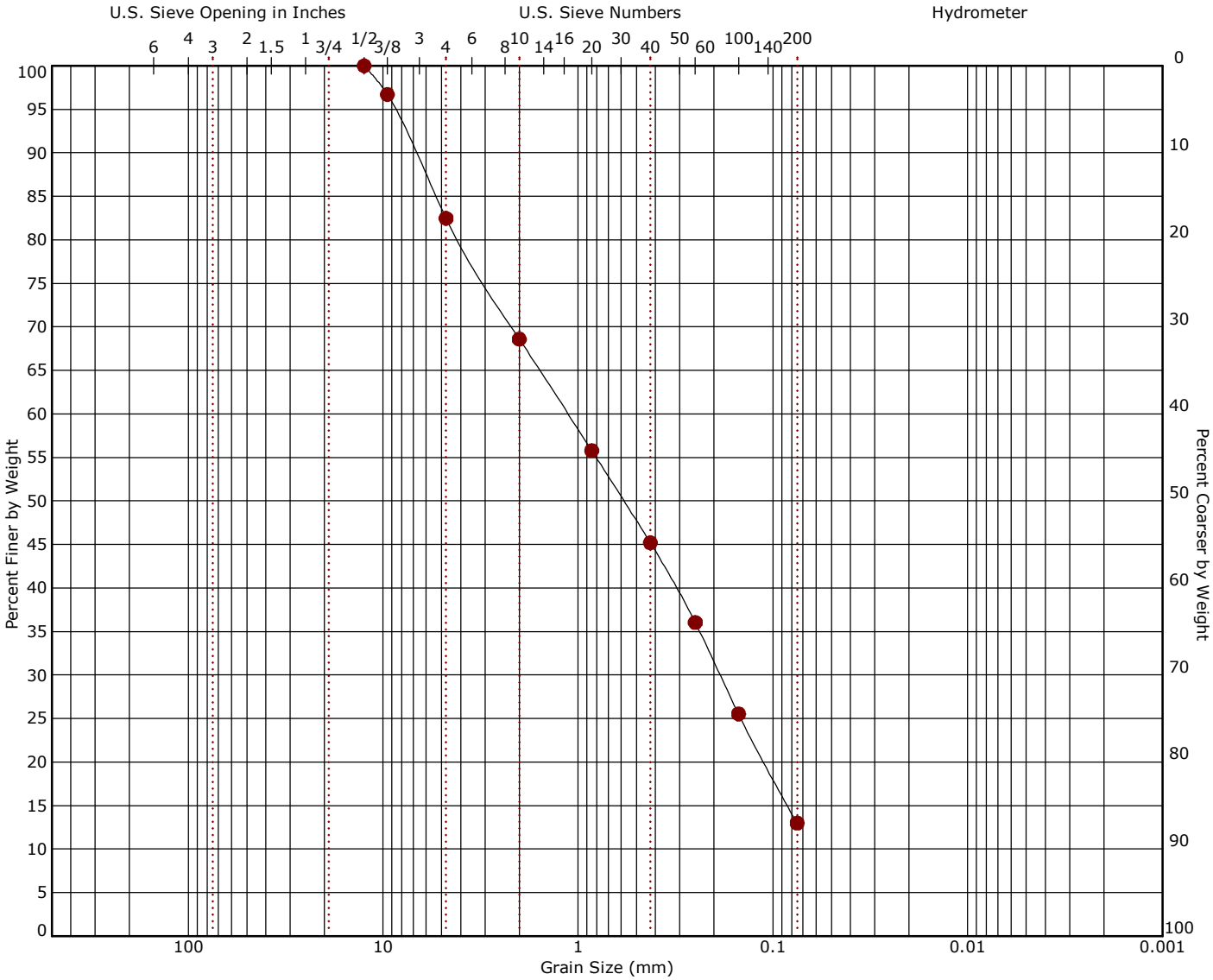
Cobbles	Gravel		Sand			Silt or Clay
	coarse	fine	coarse	medium	fine	

Boring ID	Depth (Ft)	USCS Classification	USCS	AASHTO	LL	PL	PI	Cc	Cu
● S-23-310-1	2 - 4	SILTY SAND	SM	A-2-4 (0)	NP	NP	NP		
☒ S-23-310-1	4 - 6	SILTY SAND with GRAVEL	SM	A-2-4 (0)	NP	NP	NP		
▲ S-23-310-1	6 - 8	SILTY SAND	SM	A-4 (0)	NP	NP	NP		
★ S-23-310-1	8 - 10	SILT	ML	A-4 (0)	NP	NP	NP		
⊙ S-23-310-1 Offset	0 - 5	SILTY SAND	SM	A-2-4 (0)	NP	NP	NP		

Boring ID	Depth (Ft)	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
● S-23-310-1	2 - 4	9.5	0.56	0.076		0.0	8.5	61.6	29.8		
☒ S-23-310-1	4 - 6	12.5	0.76			0.0	17.5	47.3	35.2		
▲ S-23-310-1	6 - 8	4.75	0.287	0.014		0.0	0.0	56.0		23.4	20.7
★ S-23-310-1	8 - 10	4.75	0.029	0.003		0.0	0.0	13.2		49.2	37.6
⊙ S-23-310-1 Offset	0 - 5	9.5	0.541	0.118		0.0	5.8	69.6	24.6		

