



Geotechnical Subsurface Data Report Richland County Emergency Bridge Package 2 SC 48 (Bluff Road) over Toms Creek Richland County, South Carolina F&R Project No. 65T-0191



Design-Build Section 955 Park Street Columbia, SC 29201

October 27, 2015



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Engineering • Environmental • Geotechnical

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October 27, 2015

Mr. Trapp Harris, PE South Carolina Department of Transportation Design-Build Section 955 Park Street Columbia, South Carolina 29201

Reference: Revision No. 1 - Preliminary Geotechnical Subsurface Data Report Richland County Emergency Bridge Package 2 SC 48 (Bluff Road) over Toms Creek Richland County, South Carolina F&R Project No. 65T-0191 SCDOT Project ID: P029318

Dear Mr. Harris:

The purpose of this revised geotechnical subsurface data report is to present the results of the subsurface investigation program undertaken by Froehling & Robertson, Inc. (F&R) in connection with the Richland County Emergency Bridge Package 2 at State Route SC 48 (Bluff Road) over Toms Creek in Richland County, South Carolina. Our services were performed in general accordance with your work order request emailed to F&R on October 16, 2015, and as authorized by your office per our On-Call Contract with SCDOT. The attached report presents our understanding of the project, reviews our investigation procedures, and describes existing site and general subsurface conditions.

The laboratory test results have been included with this revision of the report.



We have enjoyed working with you on this project. Please contact us if you have any questions regarding this report or if we may be of further service.

Sincerely, **FROEHLING & ROBERTSON, INC.**

calon Gary R. Taylor, PE

Senior Geotechnical Engineer Registered SC No. 27330



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Benedictus K. Azumah, PE Geotechnical Engineer Registered VA No. 052166

SCDOT - Design-Build Section SC 48 Bluff Road over Toms Creek F&R Record No. 65T-0191



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1.0 PURPOSE & SCOPE OF SERVICES

The purpose of this Geotechnical Investigation was to explore the subsurface conditions at the site and perform soil laboratory tests on selected soil samples obtained from the investigation. F&R's scope of services included the following:

- Completion of four soil test borings to depths of approximately 30 or 100 feet below the existing ground surface and as close as possible to the previous culvert location or washed-out pavement shoulder.
- Preparation of typed SCDOT Soil Boring Logs;
- Performing soil laboratory tests including natural moisture contents, Atterberg Limits and Wash No. 200 Gradation tests;
- Preparation of this geotechnical subsurface data report by professional engineers.

This report was prepared in general accordance with the 2010 SCDOT Geotechnical Design Manual (GDM), Version 1.1.

Our scope of services did not include identification and evaluation of appropriate foundation systems for the proposed replacement culvert, design capacities and other environmental aspects of the project site.

2.0 PROJECT INFORMATION

2.1 Site Description

The project site is on Bluff Road (State Route SC 48) located between Congaree Church Road and Griffins Creek Road in Richland County, South Carolina. Toms Creek flows in a north-south direction and crosses below Bluff Road at approximately 130 ft east of the intersection with Congaree Church Road. The area around the creek and the road is generally wooded. Project surroundings are shown on the attached Site Vicinity Map in Appendix I, Figure No. 1.



2.2 Project Description

Recent flooding in the region has caused extensive erosion around the culvert and resulted in a complete loss. Emergency replacement of portions of the roadway and culvert is planned. For this purpose, the geotechnical subsurface investigation and laboratory testing is required by SCDOT and will form part of a preliminary investigation of the site.

3.0 SUBSURFACE INVESTIGATION

3.1 Subsurface Investigation Program

The subsurface investigation program consisted of four soil test borings (STB). The borings, designated as B-01 and B-03 were advanced to a termination depth of 100 ft below the existing ground surface on the approaches to the previous culvert location. The borings designated as B-02 and B-04 were advanced to a termination depth of 30 ft below the existing ground surface close to the washed-out pavement shoulders near the previous culvert location. Approximate boring locations are identified on the Boring Location Plan included in Appendix I as Figure No. 2. Photographs of Borings B-01 through B-04 being drilled are also included in Appendix I as Figure No. 3A through 3D.

3.2 Location Control

The STB locations were staked in the field by personnel from our office following instructions from your office. Borings B-01 and B-03 were drilled close to the centerline of the existing alignment at approximately 23 to 24 feet from the remaining edge of the culvert approach closest to the creek. Borings B-01 and B-04 were drilled in the pavement adjacent to the washed-out shoulder areas. The ground surface elevation at the borings locations were not provided to us at the time of this writing. GPS coordinates of Borings B-01 (Latitude 33.841829 / Longitude - 80.731278), B-02 (Latitude 33.841713 and Longitude -80.730175), B-03 (Latitude 33.841876 and Longitude -80.731754) and B-04 (Latitude 33.841944 and Longitude -80.732346) were obtained with a portable hand-held GPS and are recorded on the soil boring logs included in Appendix II of this report. Boring locations should be considered no more accurate that the methods and plans used to obtain them.



3.3 Subsurface Investigation Procedure

Subsurface investigation was performed on October 20, 2015 using an ATV-mounted CME/550X and truck-mounted CME/550 drill rigs. The drill rigs used for this project were equipped with an automatic hammer and the drilling method used was the wash rotary boring. The energy ratio of this ATV-mounted hammer reported to us indicates 86% and that for the truck-mounted hammer indicates 74%. SPT tests at boring locations were performed continuously from the existing ground surface to a depth of 10 feet. Thereafter, boreholes were advanced and SPT performed at approximate 5-foot intervals to their termination depths. The Standard Penetration Test (SPT) was performed at the boring locations in general accordance with ASTM D1586.

Soil samples were obtained with a long split-spoon sampler with each SPT being driven with a 140-lb automatic hammer falling 30 inches. The number of blows required to drive the sampler each 6-inch increment of penetration was recorded and are shown on the boring logs. The first six-inch increment is used to seat the sampler with the sum of the second and third penetration increments being termed the SPT value, "N". A representative portion of each disturbed split-spoon sample was collected with each SPT, placed in a glass jar, and returned to our laboratory for review and testing.

The recovered split-spoon samples were visually classified by F&R engineers in general accordance with the ASTM D2488. The boring logs provided in Appendix II show the subsurface conditions encountered on the dates and at the approximate locations indicated.

By the nature of the work performed, the drilling activities result in disturbances to the site. The completed boreholes performed were backfilled upon completion. The borehole backfill may subside at some time following our work. F&R assumes no responsibility for borehole subsidence after completion of the field investigation and departing the site.



