



Preliminary Geotechnical Subsurface Data Report Richland County Emergency Bridge Package 4 S-69 Congress Road over Jumping Run Creek Richland County, South Carolina F&R Project No. 65T-0191



Columbia, SC 29201

November 11, 2015



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Engineering • Environmental • Geotechnical

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November 11, 2015

Mr. Trapp Harris, PE South Carolina Department of Transportation Design-Build Section 955 Park Street Columbia, South Carolina 29201

Reference: Preliminary Geotechnical Subsurface Data Report Richland County Emergency Bridge Package 4 SC 69 Congress Road over Jumping Run Creek Richland County, South Carolina F&R Project No. 65T-0191

Dear Mr. Harris:

The purpose of this geotechnical subsurface data report is to present the results of the subsurface investigation program undertaken by Froehling & Robertson, Inc. (F&R) in connection with the Richland County Emergency Bridge Package at State Route S-69 Congress Road over Jumping Run Creek in Richland County, South Carolina. Our services were performed in general accordance with your work order request emailed to F&R on October 16, 2015, and as authorized by your office per our On-Call Contract with SCDOT. The attached report presents our understanding of the project, reviews our investigation procedures, describes existing site and general subsurface conditions, and presents the results of our soil laboratory tests.

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We have enjoyed working with you on this project. Please contact us if you have any questions regarding this report or if we may be of further service.

Sincerely, **FROEHLING & ROBERTSON, INC.**

Gary Ř. Taylor, PÉ Senior Geotechnical Engineer Registered SC No. 27330



Benedictus K. Azumah, PE Geotechnical Engineer Registered VA No. 052166





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1.0 PURPOSE & SCOPE OF SERVICES

The purpose of this Geotechnical Investigation was to explore the subsurface conditions at the site and perform soil laboratory tests on selected soil samples obtained from the investigation. F&R's scope of services included the following:

- Completion of two soil test borings to a depth of approximately 100 feet below the existing ground surface and as close as possible to each previous bridge abutment location.
- Preparation of typed SCDOT Soil Boring Logs;
- Performing soil laboratory tests including natural moisture contents, Atterberg Limits and Wash No. 200 Gradation tests;
- Preparation of this geotechnical subsurface data report by professional engineers.

This report was prepared in general accordance with the 2010 SCDOT Geotechnical Design Manual (GDM), Version 1.1.

Our scope of services did not include identification and evaluation of appropriate foundation systems for the proposed bridge, design capacities and other environmental aspects of the project site.

2.0 PROJECT INFORMATION

2.1 Site Description

The project site is on Congress Road (State Route S-69) located about 250 ft north from the intersection with Old Leesburg Road in Richland County, South Carolina. Cedar Creek flows in an east-west direction and crosses below the road at the site. The area around the creek and the road is generally wooded. Project surroundings are shown on the attached Figure No. 1 - Site Vicinity Map, Included in Appendix I.



2.2 Project Description

Recent flooding in the region has caused extensive erosion around the bridge and resulted in a complete loss of the bridge. Emergency replacement of the roadway and bridge is planned. For this purpose, the geotechnical subsurface investigation and laboratory testing is required by SCDOT and will form part of a preliminary investigation of the site.

3.0 SUBSURFACE INVESTIGATION

3.1 Subsurface Investigation Program

The subsurface investigation program consisted of two soil test borings (STB). The borings, designated as B-01 and B-02 were advanced to a termination depth of 100 ft below the existing ground surface on the approaches adjacent to the previous location of the bridge. Approximate boring locations are identified on Figure No. 2 - Boring Location Plan, included in Appendix I. Photographs of Borings B-01 and B-02 being drilled are also included in Appendix I as Figure No. 3 and No. 4, respectively.

3.2 Location Control

The STB locations were staked in the field by personnel from our office following instructions from your office. The borings were drilled in the centerline of the existing alignment a few feet from the remaining edge of the bridge approach closest to the creek. The ground surface elevation at the borings locations were not provided to us at the time of this writing. GPS coordinates of Borings B-01 (Latitude 33.999907 / Longitude -80.742296) and B-02 (Latitude 34.000115 / Longitude -80.742316) were obtained with a portable hand-held GPS and are recorded on the soil boring logs included in Appendix II of this report.



3.3 Subsurface Investigation Procedure

Subsurface investigation was performed on October 19, 2015 using an ATV-mounted CME/550X and truck-mounted CME/55 drill rigs. The drill rigs used for this project were equipped with an automatic hammer and the drilling method used was the wash rotary boring. The energy ratio of this ATV-mounted hammer reported to us indicates 86% and that for the truck-mounted hammer indicates 74%. SPT tests at boring locations were performed continuously from the existing ground surface to a depth of 10 feet. Thereafter, boreholes were advanced and SPT performed at approximate 5-foot intervals to their termination depths. The Standard Penetration Test (SPT) was performed at the boring locations in general accordance with ASTM D1586.

Soil samples were obtained with a long split-spoon sampler with each SPT being driven with a 140-lb automatic hammer falling 30 inches. The number of blows required to drive the sampler each 6-inch increment of penetration was recorded and are shown on the boring logs. The first six-inch increment is used to seat the sampler with the sum of the second and third penetration increments being termed the SPT value, "N". A representative portion of each disturbed split-spoon sample was collected with each SPT, placed in a glass jar, and returned to our laboratory for review and testing.

The recovered split-spoon samples were visually classified by F&R engineers in general accordance with the ASTM D2488. The boring logs provided in Appendix II show the subsurface conditions encountered on the dates and at the approximate locations indicated.

By the nature of the work performed, the drilling activities result in disturbances to the site. The completed boreholes performed were backfilled upon completion. The borehole backfill may subside at some time following our work. F&R assumes no responsibility for borehole subsidence after completion of the field investigation and departing the site.