Grain Size Distribution

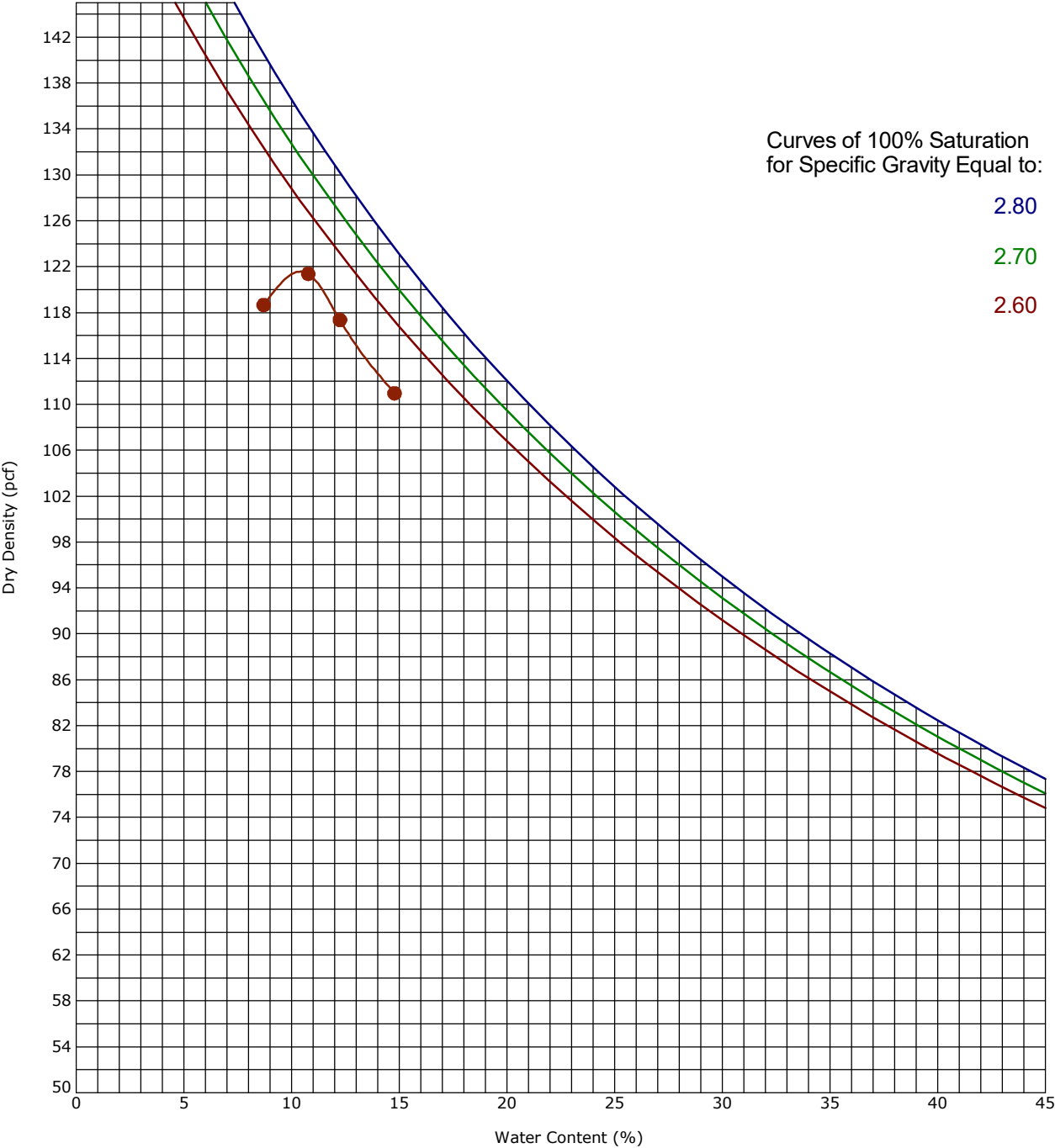
ASTM D422 / ASTM C136



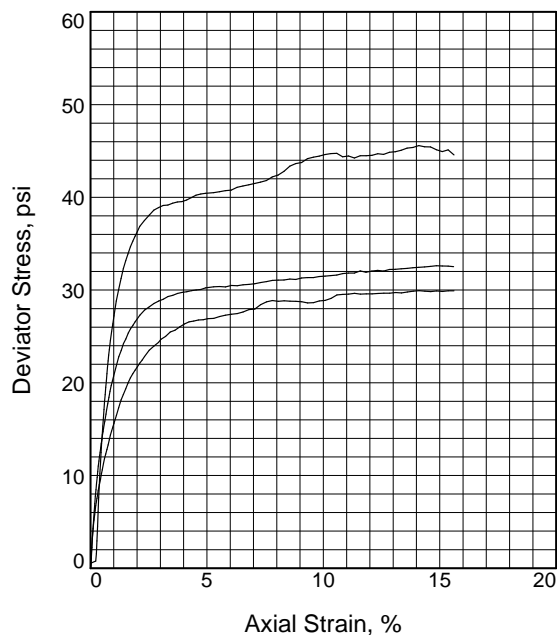
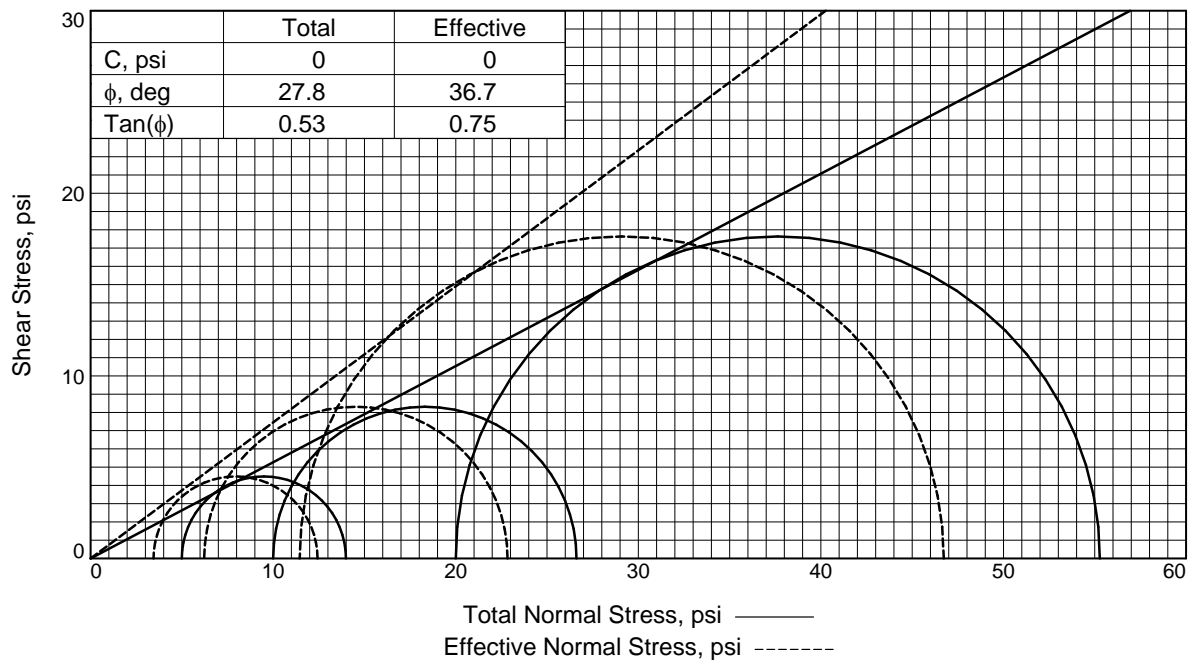
Cobbles		Gravel		Sand			Silt or Clay				
		coarse	fine	coarse	medium	fine					
Boring ID	Depth (Ft)	USCS Classification			USCS	AASHTO	LL	PL	PI	Cc	Cu
● S-23-310-2	0.5 - 2	SILTY SAND with GRAVEL			SM	A-1-b (0)	NP	NP	NP		
Boring ID	Depth (Ft)	D ₁₀₀	D ₆₀	D ₃₀	D ₁₀	%Cobbles	%Gravel	%Sand	%Fines	%Silt	%Clay
● S-23-310-2	0.5 - 2	12.5	1.127	0.186		0.0	17.5	69.5	13.0		

Moisture-Density Relationship

ASTM D698-Method B



Boring ID		Depth (Ft)		Description of Materials			
S-23-310-1 Offset		0 - 5		SILTY SAND(SM)			
Fines (%)	Fraction > mm size	LL	PL	PI	Test Method	Maximum Dry Density (pcf)	Optimum Water Content (%)
25	0.0	NP	NP	NP	ASTM D698-Method B	121.6	10.4



Sample No.		1	2	3
Initial	Water Content, %	10.3	10.2	10.3
	Dry Density, pcf	115.7	116.0	115.9
	Saturation, %	61.0	60.7	61.4
	Void Ratio	0.4572	0.4537	0.4547
	Diameter, in.	2.80	2.80	2.80
	Height, in.	5.62	5.62	5.62
At Test	Water Content, %	15.2	15.1	14.8
	Dry Density, pcf	119.6	119.7	120.5
	Saturation, %	100.0	100.0	100.0
	Void Ratio	0.4095	0.4081	0.3985
	Diameter, in.	2.76	2.76	2.75
	Height, in.	5.58	5.59	5.58
Strain rate, in./min.		0.001	0.001	0.001
Back Pressure, psi		50.0	50.0	50.0
Cell Pressure, psi		55.0	60.0	70.0
Fail. Stress, psi		9.0	16.6	35.3
Excess Pore Pr., psi		1.5	3.8	8.5
Ult. Stress, psi		29.8	32.6	44.9
Excess Pore Pr., psi		-4.8	-1.2	6.2
$\bar{\sigma}_1$ Failure, psi		12.4	22.8	46.7
$\bar{\sigma}_3$ Failure, psi		3.5	6.2	11.5

Type of Test:

CU with Pore Pressures

Sample Type: Remolded

Description: Silty Sand (SM)

LL= NP

PI= NP

Specific Gravity= 2.7

Remarks: Specimens were remolded to approximately 95% of MDD at optimum water content.

Figure _____

Client: HNTB North Carolina PC

Project: S-23-310 (Crestwood Drive) over Langston Tributary

Source of Sample: S-23-310-1 Offset **Depth:** 0-5'

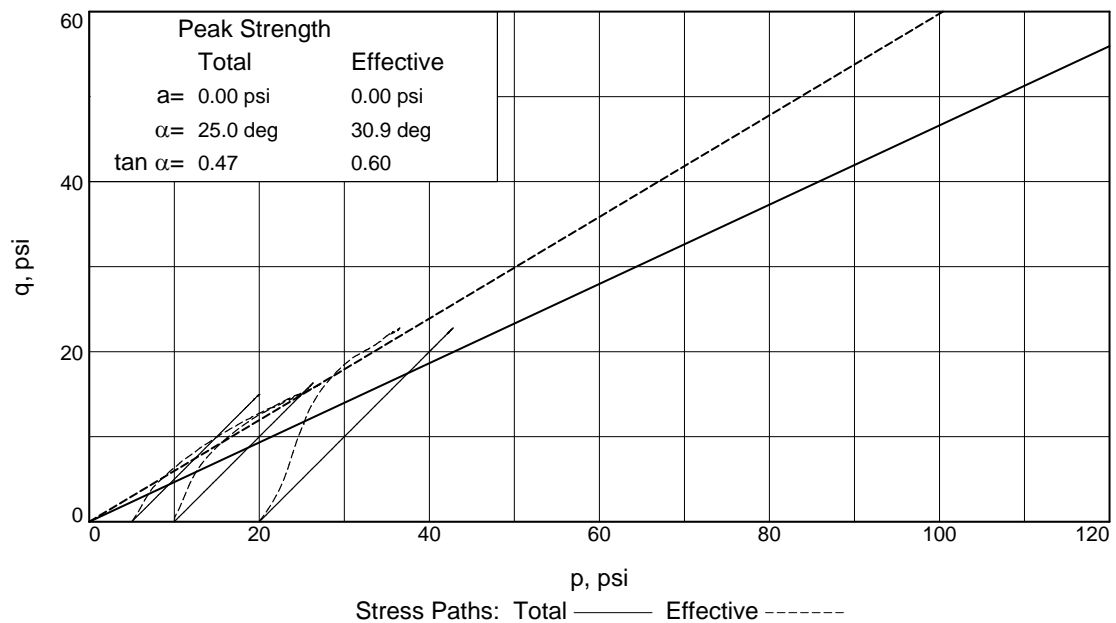
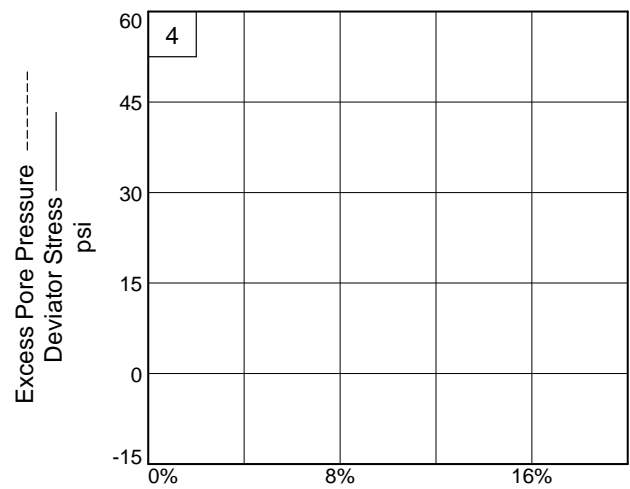
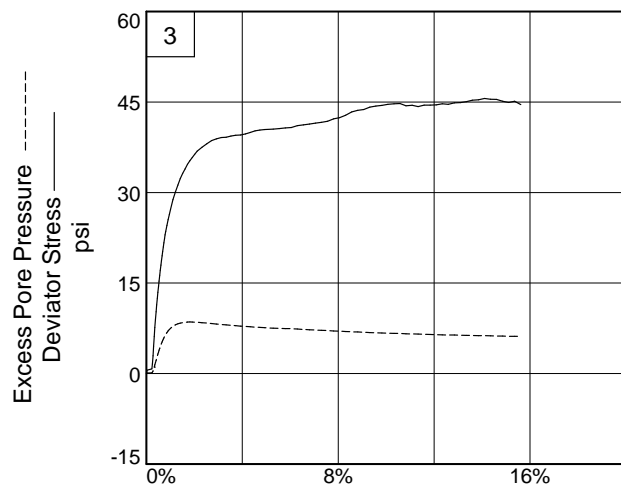
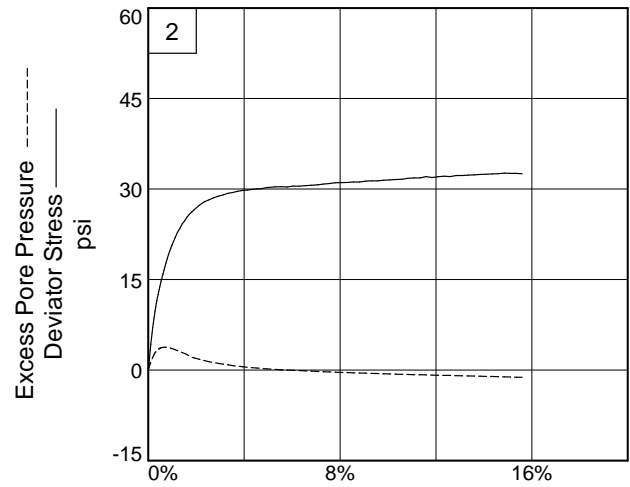
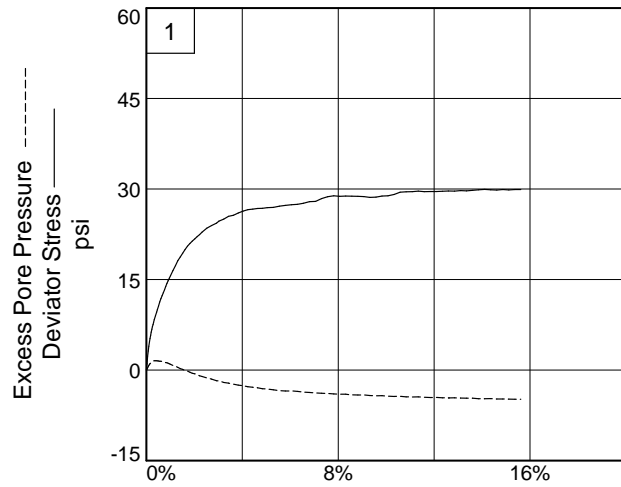
Proj. No.: 8623P180

