S-21-57 (North Old River Road) Bridge Replacement over Barfield Mill Creek Florence County, South Carolina

November 13, 2015 Terracon Project No. 73155050F

## Prepared for:

South Carolina Department of Transportation Columbia, South Carolina

## Prepared by:

Terracon Consultants, Inc. Columbia, South Carolina

Offices Nationwide Employee-Owned Established in 1965 terracon.com





South Carolina Department of Transportation 955 Park Street, Room 421 Columbia, South Carolina 29201

Attn: Mr. Trapp Harris, P.E.

Geotechnical Design Engineer – Design-Build Section

Re: Preliminary Geotechnical Data Report

Emergency Bridge Package 3 -

S-21-57 (North Old River Road) Bridge Replacement over Barfield Mill Creek

Florence County, South Carolina Terracon Project Number: 73155050F

Dear Mr. Harris:

Terracon Consultants Inc. (Terracon) has completed the geotechnical exploration and testing services for the above referenced project. These services were conducted in general accordance with the SCDOT Request for Subsurface Exploration and Laboratory Testing dated November 5, 2015. This geotechnical data report presents the findings of the subsurface exploration and laboratory testing along with an overview of testing activities.

### 1.0 INTRODUCTION

The South Carolina Department of Transportation (SCDOT) has contracted Terracon to perform the subsurface exploration and laboratory testing for the Emergency Bridge Package 3 - S-21-57 (North Old River Road) Bridge Replacement over Barfield Mill Creek in Florence County, South Carolina. The purpose of this work is to develop information relative to subsurface soil and groundwater conditions at the bridge location. This report presents the results of that work. No geotechnical recommendations are associated with the requested scope of study.

The following sections of this report contain a summary of the activities our field exploration and laboratory testing. The logs of the borings and CPT soundings, the Site Location Map and the Exploration Plan are included in Appendix A of this report. The results of the laboratory testing performed on soil samples obtained from the site during the field exploration are included in Appendix B of this report. Descriptions of the field exploration and laboratory testing are included in their respective appendices.



Terracon Consultants, Inc. 521 Clemson Road Columbia, South Carolina 29229
P [803] 741 9000 F [803] 741 9900 terracon.com



### 2.0 PROJECT DESCRIPTION

The project site is located on S-21-57 (North Old River Road) in Florence County, SC. Site location and boring locations plans are presented in Appendix A of this report. It is our understanding that the project will include the demolition/removal of the existing bridge structure and the replacement with a new bridge on the existing or similar horizontal alignment. The existing bridge is a 3-span structure apparently supported by wooden piles. It crosses Barfield Mill Creek. At the time of the field exploration, the surface of the stream flow was about 13 feet below the existing bridge deck.

## 3.0 GEOTECHNICAL TESTING

Between November 6 and November 9, 2015, two (2) soil test borings (designated STB-1 and STB-2) and two cone penetration test soundings (designated CPT-1 and CPT-2) were performed at the bridge location. Borings STB-1 and STB-2 were performed approximately 10 feet to the north and south of the ends of the existing bridge, respectively, as shown on Exhibit A-2 in Appendix A. The CPT soundings were performed slightly further away in the adjacent lanes to the soil borings.

## 3.1 Field Exploration

Our field exploration at the site consisted of two (2) Standard Penetration Test (SPT) Borings (STB-1 and STB-2) and two cone penetration test (CPT) soundings (CPT-1 and CPT-2) at the general test locations provided to Terracon by the SCDOT. A description of our testing methods and graphical logs outlining the soil conditions at each test location are presented in Appendix A. The test locations were established in the field by Terracon and surveyed by Construction Support Services, LLC, after completion. At the time of our field exploration, the roadway in the area of the bridge was closed due to a washout of each of the approaches. The set up at each boring is provided in the photographs below.



Photo 1. Drill rig at STB-1 (S-21-57)



Photo 2. Drill rig at STB-2 (S-21-57)

EBP-3 - S-21-57 BRO Barfield Mill Creek Florence County, SC November 13, 2015 Terracon Project No. 73155050F



## 3.2 Laboratory Testing

The following laboratory tests were performed on the soil samples collected at the site.

- Twelve (12) Natural Moisture Content Tests (ASTM D2216)
- Three (3) Atterberg Limits Tests (ASTM D4318)
- Twelve (12) Wash #200 Tests

The laboratory procedures and results of the laboratory tests are presented in Appendix B.

## 4.0 CLOSURE

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or we may be of further service, please contact us.

Sincerely,

**Terracon Consultants, Inc.** 

Joseph D.M. Fredendall, E.I.T. Field Engineer

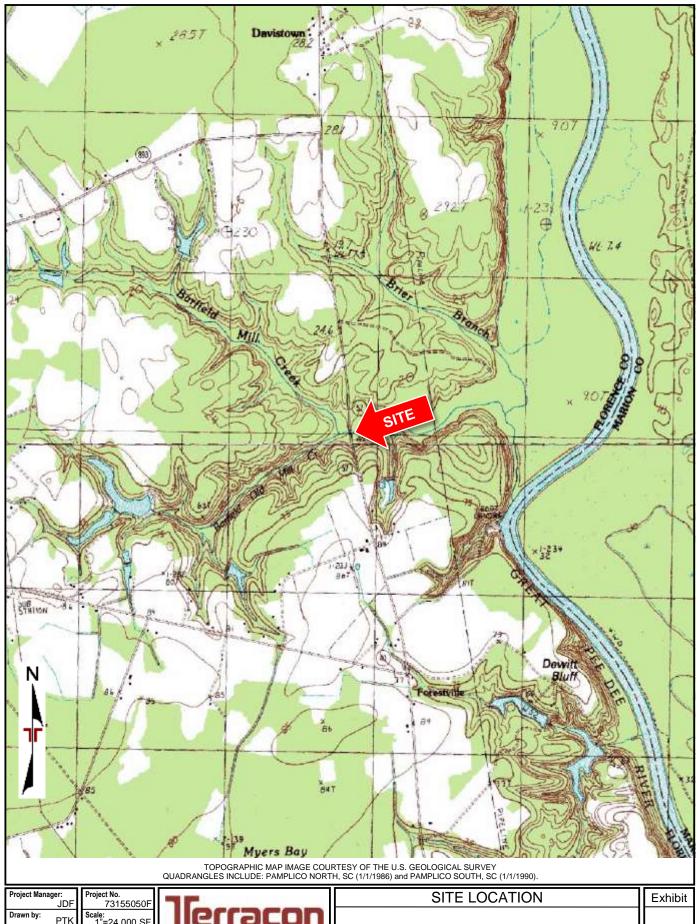
Phillip A. Morrison, P.E. Geotechnical Department Manager SC Registration No. 17275

Attachments

Copies: Addressee (1 via email) File (1)

## **APPENDIX A - FIELD EXPLORATION**

Exhibit A-1 – Site Location Map
Exhibit A-2 – Exploration Plan
Exhibit A-3 – Summary of Boring Data
Exhibit A-4 – Field Testing Description
Exhibit A-5 – Soil Description Terms
Exhibits A-6 and A-7 - Boring Logs
Exhibits A-8 to A-11 – CPT Sounding Logs