3.4 Groundwater

Groundwater was encountered in all borings at depths of approximately 8 and 17.5 feet. The test borings were backfilled after completion of drilling for safety. The depth at which groundwater was encountered in each individual boring is indicated on the attached soil boring logs in Appendix II.

The groundwater levels at the boring locations were determined based on our observation of free water in the split-spoon soil samples following removal of the sampler. Upon completion of drilling, the boreholes were backfilled for safety, hence the absence of 24-hour water level readings on the boring logs.

The groundwater levels on the soil boring logs indicate our estimate of the hydrostatic water table at the time of our investigation. The final design should anticipate the fluctuation of the hydrostatic water table depending on variations in precipitation, surface runoff, evaporation, creek levels and similar factors.

4.0 LABORATORY TESTING

Laboratory testing consisted of Atterberg Limits Tests, No. 200 Sieve Cut grain size analyses (Wash #200), and Natural Moisture Content tests performed on specific soil samples. The specific tests performed on the selected samples are listed in Table No. 1 below.



Item	Boring	Depth (ft)	Atterberg Limits	Percent Fines Wash # 200	Natural Moisture Content
1	B-01	4 - 6	-	yes	yes
2	B-01	18.5 – 20	-	yes	yes
3	B-01	28.5 – 30	-	yes	yes
4	B-01	38.5 – 40	yes	yes	yes
5	B-01	48.5 – 50	yes	yes	yes
6	B-01	58.5 – 60	yes	yes	yes
7	B-01	68.5 – 70	-	yes	yes
8	B-01	73.5 – 75	-	yes	yes
9	B-02	13.5 – 15	-	yes	yes
10	B-02	23.5 – 25	yes	yes	yes
11	B-03	18.5 – 20	-	yes	yes
12	B-03	28.5 – 30	-	yes	yes
13	B-03	33.5 – 35	yes	yes	yes
14	B-03	43.5 – 45	-	yes	yes
15	B-03	53.5 – 55	-	yes	yes
16	B-03	63.5 – 65	yes	yes	yes
17	B-03	68.5 – 70	-	yes	yes
18	B-03	78.5 – 80	-	yes	yes
19	B-04	6 – 8	-	yes	yes
20	B-04	23.5 – 25	-	yes	yes

Table No. 1. Soil Laborator	y Tests Performed on Selected Soil Samples
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The laboratory testing results are presented in Appendix III and the laboratory test data sheets are presented in Appendix IV.

F&R greatly appreciates the opportunity to work with you on this project. If there are any questions concerning this report or if any additional information is required, please do not hesitate to contact us.



6.0 LIMITATIONS

This report has been prepared for the exclusive use of South Carolina Department of Transportation or their agent, for specific application to the referenced site in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. Our investigation is based on site location information furnished to us; and generally accepted geotechnical engineering practice. The subsurface investigation logs included herein, do not reflect variations in subsurface conditions which could exist intermediate of the boring locations or in unexplored areas of the site. Should such variations become apparent during construction, it will be necessary to perform additional subsurface investigations based upon on-site observations of the conditions.



APPENDIX I





Figure No. 3A - Photograph of Boring B-01 Being Drilled

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Figure No. 3B - Photograph of Boring B-02 (Red dot on pavement

Figure No. 3C - Photograph of Boring B-03 Being Drilled

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APPENDIX II

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Boring		B-02	_	ing Locati					Offse						nmen		Existing]
lev.:	ft	Latit			341713	Longi).730				Start			_	20/15	
Total D	-	30 ft	Soi	I Depth:	30 ft		ore De		0 f			Date	Com				20/2015	5
Bore Ho	ole Dia	meter (in):		San	pler Con	figurati	on	Line	er Re	quir	ed:	Y	N		Liner	Used	: Y	C
Drill Ma	chine:	CME-55)X	Drill Meth	od: Ro	tary Wa	ash	Hamm	er Ty	/pe:	Auto	omat	ic	En	ergy	Ratio:	86%	
Core Si	ze:			Driller:	SCI			Ground	dwat	er:	TOE	3 8	B ft		24	HR	N/A	
															SPT	N VALI	JE	
E	_					U	e _	e e				Ð		PL		MC		
(ft)	Depth (ft)	MATE	RIAI	DESCRIP	NOIT	Graphic Log	Sample Depth	Sample No./Type	=_	ļ0	.9	Value				0		
Elevation (ft)	ے 0.0			DECON	non	5	S Q ,	No.	st 6"	2nd 6"	3rd 6	ź	0 10				NT (%)	0
	0.0	Asphalt Pave	ment 6	5"			0.0			N	e		0 10	20 3	<u>30 40</u>	50 60	<u>70 80</u>	9
	-	FILL, dense,			wn fine to			- 1/SS	7	14	10	24		•				
	2.0	coarse, CLA	EY S	AND (SC/A-2	2-6)		2.0						-	-	: :			
	4	Munsel: 10Y	R 5/2					- 2/SS	1	2	4	6			: :	÷		
	4.0	└	rav an	d brown SA	NDY FAT		4.0						-	-				
	-	CLAY (CH/A		, .				- 3/SS	1	5	10	15						
	6.0	Munsel: 10YF	R 5/2				6.0						-		: :			
	4	medium dens	e, moi	ist, brownish	gray. fine	-		- 4/SS	4	4	3	7						
	8.0	≵to coarse, SI I	TY S	AND (SM/A-	2-4)		8.0							:				
		Munsel: 10Y	R 6/2			<i> </i>		- 5/590	ОН/1	8"		1						
		loose, moist,	brown	ish arav. fine	 e to	-¦ <i>V///</i>				-								
		medium, PO	ORLY	GRĂDĖD SA	AND with	` /////							:		: :			
	7	SILT (SP-SM)		¦ [////							:		: :			
	12.5	Munsel 10YF	. 6/2					1							: :			
	1		ery so	oft, wet, gray	, SANDY		13.5	1					-		: :			
	-	LEAN CLAY	•	·v)				6/SS	6	10	13	23	:		: :			
	-		<u> </u>			_/ [+										
	-	medium dens	e, wet	, gray, fine,	SILTY			1					:		: :			
	17.5	SAND (SM/A Munsel: 10YF						-						-				
	-	SS-6: NMC=		%200=18 3	ł		18.5	-							: :			
	-				, 	_` ///		- 7/SS	6	10	19	29	:					-
	-	hard, moist, l						+							· · ·			
	-	CLAY TRAC		•)			-										
	-	Munsel: 10YF			10-06 50/			-					:		: :			
	-	SS-8: LL=62, %200=85.2	г L=2	ש, רו−סס, INI\	//U-20.3%,		23.5	-						-				
	-							8/SS	9	16	25	41		÷C	: : * •	>	< · · ·	
	-								Ļ					. *			<u> </u>	_
	4							-						÷				į
	4							_										
	_						00.5	_						÷				
							28.5			4.5			1					
	30.0							9/SS	11	15	22	37						
													:					
]													÷	: :			
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	1							1							: :			
	1							1							: :			
						LE	GENI	 D	1			1	· ·		•	•		
		SAM	IPLER	TYPE					_			RILLIN	IG ME					
	Split Spo Shelby Ti			NQ - Rock C CU - Cuttings				SA - Hollo				gers			Rotary	Wash ore		