Date Sampled: N/A

TRIAXIAL SHEAR TEST REPORT

Terracon Consultants, Inc.

Chattanooga, TN



Client: HNTB North Carolina PC

Project: S-23-310 (Crestwood Drive) over Langston Tributary

Source of Sample: S-23-310-1 Offset **Depth:** 0-5'

Project No.: 8623P180

Figure _____

Terracon Consultants, Inc.

750 Pilot Road, Suite F
Las Vegas, Nevada 89119
(702) 597-9393



Client

HNTB North Carolina PC

Project

S-23-310 (Crestwood Drive) over Langston Tributary

Sample Submitted By: Terracon (86)

Date Received: 8/16/2024

Lab No.: 24-0279

Results of Corrosion Analysis

Sample Number	S-23-310-1
Sample Location	--
Sample Depth (ft.)	2.0-10.0
pH Analysis, AASHTO T289	5.26
Water Soluble Sulfate (SO4), AASHTO T290 (mg/kg)	90
Chlorides, AASHTO T291, (mg/kg)	132
Saturated Minimum Resistivity, AASHTO T288, (ohm-cm)	2231

A handwritten signature in black ink, appearing to read 'N. Campo'.

Analyzed By _____
Nathan Campo
Laboratory Coordinator

The tests were performed in general accordance with applicable ASTM and AWWA test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.



Rock Coring Summary

PAGE 1 OF 1

PROJECT ID P041162

PROJECT NAME S-23-310 over Langston Tributary

PROJECT COUNTY Greenville

Borehole	Core Run Number	Core Run Top Depth	REC (%)	RQD (%)	q _u (psi)	Poisson's Ratio	Secant Modulus (ksi)	Unit Weight (pcf)	RMR	GSI
S-23-310-1	NQ-1	13.0	94	88						80
S-23-310-1	NQ-2	18.0	100	58	6127	0.07	817	173	67	70
S-23-310-1	NQ-3	23.0	97	73						75
S-23-310-1	NQ-4	28.0	95	65	5190	0.28	177	158	67	70
S-23-310-1	NQ-5	33.0	100	78						75
S-23-310-1	NQ-6	38.0	100	86	10054	0.04	1204	182	74	80
S-23-310-1	NQ-7	43.0	98	58						70
S-23-310-1	NQ-8	48.0	94	82	6048	0.03	1065	168	71	75
S-23-310-2	NQ-1	12.1	93	53	12545	0.46	1243	164	70	70
S-23-310-2	NQ-2	17.0	95	77						75

Client

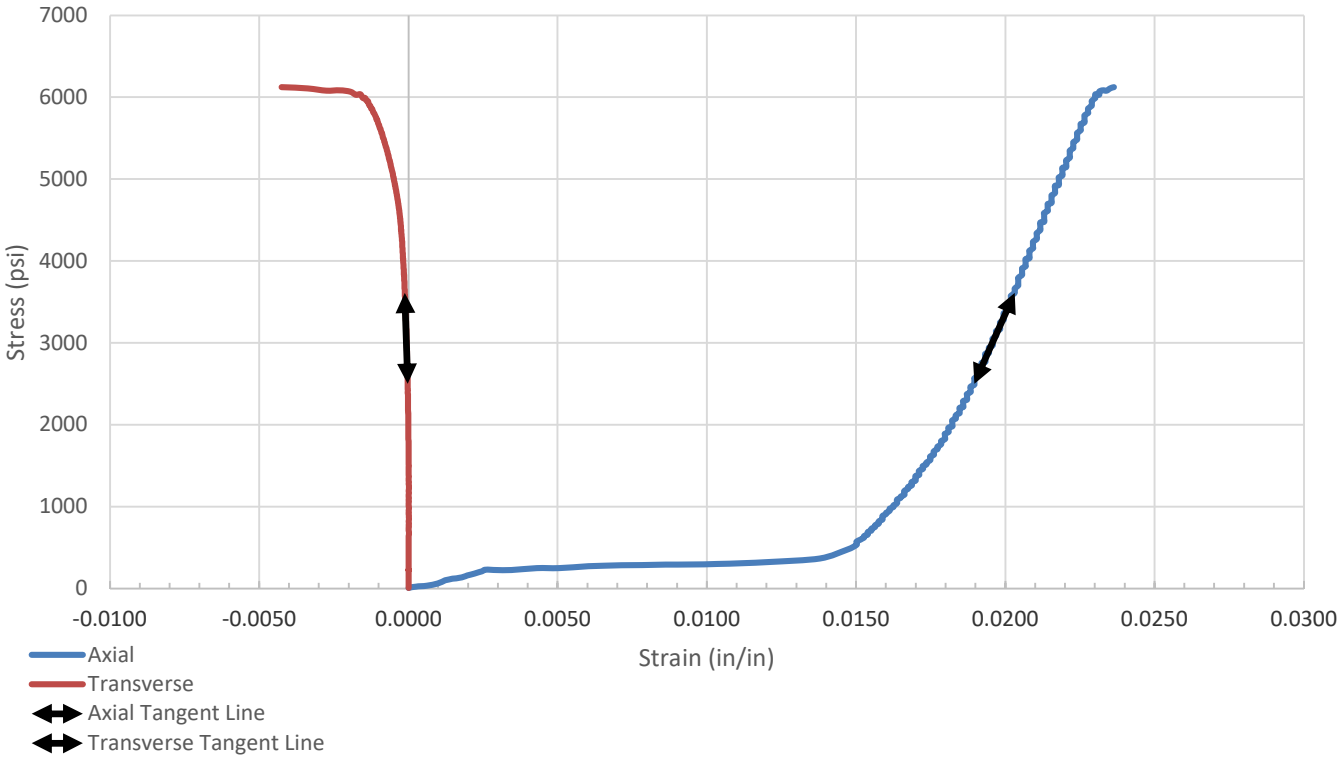
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 19

Project No. 8623P180

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	SCDOT Bridge Package 19		
Description:	Schist		
Boring:	S-23-310-1	Depth (feet):	21.3-22.8

SPECIMEN INFORMATION

Sample No.:	R2	Mass (g):	550.91
Length (in.):	4.07	Diameter (in.):	1.95
L/D Ratio:	2.09	Density (pcf):	172.67

TEST RESULTS

Failure Load (lbs):	18297
Failure Strain (%):	2.72
Unconfined Compressive Strength (psi):	6,127
Elastic Modulus, E, (ksi):	817
Poisson's Ratio, u:	0.069
Time of Failure (min):	00:56
Rate of Loading (psi/sec):	0.041
Moisture Content Post-break:	0.0021



Client	Project
HNTB North Carolina PC Attn: Spencer Franklin 343 E Six Forks Rd Ste 200 Raleigh, NC 27609	SCDOT Bridge Package 19
	Project No. 8623P180

Equipment:	TICCS ID:
Calipers	W-54522
Scale	B-71466
Dial Indicator	C-70608
Compression (spherically seated)	C-48999

Samples were prepared and tested in accordance with ASTM D4543 and D7012. Deviations, if any, are noted below:

Notes:

Per ASTM D4543, this specimen has not met the requirements for straightness, by exceeding 0.02 inches.