Checked by: PAM Approved by: PAM

File Name: A-1 & A-2 Nov. 2015

521 Clemson Road Columbia, SC 29229 S-21-57 over Barfield Mill Creek Florence County, South Carolina

A-1



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

AERIAL PHOTOGRAPHY PROVIDED BY MICROSOFT BING MAPS

Project Manager: Drawn by: Checked by: PAM Approved by: PAM

Project No. 7<u>3155050F</u> ÄS SHOWN File Name: A-1 & A-2 Nov. 2015



## **EXPLORATION PLAN**

S-21-57 over Barfield Mill Creek Florence County, South Carolina

Exhibit

A-2

Preliminary Geotechnical Data Report
EBP-3 - S-21-57 BRO Barfield Mill Creek■ Florence County, SC November 13, 2015 Terracon Project No. 73155050F



## **Summary of Boring Data**

Boring No.	Ground Elevation (ft.)	Test Depth (ft.)	Northing	Easting	Latitude	Longitude
STB-1	43.5	100	791828.8	2445714.1	34.0009704	-79.5291347
STB-2	44.1	100	791754.1	2445714.0	34.0007651	-79.5291388
CPT-1	43.7	33.1	791841.0	2445702.7	34.0010045	-79.5291719
CPT-2	44.2	30.7	791742.5	2445724.4	34.0007331	-79.5291048

EBP-3 - S-21-57 BRO Barfield Mill Creek■ Florence County, SC November 13, 2015 ■ Terracon Project No. 73155050F



#### FIELD EXPLORATION DESCRIPTION

#### Overview

The testing locations were provided by the SCDOT and located in the field by Terracon by taking measurements from the existing bridge. The borings were surveyed by Construction Support Services, LLC after drilling was complete. The locations are shown on the Exploration Plan (Exhibit A-2).

A field log of each test location was prepared by our field engineer. The final boring logs included with this report represent the engineer's description of the encountered conditions modified as necessary based on laboratory test results of the individual samples.

#### **Soil Test Borings (STB)**

All boring and sampling operations were conducted in general accordance with the following procedures:

- SCDOT Geotechnical Design Manual 2010
- ASTM D5783, "Standard Guide for Use of Direct Rotary Drilling with Water-Based Drilling Fluid for Geo-environmental Exploration"
- ASTM D1586 "Test Method for Penetration Test and Split-Barrel Sampling of Soils"
- ASTM D4220 "Standard Practices for Preserving and Transporting Soil"

Each boring was advanced using rotary wash drilling techniques. Five samples were collected in the upper 10 feet. Below that depth, samples were obtained at 5-foot intervals. Soil samples were obtained with a standard 1.4-inch I.D., 2-inch O.D., split-barrel sampler, also known as a standard split-spoon. The sampler is advanced into the soil a total of 18 inches (24-inches in the upper 10 feet) by striking the drill rod using a 140-pound automatic hammer falling 30 inches. The number of blows required to advance the sampler for each of the three, 6-inch increments is recorded. A fourth reading was recorded in the upper 10 feet. The sum of the number of blows for the second and third increments is called the "Standard Penetration Value", or N-value (N<sub>meas</sub>, blows per foot). The N-value, when properly evaluated, is an index to the soil strength.

Soil Classification provides a general guide to the engineering properties of various soil types and enables the engineer to apply his experience to current situations. In our exploration, samples obtained during drilling operations are examined and visually classified by a geotechnical engineer using the procedures outlined in ASTM D2487 - Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System). Laboratory testing was also performed on select split-spoon samples to evaluate index properties for further classification. The soils are described according to color, texture, and relative density or consistency (based on standard penetration resistance). An explanation of the soil descriptions shown on the logs is provided on Exhibit A-5.

EBP-3 - S-21-57 BRO Barfield Mill Creek■ Florence County, SC November 13, 2015 ■ Terracon Project No. 73155050F



Due to the drilling method, time-of-drilling water levels could not be recorded as it as well as the rock coring introduces water into the borehole. The 24-hour groundwater readings were collected from the borings, where possible. These are indicated on the boring logs. At the conclusion of the work, the boreholes were backfilled and capped with cold-patch asphalt.

### **Cone Penetration Test Soundings (CPT)**

Cone Penetration Test soundings were conducted in general accordance with ASTM D5778 Standard Test Method for Performing Electronic Friction Cone and Piezocone Penetration Testing of Soils.

### **SOIL DESCRIPTION TERMS**

#### **Relative Density/Consistency Terms**

Relative Density <sup>1</sup>			Consistency <sup>2</sup>		
				Unconfined	SPT Blow
Descriptive Term	Relative Density	SPT Blow Count	Descriptive Term	Compression	Count
				Strength (q <sub>u</sub> ) (tsf)	
Very Loose	0 to 15%	4 and less	Very Soft	0.25 and less	2 and less
Loose	16 to 35%	5 to 10	Soft	0.26 to 0.50	3 to 4
Medium Dense	36 to 65%	11 to 30	Firm	0.51 to 1.00	5 to 8
Dense	66 to 85%	31 to 50	Stiff	1.01 to 2.00	9 to 15
Very Dense	86 to 100%	51 and more	Very Stiff	2.01 to 4.00	16 to 30
			Hard	4.01 and more	31 and more

#### **Moisture Condition**

Descriptive Term Criteria

Dry Absence of moisture, dusty, dry to the touch

Moist Damp but no visible water

Wet Visible free water, usually in coarse-grained soils below the water table

#### Color

Describe the sample color while sample is still moist.

#### Angularity<sup>1</sup>

Descriptive Term Criteria

Angular Particles have sharp edges and relatively plane sides with unpolished surfaces.

Subangular Particles are similar to angular description but have rounded edges.

Subrounded Particles have nearly plane sides but have well-rounded corners and edges.

Rounded Particles have smoothly curved sides and no edges.

#### **HCI Reaction<sup>3</sup>**

<u>Descriptive Term</u> <u>Criteria</u>

None Reactive No visible reaction

Weakly Reactive Some reaction, with bubbles forming slowly Strongly Reactive Violent reaction, with bubbles forming immediately

#### Cementation<sup>3</sup>

Descriptive Term Criteria

Weakly Cemented Crumbles or breaks with handling or little finger pressure
Moderately Cemented Crumbles or breaks with considerable finger pressure
Strongly Cemented Will not crumble or break with finger pressure

#### Particle-Size Range<sup>1</sup>

Gravel	Diameter, mm	Sieve Size	<u>Sand</u>	Diameter, mm	Sieve Size
Fine	4.76 to 19.1	#4 to ¾ inch	Fine	0.074 to 0.42	#200 to #40
Coarse	19.1 to 76.2	3/4 inch to 3 inch	Medium	0.42 to 2.00	#40 to #10
			Coarse	4.00 to 4.76	#10 to #4

### Primary Soil Type<sup>1, 2</sup>

The primary soil type will be shown in all capital letters.

