



Revised Geotechnical Subsurface Exploration and Laboratory Testing Data Report

***2016-1A Emergency Bridge Replacement Package
SCDOT PIN P031750
S-45 (Lester Road) Bridge over Little Pee Dee Swamp
Dillon County, South Carolina
F&R Project No. 65U-0177***

Prepared For:



***South Carolina Department of Transportation
Design Build Section
955 Park Street
Columbia, South Carolina 29201***

Prepared By:

***Froehling & Robertson, Inc.
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December 15, 2016



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December 15, 2016

Mr. Trapp Harris, PE
South Carolina Department of Transportation
Design Build Section
955 Park Street
Columbia, South Carolina 29201

Subject: Revised Geotechnical Subsurface Exploration and Laboratory Testing Data Report
2016-1A Emergency Bridge Replacement Package
SCDOT PIN P031750, S-45 (Lester Road) Bridge over Little Pee Dee Swamp
Dillon County, South Carolina
F&R Project No. 65U-0177

Dear Mr. Harris:

The purpose of this data report is to present the results of the subsurface exploration program and laboratory testing undertaken by Froehling & Robertson, Inc. (F&R) in connection with the 2016-1A Emergency Bridge Package which includes the S-45 (Lester Road) Bridge over Little Pee Dee Swamp in Dillon County, South Carolina. Our services were performed in general accordance with your work order Number FR#10-18-P031750 emailed to F&R on November 22, 2016, and as authorized by your office per our On-Call Contract with SCDOT (Contract Number S-147-14). The attached report presents our understanding of the project, reviews our exploration procedures, describes existing site and general subsurface conditions, and presents the results of our laboratory tests.



We have enjoyed working with you on this project. Please contact us if you have any questions regarding this report or if we may be of further service.

Sincerely,
FROEHLING & ROBERTSON, INC.

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Geotechnical Engineer
SC PE License No. 33654



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Emergency Bridge Package.docx





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1.0 PURPOSE & SCOPE OF SERVICES

The purpose of the subsurface exploration and soil laboratory testing was to obtain preliminary subsurface condition information for use as geotechnical baseline information in connection with the proposed bridge replacement, a design build project.

F&R's scope of services included the following:

- Coordination of underground utility clearance with SC 811;
- Review of readily available geologic and subsurface information relative to the project site;
- Completion of two soil test borings to a depth of approximately 100 feet below the existing ground surface;
- Preparation of typed boring logs presented on The SCDOT soil test boring log template along with raw electronic data files in gINT format;
- Performing laboratory testing including up to eight natural moisture content tests, up to eight amount finer than No. 200 Sieve, and up to four Atterberg Limit tests on selected soil samples;
- Completion of two cone penetration tests to a depth of approximately 50 feet below the existing ground surface;
- Preparation of graphically illustrated CPT sounding logs and raw electronic CPT data files. We have provided these electronic data files in dot DAT, Comma-Separated Values (.CSV) and gINT (.GPJ) formats;
- Completion of one geophysical test at the bridge site using a Multi-Channel Analysis of Surface Waves (MASW) method;
- Completion of field surveys of subsurface test location to include stations, offsets, GPS coordinates in horizontal state plane coordinates (northings and eastings), and ground surface elevations at each test hole location;
- Preparation of this geotechnical data report by professional engineers.

F&R's geotechnical services did not include development of quantity estimates, preparation of plans and specifications, or the identification and evaluation of wetlands or other environmental aspects of the project site.



2.0 PROJECT INFORMATION

2.1 Site and Project Description

The project site is on South Carolina Highway S-45 (Lester Road) at the middle of the three bridges over Little Pee Dee Swamp in Dillon County, South Carolina. Highway S-45 is an asphalt paved two-lane highway. The area around the bridge and roadway is generally swampy, wooded or partly covered with brush. The ground surface elevation on the paved area is at approximately EL 79 and the elevations around the river banks and the immediately adjacent areas range from approximately El 70 to El 73. A site vicinity map is shown as Figure No. 1 and included in Appendix 1 of this report.

As a result of recent storm events, damage to portions or all of the bridge has occurred and therefore replacement of the existing bridge is planned. For this purpose, subsurface exploration at the bridge site is required.

F&R performed our subsurface exploration in accordance with the scope of services as described in your work order request to F&R which you submitted to us on November 22, 2016. F&R obtained the site location information from the Emergency Bridge Package 7 dot KMZ file dated November 21, 2016, which we received from your office on the same date. The project development information was provided to us through our communication with you and included in the work order request referenced above. Additional site details were obtained through our site visit.

2.2 Location Control

The SPT borings, CPT soundings and geophysical testing locations were staked in the field by F&R personnel at locations close to the existing bridge. After completion of the subsurface explorations our licensed surveying subcontractor, Chao and Associates, Inc., of Columbia, South Carolina obtained the station, offset, GPS coordinates (latitude and longitude), horizontal state plane coordinates (northings and eastings), and ground surface elevations at each test hole location. Surveying was performed in accordance with the rules and regulations governing the practice of surveying in the State of South Carolina. Horizontal datum was referenced to SCSPCS and Vertical datum was referenced to NGVD88. These locations and elevations should be considered no more accurate than the methods and plans used to obtain them.



3.0 SUBSURFACE EXPLORATION PROCEDURES

3.1 Soil Test Borings

The soil test borings were conducted by a joint effort between personnel from our firm and our drilling subcontractor, William Walker Environmental Services LLC, of West Columbia, South Carolina. The drilling was performed from November 22nd through 30th, 2016. The Standard Penetration Test (SPT) was performed at the boring locations in general accordance with ASTM D1586.

The drill rigs used for this project were an ATV-mounted CME-550X equipped with an automatic hammer and a truck-mounted CME-45B equipped with a safety hammer. The test holes were advanced using the mud rotary drilling technique.

The subsurface exploration program included two Standard Penetration Test (SPT) borings, each located as close as possible to opposite ends of the existing bridge. The borings are designated as Soil Test Borings STB-101 and STB-102. The SPT tests were performed continuously from the existing ground surface to a depth of 10 feet and at approximate 5-foot intervals thereafter until termination at a depth of approximately 100 feet below the existing ground surface. Approximate boring locations are identified on Figure No. 2 - Location Plan included in Appendix 1 of this report. Photographic documentation of the drill rigs in operation at the locations of STB-101 and STB-102 are also included in Appendix 1 and presented as Figure No. 3 and 4, respectively.

Soil samples were obtained with a standard 2" O.D. and 30" long split-spoon sampler with each SPT being driven with a 140-lb automatic hammer falling 30 inches. The number of blows required to drive the sampler each 6-inch increment of penetration was recorded and are shown on the boring logs. The first six-inch increment is used to seat the sampler with the sum of the second and third penetration increments being termed the SPT N-value. A representative portion of each disturbed split-spoon sample was collected with each SPT, placed in a bag, and returned to our laboratory for review.

The recovered split-spoon samples were visually classified by F&R engineers in general accordance with the ASTM D2488. The boring logs provided in Appendix II show the subsurface conditions encountered on the dates and at the approximate locations indicated. Groundwater observations at the time of drilling and after 24 hours are recorded on the boring logs.



By the nature of the work performed, the drilling activities result in disturbances to the site. The completed boreholes performed were backfilled with on-site soils. The borehole backfill may subside at some time following our work. F&R assumes no responsibility for borehole subsidence after completion of the field exploration and departing the site. For continued safety, the boreholes should be occasionally observed by others with any needed additional backfilling then being performed. The test boring logs are included with this report and presented in Appendix II.

3.2 Cone Penetration Testing

The Cone Penetration Test (CPT) soundings conducted for our subsurface exploration were performed by our sub-contractor Palmetto Insitu, LLC, of Charleston, South Carolina, on November 22, 2016. The two CPTs were performed close to each of the existing bridge abutments in general accordance with ASTM D5778. The CPTs are designated as CPT-101 and CPT-102 and are identified on Figure No. 2 - Location Plan included in Appendix 1 of this report. Cone Penetration Tests CPT-101 and CPT-102 encountered refusal at depths of 37 feet and 42 feet, respectively.