3.4 Groundwater

Groundwater was encountered in Borings B-01 and B-02 at depths of approximately 18.5 and 13.5 feet, respectively. The depth at which groundwater was encountered in each individual boring is indicated on the attached soil boring logs in Appendix II.

The groundwater levels at the boring locations were determined based on our observation of free water in the split-spoon soil samples following removal of the sampler. Upon completion of drilling, the boreholes were backfilled for safety, hence the absence of 24-hour water level readings on the boring logs.

The groundwater levels on the soil boring logs indicate our estimate of the hydrostatic water table at the time of our investigation. The final design should anticipate the fluctuation of the hydrostatic water table depending on variations in precipitation, surface runoff, evaporation, creek levels and similar factors.

4.0 LABORATORY TESTING

Laboratory testing consisted of Atterberg Limits Tests, No. 200 Sieve Cut grain size analyses (Wash #200), and Natural Moisture Content tests performed on specific soil samples. The specific tests performed on the selected samples are listed in Table No. 1 below.



Boring	Sample Number	Depth (ft)	Atterberg Limits	Percent Fines Wash # 200	Natural Moisture Content
B-01	SS-4	6.5-8.5	Х	X	Х
B-01	SS-7	18.5-20.0	Х	X	Х
B-01	SS-9	28.5-30.0		X	Х
B-01	SS-10	33.5-35.0		X	Х
B-01	SS-12	43.5-45.0		X	Х
B-01	SS-14	53.5-55.0	Х	X	Х
B-01	SS-16	63.5-65.0		X	Х
B-01	SS-18	73.5-75.0		X	Х
B-01	SS-20	83.5-85.0		X	Х
B-01	SS-21	88.5-90.0		X	Х
B-02	SS-5	8.0-10.0		X	Х
B-02	SS-6	13.5-15.0	Х	X	Х
B-02	SS-8	23.5-25.0		X	Х
B-02	SS-10	33.5-35.0	Х	X	Х
B-02	SS-12	43.5-45.0		X	Х
B-02	SS-14	53.5-55.0	Х	X	Х
B-02	SS-15	58.5-60.0		X	X
B-02	SS-17	68.5-70.0	Х	X	X
B-02	SS-20	83.5-85.0		X	Х

Table No. 1: Soil Laboratory Tests Performed on Selected Soil Samples

The laboratory testing results are presented in Appendix III and the laboratory test data sheets are presented in Appendix IV.

F&R greatly appreciates the opportunity to work with you on this project. If there are any questions concerning this report or if any additional information is required, please do not hesitate to contact us.



6.0 LIMITATIONS

This report has been prepared for the exclusive use of South Carolina Department of Transportation or their agent, for specific application to the referenced site in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. Our investigation is based on site location information furnished to us; and generally accepted geotechnical engineering practice. The subsurface investigation logs included herein, do not reflect variations in subsurface conditions which could exist intermediate of the boring locations or in unexplored areas of the site. Should such variations become apparent during construction, it will be necessary to perform additional subsurface exploration based upon on-site observations of the conditions.



APPENDIX I











APPENDIX II



KEY TO SOIL CLASSIFICATION

Correlation of Penetration Resistance with

Relative Density and Consistency

Sands and	d Gravels	Silts and C	<u>Clays</u>
No. of	Relative	No. of	
<u>Blows, N</u>	<u>Density</u>	<u>Blows, N</u>	<u>Consistency</u>
0 - 4	Very loose	0 - 2	Very soft
5 - 10	Loose	3 - 4	Soft
11 - 30	Medium dense	5 - 8	Firm
31 - 50	Dense	9 - 15	Stiff
Over 50	Very dense	16 - 30	Very stiff
		31 - 50	Hard
		Over 50	Very hard

Particle Size Identification

	(Unified Classification System)
Boulders:	Diameter exceeds 12-in. (300-mm)
Cobbles:	3-in. (75-mm) to 12-in. (300-mm) diameter
Gravel:	Coarse - ¾-in. (19-mm) to 3 in. (75-mm) diameter Fine - No. 4 (4.75-mm) sieve to ¾-in. (19-mm) diameter
Sand:	<u>Coarse</u> – No. 10 (2.0-mm) to No. 4 (4.76 mm) sieve <u>Medium</u> – No. 40 (0.425-mm) to No. 10 (2.0-mm) sieve <u>Fine</u> - No. 200 (0.075-mm) to No. 40 (0.425-mm) sieve
Silt and Clay:	Less than No. 200 (0.075-mm) sieve



Modifiers

The modifiers provide our estimate of the amount of silt, clay or sand size particles in the soil sample.

Approximate	
<u>Content</u>	<u>Modifiers</u>
≤ 5%:	Trace
5 to 10%:	Few
15 to 25%:	Little
30 to 45%:	Some
50 to 100%	Mostly

	Field Moisture
	Description
Dry	Absence of moisture, dusty, dry to touch
Moist	Damp but no visible water
Wet	Visible free water, usually soil is below water table

SOIL CLASSIFICATION CHART

м			SYM	BOLS	TYPICAL
	AJOR DIVISI		GRAPH	LETTER	DESCRIPTIONS
	GRAVEL AND	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
	GRAVELLY SOILS	(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
COARSE GRAINED SOILS	MORE THAN 50% OF COARSE FRACTION	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
	RETAINED ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
MORE THAN 50% OF MATERIAL IS	SAND AND	CLEAN SANDS		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
LARGER THAN NO. 200 SIEVE SIZE	SANDY SOILS	(LITTLE OR NO FINES)		SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION	SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES
	PASSING ON NO. 4 SIEVE	(APPRECIABLE AMOUNT OF FINES)		SC	CLAYEY SANDS, SAND - CLAY MIXTURES
				ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE				МН	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
SIZE	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		СН	INORGANIC CLAYS OF HIGH PLASTICITY
				ОН	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HI	GHLY ORGANIC S	SOILS		PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