#### **USCS Soil Designation**

Indicate USCS soil designation as defined in ASTM D-2487 and D-2488

#### **AASHTO Soil Designation**

Indicate AASHTO soil designation as defined in AASHTO M-145 and ASTM D-3282

<sup>&</sup>lt;sup>1</sup> Applies to coarse-grained soils (major portion retained on No. 200 sieve)

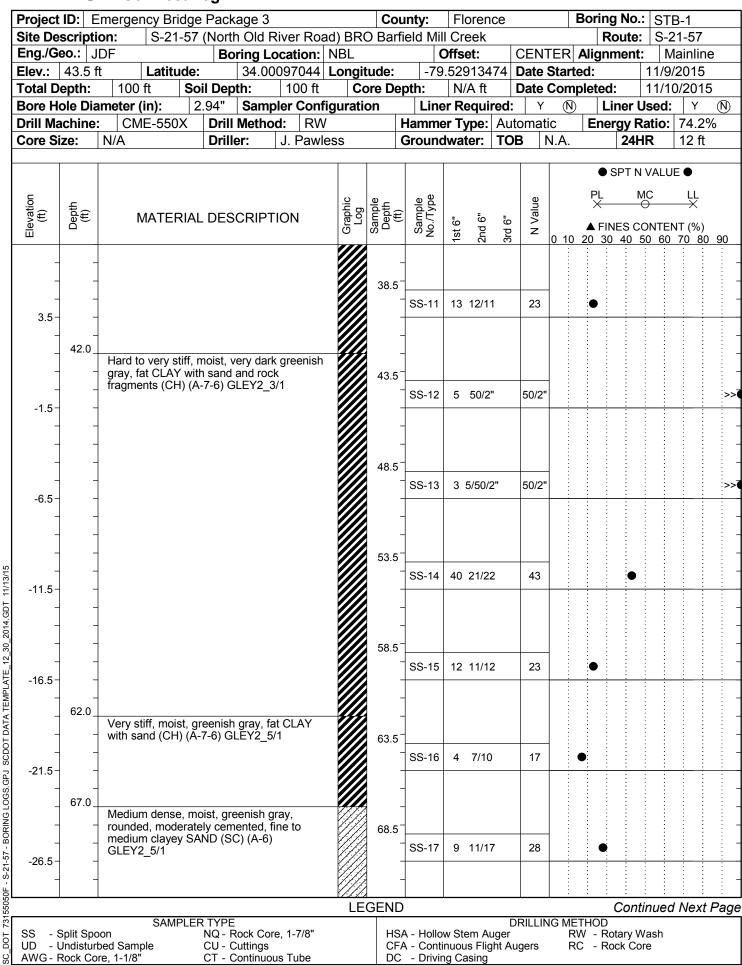
<sup>&</sup>lt;sup>2</sup> Applies to fine-grained soils (major portion passing No. 200 sieve)

<sup>&</sup>lt;sup>3</sup>Use as required



		mergency Bridge			=		unty:	Flore				В			: ST		
	scripti		(North Old			Bar								oute:		21-57	
	eo.: J		Boring					Offset:		CEN				nent		<u>lainli</u>	
lev.:					4 Longitu			.52913		Date					11/9		
otal D	•		il Depth:	100			epth:	N/A ft		Date					11/1		
		. ,		-	nfiguratio			r Requ	_			<u>D</u>			Jsed:		(
	achine:		Drill Meth		:W		Hamme			toma			Ene		Ratio:	+	
Core Si	ize:	N/A	Driller:	J. Pa	wless		Ground	lwater:	TO	В	N.A			24F	łR	12	ft
													• 9	SPT N	I VALU	JE $lacktrian$	
Elevation (ft)	Depth (ft)	MATERIAL	. DESCRIP	PTION	Graphic Log	Sample Depth (#)	Sample No./Type	1st 6" 2nd 6"	3rd 6"	N Value			PL ×		MC ⊖ ONTE	LL X	.)
ⅲ	0.0	Roadway			0 0	<i>J</i> ) —	o ž	1st 2nd	3rd		0 1				50 60		
	0.1	Asphalt (2 inches)				0.0	00.4	F 0//	. 40	1 47				:			:
7	0.4	Aggregate Base Co	ourse (3 inch	es)	/ <b>XXX</b>	2.0	SS-1	5 8/9	9 10	17		•	:	:		Ė	:
_	4.0	FILL -Medium dens angular, weakly cel silty SAND (SM) (A	mented, fine	to mediur	n 🔛	4.0	- SS-2	10 10/	10 15	20							
38.5-	6.0	%#200=16.2, NMC	=12.4			6.0	- SS-3	5 9/1	1 14	20		<b>4</b> ○(	-	:			:
_	8.0	angular, weakly cell well graded SAND fragments (SW) (A	mented, fine with silt and	to mediur asphalt	n [	8.0	- SS-4	4 3/3	3 4	6	•	C	<b>A</b>				
-		%#200=11.8, NMC	C=15.1			3.3		WOH WO	DH/1 2	2 1	• 4	•	) :			:	
33.5 - - -	12.0	FILL -Loose, dry, y angular, weakly ce silty SAND with asp (A-2-4) 5YR5/6 to 7	mented, fine phalt fragmei 7.5YR4/1	to mediur	n   No.		-										
_		%#200=25.2, NMC		red		13.5	SS-6	3 2/	1	3	<b>*</b>		0				
28.5 - - -	17.0	angular, weakly ce well graded SAND fragments (SW) (A	mented, fine with silt and -2-4) 5YR5/6	to mediur asphalt	n												:
_		%#200=10.1, NMC				18.5	_						:	:		:	:
23.5	_	Very loose, dry, gra with silt (A-3) 7.5YF LL=NP, PL=NP, PI	R3/1				SS-7	3 5/4	1	9	•	•					
_	22.0	NMC=28.3  Loose, moist, green weakly cemented, to	fine to mediu	ım clayey	, <b>////</b>	00.5											
_		SAND (SC) (A-2-4)	) GLEY2_5/1			23.5	SS-8	6 7/9	 a	16		•	X		<u> </u>		•
18.5	-	Very stiff, moist, gr with sand (CH) (A- LL=63, PL=22, PI= NMC=33.4	7-6) GLEY2_	_5/1					-	10				:			: <b>-</b>
-	-					28.5	- - SS-9	7 8/1	1	19							
13.5- -							-					_		:		:	
_	32.0	Very stiff, moist, ve fat CLAY with sand GLEY2_3/1				33.5											
8.5-	]	_					SS-10	7 11/	13	24			•	<u>:</u>	<u> </u>		:
-						) ENIT							:	Cont	inus:	: 1 N/a:	: /+ D
					LEC	SENE	,							cont	лиес	d Ne	KL P