The equipment used for the exploration includes an electronic 15 cm² Vertek seismic cone, hydraulically advanced into the soil using a Vertek S4 Scorpion CPT rig capable of 20 tons of thrust. The collected raw data was processed by Palmetto Insitu, LLC using Bentley's gINT V8i SS2 software (version 08.30.04.206) and Dataforensics, RapidCPT software (version 4.2.2.0). The legend used for the SBT correlations is based on Robertson and Campanella: 1990 and is included with the CPT results provided in Appendix III. An electronic file (in .CSV file format) containing the CPT results is being submitted under separate cover. Photographic documentation of the CPT rigs in operation at the locations of CPT-101 and CPT-102 are included in Appendix 1 and presented as Figure No. 5 and 6, respectively.

3.3 Geophysical Testing

A Refraction Microtremor (ReMi) survey was performed at one location (array) longitudinal to the road and just to the north side of the bridge. The ReMi survey was conducted to provide estimated measurements of the soil shear wave velocity in the upper 100 feet. The dispersive characteristic of Rayleigh waves when traveling through a layered medium is measured from the surface, which makes the method nondestructive and nonintrusive. A seismic source (ambient "noise") is applied at the ground surface where vertical transducers record the propagation of surface waves. By analyzing the phase information for each frequency contained in the wave train, the Rayleigh and shear wave velocity can be determined. The data was processed using



SeisOpt® ReMi™ software to reveal a one-dimensional average shear-wave (S-wave) velocity structure for the array. The survey was performed to provide the average shear wave velocity to a depth of 100 feet used to determine the seismic Site Classification in accordance with Chapter 16 of the 2015 International Building Code (IBC). The result of the geophysical test is included in Appendix IV of this report.

4.0 LABORATORY TESTING

4.1 Laboratory Testing

Laboratory testing consisted of ten natural moisture content test (ASTM D2216), ten amount finer than No. 200 Sieve tests (ASTM D1140), and four Atterberg Limit tests (ASTM D4318) on several samples obtained from the borings.

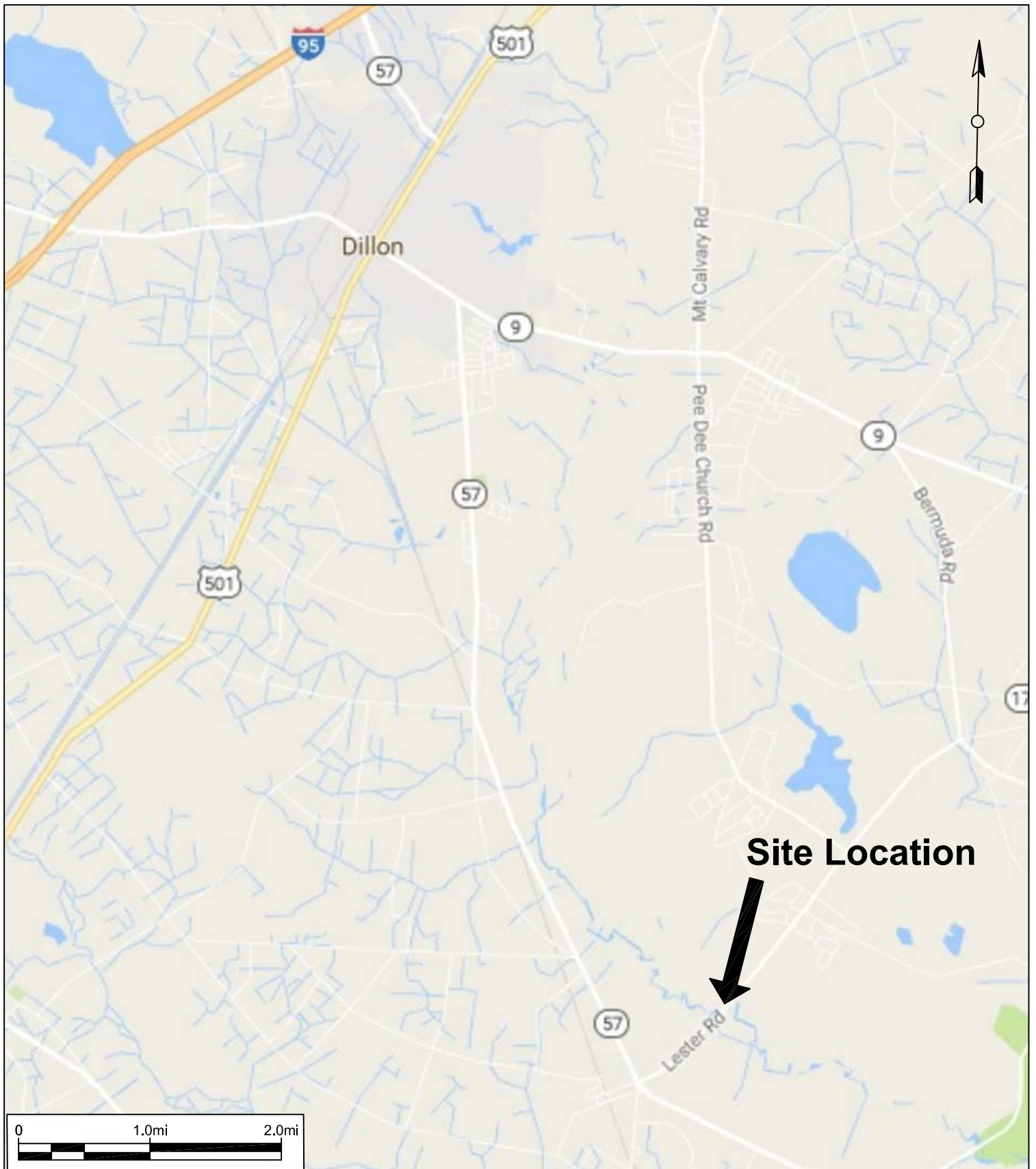
Laboratory test results are included in Appendix V of this report revision.

5.0 LIMITATIONS

This report has been prepared for the exclusive use of South Carolina Department of Transportation – Design Build Section or their agent, for specific application to the S-45 (Lester Road) Bridge over Little Pee Dee Swamp project, in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. Our investigation is based on site location information furnished to us; and generally accepted geotechnical engineering practice. The subsurface investigation logs included herein, do not reflect variations in subsurface conditions which could exist intermediate of the boring locations or in unexplored areas of the site. Should such variations become apparent during construction, it will be necessary to perform additional subsurface exploration based upon on-site observations of the conditions.



APPENDIX I



FROEHLING & ROBERTSON, INC.
GEOTECHNICAL • ENGINEERS • MATERIALS

DATE: 12/7/2016

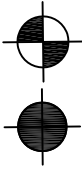
CLIENT: SCDOT

PROJECT NO.: 65U-0177

Site Vicinity Map
Emergency Bridge Replacement - S-45 (Lester Rd)
Dillon County, South Carolina

FIG NO. 1

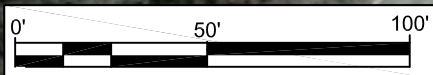
Drawing Legend:



SPT Boring



CPT Boring



FROEHLING & ROBERTSON, INC.
GEOTECHNICAL • ENGINEERS • MATERIALS

DATE: 12/7/2016

CLIENT: SCDOT

PROJECT NO.: 65U-0177

Boring Location Map
Emergency Bridge Replacement - S-45 (Lester Rd)
Dillon County, South Carolina

FIG NO. 2



Figure No. 3: Photograph of Soil Test Boring STB-101 Being Drilled



Figure No. 4: Photograph of Soil Test Boring STB-102 Being Drilled



Figure No. 5: Photograph of Cone Penetration Test CPT-101 Being Performed



Figure No. 6: Photograph of Cone Penetration Test CPT-102 Being Performed



APPENDIX II



KEY TO SOIL CLASSIFICATION
Correlation of Penetration Resistance with
Relative Density and Consistency

<u>Sands and Gravels</u>		<u>Silts and Clays</u>	
No. of <u>Blows, N</u>	Relative <u>Density</u>	No. of <u>Blows, N</u>	<u>Consistency</u>
0 - 4	Very loose	0 - 2	Very soft
5 - 10	Loose	3 - 4	Soft
11 - 30	Medium dense	5 - 8	Firm
31 - 50	Dense	9 - 15	Stiff
Over 50	Very dense	16 - 30	Very stiff
		31 - 50	Hard
		Over 50	Very hard