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AWG-I	Rock Cor	r <u>e, 1-1</u> /8"		<u>CT</u> - C	<u>Continuo</u>	ous Tube			<u>A - Cont</u> <u>C - D</u> rivi	ng Ca	s Flight /	-ugers		- KUC			

File No	b.: 65	5T-0191	Project	t No. (P	IN):			Co	ounty:	Ric	chlan	d			En	g./Ge	90.:	SCI		
Site De	escripti	on:	Emerger	ncy Bric	lge Pa	ackage	4 - Cor	ngress	Rd ove	er Ju	mpin	g Rui	n Ci	reeł	<	Ro	ute:	SC 6	69	
Boring	No.:	B-01	Bo	oring Lo	catio	ו:			(Offse	et:				A	ignm	nent:	Ex	isting	
Elev.:	ft		Latitude:		-80.74	2296	Longi	tude:	33	9999	907	Da	ate	Star	rted	:	-	10/19	/15	
Total D	Depth:	100 f	ft So	il Depth	า:	100 f	t C	ore De	epth:	0 f	t	Da	ate	Con	nple	eted:	-	10/19	2015	
Bore H	lole Dia	meter (i	n):		Samp	er Cor	nfigurati	on	Line	er Re	quire	ed:	Y	()	J)	Lir	ner Ús	sed:	Y	(N)
Drill M	achine:	CME	E-55X	Drill N	letho	l: R	otary Wa	ash	Hamm	er Ty	pe:	Autor	mati	ic		Ener	gy Ra	atio:	86%	
Core S	Size:			Driller	:	SCI			Ground	dwat	er:	тов	-	18.5	ft		24HF	२	N/A	
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	97.5		<u> </u>						-						÷	÷	: :	÷	: :	
		(CH/A-7	ff, pinkish <u>(</u> 7 -6)	gray, moi	st, FAI	CLAY		98.5	; -							:		÷		-
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ss -	Split Spo	on		NQ - Ro	ck Core	e, 1-7/8"		HS	SA - Hollo	w Ste	m Aug	jer			RW	- Rot	ary Wa	ash		
ST -	Shelby T	ube		CU - Cu	ttings	e Tubo			A - Cont	inuou:	s Fligh	t Auge	ers	l	RC	- Roc	k Core	Э		
<u>δ Λ₩Θ-</u>	I YOUK CO	5, 1-1/0		01 - 00	nunuou	a rube			וועווס - כ	ny Ud	sing									

File No	o.: 65	5T-0191	Pr	oject	No.	(PIN):				Co	unty:	Ric	chlan	d			En	g./G	eo.:	SC		
Site De	scripti	on:	Eme	ergen	ісу В	ridge F	Package	- 4 -	Con	gress	Rd ove	er Ju	mpin	g Ru	n C	reel	<	Ro	oute:	ŞC	69	
Boring	No.:	B-02		Bo	ring l	ocatio	on:				(Offse	et:				A	ignn	nent:	E	xisting	9
Elev.:	ft		Latit	ude:		-80.7	42316	Lo	ongi	tude:	34.	0011	15	D	ate	Star	rted	:		10/1	9/15	
Total D	epth:	100	ft	Soi	il Dep	oth:	100 f	t	C	ore De	epth:	0 f	t	D	ate	Con	nple	eted:		10/1	9/201	5
Bore H	ole Dia	meter (i	in):			Sam	pler Coi	nfigu	urati	on	Line	er Re	quir	ed:	Y		<u>ا</u>		<u>ner U</u>	sed:	Y	N
Drill Ma	achine:	CM	E-55		Dril	Metho	bd: Re	otar	y Wa	ash	Hamme	er Ty	pe:	Auto	mat			Ener	gy Ra	atio:	74%	
Core S	IZE:				Drii	er:	SCI				Ground	awat	er:	IOB		13.5	π		24H	ĸ	N/A	
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ation ()) th								g	the	ype		_		alue			×		<u> </u>	—X	
(fi	Del Del	IV	1AIE	RIAL	DES	SCRIP	IION		C ag	Del	No./	t 6"	id 6'	d 6"	» Z			FIN	ES CO	ONTEI	NT (%)	
	0.0	Aanhalt	Dava	mont	6"				-	0.0		,	2	ä	_	0 1	0 20) 30	40 5	0 60	70 80	90
	0.5				0			-/		0.0	- 1/SS	10	6	9	15		•					
	2.0	[↑] to medi	um, m	i dens ioist, (clay	y pale t EY SAN	D D	e		2.0	·											
		(SC/A-2	2-6)							40	- 2/SS	6	10	11	21							
]	Munsel	=10YF	₹7/3				_i			2/00	10	Q	Q	16		_					
	6.0	Medium	n dens	e, ver	y pale	brown,	fine to		,	6.0	0,00		υ	U	10			:				
		with SI	1, MOIS LT (SF	st, PO P-SM//	ORLY A-3)	GRAD	ed Sani	וו			- 4/SS	1	2	2	4	•	÷	:				
	8.0	¦ ∫Munsel	=10YF	R 7/3	,			i,		8.0							-					
				- <u>-</u> -	drav		nedium			•	- 5/SS	2	3	3	6		0		:			
		moist, C	CLAYE	EY SA	ND (S	SC/A-2-6	5)	Ì									İ					
		Munsel	=5YR	6/1				i]						į	-				
	12.5	Loose,	gray, f	fine to	medi	um, wet	, SILTY			125							÷	÷				
		¥SAND (SM/A-	-2-4)					ш	13.5	6/99	2	5	11	16			: X				
	-	Munsel	=5YR	6/2	0/ 00	0-00.0			ш		- 0/33	2	5		10						<u> </u>	
	-	155-5: N		14.1%	, %20 	0=23.8		_i	ш		-						i	-				
	-	COAST	AL PL	AIN,	very s	tiff, brov	wnish		ш		-						÷					
		gray, m	ioist, E		FIC SI	LT (MH/	A-7-5)		ш	18.5	;-						-					
				30 070	% %2	00=07 ?	8 11 =61		ш		7/SS	7	11	15	26			•				
		PL=32,	PI=29)	/0, /02	00-07.0	, LL-01,		ш		_						i					-
	-	Hard							ш		_							:				-
	23.5									23.5	;						i				: :	-
	-	Very ha	ard, gra	ay and	d pink	ish gray	, moist, 5)		ш		- 8/SS	14	23	43	66			ЭÌ			•	
		Munsel	=5YR	6/1 &	6/2	m <i>n</i> – <i>r</i> –	5)		ш		-						:					
		SS-8: N	IMC=2	22.7%	, %20	0=58.8]											
	27.5	~								28 5	_						:					
		very pa	1 dens le brov	e, fine wn an	e to m d pink	eaium, i ., SILTY	noist, Í SAND			20.0	9/SS	7	7	13	20	1)	÷			
	-	(SM/A-1	1-b)										-	-					:		<u> </u>	
		Munsel	=10YF	R 7/3 8	& 5YR	7/3					-											
		SS-10: Non-Pla	NMC= astic	=23.9%	%, %2	00=13.7	,															
, ,]	SS-12:	NMC=	=18.19	%, %2	00=12.9)			33.5	10/00	-	40	45	05							
3		_									10/55	/	10	cı	25		A	• :				
Š		Dense									-											-
5	-										-											-
										38.5	;-											-
										•	11/SS	9	11	17	28							
									LE	GENI	2							(Conti	nuec	Next	Page
SS -	Split Spo	on	SAM	IPLER		E Rock Co	re 1-7/8"			Н	SA - Hollo	w Ste	m Au	DR	ILLIN	IG M	IETH RW	IOD - Rot	arv W	ash		
ST -	Shelby T	ube			CU - (Cuttings				CF	A - Cont	nuous	s Fligh	t Aug	ers		RC	- Ro	ck Cor	e		
AWG-	Rock Co	e, 1-1/8"			CT - (Continuc	us Tube			D(C - Drivir	ng Ca	sing									