Per ASTM D4543, this specimen has not met the requirements for perpendicularity, by exceeding 0.250°.

Per ASTM D4543, this specimen has not met the requirements for flatness, by exceeding 0.001 inches.

Per ASTM D4543, this specimen has not met the requirements for parallelism, by exceeding 0.25°.

According to ASTM D7012 Section 8.2.1, this specimen, although not meeting all requirements of ASTM D4543 is acceptable for testing. However, the results reported may differ from results obtained from a test specimen that meets the requirements of D4543.

Client

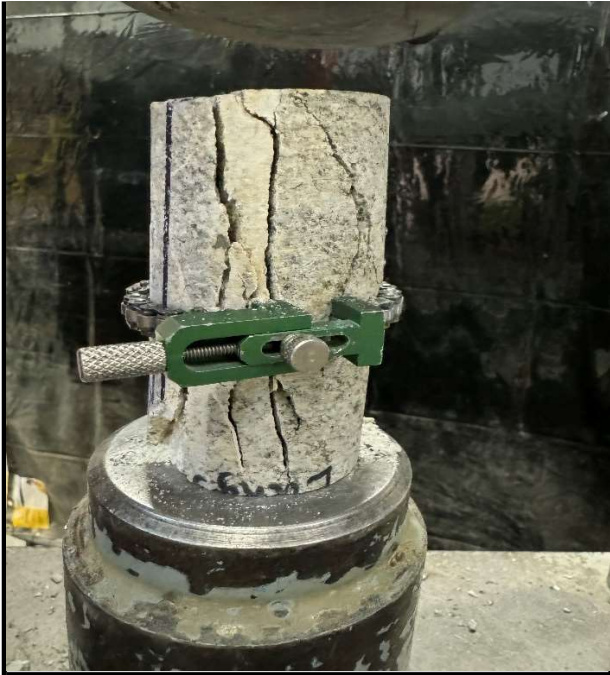
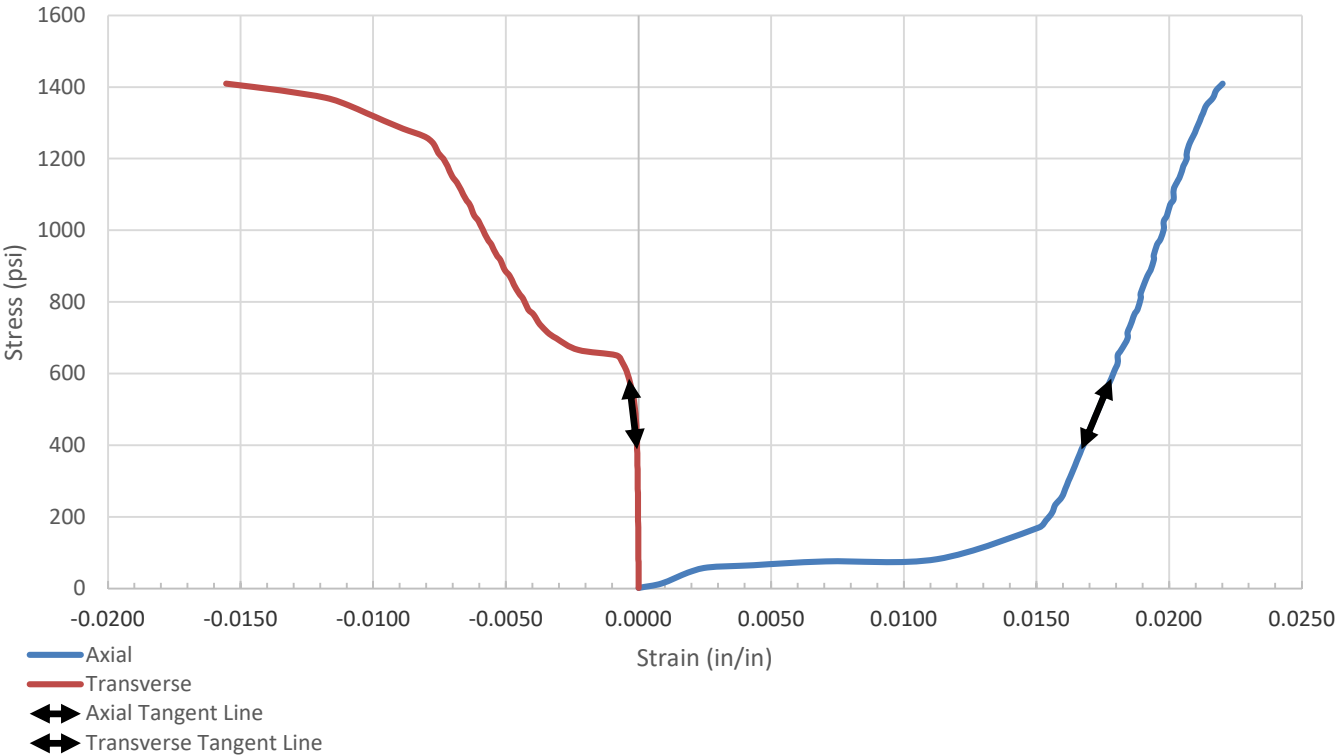
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 19

Project No. 8623P180

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	SCDOT Bridge Package 19		
Description:	Granite		
Boring:	S-23-310-1	Depth (feet):	30.4-31.1

SPECIMEN INFORMATION

Sample No.:	R4	Mass (g):	507.69
Length (in.):	4.05	Diameter (in.):	1.96
L/D Ratio:	2.07	Density (pcf):	158.28

TEST RESULTS

Failure Load (lbs):	15658
Failure Strain (%):	2.33
Unconfined Compressive Strength (psi):	5,190
Elastic Modulus, E, (ksi):	177
Poisson's Ratio, u:	0.278
Time of Failure (min):	00:13
Rate of Loading (psi/sec):	0.041
Moisture Content Post-break:	0.0050



Client	Project
HNTB North Carolina PC Attn: Spencer Franklin 343 E Six Forks Rd Ste 200 Raleigh, NC 27609	SCDOT Bridge Package 19
	Project No. 8623P180

Equipment:	TICCS ID:
Calipers	W-54522
Scale	B-71466
Dial Indicator	C-70608
Compression (spherically seated)	C-48999

Samples were prepared and tested in accordance with ASTM D4543 and D7012. Deviations, if any, are noted below:

Notes:

Per ASTM D4543, this specimen has not met the requirements for perpendicularity, by exceeding 0.250°.

Per ASTM D4543, this specimen has not met the requirements for flatness, by exceeding 0.001 inches.

Per ASTM D4543, this specimen has not met the requirements for parallelism, by exceeding 0.25°.

According to ASTM D7012 Section 8.2.1, this specimen, although not meeting all requirements of ASTM D4543 is acceptable for testing. However, the results reported may differ from results obtained from a test specimen that meets the requirements of D4543.

Client

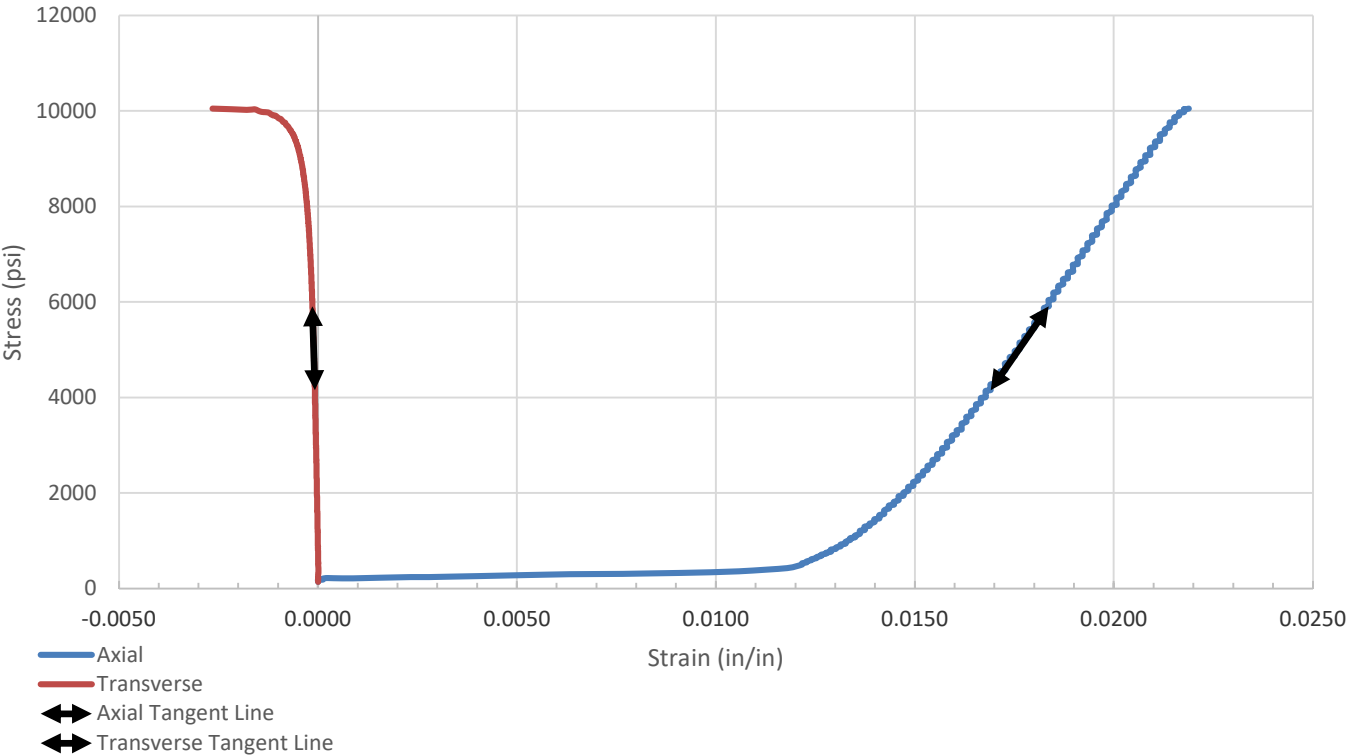
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 19