Particle Size Identification

(Unified Classification System)

Boulders:	Diameter exceeds 12-in. (300-mm)
Cobbles:	3-in. (75-mm) to 12-in. (300-mm) diameter
Gravel:	<u>Coarse</u> - ¾-in. (19-mm) to 3 in. (75-mm) diameter <u>Fine</u> - No. 4 (4.75-mm) sieve to ¾-in. (19-mm) diameter
Sand:	<u>Coarse</u> – No. 10 (2.0-mm) to No. 4 (4.76 mm) sieve <u>Medium</u> – No. 40 (0.425-mm) to No. 10 (2.0-mm) sieve <u>Fine</u> - No. 200 (0.075-mm) to No. 40 (0.425-mm) sieve
Silt and Clay:	Less than No. 200 (0.075-mm) sieve

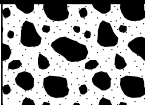




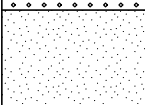
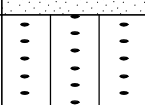
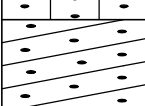
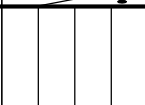
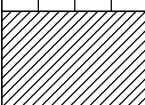
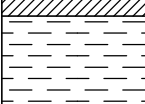
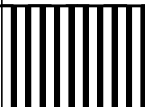

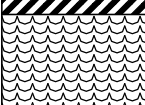
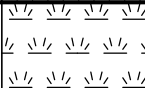
Modifiers

The modifiers provide our estimate of the amount of silt, clay or sand size particles in the soil sample.

<u>Approximate Content</u>	<u>Modifiers</u>
≤ 5%:	Trace
5 to 10%:	Few
15 to 25%:	Little
30 to 45%:	Some
50 to 100%	Mostly

<u>Field Moisture Description</u>	
Dry	Absence of moisture, dusty, dry to touch
Moist	Damp but no visible water
Wet	Visible free water, usually soil is below water table

SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS
			GRAPH	LETTER	
COARSE GRAINED SOILS MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	GRAVEL AND GRAVELLY SOILS MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	CLEAN GRAVELS (LITTLE OR NO FINES)		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
				GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		GRAVELS WITH FINES (APPRECIABLE AMOUNT OF FINES)		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
				GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
	SAND AND SANDY SOILS MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	CLEAN SANDS (LITTLE OR NO FINES)		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
				SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
		SANDS WITH FINES (APPRECIABLE AMOUNT OF FINES)		SM	SILTY SANDS, SAND - SILT MIXTURES
				SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS LIQUID LIMIT LESS THAN 50			ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
	SILTS AND CLAYS LIQUID LIMIT GREATER THAN 50			MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				CH	INORGANIC CLAYS OF HIGH PLASTICITY
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HIGHLY ORGANIC SOILS				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

SCDOT Soil Test Boring Log

File No.:	727.615	Project No. (PIN):	23546	County:	Beaufort/Jasper	Eng./Geo.:	A. Bore
Site Description:	RBO New River					Route:	SC 170/46
Boring No.:	B-722	Boring Location:	722+00	Offset:	5 ft LT	Alignment:	Mainline
Elev.:	1,500 ft	Latitude:	34.3750	Longitude:	81.0944	Date Started:	07/15/03
Total Depth:	45 ft	Soil Depth:	39 ft	Core Depth:	6 ft	Date Completed:	07/16/03
Bore Hole Diameter (in):	4.5	Sampler Configuration		Liner required:	Y N	Liner used:	Y N
Drill Machine:	CME-750	Drill Method:	Wash Rotary	Hammer Type:	Automatic	Energy Ratio:	100%
Core Size:	NQ Wireline	Driller:	I. Core	Groundwater:	TOB 7.5 ft	24 hr	15 ft

Depth (feet)	Elevation (ft msl)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (feet)	Sample Type / No.	1 st	2 nd	3 rd	SPT N-Value	• - SPT N-Value (blows / foot) PL MC LL x-----o-----x ▲ - % fines									
										1	2	3	4	5	6	7	8	9	10
		Soil Description a . b . c . d . e . f . g h . i . j . Munsell . LL PL . PI . NMC . %200 Munsell = Munsell Color Chart Designation LL = Liquid Limit PL = Plastic Limit PI = Plasticity Index NMC = Natural Moisture Content %200 = Percent Passing #200 Sieve																	
		Rock Description (as required) Lithologic description: rock type, color, texture, grain size, foliation, weathering and strength with k . l . m . n . o . p . q r . Munsell . RQD . %REC RMR Munsell = Munsell Color Chart Designation RQD = Rock Quality Designation %REC = Percent Recovery RMR = Rock Mass Rating																	

Figure 6-10, SCDOT Soil Test Boring Log

SCDOT Soil Test Boring Log Descriptors

a

Relative Density / Consistency Terms

Relative Density¹

Consistency²

Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Unconfined Compression Strength (q _u) (tsf)	SPT Blow Count
Very Loose	0 to 15%	< 4	Very Soft	<0.25	<2
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	>51	Very Stiff	2.01 to 4.00	16 to 30
			Hard	>4.01	> 31

b

Moisture Condition

Descriptive Term

Criteria

Dry	Absence of moisture, dusty, dry to the touch
Moist	Damp but no visible water
Wet	Visible free water, usually in coarse-grained soils below the water table

c

Color

Describe the sample color while sample is still moist, using Munsell color chart.

d

Angularity¹

Descriptive Term

Criteria

Angular	Particles have sharp edges and relatively plane sides with unpolished surfaces
Subangular	Particles are similar to angular description but have rounded edges
Subrounded	Particles have nearly plane sides but have well-rounded corners and edges
Rounded	Particles have smoothly curved sides and no edges

e

HCl Reaction³

Descriptive Term

Criteria

None Reactive	No visible reaction
Weakly Reactive	Some reaction, with bubbles forming slowly
Strongly Reactive	Violent reaction, with bubbles forming immediately

f

Cementation³

Descriptive Term

Criteria

Weakly Cemented	Crumbles or breaks with handling or little finger pressure
Moderately Cemented	Crumbles or breaks with considerable finger pressure
Strongly Cemented	Will not crumble or break with finger pressure

g

Particle-Size Range¹

Gravel

Sand

	mm	Sieve size		mm	Sieve size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	¾ inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4.00 to 4.76	#10 to #4

h

Primary Soil Type^{1,2}

The primary soil type will be shown in all capital letters

i

USCS Soil Designation

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

j

AASHTO Soil Designation

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

¹Applies to coarse-grained soils (major portion retained on No. 200 sieve)

²Applies to fine-grained soils (major portion passing No. 200 sieve)

³Use as required

Figure 6-11, SCDOT Soil Test Boring Log Descriptors - Soil

SCDOT Soil Test Log

Project ID:	P031750	County:	Dillon County	Boring No.:	STB-101
Site Description:	2016-1A Emergency Bridge Package			Route:	S-45
Eng./Geo.:	B. Azumah	Boring Location:	46+48.87	Offset:	12.0 ft LT
Elev.:	78.5 ft	Latitude:	34.3329333	Longitude:	-79.3243083
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	N/A ft
Date Started:	11/22/2016			Date Completed:	11/28/2016
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)	Drill Machine:	CME-45B	Drill Method:	RW
Hammer Type:	Safety	Energy Ratio:	83.3%	Core Size:	N/A
Driller:	WWES, LLC	Groundwater:	TOB	6 ft	24HR
					7.5 ft