File No).: 65	5T-0191	Proj	ect N	No. (I	PIN):				Co	unty	/:	Ric	hlar	nd			Er	ng./(Geo.	: S	CI	
Site De	escripti	on:	Emerg	genc	y Bri	dge F	Package	e 4 -	Con	gress	Rd	over	⁻ Jur	npir	ig Ri	un C	ree	k	F	Rout	e: S	C 69	
Boring	No.:	B-02	I	Boriı	ng L	ocatio	on:					0	ffse	t:				A	ligr	mer	nt:	Existir	ng
Elev.:	ft		Latitud	de:	_	-80.7	42316	Lo	ongit	ude:		34.0	011	5	1	Date	Sta	rteo	1:		10/	19/15	_
Total D	Depth:	100 f	t s	Soil	Dept	:h:	100 1	ft	Co	ore De	epth	:	0 ft		1	Date	Cor	mpl	ete	d:	10/	19/20	15
Bore H	lole Dia	meter (i	n):			Sam	pler Co	nfigu	iratio	on		Line	r Re	quir	ed:	Y	(N)	L	iner	[.] Use	d: Y	N
Drill Ma	achine:	CME	E-55	1	Drill	Metho	od: R	otary	/ Wa	ish	Har	nme	r Ty	pe:	Auto	omat	ic		Ene	ergy	Ratio): 749	6
Core S	ize:	ł		0	Drille	er:	SCI				Gro	und	wate	er:	TOE	3	13.5	5 ft		24	4HR	N/A	1
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(atic	epth ft)	М		ίδι Γ					ihde og	pth #		d K K		5	=	/alu			X		-0	<u> </u>	
		111				JINI			5	Sal	Sal	No.	st 6	pu (rd 6	ź			▲ FI	NES	CONT	ENT (%)
_	-										_	_	÷	N	ē		0 1	02	03	0 40	50 6	0 70 8	<u>30 90</u>
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										43.5	-						_	-			•		
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	47.5-																	-					
	41.5	Verv de	nse, dar	rk bro	wn. f	ne to r	 nedium.		////	10 -	_							:					: : :
		moist, C	LAYEY	SAN	D (SO	C/A-2-6	5) ⁽			40.5	13	/99/	16	22	23	15	-	-					
	_	Munsel=	=7.5YR 3	3/2								/33	10	22	25	43		<u>.</u>				<u> </u>	<u> </u>
	-										_							-					
	52.5										-							:					
	-	Hard, pi	nkish gr	ray, m	noist,	SAND	Y SILT			53.5	-							:					
	-		9) -7 5VD '	7/0							14	/SS	5	11	17	28		-	Ø	\rightarrow			
	-	SS-14	-7.511X NMC=24	4.6%	%20	0=67 () =37											:					
		PL=25,	PI=12	4.070,	, /020	0-07.0	, LL-07,				1							:			:		: : :
	57.5																	-					
		Dense, v medium	very pal SILTY	le bro' SAN	wn ai I D (SI	nd pink //A-1- ł	, fine to			58.5	<u> </u>	/0.0					-						
		Munsel=	=10YR 7	7/3 &	5YR	7/3	-,	÷			15	/55	10	14	18	32		: ▲ (<u> </u>	<u> </u>
		SS-15: I	NMC=20	0.9%,	, %20	0=14.1	1				_												
	62.5										_							-			:		
	-	Hard, gr	ay, moi	st, FA	AT CL	AY (C	H/A-7-6)			63.5	-							:				: :	÷ ÷ •
	-	Munsel=	=5YR 6/	1							16	/ss	8	9	16	25			•				
	-										+							:					
	-										-							-					
	67.5					un fin												:			:	:	: : -
		medium	, POOR	RLY G	RAD	ED SA	ND with			68.5]												
		SILT (SI	P-SM/A-	-1-b)				:			17	/SS	20	31	42	73	· ·	A	0				<u> </u>
2		Munsel=	=10YR 5	5/6				:			_							-					÷ ; .
	_	SS-17: ľ Non-Pla	NMC=25 Istic	5.5%,	, %20	0=10.8	3,	:			_							-					
-	-		lotio					:		73.5	-							-					
5.	-										18	/SS	23	45	50/5	95		:				: :	•
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5								į.			-							-					
	1	Dinkich	arav					i.		78.5]—						-	:					
3			gray								19	/SS	21	35	36	71		:				•	<u> </u>
									LE	GEN	2									Со	ntinue	ed Ne	kt Page
ss -	Split Spo		SAMPL		YPE Q - R	ock Co	re 1-7/8"			HS	SA - H		/ Ste	m Au	Di	RILLI	NGN	/ETI RW	HOD - R	otarv	Wash		
ST -	Shelby T	ube		C	Ũ - C	uttings	,			CF	A - (Contin	uous	Flig	ht Au	gers		RC	- R	lock (Core		
AWG-	Rock Co	re, 1-1/8"		<u> </u>	T - C	ontinuc	ous Tube				<u> </u>	Driving	g Cas	sing									