Site De Boring	-	B-03	SCD			ocatio					Offs						gnm	ute:		2 48 Existing	
Elev.:	ft		Latitu		ing L		41876	Lon	gitude:		80.73			Data	Star		JIIII			9/15	J
otal D		100 f			Dep		100 fl		Core D			ft			Com		ad.			20/201	
				301	Бер											•					
		meter (ii	,		Duill		pler Cor				iner R				\sim				sed:		(
	chine:		-550			Metho	SCI	otary V	asn		mer T Indwa								atio:		
Core Si	ze:				Drille	er:	501			Grou	nawa	iter:	TOE	5	8 ft			24H	ĸ	N/A	
																	• SF		VALU	IF ●	
Elevation (ft)	÷							hic	the d	(π) Sample	Ahe			Value		P >	L 	M	<u> </u>	\xrightarrow{LL}	
eva (ft)	Depth (ft)	M	ATEF	RIAL	DES	CRIP	TION	Graphic	Log Sample Depth		. 0	2nd 6"	6"	Va			FINE	sco		NT (%)	
Ē	0.0							0	– رہ	0	1st	2nc	3rd 6"	z	0 10					70 80	
	0.5	Asphalt	Paven	nent 6) <u> </u>			/ []]	0.0			0	4	10		:	:	:			
		FILL, me							2.0	1/S	S 6	6	4	10							:
		gray, fin (SM/A-2		barse,	SILT	Y SAN	D									÷		: :			
	4.0	Munsel:		5/3 &	. 6/1				4.0	_ 2/S	S 1	1	1	2							:
	<u>-</u>	\sim															:	:			
	-	loose, m					se,		6.0	_ 3/S	S 2	2	3	5			;				
	-	Munsel:		•	_ •	,			0.0						1						
	_			-						_ 4/S	S 2	2	3	5				-			
	8.0	_Z gray ∫Munsel:	7.5YR	8 6/1					8.0	' 	+				1		:	: :			
	-	/						-' 🕢		- 5/S	S 2	2	4	6				-			
	-	loose, w	et, gra ∕ S∆NI	iyish l D (SC	brown,	, fine to	o coarse,								:		:	: :	:	<u>: :</u> : :	
	-	Munsel:		•	// // -2-0	"				-								-			
	12.5	Manool.	10110	0/2						-					:	÷	:	: :	:	: :	
	12.0					, brow	nish gray,	1	13.	_											
	_	FAT CL	•		-6)				15.3	, 6/S	S 1	1	1			÷	-				
	_	Munsel:	10YR	6/2						0/3		1	-	2							
	_									_											
																÷	-	-			
	17.5	 medium	dense	e wet		 sh brov	wn, fine to			_								:			
		coarse,	POOR	LY G			ID WITH		18.						-						
	1	SILT (SI								7/S	S 2	7	8	15							
	1	Munsel:			0/ 000	-7 40/										:			:		
	1	SS-7: N		J.J%,	70ZUU	<i>i−1</i> .4%)			1						÷	:	: :			
	-									1							:	-			
	-	dens - "		0.4.55					23.	57					-			:			
	-	dense, li Munsel:	-							8/S	S 10	11	13	24							:
	-	1011301.	1.010	. 0/0						+											:
	-									1						÷	÷				
	27.5			.			_ 			1											
	-	dense, v POORL							28.	5-						-		:			
	-	(SP-SM/								9/S	S 10	15	17	32		Ó		-			
	-	Munsel:	7.5YR	R 6/3						+											
	-	SS-9: N	MC=20	0.3%,	%200	8.8=				-							:				
	32.5									-							:	: :			
	-						ight gray		33.5	5-						÷	÷				
	-	and light (CH/A-7		n, SA	NDY F	ATCL	.AY			10/5	S 8	10	18	28	1 :	0			×		
			•,									10	10	20		:	:	:	<u></u>	<u> </u>	
			0 4 7 1 -		T\/P-			L	EGEN	D			-					onti	nuec	d Next	Pa
SS - 5	Split Spo	on	SAM		TYPE		ore, 1-7/8"			<u></u>	bllow St	۰.		KILLI			DD Rota		ach		