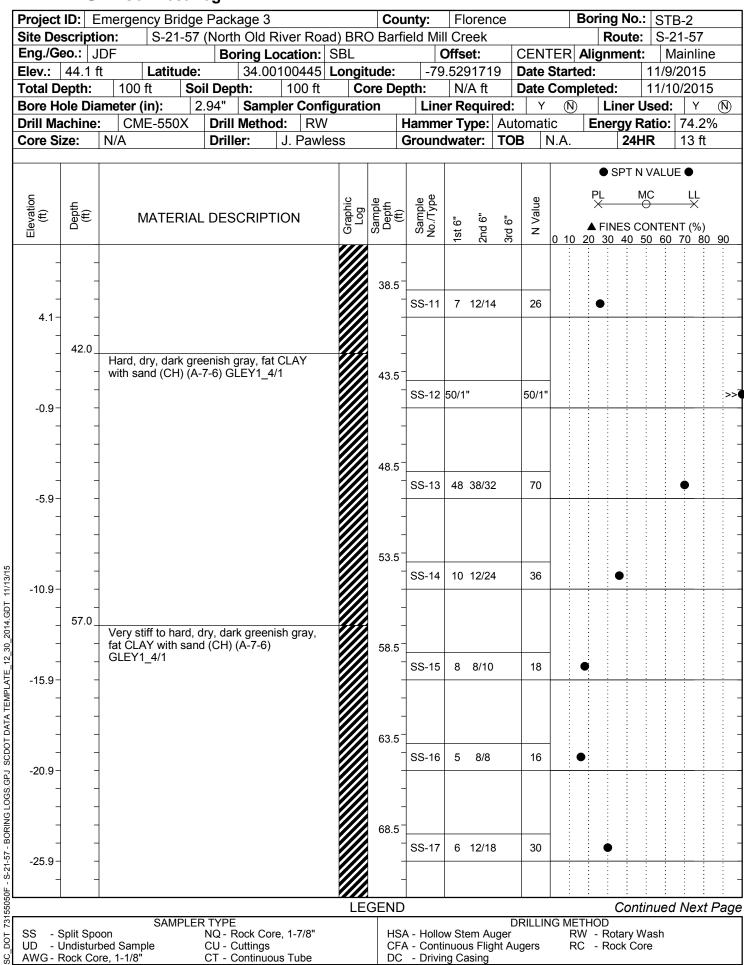


roject	ID:	mer	gency E	<u> Bridg</u> e	e Pa	ckag	e 3				Co	unty:	F	loren	ice			Во	ring	No.:	STI	B-1_	
ite De	scripti	on:	S-2	21-57	(No	rth C	ld F	River	Roa	d) BR	O Bar	field Mi	ill Cı	reek						ute:		21-57	
	eo.: J				_				on:				Offs			CEN	TER	Al	ignn	nent:	M	lainlin	e
lev.:	43.51	t	Lati	tude:			_			Longi	tude:	-79	9.52	9134	174	Date						2015	
otal D	epth:	10	00 ft			epth:		_	0 ft		ore De		_	/A ft		Date						0/201	
	ole Dia				2.94"					gurati				equi	red:		(N				lsed:	Y	(1)
	achine		ME-55		_	ill Me			RW	J		Hamm			_	tomat						74.2	$\overline{}$
ore S		N/A				iller:		_	Pawle	SS		Groun			TO		N.A.			24H		12 ft	
								1															
																			• s	PT N	VALU	E●	
_																			PL ×—	M	IC	LL	
Elevation (ft)	Depth (ft)		N 4 A T F				דחו	TION!		Graphic Log	Sample Depth	Sample No./Type				N Value			×—			—X	
je E	D D		MATE	EKIAL	LDE	:5CF	KIPI	ION		Gra	San	San No./	1st 6"	2nd 6"	3rd 6"							NT (%)	
ш	_									77.77			18	7	<u>ب</u>		0 1	0 20	30	40 5	0 60	70 80	) 9
_	-										73.5							:	:	:			:
_	-										1 0.0	SS-18	11	9/18	3	27	] :	:		:	:		:
-31.5-	-											100-10	+ ''	5, 10			<del> </del>	- :	<u> </u>	<u>:</u>	:		
-	-											_						:	:	:			
_	77.0_										1	_						:	:				
_	-		y dense, derately d						iaed,		78.5							:	:	:	: :		
_	-		ey SANI								10.5	SS-19	1	11/50	/4"	50/4'	; ;	:	:	:		: :	
-36.5	-											33-19	9	11/50	/4	30/4	<u> </u>		:	:		<u> </u>	
_	_											_						:	:	:			
_	82.0_											_						:	:	:			
_	_		d, moist, d (CH) ( <i>A</i>					LAY v	vith			_						:	:	:			
_	_	Sail	u (OII) ( <i>F</i>	(-1-U)	OLE	12_3/					83.5		+	4011		+	1 :	:	:	:			
-41.5-	_											SS-20	9	12/4	/	59		<u> </u>	<u>:</u>	:	•		
71.5																		:	:				
	87.0_																	:	:	:	: :		
_		Ver	y dense,	moist.	gree	enish	gray	, rour	ided,									:	:	:			
_	_		ikly ceme 2-4) GLE			SIILY S	MINL	אופ) ר	)		88.5		+				-	i	:	:			
-46.5	_	, -	,									SS-21	50/2	2"		50/2'	<u>'</u>	:	:	<u>:</u>	<u> </u>	<u> </u>	:
- <del></del> 0.5	_									144	:							:	:				
_	92.0																	:	:	:	:		:
_	]	Ver	y dense,	moist	gree	enish	gray	, rour	ided,		]	1						:	:	:			
	] -	wea (SM	ikly ceme I-SC) (A-	ented, -2-4) (-	tine : SLEY	siity c 2 6/1	aye	y SAN	טא		93.5		+				-	:	:	:			
-	] -	,	/ (* *	٠, ح							:	SS-22	50/1	1"		50/1'		:	:	:			
-51.5	1 -																	:	:	:			
_	97.0																	i	:	:	: :		
_	]	No	Recovery	у						1-1-1-	1							:	:	:	:		:
_	1 -										98.5	1	-					:	:				
	100.0											SS-23	50/5	5"		50/5'	'  :	:	:	:			
-56.5	1 .30.0_	Bor	ing Tern	ninate	ed at	100 fe	eet				1							i	:	:			
-	1 -											1						:	:	:			
_	1 -											1						:	:	:			
_	1 -											1						:	:	:			
_	1 -											1						:	:	:			
-61.5	-											1						÷	:	:			
_	-											-					:	i	:	:			
-	-											-						:	:	:			
	<u> </u>									IF	GENI	<u> </u>					1 :	•	•	•	•	. :	
			SAI	MPLEF												RILLII	NG M	ETH	OD				_
SS - S JD - 1	Split Spo Undistur	on C	amela			- Rock		re, 1-7	7/8"			SA - Hollo				Idore				ary W			
		re, 1-1				<ul><li>Cutti</li><li>Cont</li></ul>		<b></b>	<b>.</b> .			A - Con C - Drivi			JIIL A	ayers		1	- 100	V C01	C		