File No).: 6	5T-0191	Pro	ject	No.	(PIN):				Co	bun	ty:	Ric	chla	nd			Er	ng./C	Geo.:	S	CI	
Site De	escripti	on:	Emer	rgen	ісу Е	Bridge F	Packa	age 4	4 - Con	gress	s Ro	d ove	r Ju	mpiı	ng R	un C	ree	k	R	loute	: S	C 69	
Boring No.: B-02 Boring Location:			Offset:				Alignment:			t:	Existin	g											
Elev.:	ft		Latitu	de:		-80.7	4231	16	Longit	tude:		34.0)01 ⁻	15	I	Date	Sta	rteo	d:		10/	19/15	
Total D	Depth:	100 f	ft	Soi	l De	pth:	10	0 ft	Co	ore De	ept	h:	0 f	t	I	Date	Co	mpl	etec	1:	10/	19/201	5
Bore H	lole Dia	meter (i	n):			Sam	pler (Conf	igurati	on		Line	r Re	qui	red:	Y	(N	L	iner	Usec	l: Y	N
Drill M	achine:	CME	E-55		Dri	I Metho	od:	Rot	ary Wa	ish	Ha	amme	r Ty	pe:	Aut	omat	tic		Ene	ergy I	Ratio	: 74%)
Core S	ize:				Dri	ler:	SC				Gr	ound	wat	er:	TO	3	13.	5 ft		24	HR	N/A	
										1													
																			•	SPT	N VAL	UE	
E									U	۵_		e e				e			ΡĻ		MC	ĹĹ	
(ft)	epth	M		2141	DE	SCRIP	ΓΙΟΝ		ida og	=pth	Ê	d K	-	50	=_	/alu			\mathbf{X}		0	X	
Ше́	ŏ					o or an			5	° Sa		No.	st 6	pq	rd 6	ź			▲ FI	NES C	CONTE	ENT (%)	0 00
	-	Munsel	=7.5YR	2 7/2					िलग		_		~	2	с С		0	:	0 30	<u>J 40</u>	50 6	<u>0 70 8</u> : :	<u>90</u>
	-										-							-					
	82.5										-							:		-			
	-	Dense, SAND (yellowi: SC/∆-2	sn br 2-61	rown,	tine, CL	AYEY.	ſ		83.5	54					<u> </u>	-	:		÷			-
	1	Munsel	=10YR	5/8							72	20/SS	11	13	22	35		-	E AQ	•			
	1	SS-20:	NMC=2	28.9%	6, %2	200=26.6	6											:		÷		· · ·	
				/	,													-					
	87.5	Verv ha	rd grav		TCI	AY (CH	A-7-6	<u>,</u>		00.5								-					
		Munsel	=5YR 6	5/2				,		88.5		1/00	40	25	50	05		-					
											1	21/55	18	35	50	85							•
											_							-					-
											_							-					-
	-									93.5	5-							-					-
	-										- 2	2/55	33	46	50/4	96		-					•
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) SS	Solit Spa		SAMF	PLER	TYP	E Bock Co	ro 1_7	7/8"		Ц	SA	Hollow	N Sto	m Aı	D	RILLII	NG	METI		otary	Wash		
ST -	Shelby T	ube		(CU -	Cuttings	ie, I-/	10			5A - FA -	Contir	านอนะ	s Flig	ht Au	gers		RC	- R	ock C	ore		
AWG-	Rock Co	re, 1-1/8"		(<u>CT -</u>	Continuo	ous Tul	be		D	<u>- C</u>	Drivin	g Ca	sing		-							



APPENDIX III



FROEHLING & ROBERTSON, INC.