Project No. 8623P180

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	SCDOT Bridge Package 19		
Description:	Gneiss		
Boring:	S-23-310-1	Depth (feet):	41.5-42.7

SPECIMEN INFORMATION

Sample No.:	R6	Mass (g):	586.63
Length (in.):	4.12	Diameter (in.):	1.95
L/D Ratio:	2.11	Density (pcf):	181.63

TEST RESULTS

Failure Load (lbs):	30025
Failure Strain (%):	2.69
Unconfined Compressive Strength (psi):	10,054
Elastic Modulus, E, (ksi):	1204
Poisson's Ratio, u:	0.040
Time of Failure (min):	01:31
Rate of Loading (psi/sec):	0.041
Moisture Content Post-break:	0.0011



Client	Project
HNTB North Carolina PC Attn: Spencer Franklin 343 E Six Forks Rd Ste 200 Raleigh, NC 27609	SCDOT Bridge Package 19
	Project No. 8623P180

Equipment:	TICCS ID:
Calipers	W-54522
Scale	B-71466
Dial Indicator	C-70608
Compression (spherically seated)	C-48999

Samples were prepared and tested in accordance with ASTM D4543 and D7012. Deviations, if any, are noted below:

Notes:

Per ASTM D4543, this specimen has not met the requirements for perpendicularity, by exceeding 0.250°.

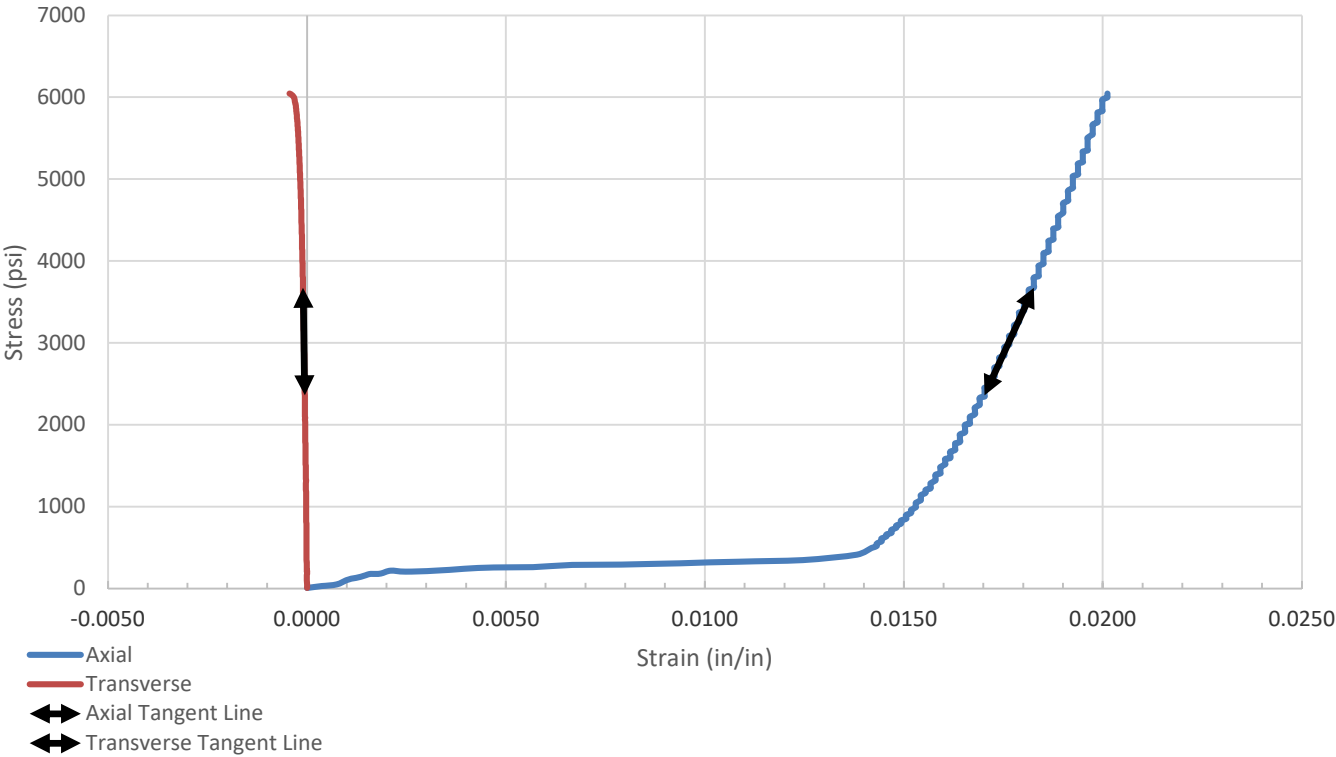
Per ASTM D4543, this specimen has not met the requirements for flatness, by exceeding 0.001 inches.

Per ASTM D4543, this specimen has not met the requirements for parallelism, by exceeding 0.25°.

According to ASTM D7012 Section 8.2.1, this specimen, although not meeting all requirements of ASTM D4543 is acceptable for testing. However, the results reported may differ from results obtained from a test specimen that meets the requirements of D4543.

Client	Project
HNTB North Carolina PC	SCDOT Bridge Package 19
Attn: Spencer Franklin	
343 E Six Forks Rd Ste 200	
Raleigh, NC 27609	Project No. 8623P180

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	SCDOT Bridge Package 19		
Description:	Gneiss		
Boring:	S-23-310-1	Depth (feet):	49.8-50.4

SPECIMEN INFORMATION

Sample No.:	R8	Mass (g):	535.11
Length (in.):	4.06	Diameter (in.):	1.95
L/D Ratio:	2.08	Density (pcf):	168.13

TEST RESULTS

Failure Load (lbs):	18061
Failure Strain (%):	2.32
Unconfined Compressive Strength (psi):	6,048
Elastic Modulus, E, (ksi):	1065
Poisson's Ratio, u:	0.034
Time of Failure (min):	00:55
Rate of Loading (psi/sec):	0.041
Moisture Content Post-break:	0.0029



Client	Project
HNTB North Carolina PC Attn: Spencer Franklin 343 E Six Forks Rd Ste 200 Raleigh, NC 27609	SCDOT Bridge Package 19
	Project No. 8623P180

Equipment:	TICCS ID:
Calipers	W-54522
Scale	B-71466
Dial Indicator	C-70608
Compression (spherically seated)	C-48999

Samples were prepared and tested in accordance with ASTM D4543 and D7012. Deviations, if any, are noted below:

Notes:

Per ASTM D4543, this specimen has not met the requirements for straightness, by exceeding 0.02 inches.

Per ASTM D4543, this specimen has not met the requirements for perpendicularity, by exceeding 0.250°.

Per ASTM D4543, this specimen has not met the requirements for flatness, by exceeding 0.001 inches.

Per ASTM D4543, this specimen has not met the requirements for parallelism, by exceeding 0.25°.

According to ASTM D7012 Section 8.2.1, this specimen, although not meeting all requirements of ASTM D4543 is acceptable for testing. However, the results reported may differ from results obtained from a test specimen that meets the requirements of D4543.