Boring	scription No.:	B-03	Boring I					Bluff Ro	Offse					Aligr	nmen	t:	Existin	g
Elev.:	ft	Latit			1876	Longit	ude:	-80	.731	754	0	Date	Starte	ed:		10/	19/15	-
Total D	epth:	100 ft	Soil Dep	oth:	100 ft		re De	oth:	0 f	t	0	Date	Com	olete	d:	10/:	20/201	5
		meter (in):			oler Conf			Line				Y	(N)		iner			()
	chine:) Drill	Metho		ary Wa		lamme		-		mati	$\overline{}$	-			: 79%	
Core Si			Drill		SCI			Ground		-	TOE		3 ft			HR	N/A	
					1.00.													
														•	SPT	N VAL	UE ●	
_														DI		MC		
Elevation (ft)	÷					Graphic Log	Sample Depth (ft)	Sample No./Type		_		Value		PL ×			——X	
(ft (ft	Depth (ft)	MATE	RIAL DES	CRIPT	ION	Lo	Dep (ff	sam o./T	st 6"	2nd 6"	3rd 6"	× N		▲ F	INES (ONT	ENT (%))
ш							0)	°'Z	1st	2n	3rc	~	0 10				<u>0 70 8</u>	
		Munsel: 7.5Y					_											
]	SS-10: LL=52 %200=74.7	2, PL=27, PI	=25, NN	1C=21.7%,									÷				:
	37.5	~				- HA	-							-				
	-	hard, moist, g	gray, SAND	Y LEAN	CLAY		38.5							÷	÷			
	-	(CL/A-4)					-	11/SS	9	12	18	30	÷		•			
	-	Munsel: 7.5Y	r 0/1				-						:	÷		: :		
	-						-						÷		÷			
	42.5						-							:	÷	: :		
	-	medium dens					43.5						:	-		: :		
	-	fine to mediu Munsel: 5YR		•	/⊮A-1-D)		-	12/SS	8	9	9	18		÷ •		: :		:
	-						-	,00		5	5				<u>.</u>			
	_	SS-12: NMC=	=21.1%,%2	00=18.3			_											
							_						÷	-	: :	: :		
							_							-				
	1						48.5											
	1						-	13/SS	8	10	11	21		۲				į
	-						-							:				
	-						-							-				
	52.5						-						:	÷				
	-	medium dens to medium, S	e, wet, brov ערא אין דע	vn and g (SM/∆ ₋1	ray, fine 1-b)		53.5							:	: :			•
	-	Munsel: 10YF		(Sum-1	. ~,		-	14/SS	8	8	10	18	÷	ė	÷			
	-	SS-14: NMC=)=19.3			-							:				
	-		, /0200				-						÷		÷			
	57.5						-											
	-	medium dens					58.5									: :		
	_	medium, POC SILT (SP-SM		DED SAN	ND with			15/SS	8	7	8	15						
	_	Munsel: 7.5Y					_	13/33	0	I	0	10						
							_							-				: :
							_											:
	62.5	hard, moist, li	ight brown											-				
	7	(SM/A-2-4)					63.5							÷				
	1	Munsel: 7.5Y	R 6/3				-	16/SS	7	13	18	31						:
	-	SS-16: NMC=	=28.0%, %2	00=27.3			-							:				:
	-						-						:	-		: :		
	67.5						-							÷		:		:
	-	very dense, n gray, fine to r	noist, light b	rown an	d light		68.5							-		: :		
	-	SAND with S	ILT (SP-SM	/A-1-b)			-	17/SS	17	21	21	42		Ö	•	: :		
				.,									:	:	<u>: :</u>	: :		<u>. </u>
		0.00				LE	GEND					<u> </u>				tinue	ed Nex	t Pa
SS - 5	Split Spo		IPLER TYPE	<u>-</u> Rock Cor	. 1 7/0"			A - Hollo	w Sto			VILLIN	IG ME) Rotary '	Mach		

Site De Boring	-	B-03	DOT - Eme Boring L				•		Offse						Route nmer		C 48 Existing	g
Elev.:	ft		ude:	33.841		Longi	tude:			754	1	Date	Start				19/15	<u> </u>
Total D		100 ft	Soil Dep	-	100 ft		ore De		0 f			Date			d:	_	20/201	5
		meter (in):		Sample					_	quir		Y		•		Usec		1)
Drill Ma			0 Drill	Method		tary Wa		Hamme					$\overline{}$: 79%	_
Core Si			Drill		SCI			Ground		-			3 ft			HR	N/A	
	20.				001			oreand	inat				<u> </u>				110/7	
															SPT	N VAL	UE ●	
_														Ы		MC		
Elevation (ft)	t					Graphic Log	Sample Depth	Sample No./Type				Value		PL		MC	—X	
eva (ft	Depth (ft)	MATE	RIAL DES	CRIPTIC	NC	Lo	Dep	o./T	1st 6"	2nd 6"	3rd 6"	\a		A F	INES	CONT	ENT (%)	
Ξ						0	0) —	٥ź	1st	2nc	3rd	z	0 10				0 70 80) 9
		Munsel: 7.5	YR 6/3											÷	: :			
	1	SS-17: NMC	=19.0%, %20	00=11.0			•											
	-	SS-19: NMC	=19.8%, %20	00=7.7				1										
	-						73.5	1					1	-	:			
	-						:]	18/SS	14	15	18	33		-	•			
	-																	
	-							-						-				
	-							-					:	:	: :			
	-						78.5	-						÷	· · ·			
	_						10.5	19/SS	18	04	20	47		: •				
	_							19/55	10	21	26	47		<u> </u>		•		
							:							-	: :	: :		
	1													-				
	82.5	hord maint					-	1						-				
	-	hard, moist, g Munsel: 10YI		SILI (IVIL	∟/ A -4)		83.5	1										
	-		N 0/ 1					20/SS	25	35	50	85		-	· · ·			•
	-													÷				
	-							-										
	-							-						÷				
	-						88.5	-										
	_						00.0	21/SS	20	37	47	84	1					
	_							21/00	20	57	+/	04						-
	_													÷		:		
	92.5	 dense, wet, li	ight gravish l	hrown fin										-		:		
	1	medium, PO	ÖRLY GRAD	DED SAND) with		93.5											
	1	SILT (SP-SM	-					22/SS	16	15	16	31		-	•			
	-	Munsel: 10YI	R 5/2												· ·			
	-							1						Ē				
	-							1						÷				
	-						98.5	-										
	-							23/SS	14	12	11	23		•				
	100.0								· ·	_								
	_							-						:				
	_							-						-				
														-	: :			
	1													-				
						LE	GENE)				•	•					
									_			RILLIN	IG ME	THO	2			
	Split Spo Shelby Ti			Rock Core, Cuttings	I-1/8"			A - Hollo A - Conti				aore			Rotary Rock C	Wash		