LABORATORY TEST SUMMARY SHEET

F&R Project No: 65T-0191 Client: South Carolina Department of Transportation Project: S-69 Congress Road over Jumping Run Creek SCDOT Proj ID City/State:

Richland County, SC

Baring/Comple No	Donth (ft)		ы	ы	USCS/AASHTO	Motor Contont (%)	Percent Passing No. 200
Bornig/Sample No.	Deptil (It)	LL	PL	PI	Classification	water content (%)	Sieve
B-01/SS-4	6.5-8.5	38	20	18	SC/A-6	20.6	37.1
B-01/SS-7	18.5-20.0	40	28	12	SM/A-2-6	36.9	30.7
B-01/SS-9	28.5-30.0				CH/A-7-6	21.9	76.9
B-01/SS-10	33.5-35.0				SM/A-2-4	30.4	21.9
B-01/SS-12	43.5-45.0				SM/A-2-4	28.6	15.9
B-01/SS-14	53.5-55.0	NP	NP	NP	SM/A-1-b	27.9	22.2
B-01/SS-16	63.5-65.0				SC/A-2-7	27.9	35.3
B-01/SS-18	73.5-75.0				SM/A-1-b	27.0	15.0
B-01/SS-20	83.5-85.0				SM/A-1-b	20.5	13.7
B-01/SS-21	88.5-90.0				SM/A-1-b	27.9	12.0
B-02/SS-5	8.0-10.0				SM/A-2-4	14.1	23.8
B-02/SS-6	13.5-15.0	61	32	29	MH/A-7-5	30.7	97.3
B-02/SS-8	23.5-25.0				MH/A-7-5	22.7	58.8
B-02/SS-10	33.5-35.0	NP	NP	NP	SM/A-1-b	23.9	13.7
B-02/SS-12	43.5-45.0				SM/A-1-b	18.1	12.9
B-02/SS-14	53.5-55.0	37	25	12	MH/A-6	24.6	67.0
B-02/SS-15	58.5-60.0				SM/A-1-b	20.9	14.1
B-02/SS-17	68.5-70.0	NP	NP	NP	SP-SM/A-1-b	25.5	10.8
B-02/SS-20	83.5-85.0				SC/A-2-6	28.9	26.6

NP: non-plastic

Date: 11/11/15



APPENDIX IV

CONGRESS RD

	×		***		· /0 , ·			-1
	; #1	#2	#3	<u>#4</u>	#5	₩6 ±	<u></u>	7
Sample I.D	B-1 6.5-	8	B-1 18.5-2	P	B-1 28.5	30	<u>B-1 335</u>	
Wet Soil + Tare	295,89	Y	444.84		389,83		458.11	·
Dry Soils + Tare	270,44		365.10		346.12	 	385.76	{ -1
Tare # & Weight	5-7 147,14		53 149.17		L 146.18	1	521 147.7	°
Weight of Water	25.45	- :	79.74		43,71		72.35	-
Weight of Dry Soils	123.3	!	215.93	1	199.94		238.06	
Ministure Coptent	0.206		0.369		0.219		0.304	ļ
Mulature Contern			*		,		•	-1
	#8	#9	#10	#11	#12	#13	<u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u>	
Sample I D.	B-1 43.5.	45	B-1 53,5-5	5	B-1 63.5-	65	B-1 73.5-	7
Wet Soil + Tare	436.65		443, 58		484.52	1	477.92	
Dry Soils ÷ Tare	372.84		379,45		411,10		407,91	
Tare # & VVeight	G 149,47		5 149.47		58 148.06		59 148.5	8
Weight of Water	63.81		64.13		73,42		70.01	
Weight of Dry Soils	223.37		229.98		263.04		259.33	
Vioisture Content	0.286		0.279		0.279		0.270	
				•				
<i>2</i> -	#15	#15	#17	#18	i #19	#20		
Sample I.D.	B-1 83.5-	85	B-1 88.5-	10		<u> </u>		
Vet Soil + Tare	520,50		523.84					
Dry Soils + Tare	466.64		441.49					
are # & Weight	AE 204.20	0	B 146.04					
Veight of Water	53.86		82.35			!	· · · · ·	
Veight of Dry Soils	262.38		295.45		.		.]	
ioisture Content	0.205	•	0.279		1	:		
		1100	1 404 1	#25	<u> </u>	 #27	#28	
	#22	#23	<u> </u>	<u>#</u> 20	<u>π</u> 20			
ample I.D.							}	
let Soil + Tare				ι				
ry Soils ÷ Tare						i		
are # & Weighi	1. [·		1.	
eight of Water					· · · · · · · · · · · · · · · · · · ·			
eight of Dry Spils	}				i			
oisture Content 🛛					!	1		





PI =

USCS =



Program Administrator



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PL =

PI =

USCS =

Laboratory Technician: ______ Reviewed By:

Program Administrator



SP3



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CLIENT PROJECT RECORD NO.	55	- B-1 - 6.5	-B	TECHNICIAN _ TEST DATE _	
		SOIL CLASSIF	ICATION		
TARE # N2	_	TARE WEIGHT	319.9	18	
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	443,6 123.64 397.71 77.73 45.91	2	-	, ·

Percent Passing #200

37.1%



CLIENT		- B \	TECHNICIAN		,
PROJECT CONGRE	255		TEST DATE		
RECORD NO		18.5-20	·		
		SOIL CLASSIFICATION			
TARE #KC		TARE WEIGHT 374	.65		
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	598,70 216,05 524,27 149,62 66,43		. ·	
• • • · · ·	.*				

Percent Passing #200

30,7%



CLIENT		- β-1	TECHNICIAN
PROJECT CONGRE	55	- 785-30	TEST DATE
RECORD NO		-	
		SOIL CLASSIFICATION	
TARE #B		TARE WEIGHT 397.	87
A Tare and Dry Soil		598,30	
B Dry Soil C Taro and Dry Soil After Wash	(A-Tare)	444.10	— , .
D Dry Soil After Wash	(C-Tare)	46,23	-
E Material Lost	(B-D)	154.2	_
· · · ·	.'		