	mergency Bridge					unty:		ence				g No.:			
					Barl								_		
															<u>e_</u>
epth:	100 ft <b>So</b>	il Depth:	100 ft	Cor	re De	epth:	N/A	ft	Date	Comp	oleted	l: 1	1/10	/2015	5
ole Dia	meter (in): 2	2.94" <b>San</b>	npler Confi	iguratio	n	Line	r Req	uired	: Y	N	L	iner Us	ed:	Υ	(
chine:	CME-550X	Drill Meth	od: RW			Hamme	r Typ	e: Au	toma	tic	Ene	rgy Rat	tio:	74.29	%
ze:	N/A	Driller:	J. Pawle	ess		Ground	lwate	r: TC	В	N.A.		24HR	{   ·	13 ft	
											•	SPT N V	ALUE	•	
Depth (ft)		L DESCRIF	PTION	Graphic Log	Sample Depth (ff)	Sample No./Type	st 6"	nd 6" rd 6"	N Value	0.40		NES CON	NTEN <sup>-</sup>		
					0.0		_	<u>α</u> α		0 10	20 30	40 50	60 /	70 80	
0.4	, , ,	ouroo /2 inch	00)	// <b>XXX</b>	0.5		44 4	0/7	40	] :		: :	:	: :	
2.0	7 60 0	,		J <b>XXX</b>	2.0	55-1	11 1	211	19	4 !	•		:		
4				1144		- SS-2	10 9	9/9 1	18	7 :			:		
4	to medium silty SA				4.0					4 !			:		
	2.5YR3/3					SS-3	2 4	1/5 7	9		ə <u>:                                      </u>			<u>: :</u>	
6.0	%#200=16.8, NMC	C=13.6			6.0		_			<b>↓</b> ••••			:	: :	
	Medium dense to l	loose, dry, br	own,				4	1/2 2	6		, ! !		:		
8.0	subrounded, weak	ly cemented,	fine to		8.0		<del>-</del> -	T/				: :	:		
	1		I) 7.5YR5/3		5			211/24:2:		1 :			:		
Ī	%#200=18.0, NMC	7=14.7				¬ SS-5 ₩	VOH WO	JH/WOH	1 0	<b>T</b>	<b>A</b> Q		:		
7	%#200=14.2, NMC	C=17.4				1				1 :	: :		<del>-</del> :		_
12.0	cemented, fine to r SAND with silt(SW	medium well /) (A-2-4) 7.5	graded		13.5	- - -									
4	%#200=10.8, NMC	C=14.5			10.0		1 1	1 /1	7						
_	cemented, fine to r					- 33-0	1	1/ 1							
17.0	Γ΄ ΄	C=29.4				-				:	: :		:		
4			-194.		18 5	<u> </u>							:		
4	very loose, moist,	greenish blad cs. (SM) (A-2	ck, silty -4)		10.5		1 1	2/2	6	<b> </b> _ :			:		
4	GLEY1_2.5/1	, (•) (, ( 2	• /			33-1	1 3	) J	<u> Р</u>		<u> </u>				
		I=NP, %#200	=15.0,			_				:			:		
	NMC=34.1					_							i		
						]				:			Ė		
		nd roots (CH)	(A-7-6)		23.5					<b>∃</b>	_		:		
7	GLL 12_3/1					SS-8	6 8	3/9	17		•		Ė	: :	
7													:		
1						1							:	: :	
7						┦							:		
7					28.5	1				-		: :	Ė		
7						SS-9	5 8	/13	21		•		:		
=						+ -				1 :	: :	<del>: :</del>	<del>-</del> :-	: :	
													:		
32.0	Hard to very stiff	drv. dark gree	nish grav									: :	:		
-	fat CLAY with sand				33.5								:		
4	GLEY1_4/1						13 50	0/4"	50/4	<b>.</b>			:		
4						30-10	.0 0	<b>-</b> 1 -1	30/4	+ :-	<u> </u>			<u>: :</u>	_
										:	: :	<u>: :</u>	:	: :	_
				LEG	ENE	)						Contin	ued	Next	P
		R TYPE NQ - Rock C	4 7/0"		,,,	A - Hollo	04		JRILLI			otary Was	_  _		
	scripti eo.: J 44.1 f epth: ole Dia achine: ze: 	scription: S-21-57 eo.: JDF  44.1 ft Latitude: epth: 100 ft Scoole Diameter (in): 2 chine: CME-550X ze: N/A  MATERIAL O.0 Roadway  0.1 Asphalt (2 inches) Aggregate Base Condum silty SA 2.5 YR3/3 %#200=16.8, NMC %#200=18.0, NMC %#200=14.2, NMC Loose, dry, brown cemented, fine to subrounded, weak medium silty SA 12.0 SAND with silt(SW %#200=10.8, NMC W#200=10.8, NMC Very loose, dry, brown cemented, fine to (A-2-4) 7.5 YR5/3 %#200=24.7, NMC Very loose, moist, SAND with organic GLEY1_2.5/1 LL=NP, PL=NP, PNMC=34.1  Firm to very stiff, or CLAY with sand and GLEY2_5/1  Hard to very stiff, or CLAY with sand and GLEY2_5/1  Hard to very stiff, or CLAY with sand and GLEY2_5/1	scription: S-21-57 (North Old eo.: JDF Boring 44.1 ft Latitude: 34.0 epth: 100 ft Soil Depth: ole Diameter (in): 2.94" San Inchine: CME-550X Drill Methods 2e: N/A Driller:  MATERIAL DESCRIP O.0 Roadway 0.1 Asphalt (2 inches) Aggregate Base Course (3 inches) Aggreg	Scription: S-21-57 (North Old River Roago): JDF Boring Location: 44.1 ft Latitude: 34.00100445 epth: 100 ft Soil Depth: 100 ft ole Diameter (in): 2.94" Sampler Confinchine: CME-550X Drill Method: RW Ze: N/A Driller: J. Pawle Dri	Scription:   S-21-57 (North Old River Road) BROBO:   JDF	Secription:   S-21-57 (North Old River Road) BRO Bar	Scription:   S-21-57 (North Old River Road) BRO Barfield Mile   Soi.   DF   Boring Location:   SSL   C   C   C   C   C   C   C   C   C	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Cree	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Creek	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) BRO Barfield Mill Creek   S-21-57 (North Old River Road) Broth Road River Road) Broth Road River Road Rive	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   Sept.   Sol.   JDF	Secription:   S-21-57 (North Old River Road)   BRO Barfield Mill Creek   Ralign	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   Route:	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   Route:   S-21 80e.   JDF   Boring Location:   SBL   Offset:   CENTER Alignment:   Market   Latitude:   34.00100445   Longitude:   79.5291719   Date Started:   1179/2   Date Completed:   1179/2   Date Completed:	Scription:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   Route:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   Route:   S-21-57 (North Old River Road) BRO Barfield Mill Creek   Road   R