Elevation (ft)	Depth (ft)	MATERIAL DESCRIPTION	Graphic Log	Sample Depth (ft)	Sample No./Type	1st 6"	2nd/3rd 6"	4th 6"	N Value	● SPT N VALUE ●	PL X MC O LL X	▲ FINES CONTENT (%)
	0.0											
	0.5	6 inches topsoil		0.0	SS-1	7	11/15	8	26			
	2.0	FILL , medium dense, moist, grayish brown (5Y 4/3), fine to medium grained, SILTY SAND (SM/A-2-4) SS-1: NMC=9.8%, %200=17		2.0	SS-2	4	4/5	5	9			
	6.0	Loose to very loose, moist, grayish brown (5Y 4/3), fine to medium grained, SILTY SAND (SM/A-2-4) , contains lean clay pockets SS-3: NMC=15.3%, %200=14		6.0	SS-3	3	2/1	1	3			
	8.0	Changes to wet		8.0	SS-4	3	3/2	2	5			
	68.5	Loose, wet, brownish gray (5YR 5/2), fine to coarse grained, POORLY GRADED SAND WITH SILT (SP-SM/A-1-a)			SS-5	2	2/2	2	4			
	13.0	ALLUVIUM , very loose, wet, dark brown (5Y 4/3), fine to medium grained, CLAYEY SAND (SC/A-2-7) , contains root fragments and organics		13.0	SS-6	2	2/5	5	7			
	63.5	Very loose, wet, brown (5Y 4/3), fine to coarse grained, POORLY GRADED SAND with SILT (SP-SM/A-1-a) , contains organics										
	18.0	SS-6: NMC=38.2%, %200=6		18.0	SS-7	5	10/12	13	22			
	58.5	PEE DEE FORMATION , medium dense, wet, brownish gray (5YR 5/2), fine to medium grained, POORLY GRADED SAND WITH SILT (SP-SM/A-1-a) , trace gravel										
	23.0	Dense, wet, brownish gray (5YR 5/2), fine to medium grained, POORLY GRADED SAND WITH SILT (SP-SM/A-1-a) , trace gravel		23.0	SS-8	2	20/22	23	42			
	53.5											
	48.5				SS-9	13	15/17	14	32			
				33.0	SS-10	10	15/17	20	32			

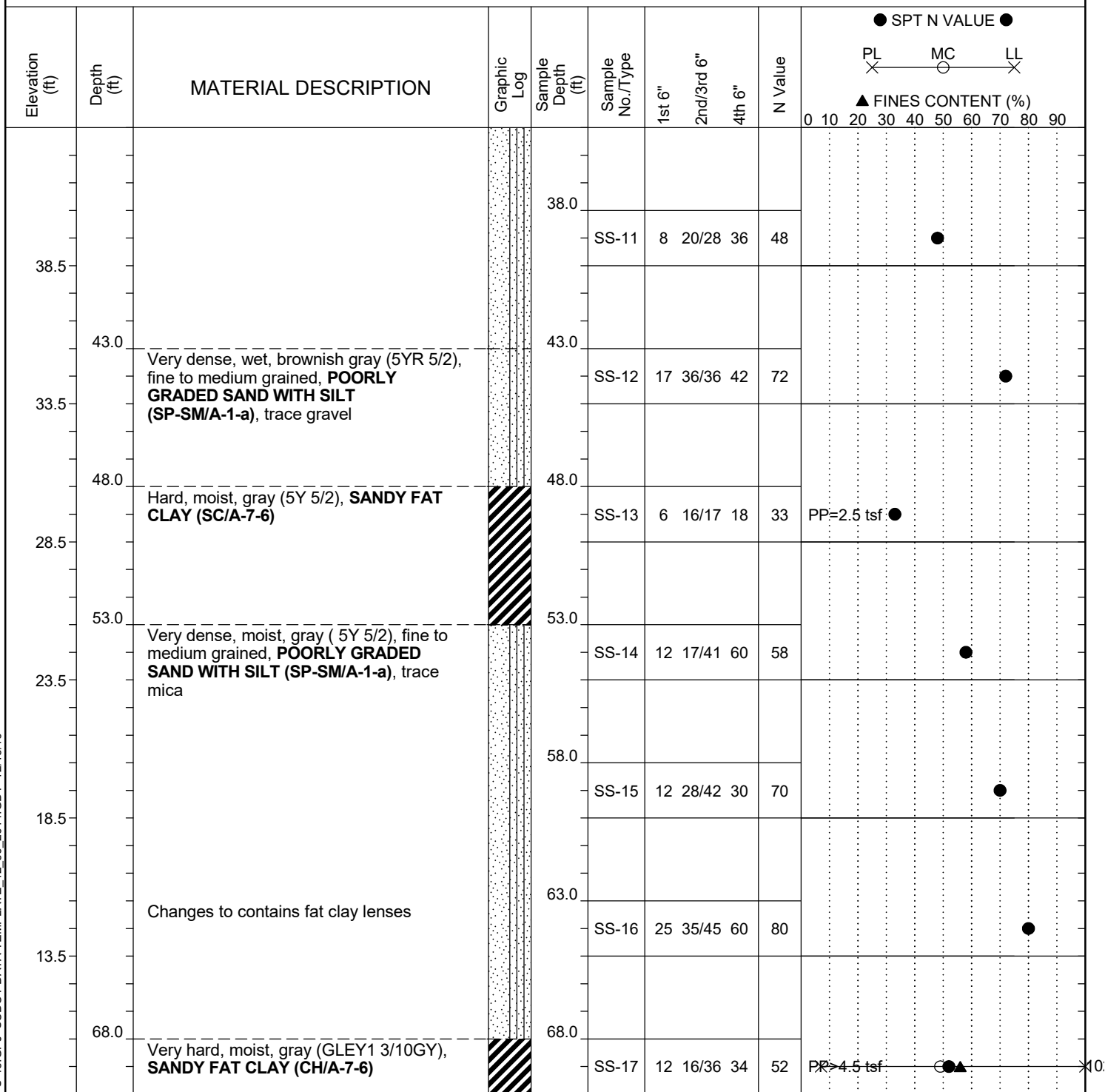
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SCDOT Soil Test Log

Project ID:	P031750	County:	Dillon County	Boring No.:	STB-101
Site Description:	2016-1A Emergency Bridge Package			Route:	S-45
Eng./Geo.:	B. Azumah	Boring Location:	46+48.87	Offset:	12.0 ft LT
Elev.:	78.5 ft	Latitude:	34.3329333	Longitude:	-79.3243083
Date Started:	11/22/2016				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	N/A ft
Date Completed:	11/28/2016				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)	Drill Machine:	CME-45B	Drill Method:	RW
Hammer Type:	Safety	Energy Ratio:	83.3%		
Core Size:	N/A	Driller:	WWES, LLC	Groundwater:	TOB 6 ft
24HR	7.5 ft				