Site De Boring	-	B-04	DOT -	ina Lo	cation	:			(Offs					Alig	nme	nt:	Exi	isting	
Elev.:	ft		titude:		33.841		Longit	ude:			2346	1	Date	Start)/20/	<u> </u>	
Fotal D		30 ft		Depth		30 ft		ore De		01			Date			d.			/2015	;
		meter (in):					figuratio			_	equir			(N)			r Use		Y	, ()
Drill Ma					lethod		tary Wa		Hamme		-			\sim			Rati			
Core Si				Driller		SCI			Ground	_		TOE		3 ft			4HR		N/A	
	20.			Dimer	•	001			oround	ama		101		<i>,</i>		-	<u>+1 II X</u>		•// \	
																SPT	N VA	LUE	•	
															וס		MC			
Elevation (ft)	t						Graphic Log	Sample Depth (ft)	Sample No./Type				Value		PL ×	•			⊥L →X	
(ft)	Depth (ft)	MAT	ERIAL	DESC	RIPTI	ON	Sraphi Log	Sam Dep (ft	Sam o./T	1st 6"	2nd 6"	3rd 6"	N <		▲ F		CON	TENT	Γ(%)	
Ξ	0.0						0	0)	°,z	1st	2nc	3rc	2	0 10					70 80	9
	0.5	Asphalt Pa	vement 6)"				0.0	1/SS	5	7	12	19			-				-
		FILL, medi						2.0	1/00		'	12	13	÷		: :	÷	÷	: :	ļ
	1	gray, fine to (SC/A-2-6)	o coarse,	CLATE	IT SAN	D			0/00		4	4	_			-		-		
	1	Munsel: 10	YR 6/2					4.0	2/SS	1	1	4	5		:		÷	-	1	
	-	loose						4.0	1						-	: :				
	6.0	10000						6.0	- 3/SS	3	2	2	4	•		: :		<u> </u>		
	0.0	medium de						0.0										:	: :	
		to medium	SILTY S						4/SS	1	2	9	11	• :	Å			÷	: :	
	8.0	Munsel: 10						8.0								: :			: :	
	-	SS-4: NMC	=10.3%,	%200=	18.9		`\ <i>\\\\</i>	-	5/SS	3	2	2	4	\bullet		: :		-		
	10.0		l firm we	et. arav	SAND	Y LEAN	- , 644	-		-					<u>:</u>	<u> </u>	<u>:</u>	<u>:</u>	: :	_
	-	CLAY (CL/	A-6)	, gruy,	.	/\\	1000	.	-							: :				
	12.5	Munsel: 10	YR 6/1				1 top		-						÷		:	÷		
	12.5	medium de	nse wet	hrown				13.5	_							1				
	_	GRAVEL (GM/A-1-b))	, 0.2			13.5			10	<u> </u>	10			: :		-		
	_	Munsel: 10	YR 5/3						6/SS	9	10	6	16							
	_	dense, wet		brown.	 fine to r	nedium.	-													
		POORLY (RADED												-			÷		
		(SP-SM/A-			10												:		: :	
]	Munsel: 5Y						18.5		-								į		
	1	SS-8: NMC	<i>,</i> −∠U.1%,	%ZUU=	10.4				7/SS	9	9	16	25		•		:	÷		
	1														-		:	:		
	-							-	1						-	: :		-		
	-							-	1									:	: :	
	-							23.5	1	<u> </u>							:	:	: :	
	-							-	8/SS	4	16	18	34		: O	ullet	•	÷		
	-							-		-					÷	: :		:	: :	
	-							-	-					:				÷	: :	
	-							.	-								:			
	-							28.5	-							: :	:	ł	: :	
	-	medium de	nse						9/SS	7	10	11	21				•	:		
	30.0							-		Ļ.		• •				<u>:</u> : :		<u> </u>	+ +	
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															:	:			<u>:</u>	
							LE	GEND)											
	Split Spo		AMPLER	TYPE NQ - Roo						w Ste			RILLIN				y Was			



APPENDIX III



FROEHLING & ROBERTSON, INC.

LABORATORY TEST SUMMARY SHEET

F&R Project No:65T-0191Client:South Carolina Department of TransportationProject:SC 48 Bluff Road Bridge over Toms CreekSCDOT Proj IDP029341City/State:Richland County, SC

Boring/Sample No.	Depth (ft)	ш	PL	PI	USCS Classification	Water Content (%)	Percent Passing No. 200 Sieve
B-01	4 - 6					21.0	29.7
B-01	18.5 – 20					18.6	3.9
B-01	28.5 - 30					25.7	14.2
B-01	38.5 – 40	31	19	12	CL	18.4	67.1
B-01	48.5 – 50	NP	NP		SM	30.7	42.0
B-01	58.5 – 60	NP	NP		SP-SM	19.0	10.2
B-01	68.5 – 70					30.1	17.7
B-01	73.5 – 75					20.8	26.9
B-02	13.5 – 15					20.5	18.3
B-02	23.5 – 25	62	29	33	СН	26.5	85.2
B-03	18.5 – 20					16.3	7.4
B-03	28.5 – 30					20.3	8.8
B-03	33.5 – 35	52	27	25	СН	21.7	74.7
B-03	43.5 – 45					21.1	18.3
B-03	53.5 – 55					23.1	19.3
B-03	63.5 – 65	NP	NP		SM	28.0	27.3
B-03	68.5 – 70					19.0	11.0
B-03	78.5 – 80					19.8	7.7
B-04	6 – 8					10.3	18.9
B-04	23.5 – 25					20.7	10.4

NP: non-plastic

Date: 10/26/15



APPENDIX IV







BLUFF RD.

			VOSTURE	COMTEN	- '% ;	، ۰	×
		; #2	#3		45	¦ #6	异7
Sample I.D	B-14-5.5		B-1 18,5-2	1.	B-1 28.5-3	d	B-1 38.5-
Wet Scil + Tare	478.88		526.13		658.81		478.43
Dry Soils + Tare	421.70		466:20		554.385		435.76
Tare # & Weight	53 149.25		S-1 147.11		521 147.7	3	AD 204.3
Weight of Water	57.18	· ;	59.33	4	104,43	<u> </u>	42.67
Weight of Dry Soils		1	319.69	· · · · · · · · · · · · · · · · · · ·	406.65	ļ	231.40
Nioisture Content	0.210		0.186		0.257	[[0.184
	*	<u> </u>	- **	_			
	#R	ļ #9	#10	#11		<u> </u> #13	· #14
Sample I D.	B-1 48.5	-50	B-1 58.5-0	60	B-1 68.5-		B-1 735
Wet Soil + Tare	519.75		A15.75		517,10	<u>}</u>	504.54
Dry Solls ÷ Tare	431.92		372,72		431. 78	· · · · · · · · · · · · · · · · · · ·	442.96
Tare # & Weight	K 145,31		L 146,27	<u> </u>	Sid 148,27		56 146.5
Veight of Water	87.83		43.03		85.32	<u> </u>	61.58
Veight of Dry Soils	286.55		226.45		283.51	1	296.43
Aoisture Contení	6.307		0.190	·	0.301	<u> </u>	0.208
<u> </u>	<u>. </u>			, 	(#19	<u>#20</u>	#21
	#15	#15	#17 - ·	#\0	#15	<u></u>	
ample I.D.				^		1	
Vet Soil + Tare	 				1		 L
ry Soils + Tare					1		
are # & Weight							· · ·
eight of Water			<u> </u>			!	· · ·
eight of Dry Solls			ļ [•			
oisture Content			·		1	<u>.</u>	· [
					1 400 1		#28
	幕22	<i>#</i> 23	#24	#25	#26	<u>#2</u> 7	<u> </u>