		<b>3</b> 011														•		T			_
		mergency							ounty:		loren	ce			Bor			STE			
Site De			-21-57			River Ro		O Baı								Rou			1-57		
Eng./G	eo.: J	DF		Во	ring L	ocation:	: SBL			Offs	et:		CEN	TER	Alig	gnme	ent:	M	ainli	ne	
Elev.:	44.1 f	t La	titude:		34.0	0100445	Longi	tude:	-79	9.52	9171	9	Date	Start	ed:			11/9/	2015	5	
Total D	epth:	100 ft	Sc	oil Dep	th:	100 ft	C	ore D	epth:	N	/A ft		Date	Com	plet	ed:		11/10	0/201	15	
Bore H	ole Dia	meter (in)	: 2	2.94"	Sam	pler Cor	nfigurati	ion	Lin	er R	equi	red:	Y	(N)		Line	er U	sed:	Y	(Ñ	)
Drill Ma					Metho				Hamm					$\overline{}$				atio:	74 2	$\overline{}$	
Core S		N/A		Drille		J. Paw			Groun			TO		N.A.		_	24H		13 f		_
30.00		1477		<b>  -</b>		10.1 4.1			O. Guii	<u></u>				1.,				•		•	_
																● SP	TN	/ALUI	E •		_
Elevation (ft)	Depth (ft)	MA <sup>-</sup>	TERIAI	L DES	CRIP	TION	Graphic Log	Sample Depth	(π) Sample No./Type	9	9	9	Value		P >	•	M C	C ONTEN		`	
ă							٥	S	υŠ	1st 6"	2nd 6"	3rd 6"	z	0 10				0 60			_
_	1 1							73.5	57					:	:	:	: :	:	:		_
_	1 1								SS-18	8	14/1	3	30		÷	•		i	:		-
-30.9	1 1								+					<del>  :</del>	÷	:		<del>- :</del>	÷		_
-	-								4					:							-
_	-								4												-
_								70.5						:	:				:		_
_								78.5		1				1 :	:	:		:	:		
05.0									SS-19	10	15/2	5	40	:	:			:	:		
-35.9-	1 1													1		:		:			
-	1 1								-					:	:	:		:	:		-
-	1 1								+					:	÷	:		:	:		-
-								83.5													_
_								03.5		1		_		1 :	÷	i		÷	÷		_
40.0									SS-20	25	24/2	2	46	:	÷	:	•	i			
-40.9-	1 1													1	- :	:		:	-		
-	-								-					:	÷	:		i	:		-
-	87.0	\/am, dana	at a	dark ara	onioh d	~~~			4					:	:	:		:	:		-
_		Very dens rounded, v	e, wet, c veakly c	ark gre emente	enisn (	gray, silty SANI									:	:	: :	:	:		_
_		(SM) (A-2-	-4) GLE	Y1 4/1	u, iiiic	Only Or a vi		88.5						1 :	÷	:		i	:		
45.0		, , ,	•	_					SS-21	14	25/50	/3"	50/3"		÷	:	: :	÷	:	: :>	·>(
-45.9-	1 1													:	:	:		:	:		
-45.9 - -	1							:	-					:		:		į	:		-
-	92.0	No Recove	erv				- 1:1:1:1:	-	-					:	÷	:	: :	i	:		-
-		140 140004	Ci y					93.5	<u>-</u>					:	÷	:	: :	i	:		-
_								00.0	SS-22	F0/0	)!!		50/2"	1 :	÷			i			. 7
-50.9									33-22	50/2	<u>-</u>		50/2	:						: :>	->
-50.9																	: :				
_	1 1								7									:			_
-	1 1								1							:		:	:		-
_								98.5	5 -					] :	:	:		i	:	: :	-
-50.9									SS-23	50/2	2"		50/2"	:	:	:	: :	:	:	: :>	·>ī
-55.9	100.0	Davisas Ta		1 -4 40	0 f4				+					<u> </u>	<u>-</u> :-			_ <u>:</u>		<u> </u>	
_		Boring Te	rriinate	eu at 10	v reet				1					:	:	:		:			_
	]													:	÷	:	: :	i	:		_
	7													:	:			:			_
_	1 1								7					:		:		÷			-
-									1					:				÷			-
-60.9									+					<u> </u>	÷	:	: :	- :	<del>:</del>	: :	
-	-								4							:		:	:		_
-									_					:		:	: :	į			_
															<u>:</u>	:	<u>:                                    </u>	<u>:</u>	:	<u>:                                    </u>	
							LE	GEN	D												
_			AMPLE	R TYPE									RILLIN								_
SS - S	Split Spo					ore, 1-7/8"		H	SA - Hollo FA - Con	ow St	tem A	uger	10020			Rota					
UD - U AWG - F	Unaisturi Rock Co	ed Sample		CU - C		ous Tube		0	-A - Con C - Drivi	iii luol ina C	us FIIQ asina	jiit Al	ugers	K	· -	KUCK	Cor	<b>5</b>			
		-,, .		<u> </u>	J			1 5	١١٧١ ــ		ig										_

#### **CPT LOG NO. CPT-1** Page 1 of 1 PROJECT: S-21-57 (North Old River Road) RBO **CLIENT: SCDOT TEST LOCATION:** See Exhibit A-2 Barfield Mill Creek Columbia, SC Surface Elev.: 43.7 ft **SITE:** S-21-57 Latitude: 34.001° Station: SBL Offset: CENTER Florence County, South Carolina Longitude: -79.52917° **Hydrostatic Pressure** Material Description Tip Resistance, q<sub>t</sub> Sleeve Friction, f<sub>s</sub> Friction Ratio, F, Pore Pressure, u<sub>2</sub> Elev. Depth Normalized CPT (ft) (tsf) (tsf) (%) (tsf) Soil Behavior Type 270 360 10 22 12345678 40 35 30 25 20 15 CPT Refused at 33.1 Feet 10 5 0 -5 Sensitive, fine grained Organic soils - clay See Exhibit A-3 for description of field procedures. CPT sensor calibration reports available upon request. Clay - sitty clay to clay Silt mixtures - clayey silt to silty clay Sand mixtures - silty sand to sandy silt Sands - clean sand to silty sand Gravelly sand to dense sand See Appendix C for explanation of symbols and abbreviations. Very stiff sand to clayey sand Very stiff fine grained Probe no. 4526 with net area ratio of 0.83 WATER LEVEL OBSERVATION CPT Started: 11/7/2015 CPT Completed: 11/7/2015 U2 pore pressure transducer location Manufactured by Geotech A.B.; calibrated 11/12/2014 12 ft estimated water depth Rig: Pagani TG73-200 Operator: BR Tip and sleeve areas of 10 cm<sup>2</sup> and 150 cm<sup>2</sup> (used in normalizations and correlations; Ring friction reducer with O.D. of 1.875 in Project No.: 73155050F Exhibit: A-8 see Appendix C) Columbia, South Carolina

CPT LOGS.GPJ

73155050F - S-21-57

SEPARATED FROM ORIGINAL REPORT

#### **CPT LOG NO. CPT-2** Page 1 of 1 PROJECT: S-21-57 (North Old River Road) RBO **CLIENT: SCDOT TEST LOCATION:** See Exhibit A-2 Barfield Mill Creek Columbia, SC Surface Elev.: 44.2 ft **SITE:** S-21-57 Latitude: 34.00073° Station: NBL Offset: CENTER Florence County, South Carolina Longitude: -79.5291° **Hydrostatic Pressure** Material Description Tip Resistance, q<sub>t</sub> Sleeve Friction, f<sub>s</sub> Friction Ratio, F, Pore Pressure, u<sub>2</sub> Elev. Depth Normalized CPT (ft) (tsf) (tsf) (%) (tsf) Soil Behavior Type 270 360 10 22 12345678 40 35 30 25 20 15 CPT Refused at 30.7 Feet SEPARATED FROM ORIGINAL REPORT 5 0 -5 Sensitive, fine grained Organic soils - clay See Exhibit A-3 for description of field procedures. CPT sensor calibration reports available upon request. Clay - sitty clay to clay Silt mixtures - clayey silt to silty clay Sand mixtures - silty sand to sandy silt Sands - clean sand to silty sand Gravelly sand to dense sand See Appendix C for explanation of symbols and abbreviations. Very stiff sand to clayey sand Very stiff fine grained Probe no. 4526 with net area ratio of 0.83 CPT Completed: 11/7/2015 WATER LEVEL OBSERVATION CPT Started: 11/7/2015 U2 pore pressure transducer location Manufactured by Geotech A.B.; calibrated 11/12/2014 13 ft estimated water depth Rig: Pagani TG73-200 Operator: BR Tip and sleeve areas of 10 cm<sup>2</sup> and 150 cm<sup>2</sup> (used in normalizations and correlations; Ring friction reducer with O.D. of 1.875 in Project No.: 73155050F Exhibit: A-10 see Appendix C) Columbia, South Carolina