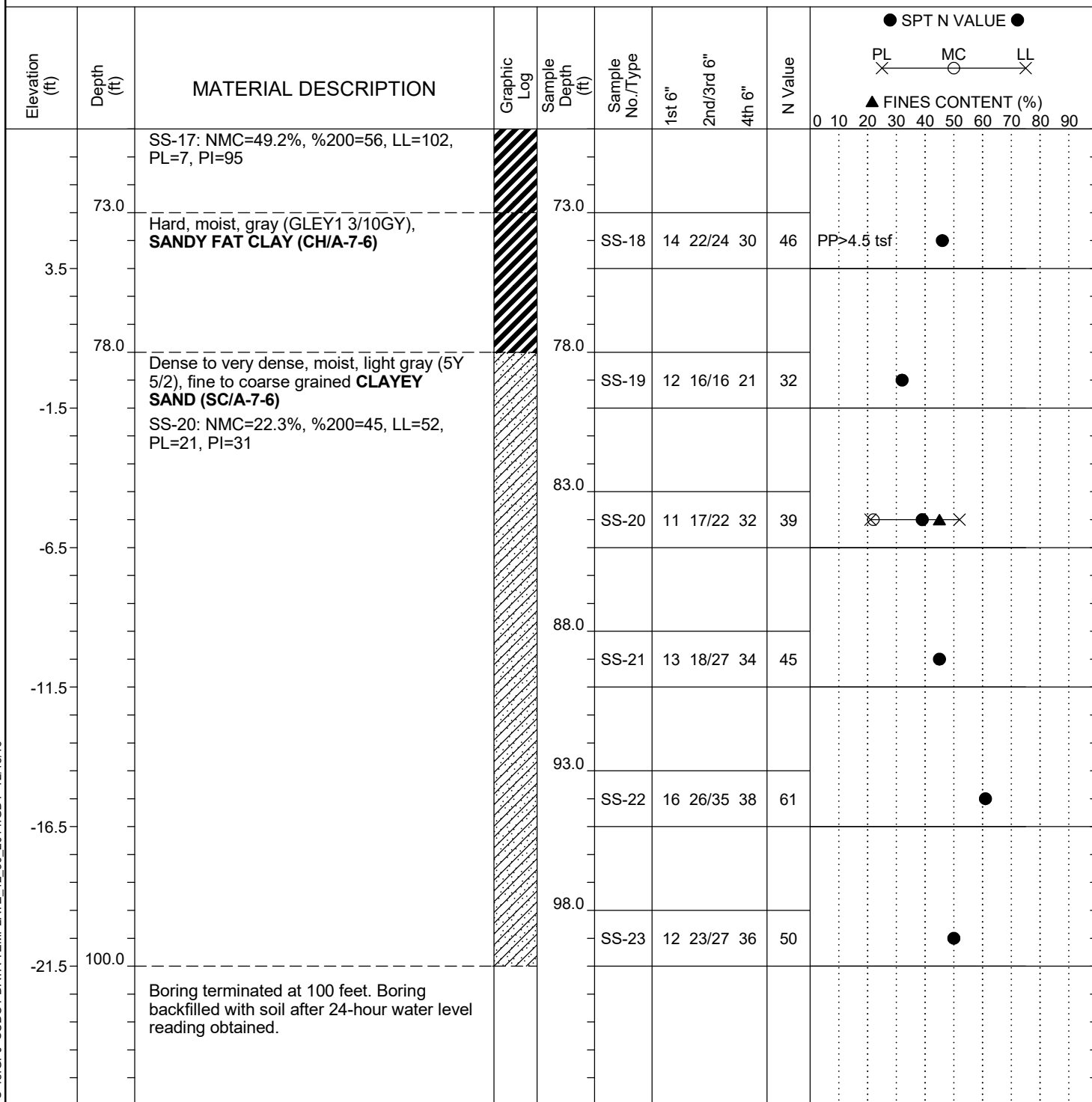
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SCDOT Soil Test Log

Project ID:	P031750	County:	Dillon County	Boring No.:	STB-101
Site Description:	2016-1A Emergency Bridge Package			Route:	S-45
Eng./Geo.:	B. Azumah	Boring Location:	46+48.87	Offset:	12.0 ft LT
Elev.:	78.5 ft	Latitude:	34.3329333	Longitude:	-79.3243083
Date Started:	11/22/2016				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	N/A ft
Date Completed:	11/28/2016				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)	Drill Machine:	CME-45B	Drill Method:	RW
Hammer Type:	Safety	Energy Ratio:	83.3%	Core Size:	N/A
Driller:	WWES, LLC	Groundwater:	TOB	6 ft	24HR
					7.5 ft

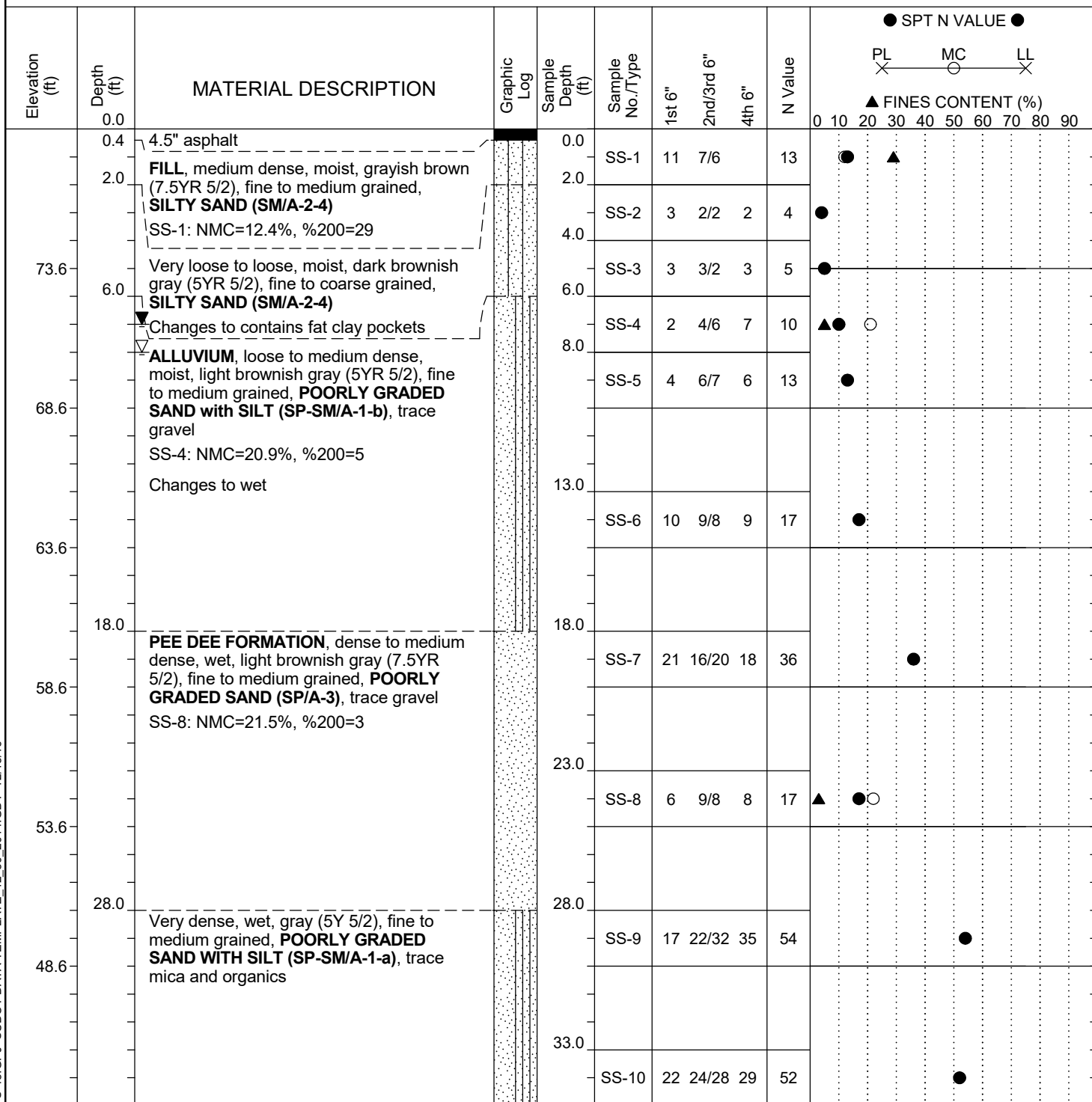


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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SCDOT Soil Test Log

Project ID:	P031750	County:	Dillon County	Boring No.:	STB-102
Site Description:	2016-1A Emergency Bridge Package			Route:	S-45
Eng./Geo.:	B. Azumah	Boring Location:	45+31.72	Offset:	5.5 ft RT
Elev.:	78.6 ft	Latitude:	34.3326528	Longitude:	-79.3245083
Date Started:	11/28/2016				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	N/A ft
Date Completed:	11/30/2016				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	CME-550X	Drill Method:	RW	Hammer Type:	Automatic
Energy Ratio:	85.5%				
Core Size:	N/A	Driller:	F&R, Inc.	Groundwater:	TOB 8 ft
24HR	7 ft				