1	<u>#22</u>	FZ3	#24	1 m20	1 1120		····
Sample I.D.			· ,			ļ	· · · · · · · · · · · · · · · · · · ·
		<u> </u>	1	, r			{ {
Wet Soil + Tare	1	<u> </u>	<u> </u>	<u></u>	i	1	
Dry Soils + Tare		<u> </u>			 	<u> </u>	
Tare # & Weighi			<u> </u>	<u> </u>	<u> </u>	1	<u> </u>
Weight of Water				<u> </u>	E	<u> </u>	<u> </u>
Weight of Dry Soils			<u>}</u>	1			·
Moisture Content			1	!		<u> </u>	· · · · · · · · · · · · · · · · · · ·

V

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BLUFF RD,

VOSTLRECONTENT (%)

			<u> </u>				·
[- #1	#2	; #3	<u> </u>	#5	#6	#7
Sample I.D	B-Z 13.5		B-2 ^{23.5}	-25.5		· · · · · · · · · · · · · · · · · · ·	
Wet Soil + Tare	561.95	ŀ	377.18		· .		
Dry Soils + Tare	491.46		329:35		· ·		
Tare # & Weight	521 147,68		53 149,14				
Weight of Water	70,49	• •	47.83	,		,	
Weight of Dry Soils	343.78		180.21				
Moisture Content	0.205		0.265				

	#8	荐9	#10	#11	#12	#13	#14
Sample I D.	B-4 6-7.5		B-4 23,5	-25	<u> </u>		}
Wet Soil + Tare	390.01		381.66		<u> </u>	<u> </u>	<u> </u>
Dry Soils + Tare	367.36		341.25		<u> </u>	<u> </u>	<u> </u>
Tare # & Weight	5-7 147.00		L 146,22		<u> </u>	<u> .</u>	
Weight of Water	22.65		40.41	·	· ·	<u> </u>	[
Weight of Dry Soils	220,28		195.03			<u> </u>	<u> </u>
Vioisture Contení	0,103		0.207	•	<u> </u>	<u> </u>	<u> </u>

·	<i>#</i> 15	#15	<u>#17</u>	#18	#19	#20	<u> </u> #21
Sample I.D.			·		1		
Wet Soil + Tare					<u> </u>	1	
Dry Soils + Tare		<u> </u>			<u> </u>	<u> </u>	<u> </u>
Tare # & Weight			1 1 1	<u>}</u>	<u>{</u>	<u> </u>	<u> </u>
Weight of Water			<u> </u>	<u> </u>	<u> </u>	<u> </u>	<u> </u>
Weight of Dry Solls			 	<u> ·</u>			<u> </u>
Moisture Content			l	<u> </u>	1	1:	· ·

	{ #22 ·	#23	#24	#25	#26	#27	#28
	(<u> </u>	1			{	1	
Sample I.D.		<u> </u>	<u> </u>	<u> </u>	·	<u> </u>	{
Wet Soil + Tare			<u> </u>			<u>_</u>	
Dry Soils + Tare		<u> </u>	· ·	<u></u>	 	<u>}</u>	<u> </u>
Tare # & Weight		Į	<u> </u>	<u> </u>	}	 	
Weight of Water	•	ļ	<u> </u>	1		<u> </u>	<u>}</u>
Weight of Dry Soils		1	¦ <u>}</u>			<u> </u>	1
Moisture Content	_		1 l			· ·	;

BLUFF RD

VOSTLRECONTENT (%

1	#1	<u>#2</u>	#3	<u>辞</u> 4	#5	「 	<u>*</u> 7
Sample I.D	B-32	0	B-3 28.5-3	»	B-3 33.5-3	\$	B-3 43.5-4
	448.16	·	415.41		310.08		509,96
Wet Soil + Tare	406,23	<u> </u>	370.54	···· ·· ··	281, 20		446,99
Dry Soils + Tare		<u> </u>	5 149.35	· · · · · · · · · · · · ·	58 147.86	 	59 148.6
Tare # & Weight	G 149,48	p 	44.87		28.88	4,	62.97
Weight of Water	41.93				;	1	298.37
Weight of Dry Soils	3		221.19	<u> </u>	133.34	 	0.211
Nioisture Content	6.163		0.203		0.217	<u> </u>	0.411
······································			- *		#12		#14
	#8	<u></u> #9					B-3 785
Sample I D.	B-3 53.5-	55	B-3 635-6	క్	B-3 68.5-	<u> 0</u>	484.54
Wet Soil + Tare	491.26		325,90		447,17	[1
Dry Soils + Tare	437,48		286.53		399,80		438.17
Tare # & Weight	L 204.25		B 146.07		5123 (47,55	• • •	AE 204.20
Weight of Water	53.78		39.37		47,97		46.37
Weight of Dry Soils	233,23		140,46		252.25		233.91
Moisture Content	0.231		0.280		0-190	····	0.198
· · · · · · · · · · · · · · · · · · ·	1		#17	#18	, #19	#20	#21
	#15	#15		#10			
Sample I.D.		······································				+	
Vet Soil + Tare							
Dry Soils + Tare						·	
are#&Weight			<u> </u>				· ·
Veight of Water					· · · · · · · · · · · · · · · · · · ·	<u>!</u>	
Veight of Dry Soils						· · · · ·	•
ioisture Content				· · · · · · · · · · · · · · · · · · ·	<u><u></u></u>	•	
· (<u>400 </u>	#23	#24	<u>#</u> 25	#26	#27	#28
	#22				1		, , ,
ample I.D.	<u> </u>			ł	1 1		
et Soil + Tare	[
y Soils + Tare							
are # & Weight	<u> </u>						····;
eight of Water	[
eight of Dry Soils			<u>;</u>			· · ·	1
bisture Content			<u>l. </u>		!		
Project: Cup & Wet Soil Cup & Dry Soil Client: F&R Project No.: Item Code/Use: Cup Weight Cup & Dry Soil are No. are No. loisture % up Weight up & Wet Soi loisture Loss oisture Loss SMO y Soil oisture % Soil Laboratory Technician: D. Durham Reviewed By: <u>ß</u> 3.19 23.14 37.31 23.03 Ő 34.12 0. 9 い、や 29.47 0 28,44 SCD0ŝ, 34 10,98 BLUFF RD. 395 Program Administrator 22,99 0.305 28.31 29,32 ده د د د 34,24 11:15 26 40 Þ Azumah 1 - 19 ы К 23.42 33.91 52,0) 0.318 œ 36 Froehling & Robertson, Inc. Laboratory Sample Analyses 0.17 11 と Sample No: 21-1-1-2 Location: Material Source: Water Content 22.0 20.0 34.0 32.0 28.0 26.0 24.0 38.0 36.0 40.0 21 6 38:5-40 USCS = 모 무 대 대 대 Number of Blows Date Processed: Date Received: 10/22/ 100