Client

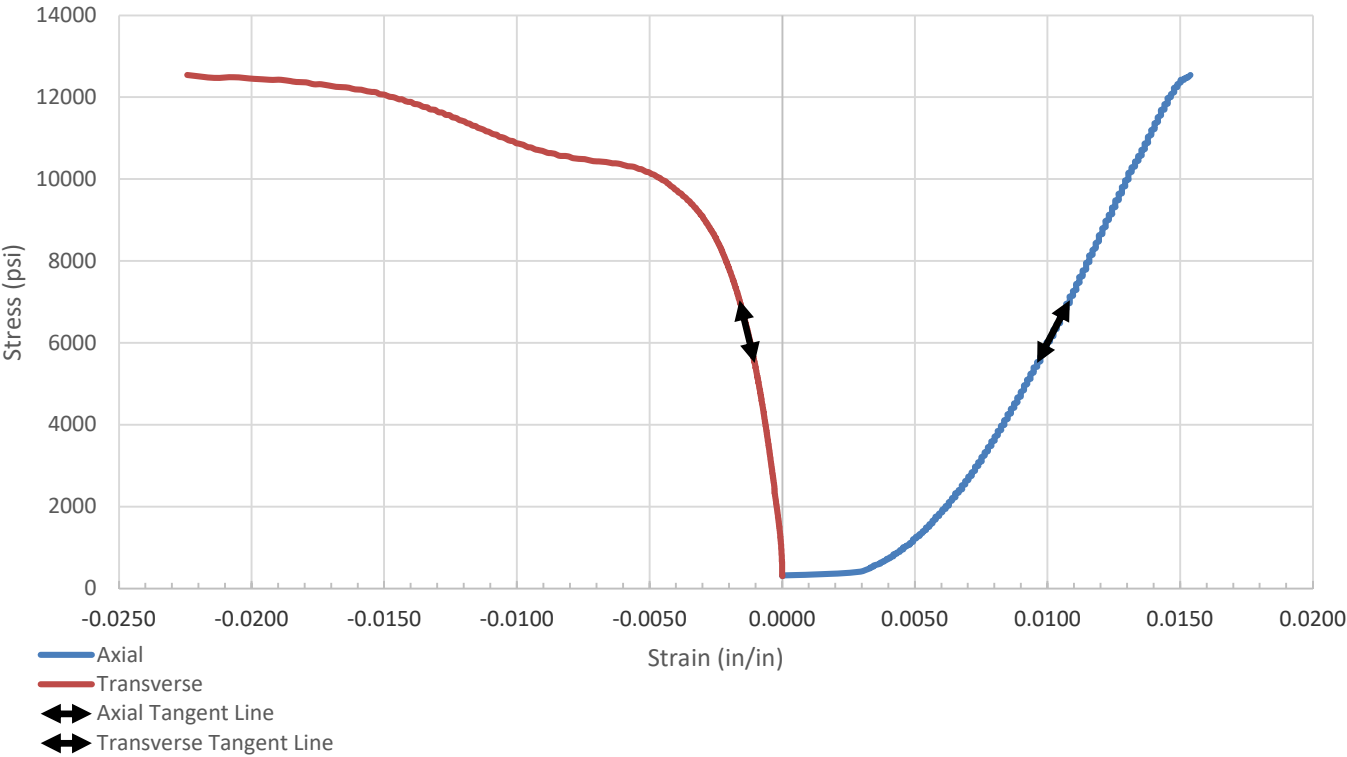
HNTB North Carolina PC
Attn: Spencer Franklin
343 E Six Forks Rd Ste 200
Raleigh, NC 27609

Project

SCDOT Bridge Package 19

Project No. 8623P180

ASTM D7012 Stress/ Strain Curve



SAMPLE LOCATION

Site:	SCDOT Bridge Package 19		
Description:	Granite		
Boring:	S-23-310-2	Depth (feet):	12.7-14.0

SPECIMEN INFORMATION

Sample No.:	R1	Mass (g):	528.04
Length (in.):	4.07	Diameter (in.):	1.96
L/D Ratio:	2.08	Density (pcf):	163.81

TEST RESULTS

Failure Load (lbs):	37852
Failure Strain (%):	1.83
Unconfined Compressive Strength (psi):	12,545
Elastic Modulus, E, (ksi):	1243
Poisson's Ratio, u:	0.463
Time of Failure (min):	01:52
Rate of Loading (psi/sec):	0.041
Moisture Content Post-break:	0.0013



Client	Project
HNTB North Carolina PC Attn: Spencer Franklin 343 E Six Forks Rd Ste 200 Raleigh, NC 27609	SCDOT Bridge Package 19
	Project No. 8623P180

Equipment:	TICCS ID:
Calipers	W-54522
Scale	B-71466
Dial Indicator	C-70608
Compression (spherically seated)	C-48999

Samples were prepared and tested in accordance with ASTM D4543 and D7012. Deviations, if any, are noted below:

Notes:

Per ASTM D4543, this specimen has not met the requirements for perpendicularity, by exceeding 0.250°.

Per ASTM D4543, this specimen has not met the requirements for flatness, by exceeding 0.001 inches.

Per ASTM D4543, this specimen has not met the requirements for parallelism, by exceeding 0.25°.

According to ASTM D7012 Section 8.2.1, this specimen, although not meeting all requirements of ASTM D4543 is acceptable for testing. However, the results reported may differ from results obtained from a test specimen that meets the requirements of D4543.

Appendix C – Supporting Documents

S-23-310 Bridge Replacement over Langston Tributary | Greenville County, SC
Terracon Project No. 8623P180 | SCDOT Project ID: P041162



Appendix C

Supporting Documents

3-Point Acceleration Design Response Spectrum By SCDOT
Rig Calibration Report – DR#554 (5 Pages)

Note: All exhibits are one page unless noted above.

3-Point Acceleration Design Response Spectrum

SCDOT v3.2 - 06/01/2023

Project ID:	P041162	Latitude:	34.8972
Route:	S-23-310-1	County:	23 - Greenville
Project:	BRO Langston Tributary		
		Longitude:	82.4002

Designer:	D. Sapkota - Support
Date:	10/15/2024

Design EQ	PGA	S _{DS}	S _{D1}	M _W	R	PGV	D ₅₋₉₅	T' _o
	g	g	g	-	km	inches/sec	sec	sec
FEE	0.01	0.02	0.00	6.63	213.43	0.18	43.94	0.17
SEE	0.02	0.04	0.01	5.66	103.17	0.39	21.83	0.13

Fundamental Period of Structure, T_0^*	Range of Interest		$V_{s,H}^*$	H	T_{NH}	
	sec				sec	
sec	0.5^*T_0	2.0^*T_0	ft/sec	ft	$(4^*H)/V_{s,H}^*$	$(6^*H)/V_{s,H}^*$
0.00	0.00	0.00	673.65	42.00	0.19	0.37
0.00	0.00	0.00	H = B-C Boundary			

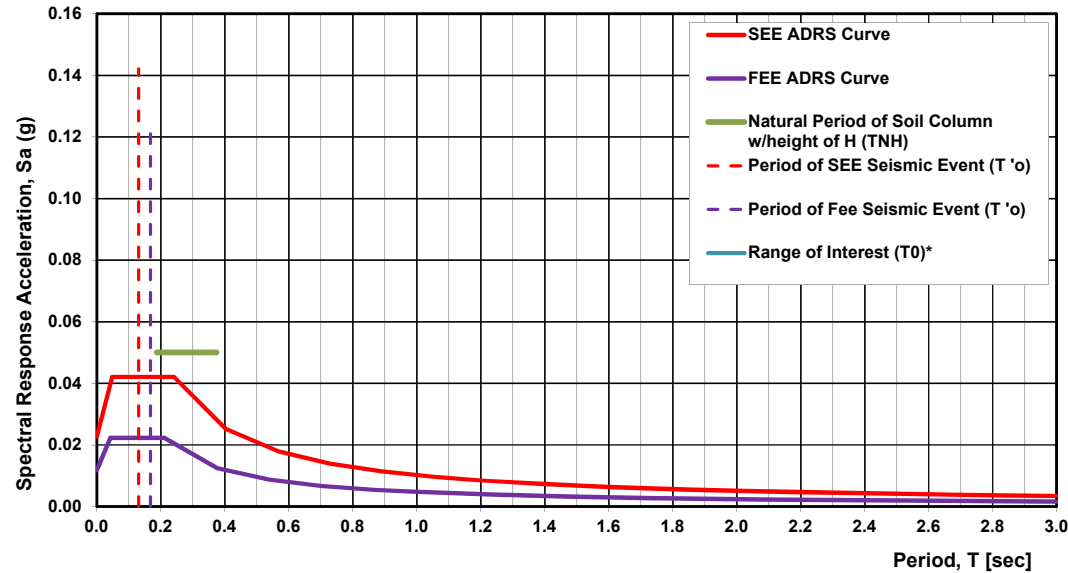
H = B-C Boundary

Damping:	5%
Geologic Condition:	Geologically Realistic (Q = 100)*
ADRS Location within Soil Column:	SCP
	At Ground Surface

South Carolina Piedmont

*Same Geologic Condition as used in SCENARIO_PC (2006)

SC Seismic ADRS Curve



FEE Data

T	S _a
0.00	0.012
0.01	0.013
0.01	0.015
0.02	0.017
0.03	0.019
0.04	0.020
0.04	0.022
0.06	0.022
0.07	0.022
0.08	0.022
0.10	0.022
0.11	0.022
0.13	0.022
0.14	0.022
0.15	0.022
0.17	0.022
0.18	0.022
0.20	0.022
0.21	0.022
0.38	0.013
0.54	0.009
0.70	0.007
0.87	0.005
1.03	0.005
1.20	0.004
1.36	0.003
1.52	0.003
1.69	0.003
1.85	0.003
2.02	0.002
2.18	0.002
2.34	0.002
2.51	0.002
2.67	0.002
2.84	0.002
3.00	0.002

SEE Data

T	S _a
0.00	0.023
0.01	0.026
0.02	0.029
0.02	0.032
0.03	0.036
0.04	0.039
0.05	0.042
0.06	0.042
0.08	0.042
0.10	0.042
0.11	0.042
0.13	0.042
0.14	0.042
0.16	0.042
0.18	0.042
0.19	0.042
0.21	0.042
0.23	0.042
0.24	0.042
0.40	0.025
0.57	0.018
0.73	0.014
0.89	0.011
1.05	0.010
1.21	0.008
1.38	0.007
1.54	0.007
1.70	0.006
1.86	0.005
2.03	0.005
2.19	0.005
2.35	0.004
2.51	0.004
2.68	0.004
2.84	0.004
3.00	0.003

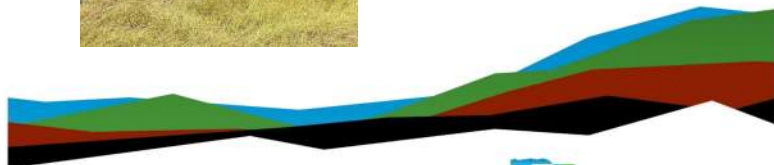
SPT Automatic Hammer Energy Measurement Report

Drill Rig Model: GeoProbe 3126

Drill Rig Serial Number: 3126TTS52010006

Asset Number: DR#554

August 21, 2023



Prepared for:

Terracon
Greenville-Spartanburg, South Carolina



July 19, 2023

Terracon
72 Pointe Circle
Greenville, South Carolina 29607

Attn: Maggie McKenney
E: m.mckenney@terracon.com

Re: SPT Automatic Hammer Energy Measurement Report
Rig Serial Number: 3126TTS52010006
Terracon Project Number: DYXX0500

Dear Ms. McKenney:

This report provides the Energy Transfer Ratio (ETR) for the Standard Penetration Testing (SPT) automatic hammer as summarized below:

Table 1: Hammer Efficiency Summary

Drill Rig Make/Model	Drill Rig Serial Number	Drill Rig Year	Asset Number	Energy Transfer Ratio (ETR)	Hammer Efficiency Correction (Ce)
GeoProbe 3126	3126TTS52010006	2021	GP#554	88.5% ± 4.2%	1.48

If you have any questions concerning this summary, or if we may be of further service, please contact us.