Percent Passing #200

76.9%



CLIENT		TECHNICIAN
PROJECT _	CONGRESS	— B-1
RECORD NO.		
		SOIL CLASSIFICATION
TARE # _	NI	TARE WEIGHT 327.20
A Tare and Dry Soil		565.75

B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash

D	Dry Soll After Wash	
Е	Material Lost	

	- q > r
(A-Tare)	238.55
Č,	513, 40
(C-Tare)	186,20
(B-D)	52.35
(B-D)	52.35

Percent Passing #200

21,9%



CLIENT		TECHNICIAN
PROJECT CONGRESS	- B-1 175-45	TEST DATE
RECORD NO	43.5	
	SOIL CLASSIFICATION	
TARE #8A	TARE WEIGHT 337.	73

A Tare and Dry Soil

- B Dry Soil
- C Tare and Dry Soil After Wash
- D Dry Soil After Wash
- E Material Lost

	561.52
(A-Tare)	223, 79
	526.01
(C-Tare)	188,28
(B-D)	35.51

Percent Passing #200

(B-D)/B x 100=

15.9%



CLIENT PROJECTONGRES	55	- B-1 53,5-55	TECHNICIAN TEST DATE	
		SOIL CLASSIFICATIO	DN	
TARE #		TARE WEIGHT 39	4.23	
A Tare and Dry Soil		624.78		
B Dry Soil	(A-Tare)	230.55		
C Tare and Dry Soil After Wash		573,50	· · · · · · · · · · · · · · · · · · ·	
D Dry Soil After Wash	(C-Tare)	179.27		
E Material Lost	(B-D)	51,28		

Percent Passing #200

22,2%



CLIENT		-R-1	TECHNICIAN
PROJECT CONGR	E55		TEST DATE
RECORD NO.			
		SOIL CLASSIFICATION	-
TARE #		TARE WEIGHT	44
A Tare and Dry Soil		652,16	
B Dry Soil C Tare and Dry Soil After Wash	(A-Tare)	559,15	
D Dry Soil After Wash	(C-Tare)	170,69	
E Material Lost	(B-D)	93.01	
		• •	

Percent Passing #200

35.3%



CLIENT PROJECT CONCRES	5	_ B-1 - 73.5-75	TECHNICIAN	
		SOIL CLASSIFICATION		
TARE # XD		TARE WEIGHT <u></u> ろいん	. 21	
Tare and Dry Soil Dry Soil Tare and Dry Soil After Wash Dry Soil After Wash Material Lost	(A-Tare) (C-Tare) (B-D)	565.87 259,66 527,05 220,84 38.82		· · · · · · · · · · · · · · · · · · ·

Percent Passing #200

A B C D E

15.0%



CLIENT _			R-1	TECHNICIAN	
PROJECT	CONGRESS		03.5-85	TEST DATE	
RECORD NO.		1994	e sta		
			SOIL CLASSIFICATION		
TARE #	R	Т	ARE WEIGHT	44	
Tare and Dry Soil			705.08	_	
Dry Soil	(A	-Tare)	262.64	_	
Tare and Dry Soil	After Wash		669, 11		· ·
Dry Soil After Was	h (C	C-Tare)	226.67	_	
Material Lost	(В	6-D)	35.97	_	

Percent Passing #200

A B C D E

13.7 %



	GRESS	- B-1 - 88.5-90	TECHNICIAN TEST DATE	
RECORD NO.				
		SOIL CLASSIFICAT	ION	
TARE # 1.18		TARE WEIGHT 53	36, 29	
Tare and Dry Soil		832,11		
Dry Soil	(A-Tare)	295. 82		
Tare and Dry Soil After Was	h	196, 48		,
Dry Soil After Wash	(C-Tare)	260,19		
Material Lost	(B-D)	35.63		
· ·				

Percent Passing #200

A B C D E

12.0%

CONGRESS RD.

			VOSTLRE X	E COMTENT	(%)	,	×	
				#4	#5	¦ #6		-) -,
Sample I.D	B-2 8-1	D	B-2 135	-15	B-2 23.5	-25	B-2 33.5	
I Wet Soil + Tare	436,36		349,44		403.20		377,75	
Dry Soils + Tare	400,76		302,19	 	355,79		344.34	-
I_are # & Weight	5123 147.5	8	SID 148.3	6	sg 146.5	4	AD 204,3	7
Weight of Water	35.60		47.25		47.41		33,41	-
Weight of Day Sails	252 10	!	153.83		209.25		139,97	
	6 141		0.307		6.227		0.239	J
Moistule Content	0.11	I	*				×	
	#8	#9	#10	#11	#12	#13	#14	
Sample I D.	B-2 A3,5	45	B-2 53,5	-55	B-2 58.5	60	B-2 68.5	-°
Wet Soil + Tare	423.53		403,89		424.13		440.47	ļ
Dry Soils ÷ Tare	380,99		361,49		376.46	1	411.16	
Tare # & Weight	K 145.37	1	12 189.3	•	5-51 149,5	3	P5 296,28	
Veight of Water	42,54		42.40		47.47		29.31	
Weight of Dry Soils	235.62		172.19		227.13		114.88	
vioisture Conteni	0.181		0.246		0.209		0.255	
-				,	. #10	[世21	
	#15	#16		#16	<u> </u>	<u> </u>		
Sample I.D.	B-2 83,5	- 85		- 				
Vet Soil + Tare	591, 53	· · · · · · · · · · · · · · · · · · ·						
ry Soils + Tare	526,33							
are # & Weight	P4 300,89					1	· · ·	
leight of Water	03.20 076 AA	ę						
leight of Dry Solis	663,77					:		
oisture Content	0.651	•			·	•		
	#22 ·	#23	#24	#25	#26	#27	#28	
mple I.D.						·		
et Soil + Tare			1				1	
y Soils ÷ īare				ŝ				
re # & Weight					 .			
eight of Water					!		1	
eight of Dry Soils							1	
isture Content					!	·	· · · · · · · · · · · · · · · · · · ·	