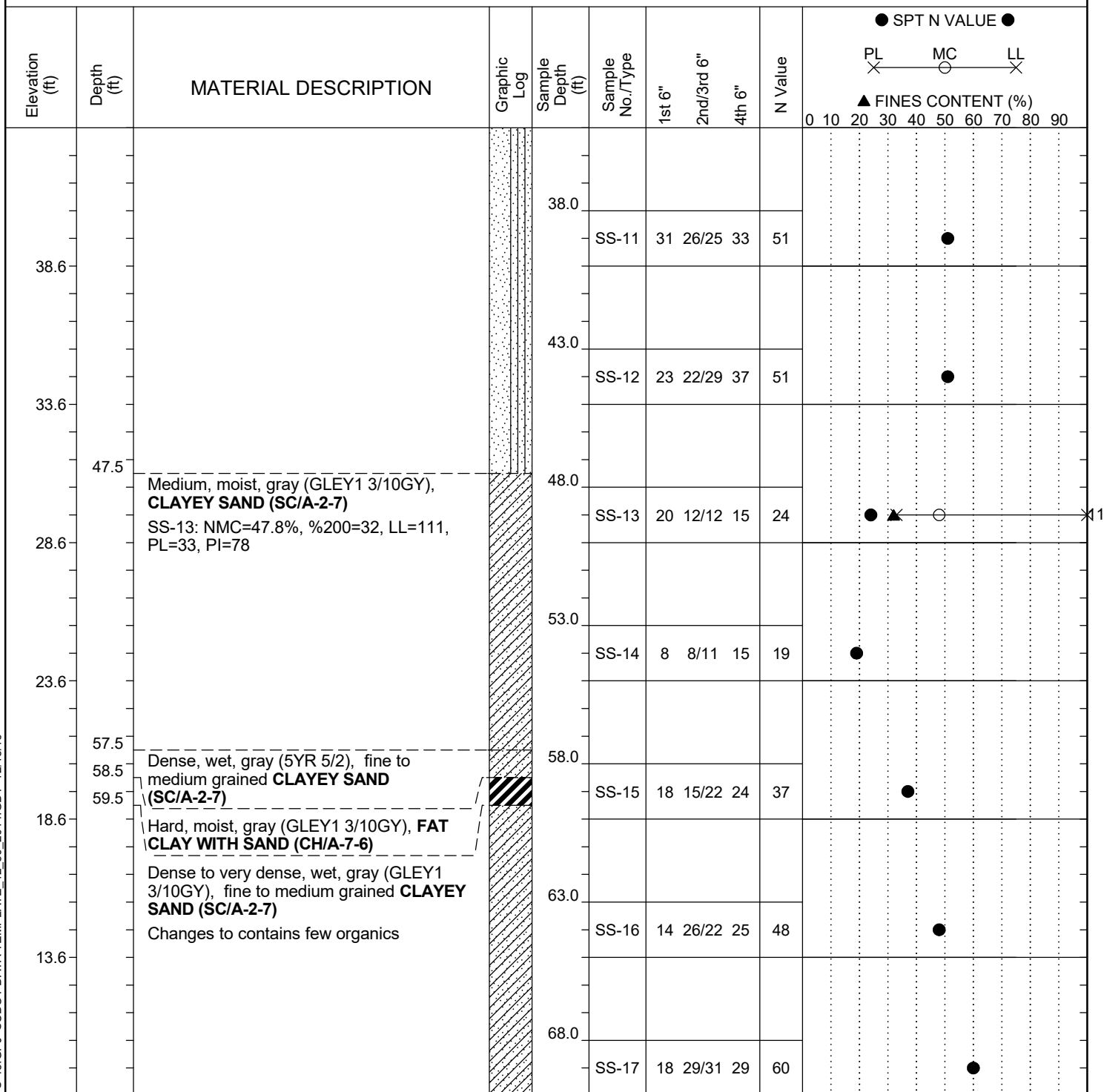
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SCDOT Soil Test Log

Project ID:	P031750	County:	Dillon County	Boring No.:	STB-102
Site Description:	2016-1A Emergency Bridge Package			Route:	S-45
Eng./Geo.:	B. Azumah	Boring Location:	45+31.72	Offset:	5.5 ft RT
Elev.:	78.6 ft	Latitude:	34.3326528	Longitude:	-79.3245083
Date Started:	11/28/2016				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	N/A ft
Date Completed:	11/30/2016				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)	Drill Machine:	CME-550X	Drill Method:	RW
Hammer Type:	Automatic	Energy Ratio:	85.5%		
Core Size:	N/A	Driller:	F&R, Inc.	Groundwater:	TOB 8 ft
24HR	7 ft				



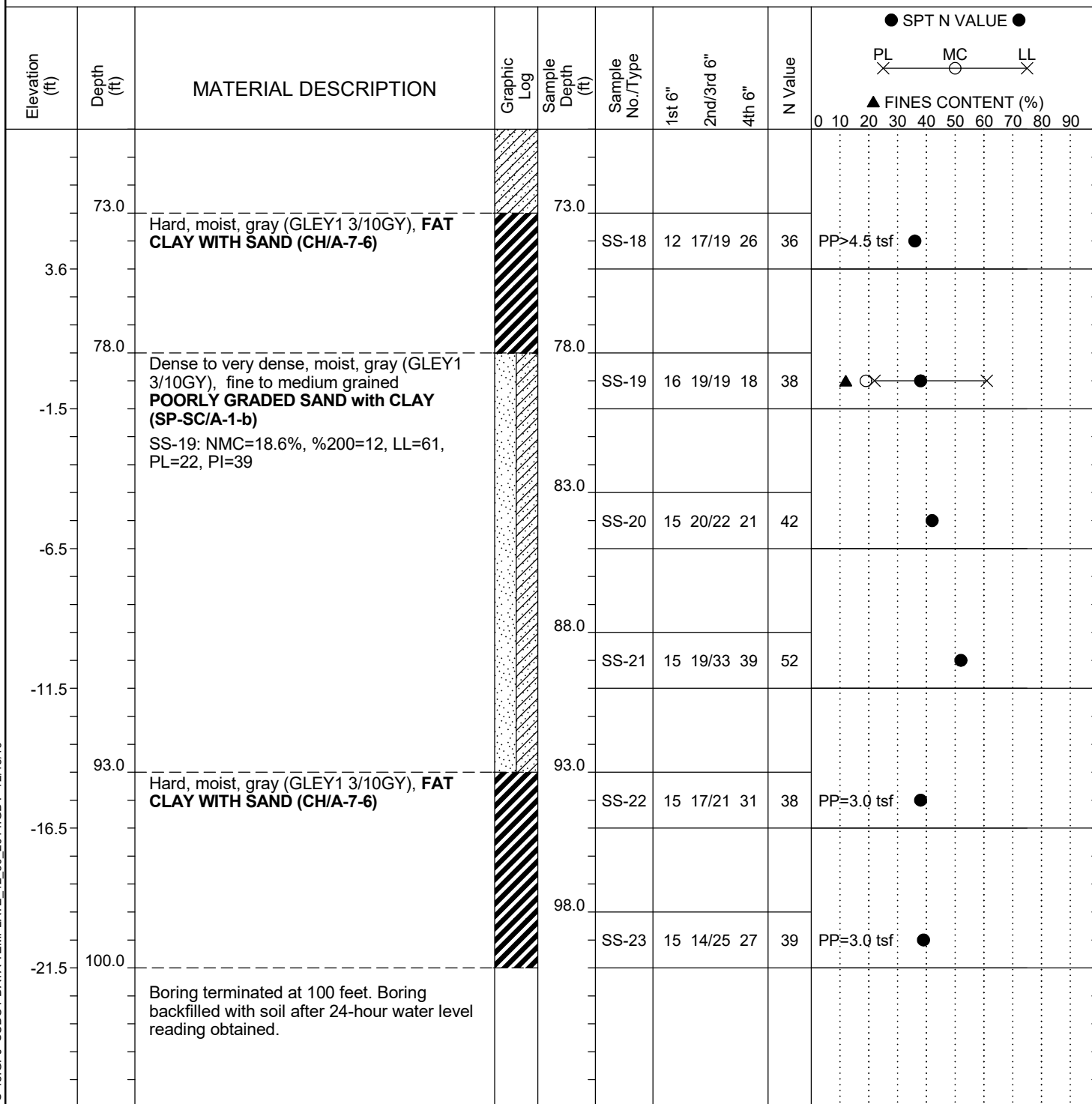
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SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	

SCDOT Soil Test Log

Project ID:	P031750	County:	Dillon County	Boring No.:	STB-102
Site Description:	2016-1A Emergency Bridge Package			Route:	S-45
Eng./Geo.:	B. Azumah	Boring Location:	45+31.72	Offset:	5.5 ft RT
Elev.:	78.6 ft	Latitude:	34.3326528	Longitude:	-79.3245083
Date Started:	11/28/2016				
Total Depth:	100 ft	Soil Depth:	100 ft	Core Depth:	N/A ft
Date Completed:	11/30/2016				
Bore Hole Diameter (in):	4	Sampler Configuration		Liner Required:	Y (N)
Liner Used:	Y (N)				
Drill Machine:	CME-550X	Drill Method:	RW	Hammer Type:	Automatic
Energy Ratio:	85.5%				
Core Size:	N/A	Driller:	F&R, Inc.	Groundwater:	TOB 8 ft
24HR	7 ft				



LEGEND

SAMPLER TYPE		DRILLING METHOD	
SS - Split Spoon	NQ - Rock Core, 1-7/8"	HSA - Hollow Stem Auger	RW - Rotary Wash
UD - Undisturbed Sample	CU - Cuttings	CFA - Continuous Flight Augers	RC - Rock Core
AWG - Rock Core, 1-1/8"	CT - Continuous Tube	DC - Driving Casing	



APPENDIX III



Geotechnical Exploration

Thank you for your trust in PalmettoINSITU, LLC to perform your field exploration.