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Program Administrator

USCS =

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29 = (52)

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22 II 22 II 24

Laboratory Technician: Reviewed By:



Client:

ScDet BLUFF

> Froehling & Robertson, Inc. Laboratory Sample Analyses

> > SP4

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Project:

F&R Project No.:

Item Code/Use:



Client: Project: Cup & Wet Soil Cup & Dry Soil F&R Project No.: Blows Moisture % Item Code/Use: Cup & Wet Soil Cup & Dry Soil Tare No. vloisture % *loisture* Loss Dry Soil are No. iry Soil up Weight <u>up Weight</u> loisture Loss Laboratory Technician: Reviewed By: SCIOT 28,13 0.48 23,25 0.273 8.01 BLUFF 23.05 51,06 351 ر. در 29.40 . 200 D 3 <u>م</u> astic Lim O. Flow Program Administrator 29,75 0.516 34.73 <u>רי י</u> גיני 1.42 23.15 0.2695 Ó M 7.64 Ø 26 0 Ę م 4 34.13 1620 30, 29 23.07 1,22 N 0c Froehling & Robertson, Inc. Laboratory Sample Analyses Sample No: Location: Material Source: Water Content 22.0 20.0 24.0 36.032.032.032.030.026.040.0 3-6 W 33.5-35.2 USCS = ק ק וווו F Number of Blows Date Processed: Date Received: 10/22/15

100

R2=70 17

52 - 10

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Tare No. Cup & Wet Soil Cup & Dry Soil Moisture Loss Client: Tare No. Cup & Wet Soil Cup & Dry Soil F&R Project No.: Blows Moisture % Project: Dry Soil Item Code/Use: Cup Weight Dry Soil up Weight loisture % loisture Loss Laboratory Technician: Reviewed By: S N N r crr Plastic Linit Liquid Link Program Administrator B ١ TT-S Froehling & Robertson, Inc. Laboratory Sample Analyses Sample No: Location: Materiai <u>·</u> Source: Water Content $\begin{array}{r} 40.0\\ 38.0\\ 34.0\\ 28.0\\ 28.0\\ 24.0\\ 22.0\\ 22.0\\ 22.0\\ 22.0\\ 20.0\\ \end{array}$ ŝ 6 J 63.5-65.0 USCS = ٦ ۲ ם וו F Number of Blows Date Processed: Date Received: 10/22/15 100

CIAN
DATE

A В

D. Е

(B-D)/B x 100=

29.7%



WASH 200 (C 117)

CLIENT	SCDOT		R-
PROJECT	BLUFF	RD.	
RECORD NO.			
			COTI

TECHNICIAN_ TEST DATE _____ 18.5-20

SOIL CLASSIFICATION

TARE # X D

TARE WEIGHT 306,18

A Tare and Dry Soil

B Dry Soil

C Tare and Dry Soil After Wash

D. Dry Soil After Wash

E Material Lost

626.10 319.92 613.61 307.43 12.49

Percent Passing #200

(B-D)/B x 100=

(A-Tare)

(C-Tare)

(B-D)

3.90%



WASH 200 (C 117)

	LDOT	B-1	TECHNICIAN	
PROJECT <u>B</u> RECORD NO.	-UFF RD	- 28.5-30	TEST DATE	
		SOIL CLASSIFICAT	TION	
	<u>.</u> C	TARE WEIGHT 3	74.60	
A Tare and Dry Soil		781,62		
B Dry Soil	(A-Tare)	407.02		
C Tare and Dry Soil Afte	r Wash	723,87		
D, Dry Soil After Wash	(C-Tare)	349.27		
E Material Lost	(B-D)	57.75		

Percent Passing #200 (B-D)/B x 100=

14,2%





WASH 200 (C 117)

CLIENT	SCDOT	-
PROJECT	BLUFF	RD,
RECORD NO.		

B-1 - 48.5-50

TECHNICIAN TEST DATE _____

TARE # NI

SOIL CLASSIFICATION

TARE WEIGHT 327.23

A Tare and Dry Soil

- B Dry Soil
- C Tare and Dry Soil After Wash
- D Dry Soil After Wash
- E Material Lost

614.08
286.85
493,47
166.24
120.61

Percent Passing #200

(B-D)/B x 100=

(A-Tare)

(C-Tare)

(B-D)

42.0%

· .	WAS	<u>б</u> н 200 (С	2 117)			
CLIENT SCOOT PROJECT BLUFF RECORD NO.		B-1 - 58,5-0 - soil classif			· · · ·	
TARE # <u>A 3</u> A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash	 (A-Tare)	TARE WEIGHT _	76	<u>}</u>		
D _. Dry Soil After Wash E Material Lost	(C-Tare) (B-D)	•	4			
Percent Passing #200	(B-D)/B x 1	.00≕ _	10.2	%	·	
					·	i i

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		F&R			
	WAS	H 200 (C	117)		
CLIENT SCOOT		B-1	TEC		
PROJECT BLUFF R RECORD NO.		68.5-	TES	ST DATE	
	······································	SOIL CLASSIFI	CATION		
TARE # D3		TARE WEIGHT	250.07		
A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost	(A-Tare) (C-Tare) (B-D)	533, 8: 283.76 483, 69 233, 61 50,15			· ·
Percent Passing #200	(B-D)/B x 10	0⇔	17.7	0/0	
					······
			•		
				•	
· · · · · · · · · · · · · · · · · · ·					