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SPZ



PL=

PI =

USCS =



Program Administrator



PL=32

PT = Z9

29/15 Client: San Date Received: Material Project: RD. Sample No: GRESS 10 F&R Project No.: 33.5-35 Location: R-7 Date Processed: Item Code/Use: Source: Liquid Limit 40.0 Tare No. Cup & Wet Soil 38.0 Cup & Dry Soil 36.0 Water Content Moisture Loss ASI 34.0 Cup Weight 32.0 Dry Soil 30.0 Blows 28.0 Moisture % 26.0 Plastic Limit 24.0 Tare No. 22.0 Cup & Wet Soil 20.0 Cup & Dry Soil Moisture Loss 10 100 Cup Weight Number of Blows Dry Soil Moisture %

Laboratory Technician: _____ Reviewed By:

Program Administrator

LL = _____ PL = _____ PI = _____ USCS = _____ T-

SPS

Froehling & Robertson, Inc.

Laboratory Sample Analyses



PI =

USCS =

Laboratory Technician: _____ Reviewed By:

Program Administrator



SP4 6



		F&R		
	WAS	SH 200 (C 11	.7)	
CLIENT PROJECTCONGRE RECORD NO	55 RD	B-2 8-10	TECHNICIAN TEST DATE	
TARE # D3		SOIL CLASSIFICATIO	50.09	
A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost	(A-Tare) (C-Tare) (B-D)	503,51 253,42 443,10 193,61 60.41		, ·
			a ph	



CLIENT PROJECT CONGRES	5	- B-Z - 13,5-15	TECHNICIAN TEST DATE	
		– SOIL CLASSIFICATIO	DN	
TARE #	-	TARE WEIGHT25	55,15	
		6-0 H		
A Tare and Dry Soil		404.46		
B Dry Soil	(A-Tare)	154.31		
C Tare and Dry Soil After Wash		259.36		· ·
D Dry Soil After Wash	(C-Tare)	4.21		
E Material Lost	(B-D)	150.10		
	4 a			

Percent Passing #200

97.3°%



CLIENT PROJECT CONGR RECORD NO	E55	- B-2 - 23,5-25	TECHNICIAN	
TARE # E	_	SOIL CLASSIFICATION	8.06	
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	457.81 209.75 334,40 86.34 123.41		

Percent Passing #200

. .

58.8 %



PROJECT CONCRES	·5	B-2 33.5-35	TECHNICIAN TEST DATE
		SOIL CLASSIFICATION	
TARE # 48-4	_	TARE WEIGHT 172.	81
Tare and Dry Soil Dry Soil Tare and Dry Soil After Wash Dry Soil After Wash	(A-Tare)	313,00 140,19 293,76 120,95	- - , ·
Material Lost	(B-D)	19.24	-

Percent Passing #200

A B C D E

13.7%

		F&R	
	WAS	5H 200 (C 117))
CLIENT PROJECT	555		TECHNICIAN
TARE # AA		SOIL CLASSIFICATION	. 73
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) (C-Tare) (B-D)	483, 65 235, 92 453, 21 205, 48 30,44	
Deverse Deceipe #200		12	9%

. .

, ,

		<u>Itski</u>	
	WA	SH 200 (C 117))
CLIENT		B-2	TECHNICIAN
PROJECT CONG	RE35	- 53,5-55	TEST DATE
RECORD NO		SOIL CLASSIFICATION	
TARE # A3		TARE WEIGHT 249.	94
Tare and Dry Soil		A22,5A	_
Dry Soil Fare and Dry Soil After Wash	(A-Tare)	172,60	— , ·
Dry Soil After Wash	(C-Tare)	57.01	_
Material Lost	(B-D)	115,59	
· .	. ·		

Percent Passing #200 (B-D)/B x 100=

А В С D Е

.

67.0%



CLIENT PROJECT <u>CONGRESS</u> RECORD NO	β-2 technician 58,5-60 test date	
	SOIL CLASSIFICATION	
TARE # N4	TARE WEIGHT 330.53	
 A Tare and Dry Soil B Dry Soil (A-Tare C Tare and Dry Soil After Wash D Dry Soil After Wash (C-Tare E Material Lost (B-D) 	$ \begin{array}{c} 557,99\\ \hline 227.46\\ \underline{525.95}\\ e) \\ 195.42\\ \underline{32.64}\\ \end{array} $	

Percent Passing #200

14.1%

WASH 200 (C 117)						
CLIENT PROJECTONGRE RECORD NO	55	B-2 68.5-70 SOIL CLASSIFICATION	TECHNICIAN TEST DATE			
TARE # 176		TARE WEIGHT 178	.21			
A Tare and Dry Soil	(A-Tare)	293,21 115.0 280,74	, · ·			
C Tare and Dry Soil After Wash						

,

FOR								
WASH 200 (C 117)								
CLIENT PROJECT RECORD NO	B-2 55 §	- 3,5-85	TECHNICIAN					
TARE #	SOIL CI TARE WEIC	ASSIFICATION	24					
 A Tare and Dry Soil B Dry Soil C Tare and Dry Soil After Wash D Dry Soil After Wash E Material Lost 	(A-Tare) 52 (A-Tare) 22 469 (C-Tare) 165 (B-D) 60	9,96 5,72 ,85 61 11						

Percent Passing #200

Z6,6%