Test Methods:

PalmettoINSITU performs in-situ testing in general accordance with the currently published ASTM procedures along with generally acceptable industry practices. Applicable procedures include:

- Piezo Cone Penetration Tests (CPTu): D5778-xx
- Marchetti Flat Plate Dilatometer (DMT): ASTM D6635-xx
- Seismic Piezo Cone Penetration Tests (SCPTu) ASTM D7400-xx

Instrumentation:

- All of PalmettoINSITU's probes are manufactured and are calibrated at least annually by Vertek.
- The equipment used for the exploration includes electronic 15 cm² cones with serial numbers listed within the electronic file.
- PalmettoINSITU's Marchetti Flat Plate Dilatometer equipment is provided by GPE, Inc and is calibrated at least annually.

Rig:

- PalmettoINSITU uses a Vertek S4 Scorpion rig capable of 20 tons of thrust. The push system is conveyed and hydraulically powered by a Bobcat T770.

Software:

- PalmettoINSITU uses Bentley's, gINT and Dataforensic's, RapidCPT to process and output the raw data collected.
- Currently, PalmettoINSITU is using version of gINT is V8i SS2 Version 08.30.04.206 and our current version of RapidCPT is 4.2.2.0.

SBT Material Correlations Legend (Robertson and Campanella: 1990):

	1 – Sensitive, Fine Grained Soils		4 – Silt Mixtures-Clay Silt to Silty Clay		7 – Gravelly Sand to Sand
	2 – Organic Soils, Peats		5 – Sand Mixtures-Silty Sand to Sandy Silt		8 – Very Stiff Clay to Clayey Sand
	3 – Clays-Clay to Silty Clay		6 – Sands-Clean Sand to Silty Sand		9 – Very Stiff Fine Grained Soils



S-45
Dillion County, SC
Project Number :16-126

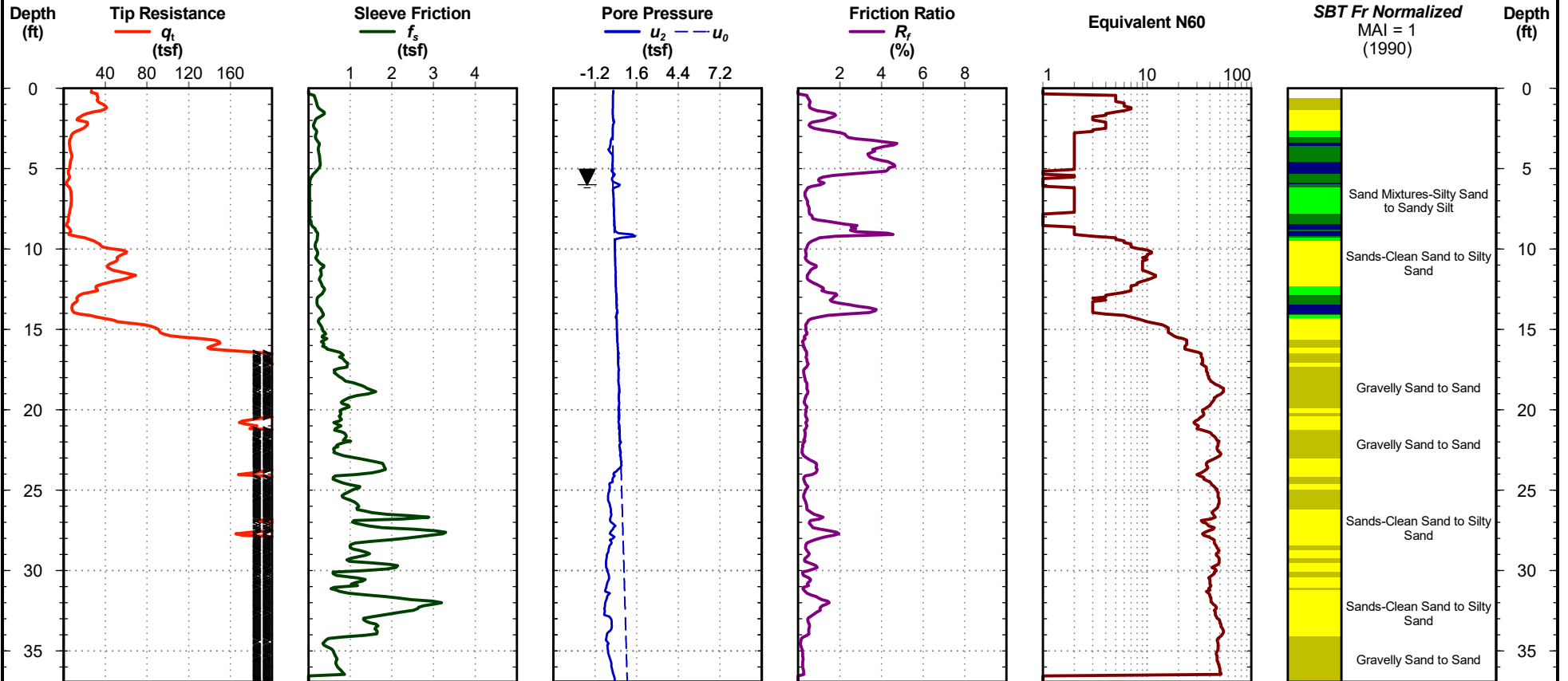
Cone Penetration Test

CPT-101

Date: Nov. 22, 2016
Estimated Water Depth: 6 ft
Rig/Operator: M. Cox | J. Croom

Northing: 913535.2610
Easting: 2505840.5120
Elevation: 78.263 ft

Total Depth: 36.9 ft
Termination Criteria: Maximum Reaction Force
Cone Size: 1.75