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		•		
		F&R		
	WAS	SH 200 (C 11	L7)	
CLIENT SCDOT	-	B-1	. TECHNICIAN	
PROJECT BLUFF R RECORD NO.		- 73.5-75		
		- SOIL CLASSIFICATI	ON	,
TARE #		TARE WEIGHT 24	18.06	
		544.81		
A Tare and Dry Soil B Dry Soil	(A-Tare)	296,75		
C Tare and Dry Soil After Wash	(C-Tare)	464,85 216.79		
E Material Lost	(B-D)	79.94		
		-		
			7 6 0 0/-	
Percent Passing #200	$(B-D)/B \times 1$	00=	26.9%	

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	WA	<u>Е</u> SH 200 (С 117	· · · · · · · · · · · · · · · · · · ·	
CLIENT SCOOT PROJECT BLUFF F		_ B-2 _ 13.5-15.0	TECHNICIAN	-
TARE # B		SOIL CLASSIFICATION	85	
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	741.92 344.07 678.97 281.12 62.95	 	

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φ,

(B-D)/B x 100=

18.3%

	WA	F SH 200 (C 117))
CLIENT <u>SCDOT</u> PROJECT <u>BLVFF</u> RECORD NO.		B-2 23.5-25.0 SOIL CLASSIFICATION	TECHNICIAN TEST DATE
TARE #AA	_	TARE WEIGHT 247.	.81
B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash	(A-Tare) (C-Tare)	180,45 274,51 26.70	
E Material Lost	(B-D)	153.75	

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(B-D)/B x 100=

85.2%

· · · · · · · · · · · · · · · · · · ·	WAS	<u>Гер</u> 5н 200 (с 117)	
	20,	B-3 (8.5-20 SOIL CLASSIFICATION TARE WEIGHT 442,	теснисіан тезт date
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	699, 43 256, 99 680, 39 237, 95 19.04	

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400 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100 - 100

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(B-D)/B x 100=

7.4%

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			· 4 4 7)	
	WAS	H 200 (0)	•
CLIENT SCOOT		B-3		TECHNICIAN
PROJECT BLUFF F	D.	28.5-	30	TEST DATE
RECORD NO		SOIL CLASSI	FICATION	L
TARE # 1.18		TARE WEIGHT	536,	27
A Tare and Dry Soil		757, 2	36	_
3 Dry Soil	(A-Tare)	221.59		_
	(C-Tare)			_
E Material Lost	(B-D)	19.51	<i>v</i>	
are and Dry Soil After Wash ry Soil After Wash aterial Lost	(C-Tare) (B-D)	<u>738.35</u> <u>202.08</u> 19:51		
Percent Passing #200	(B-D)/B x 10)0=	8.1	8 % 0

1

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		F&R	
	WAS	SH 200 (C 11)	7)
CLIENT SCOOT PROJECT BLUFF F		B-3 - 33,5-35	TECHNICIAN
TARE #		SOIL CLASSIFICATION	v +.24
A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash	(A-Tare) (C-Tare)	437.91 133.67 338.07 33.83	
E Material Lost	(B-D)	99.84	

Percent Passing #200 (B-D)/B x 100=

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i ,

74.7 %

		F&R			
	WAS	SH 200 (0	C 117)		
CLIENT SCOOT		B-3	15 5	TECHNICIAN _	
PROJECT <u>BI-UFF</u> RECORD NO.	KU .			TEST DATE _	·
TARE #	_	SOIL CLASSI		86	
A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash	(A-Tare) (C-Tare)	696. 298.0 641. 74 2.43. e	62		
E Material Lost	(B-D)	54.72			
Percent Passing #200	(B-D)/B x	100=	18	3.3%	

,

		F&R	
	WAS	SH 200 (C 117))
CLIENT SCDOT PROJECT BLUFF RECORD NO.	RD,	β-3 - 53,5-55	TECHNICIAN
TARE # A	_	SOIL CLASSIFICATION	47
 A Tare and Dry Soll B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	621.99 233.52 577.03 188.56 44.96	·
Percent Passing #200	(B-D)/B x 2	100=	1.3%

	WA	EX SH 200 (C 117)	
CLIENT <u>SC DO</u> PROJECT <u>BLUFF</u> RECORD NO.		β-3 63.5-65.0 TEST DATE	
TARE # N2		SOIL CLASSIFICATION	
Tare and Dry Soil Dry Soil Tare and Dry Soil After Wash	(A-Tare)	$ \begin{array}{r} 460.68 \\ 140.74 \\ 422.31 \\ \end{array} $	
Dry Soil After Wash Material Lost	(C-Tare) (B-D)	<u> </u>	

E Material Lost

Percent Passing #200

А В С D_.

(B-D)/B x 100≕

27.3%

· · · · · · · · · · · · · · · · · · ·		FxR			
	WAS	SH 200 (C 117)	•	
CLIENT SCOOT PROJECT BLUFF I RECORD NO. TARE # 8A		B-3 - 68.5 soil class	- 7 D	TECHNICIAN TEST DATE	
A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost	(A-Tare) (C-Tare) (B-D)	590. 252.	22 49 43 76	-	
Percent Passing #200	(B-D)/B x 1	100=		0 %	

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B-3 78.5-80 SOIL CLASSIFICATION ARE WEIGHT _ 394.	TECHNICIAN TEST DATE
SOIL CLASSIFICATION	
	20
628.30 234.10 610.21 216.01	
18.09	_
	234.10 G10.21 216.01

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	F&R
	WASH 200 (C 117)
CLIENT <u>SCDOT</u> PROJECT <u>BLOFF P</u> RECORD NO TARE # <u>AB-4</u>	B-4 TECHNICIAN TEST DATE TEST DATE SOIL CLASSIFICATION TARE WEIGHT 172.80
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	$\begin{array}{r} 393,33 \\ (A-Tare) & 220,53 \\ \underline{351} & 68 \\ (C-Tare) & 178,88 \\ (B-D) & 41.65 \end{array}$
Percent Passing #200	(B-D)/B x 100= 18.9%

	WAS	F&R 5H 200 (C 117)	
CLIENT SCOOT		B-4	TECHNICIAN
PROJECT BLUFF R	5D •		TEST DATE
RECORD NO		23.5-25	
		SOIL CLASSIFICATION	<u> </u>
TARE # 176		TARE WEIGHT 178	1 2 <u>3</u>
A Tare and Dry Soil		373.54	_
B Dry Soil	(A-Tare)	<u>(95.2)</u> 353,11	
C Tare and Dry Soil After Wash D Dry Soil After Wash	(C-Tare)	174.88	_
E Material Lost	(B-D)	20.33	

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(B-D)/B x 100=

10.4%