James P. Smith

James P. Smith
National Manager of Equipment & Training

Rob Kramer

Rob Kramer
Group Manager Geophysics

Attachments:

Exhibit A: PDA SPT Analyzer Results
Exhibit B: PDA Equipment Calibration

Facilities | Environmental | **Geotechnical** | Materials |

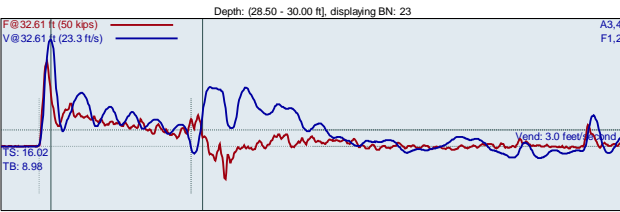
MEASUREMENT SUMMARY

ITEM	DESCRIPTION
Drill Rig Owner	Terracon Greenville-Spartanburg - Greenville, SC
Drill Rig Operator	Brett Burnett; Terracon Exploration Services
Testing Date	08/21/2023
Testing Location	Spartanburg, SC
Boring Identification	B-1
Hammer Type	140 pounds (automatic)
Boring Method	Hollow Stem Auger
Drill Rods	<ul style="list-style-type: none"> AWJ 1-3/4" outside diameter 3/16" wall thickness
Calibration Testing Equipment	<ul style="list-style-type: none"> 2-foot AWJ rod instrumented w/ two strain gauges and two accelerometers Model SPT Analyzer™ (PDA)
ASTM Methods Used	<p>ASTM D1586, Standard Test Method for Standard Penetration Test and Split-Barrel Sampling of Soils</p> <p>ASTM D4633-16, Standard Method for Energy Measurement for Dynamic Penetrometers</p>
SPT Calibration Personnel	Jim Smith, National Manager of Equipment and Training

Exhibit A

PDA SPT Analyzer Results

GP554-3126 28.5/30
JIM SMITH Interval start: 8/21/2023
TB-1
AR: 1.20 in/2 SP: 0.492 k/ft3
LE: 32.61 ft EM: 30000 ksi
WS: 16807.9 fts



F1 : [648AWJ1] 226.21 PDICAL (1) FF1
F2 : [648AWJ2] 225.58 PDICAL (1) FF1
A3 (PR): [K4483] 410.187 mv/6.4v/5000g (1) VF1
A4 (PR): [K10491] 421.907 mv/6.4v/5000g (1) VF1

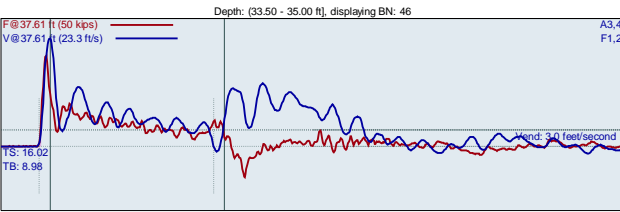
FMX: Maximum Force EFV: Maximum Energy
VMX: Maximum Velocity ETR: Energy Transfer Ratio - Rated
BPM: Blows/Minute

BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
1	6	40	19.4	1.9	294	84.1
2	6	39	19.2	51.9	292	83.4
3	6	25	16.9	52.7	274	78.2
4	6	28	17.9	52.4	273	77.9
5	6	32	19.6	52.6	294	83.9
6	6	27	17.3	53.1	268	76.5
7	8	38	19.0	52.7	289	82.5
8	8	39	19.6	52.4	305	87.2
9	8	36	19.2	52.7	290	82.8
10	8	28	18.2	52.5	292	83.4
11	8	38	19.0	53.0	293	83.8
12	8	35	19.4	52.6	282	80.4
13	8	36	19.1	52.9	299	85.3
14	8	34	19.8	52.8	307	87.7
15	11	34	19.5	52.7	307	87.6
16	11	33	19.5	52.9	299	85.6
17	11	36	19.4	52.7	308	88.1
18	11	37	18.5	52.8	320	91.4
19	11	32	19.6	52.9	301	86.1
20	11	39	18.7	52.9	301	85.9
21	11	26	17.5	52.8	277	79.1
22	11	30	19.1	52.6	306	87.4
23	11	33	19.5	52.7	298	85.1
24	11	35	19.9	52.4	303	86.5
25	11	36	19.4	53.1	313	89.6

Average	34	19.2	52.8	299	85.6
Std Dev	3	0.6	0.2	10	3.0
Maximum	39	19.9	53.1	320	91.4
Minimum	26	17.5	52.4	277	79.1
N-value: 19					

Sample Interval Time: 27.36 seconds.

GP554-3126 28.5/30
JIM SMITH Interval start: 8/21/2023
TB-1
AR: 1.20 in/2 SP: 0.492 k/ft3
LE: 37.61 ft EM: 30000 ksi
WS: 16807.9 fts

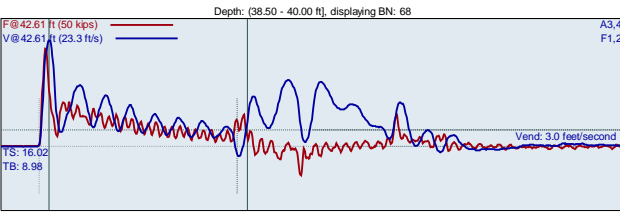


F1 : [648AWJ1] 226.21 PDICAL (1) FF1
F2 : [648AWJ2] 225.58 PDICAL (1) FF1
A3 (PR): [K4483] 410.187 mv/6.4v/5000g (1) VF1
A4 (PR): [K10491] 421.907 mv/6.4v/5000g (1) VF1

BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
26	5	38	19.1	1.9	302	86.4
27	5	35	18.9	52.0	301	86.1
28	5	29	18.8	52.0	299	85.5
29	5	35	19.2	52.7	299	85.5
30	5	37	19.4	52.5	297	84.8
31	8	37	19.5	52.4	307	87.7
32	8	26	16.4	52.7	282	80.5
33	8	34	19.5	52.4	307	87.6
34	8	40	19.1	52.2	307	87.6
35	8	37	19.4	52.6	299	85.5
36	8	40	20.6	52.4	321	91.7
37	8	41	19.6	52.8	308	87.9
38	8	40	19.8	52.7	313	89.5
39	10	34	20.2	52.2	323	92.2
40	10	32	19.4	52.8	297	84.9
41	10	36	19.8	52.6	311	88.8
42	10	37	19.7	52.5	317	90.7
43	10	35	20.0	52.6	324	92.6
44	10	38	19.5	52.7	308	88.1
45	10	34	20.1	52.4	322	92.0
46	10	35	19.7	52.4	322	92.0
47	10	37	19.9	52.6	314	89.7
48	10	37	19.8	52.7	332	94.8
Average		36	19.6	52.6	312	89.1
Std Dev		3	0.8	0.2	12	3.3
Maximum		41	20.6	52.8	332	94.8
Minimum		26	16.4	52.2	282	80.5
N-value: 18						

Sample Interval Time: 25.16 seconds.

GP554-3126 28.5-30
JIM SMITH Interval start: 8/21/2023
TB-1
AR: 1.20 in/2 SP: 0.492 k/ft3
LE: 42.61 ft EM: 30000 ksi
WS: 16807.9 ft/s



F1 : [648AWJ1] 226.21 PDICAL (1) FF1		A3 (PR): [K4483] 410.187 mm/6.4v/5000g (1) VF1				
F2 : [648AWJ2] 225.58 PDICAL (1) FF1		A4 (PR): [K10491] 421.907 mm/6.4v/5000g (1) VF1				
BL#	BC /6"	FMX kips	VMX ft/s	BPM bpm	EFV ft-lb	ETR %
49	5	34	19.6	1.9	307	87.6
50	5	34	19.3	52.0	301	86.1
51	5	27	16.5	52.7	278	79.4
52	5	33	19.9	52.5	310	88.6
53	5	29	17.7	52.7	288	82.2
54	8	29	18.6	52.5	295	84.2
55	8	23	15.6	52.9	287	82.0
56	8	34	20.1	52.6	323	92.2
57	8	28	18.1	52.8	295	84.3
58	8	38	18.8	53.1	312	89.1
59	8	35	19.2	52.6	329	94.0
60	8	36	19.3	52.9	327	93.3
61	8	40	19.7	52.8	323	92.4
62	9	35	18.8	53.0	320	91.3
63	9	37	19.1	52.7	320	91.3
64	9	35	19.9	52.9	327	93.4
65	9	29	18.8	52.7	314	89.7
66	9	35	19.7	53.0	342	97.8
67	9	36	19.9	52.8	331	94.5
68	9	38	19.3	52.8	335	95.8
69	9	36	19.9	52.5	325	92.9
70	9	39	19.5	52.9	329	94.0
Average		34	19.1	52.8	320	91.3
Std Dev		4	1.0	0.2	15	4.1
Maximum		40	20.1	53.1	342	97.8
Minimum		23	15.6	52.5	287	82.0
N-value: 17						

Sample Interval Time: 23.91 seconds.