CPT LOGS.GPJ

73155050F - S-21-57

## **APPENDIX B - LABORATORY TESTING**

Exhibit B-1 – Laboratory Testing Description Exhibit B-2 – Summary of Laboratory Data Laboratory Data Sheets

EBP-3 - S-21-57 BRO Barfield Mill Creek Florence County, SC November 13, 2015 Terracon Project No. 73155050F



#### LABORATORY TESTING DESCRIPTION

The samples collected during the field exploration were taken to our laboratory for additional testing. The laboratory testing program was developed by the SCDOT. Using the provided testing program, the laboratory tests were conducted on selected soil samples from the borings and the bulk samples locations. The test results are presented in this appendix

The laboratory test results were used to confirm the soil descriptions presented on the boring logs in Appendix A. Laboratory tests were performed in general accordance with the applicable ASTM, AASHTO, SCDOT or other accepted standards.

Selected soil samples obtained from the site were tested for the following engineering properties:

Percent Fines

Atterberg Limits

Moisture Content Determination

AASHTO T11 (ASTM D1140) AASHTO T89/T90(ASTM D4318) AASHTO T265/(ASTM D2216)

## **Summary of Laboratory Results**

Sheet 1 of 1													
BORING ID	Depth	USCS Classification and Soil Description	Compressive Strength (tsf)	Liquid Limit	Plastic Limit	Plasticity Index	% <#200 Sieve	% Gravel	% Sand	% Silt	% Clay	Water Content (%)	Dry Density (pcf)
STB-1	2 - 4	SILTY SAND (SM)					16.2	0.0	0.0			12.4	
STB-1	4 - 6	SAND with silt (SW)					11.8	0.0	0.0			15.1	
STB-1	6 - 8	SILTY SAND (SM)					25.2	0.0	0.0			19.1	
STB-1	8 - 10	SAND with silt (SW)					10.1	0.0	0.0			19.9	
STB-1	13.5 - 15	SAND with silt (SW)		NP	NP	NP	9.7	0.0	0.0			28.3	
STB-1	23.5 - 25	FAT CLAY with SAND(CH)		63	22	41	83.9	0.0	0.0			33.4	
STB-2	0 - 2	SILTY SAND (SM)					16.8	0.0	0.0			13.6	
STB-2	2 - 4	SILTY SAND (SM)					18.0	0.0	0.0			14.7	
STB-2	4 - 6	SILTY SAND (SM)					14.2	0.0	0.0			17.4	
STB-2	6 - 8	SAND with silt (SW)					10.8	0.0	0.0			14.5	
STB-2	8 - 10	SILTY SAND (SM)					24.7	0.0	0.0			29.4	
STB-2 STB-2	13.5 - 15	SILTY SAND(SM)		NP	NP	NP	15.0	0.0	0.0			34.1	

PROJECT: S-21-57 BRO Barfield Mill Creek

SITE: S-21-57

Florence, South Carolina



PROJECT NUMBER: 73155050F

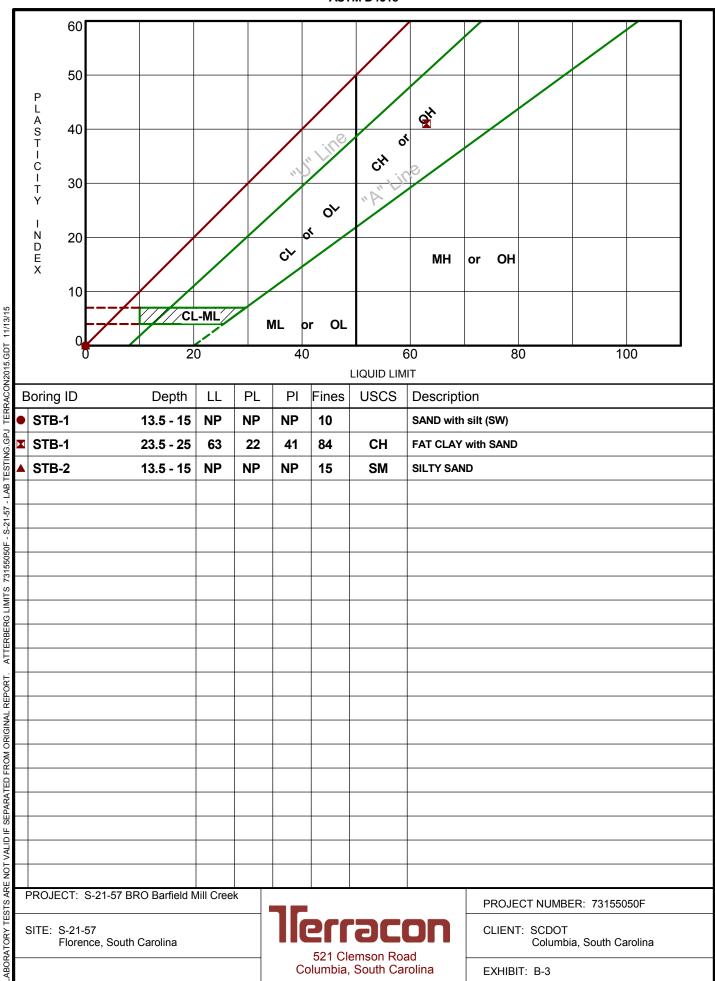
CLIENT: SCDOT

Columbia, South Carolina

EXHIBIT: B-2

## ATTERBERG LIMITS RESULTS

**ASTM D4318** 



## **APPENDIX C - SUPPORTING DOCUMENTS**

Exhibit C-1 – General Notes Exhibit C-2 – Unified Soil Classification System

## **GENERAL NOTES**

#### **DESCRIPTION OF SYMBOLS AND ABBREVIATIONS**

		$\square$		Water Initially Encountered		(HP)	Hand Penetrometer
	Auger	Split Spoon		Water Level After a Specified Period of Time		(T)	Torvane
NG	Challey Tube	Maara Cara	LEVEL	Water Level After a Specified Period of Time	ESTS	(b/f)	Standard Penetration Test (blows per foot)
IPLIN	Shelby Tube	Macro Core	<u>~</u>	Water levels indicated on the soil boring logs are the levels measured in the	D TE	(PID)	Photo-Ionization Detector
SAMPI	Ring Sampler	Rock Core	WATE	borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils,	FIEL	(OVA)	Organic Vapor Analyzer
	S. S.			accurate determination of groundwater levels is not possible with short term water level observations.			
	Grab Sample	No Recovery					

#### **DESCRIPTIVE SOIL CLASSIFICATION**

Soil classification is based on the Unified Soil Classification System. Coarse Grained Soils have more than 50% of their dry weight retained on a #200 sieve; their principal descriptors are: boulders, cobbles, gravel or sand. Fine Grained Soils have less than 50% of their dry weight retained on a #200 sieve; they are principally described as clays if they are plastic, and silts if they are slightly plastic or non-plastic. Major constituents may be added as modifiers and minor constituents may be added according to the relative proportions based on grain size. In addition to gradation, coarse-grained soils are defined on the basis of their in-place relative density and fine-grained soils on the basis of their consistency.