CPT-101



Project Number :16-126

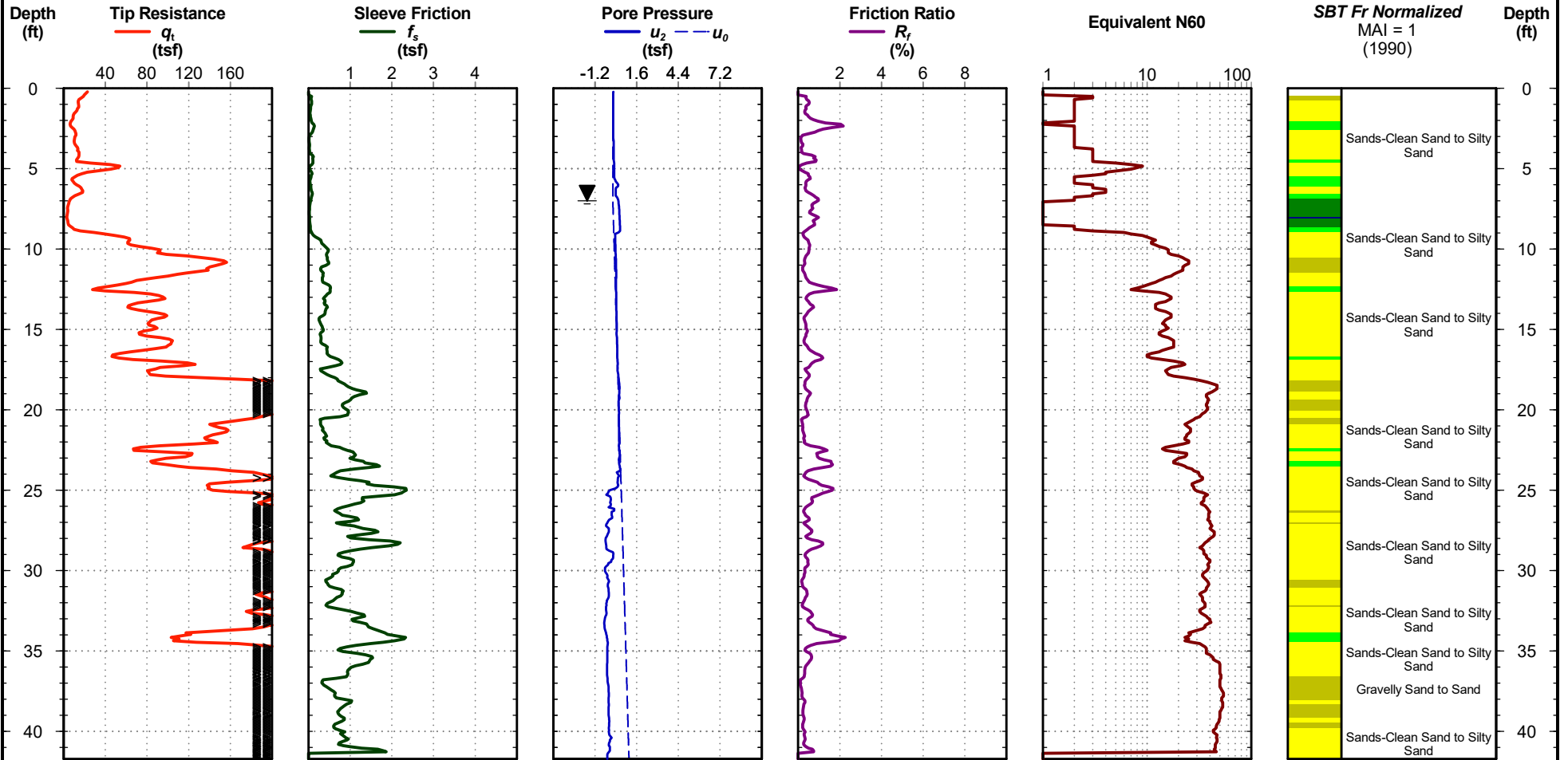
Cone Penetration Test

CPT-102

Date: Nov. 22, 2016
Depth: 7 ft
Operator: M. Cox | J. Croom

Northings: 913464.7470
Easting: 2505752.7530
Elevation: 78.442 ft

Total Depth: 41.7 ft
Termination Criteria: Maximum Reaction Force
Cone Size: 1.75



CPT REPORT - STANDARD S-45.GPJ DF STD US LAB.GDT 12/8/16

CPT-102



APPENDIX IV



FROEHLING & ROBERTSON, INC.

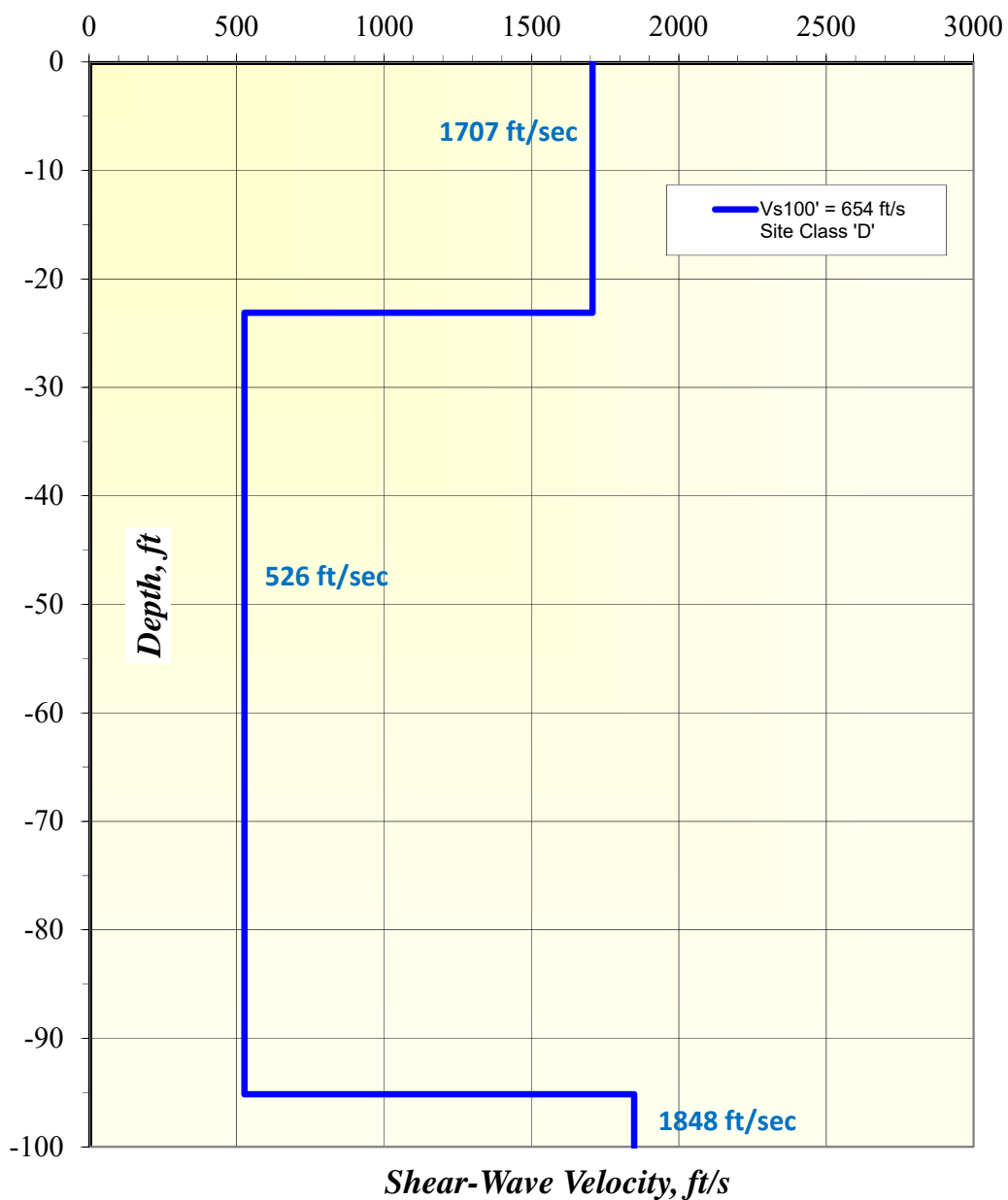
Refraction Microtremor (REMI) Results

Project: 2016-1A Emergency Bridge Package S-45, Dillon Co. SC
Client: SCDOT Geotechnical Design Group

Test Date: 12/1/2016

Report Date: 12/1/16
Record No.: 65U-0177

Vs Model





APPENDIX V



SUMMARY OF LABORATORY RESULTS

PAGE 1 OF 1

PROJECT ID P031750

PROJECT NAME 2016-1A Emergency Bridge Package

PROJECT COUNTY Dillon County

Borehole	Depth	Liquid Limit	Plastic Limit	Plasticity Index	Maximum Size (mm)	%<#200 Sieve	Classification	Water Content (%)	Dry Density (pcf)	Saturation (%)	Void Ratio
STB-101	0.0				0.075	17	SM	9.8			
STB-101	4.0				0.075	14	SM	15.3			
STB-101	13.0				0.075	6	SP-SM	38.2			
STB-101	68.0	102	7	95	0.075	56	CH	49.2			
STB-101	83.0	52	21	31	0.075	45	SC	22.3			
STB-102	0.0				0.075	29	SM	12.4			
STB-102	6.0				0.075	5	SP-SM	20.9			
STB-102	23.0				0.075	3	SP	21.5			
STB-102	48.0	111	33	78	0.075	32	SC	47.8			
STB-102	78.0	61	22	39	0.075	12	SP-SM	18.6			



HQ: 3015 DUMBARTON ROAD RICHMOND, VIRGINIA 23228 T 804.264.2701 F 804.264.1202 www.fandr.com

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