Summary of SPT Test Results

Project: GP554-3126, Test Date: 8/21/2023				EFV: Maximum Energy				
FMX: Maximum Force				ETR: Energy Transfer Ratio - Rated				
VMX: Maximum Velocity								
BPM: Blows/Minute								
Test Length ft	Blows Applied /6"	N Value	N60 Value	Average FMX kips	Average VMX ft/s	Average BPM bpm	Average EFV ft-lb	Average ETR %
32.61	6-8-11	19	28	34	19.2	52.8	299	85.6
37.61	5-8-10	18	26	36	19.6	52.6	312	89.1
42.61	5-8-9	17	25	34	19.1	52.8	320	91.3
Overall Average Values:				35	19.3	52.7	310	88.5
Standard Deviation:				4	0.8	0.2	15	4.2
Overall Maximum Value:				41	20.6	53.1	342	97.8
Overall Minimum Value:				23	15.6	52.2	277	79.1



Exhibit B

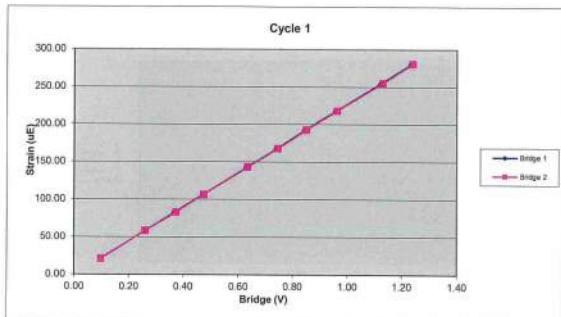
PDA Equipment Calibration



648AWJ		Cycle 1		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	799.99	21.12	0.10	0.10
3	2111.63	58.22	0.26	0.26
4	2997.39	82.70	0.37	0.37
5	3848.07	106.26	0.47	0.47
6	5131.83	143.07	0.63	0.63
7	6017.79	167.81	0.74	0.75
8	6872.07	192.74	0.85	0.85
9	7783.57	218.15	0.96	0.96
10	9136.93	255.02	1.12	1.13
11	10026.70	280.73	1.24	1.24

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8120.30	Force Calibration (lb/V)	8089.75
Offset	-4.24	Offset	-2.24
Correlation	0.999998	Correlation	0.999995
Strain Calibration (µE/V)	228.56	Strain Calibration (µE/V)	227.70
Offset	-1.57	Offset	-1.51
Correlation	0.999991	Correlation	0.999983

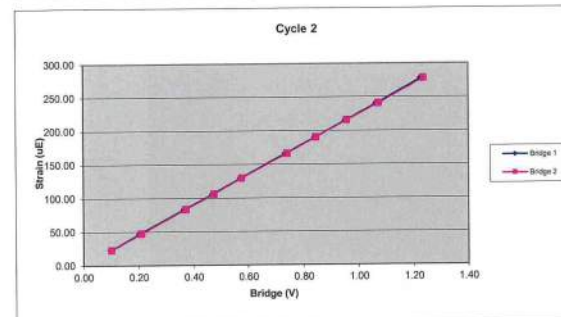
Force Strain Calibration	
EA (Kips)	35527.98
Offset	51.69
Correlation	0.999986



648AWJ		Cycle 2		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	805.54	22.23	0.10	0.10
3	1679.81	47.04	0.20	0.21
4	2989.11	83.03	0.37	0.37
5	3830.62	105.81	0.47	0.47
6	4658.00	129.50	0.57	0.58
7	5984.74	165.81	0.74	0.74
8	6848.87	189.76	0.84	0.84
9	7747.90	215.15	0.95	0.96
10	8674.21	240.08	1.07	1.07
11	9994.82	277.48	1.23	1.24

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8127.14	Force Calibration (lb/V)	8103.79
Offset	10.37	Offset	-14.59
Correlation	0.999997	Correlation	0.999997
Strain Calibration (µE/V)	225.29	Strain Calibration (µE/V)	224.64
Offset	0.36	Offset	-0.33
Correlation	0.999990	Correlation	0.999992

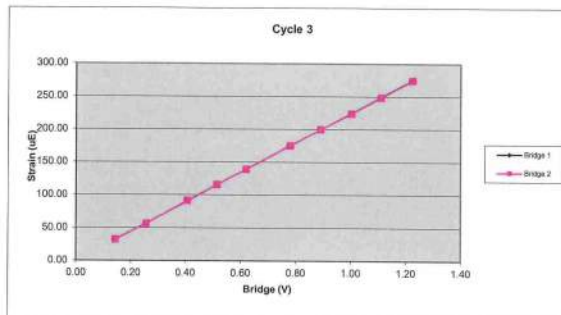
Force Strain Calibration	
EA (Kips)	36073.41
Offset	-2.66
Correlation	0.999993



648AWJ		Cycle 3		
Sample	Force (lb)	Strain (µE)	Bridge 1 (V)	Bridge 2 (V)
1	0.00	0.00	0.00	0.00
2	1153.24	31.90	0.14	0.14
3	2056.55	56.28	0.26	0.26
4	3310.19	91.18	0.41	0.41
5	4155.51	115.51	0.51	0.51
6	5035.81	139.16	0.62	0.62
7	6303.78	175.10	0.78	0.78
8	7221.91	199.87	0.89	0.89
9	8120.94	223.92	1.00	1.00
10	9001.15	246.68	1.11	1.11
11	9931.66	274.33	1.22	1.23

Bridge 1		Bridge 2	
Force Calibration (lb/V)	8132.32	Force Calibration (lb/V)	8118.57
Offset	-20.37	Offset	-15.36
Correlation	0.999998	Correlation	0.999997
Strain Calibration (µE/V)	224.79	Strain Calibration (µE/V)	224.41
Offset	-0.57	Offset	-0.43
Correlation	0.999984	Correlation	0.999985

Force Strain Calibration	
EA (Kips)	36175.62
Offset	0.42
Correlation	0.999984



Bridge Excitation (V) 5
Shunt Resistor (ohm) 60.4k

Calibration Factors		648AWJ	
Bridge 1 (µE/V)	226.21	Bridge 2 (µE/V)	225.58
EA Factor (Kips)	35925.67	Area (in ²)	1.20

Calibrated by: *Aht*
Calibrated Date: 3/3/2022

Pile Dynamics Inc
30725 Aurora Rd
Solon, OH 44139

Traceable to N.I.S.T.

Accelerometer Calibration Certificate

Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.
Calibration performed on 26Oct2021

Serial No: K4483 Temperature: 22.1 °C

Model: PR Humidity: 45%

Calibrated on: Channel 3 on 8G 5161 LE

PDA CALIBRATION FACTOR

410.2 mv/5000g
(62.0 μ v/g)
R²: 0.999973 [Chip programmed]

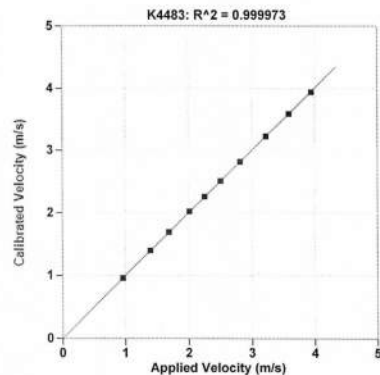
Operator: William Johnson

Signed

Ref Acc 1: 690961 Cal on: 27Jan2021
978 g's/volt

Ref Acc 2: 691321 Cal on: 09Feb2021
960 g's/volt

Reference accelerometer calibrations are traceable to the United States National Institute of Standards and Technology (NIST).



Date printed: 26Oct2021, version: 2020.30.170 9.57

Accelerometer Calibration Certificate

Pile Dynamics, Inc.



Calibrated by Pile Dynamics, Inc.
Calibration performed on 25Jan2022

Serial No: K10491 Temperature: 19.3 °C

Model: PR Humidity: 30%

Calibrated on: Channel 3 on 8G 5161 LE

PDA CALIBRATION FACTOR

421.9 mv/5000g
(84.4 μ v/g)
R²: 0.999915 [Chip programmed]

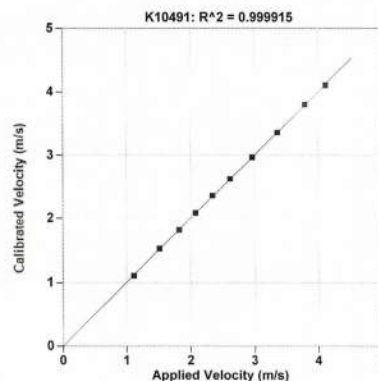
Operator: William Johnson

Signed

Ref Acc 1: 691321 Cal on: 09Feb2021
960 g's/volt

Ref Acc 2: 690961 Cal on: 27Jan2021
978 g's/volt

Reference accelerometer calibrations are traceable to the United States National Institute of Standards and Technology (NIST).



Date printed: 25Jan2022, version: 2020.30.170 9.05