#### **LOCATION AND ELEVATION NOTES**

Unless otherwise noted, Latitude and Longitude are approximately determined using a hand-held GPS device. The accuracy of such devices is variable. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

	(More than Density determine	NSITY OF COARSE-GRAI n 50% retained on No. 200 ed by Standard Penetration des gravels, sands and sil	sieve.) on Resistance		CONSISTENCY OF FIN (50% or more passing t ency determined by laborato -manual procedures or star	he No. 200 sieve.) ory shear strength testing, t	
TERMS	Descriptive Term (Density)	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.	Descriptive Term (Consistency)	Unconfined Compressive Strength, Qu, psf	Standard Penetration or N-Value Blows/Ft.	Ring Sampler Blows/Ft.
뿔	Very Loose	0 - 3	0 - 6	Very Soft	less than 500	0 - 1	< 3
	Loose	4 - 9	7 - 18	Soft	500 to 1,000	2 - 4	3 - 4
TRENGT	Medium Dense	10 - 29	19 - 58	Medium-Stiff	1,000 to 2,000	4 - 8	5 - 9
ြင	Dense	30 - 50	59 - 98	Stiff	2,000 to 4,000	8 - 15	10 - 18
	Very Dense	> 50	<u>≥</u> 99	Very Stiff	4,000 to 8,000	15 - 30	19 - 42
				Hard	> 8,000	> 30	> 42

#### **RELATIVE PROPORTIONS OF SAND AND GRAVEL**

<u>Descriptive Term(s)</u> of other constituents	Percent of Dry Weight	<u>Major Component</u> <u>of Sample</u>	Particle Size
Trace	< 15	Boulders	Over 12 in. (300 mm)
With	15 - 29	Cobbles	12 in. to 3 in. (300mm to 75mm)
Modifier	> 30	Gravel	3 in. to #4 sieve (75mm to 4.75 mm)
		Sand	#4 to #200 sieve (4.75mm to 0.075mm
		Silt or Clay	Passing #200 sieve (0.075mm)

**GRAIN SIZE TERMINOLOGY** 

PLASTICITY DESCRIPTION

#### **RELATIVE PROPORTIONS OF FINES**

Descriptive Term(s) of other constituents	Percent of Dry Weight	Term	Plasticity Index
<u>or other constituents</u>	Diy Worgin	Non-plastic	0
Trace	< 5	Low	1 - 10
With	5 - 12	Medium	11 - 30
Modifier	> 12	High	> 30



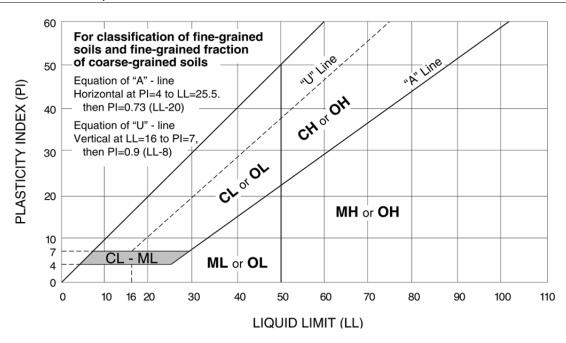
## UNIFIED SOIL CLASSIFICATION SYSTEM

				Soil Classification	
Criteria for Assign	ning Group Symbols	and Group Names	s Using Laboratory Tests A	Group Symbol	Group Name <sup>B</sup>
Coarse Grained Soils: More than 50% retained on No. 200 sieve	Gravels: More than 50% of coarse fraction retained on No. 4 sieve	Clean Gravels: Less than 5% fines <sup>C</sup>	Cu ≥ 4 and 1 ≤ Cc ≤ 3 <sup>E</sup>	GW	Well-graded gravel F
			Cu < 4 and/or 1 > Cc > 3 <sup>E</sup>	GP	Poorly graded gravel F
		Gravels with Fines: More than 12% fines <sup>C</sup>	Fines classify as ML or MH	GM	Silty gravel F,G,H
			Fines classify as CL or CH	GC	Clayey gravel F,G,H
	Sands: 50% or more of coarse fraction passes No. 4 sieve	Clean Sands: Less than 5% fines D	Cu ≥ 6 and 1 ≤ Cc ≤ 3 <sup>E</sup>	SW	Well-graded sand I
			Cu < 6 and/or 1 > Cc > 3 <sup>E</sup>	SP	Poorly graded sand I
		Sands with Fines: More than 12% fines D	Fines classify as ML or MH	SM	Silty sand G,H,I
			Fines classify as CL or CH	SC	Clayey sand G,H,I
Fine-Grained Soils: 50% or more passes the No. 200 sieve	Silts and Clays: Liquid limit less than 50	Inorganic:	PI > 7 and plots on or above "A" line J	CL	Lean clay K,L,M
			PI < 4 or plots below "A" line J	ML	Silt K,L,M
		Organic:	Liquid limit - oven dried	OL	Organic clay K,L,M,N
			Liquid limit - not dried < 0.75		Organic silt K,L,M,O
	Silts and Clays: Liquid limit 50 or more	Inorganic:	PI plots on or above "A" line	CH	Fat clay K,L,M
			PI plots below "A" line	MH	Elastic Silt K,L,M
		Organic:	Liquid limit - oven dried < 0.75	ОН	Organic clay K,L,M,P
			Liquid limit - not dried < 0.75		Organic silt K,L,M,Q
Highly organic soils:	Primarily organic matter, dark in color, and organic odor			PT	Peat

<sup>&</sup>lt;sup>A</sup> Based on the material passing the 3-inch (75-mm) sieve

<sup>E</sup> 
$$Cu = D_{60}/D_{10}$$
  $Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$ 

Q PI plots below "A" line.





<sup>&</sup>lt;sup>B</sup> If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.

Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.
 Sands with 5 to 12% fines require dual symbols: SW-SM well-graded

<sup>&</sup>lt;sup>D</sup> Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay

 $<sup>^{\</sup>text{F}}$  If soil contains  $\geq$  15% sand, add "with sand" to group name.

<sup>&</sup>lt;sup>G</sup> If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

<sup>&</sup>lt;sup>H</sup> If fines are organic, add "with organic fines" to group name.

<sup>&</sup>lt;sup>1</sup> If soil contains ≥ 15% gravel, add "with gravel" to group name.

J If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

K If soil contains 15 to 29% plus No. 200, add "with sand" or "with gravel," whichever is predominant.

<sup>&</sup>lt;sup>L</sup> If soil contains ≥ 30% plus No. 200 predominantly sand, add "sandy" to group name.

 $<sup>^{\</sup>text{M}}$  If soil contains  $\geq$  30% plus No. 200, predominantly gravel, add "gravelly" to group name.

 $<sup>^{</sup>N}$  PI  $\geq$  4 and plots on or above "A" line.

 $<sup>^{\</sup>circ}$  PI < 4 or plots below "A" line.

P PI plots on or above "A" line.