





Stormwater Management Design Report

I-77 Widening Design-Build Preparation On-Call

From Percival Road to Killian Road

Richland County, SC

May 2015

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Project Summary



1.1 Project Summary

The South Carolina Department of Transportation proposes to widen I-77 from near mile point 15 (Percival Road) on the southern terminus to approximately mile point 22 (Killian Road) on the northern terminus. The project includes adding a travel lane in each direction, improving various exit ramps, and widening ten mainline bridges. The bridges include: four over I-20 and I-20 exit ramps near mile point 16; two over Windsor Lake Boulevard (S-1196) near mile point 16.3; two over Windsor Lake near mile point 16.6; and two over Edgewater Drive (S-1722) and Jackson Creek near mile point 18. The project will also include the rehabilitation of I-77 southbound lanes from the northern terminus of the widening project near mile point 22 to mile point 27. It is assumed that the project limits and construction impacts will fall within the existing right-of-way for all improvements. The Department has contracted with HDR, Inc. to complete the drainage and outfall field surveys, pipe inspections, and preliminary hydraulic design in order to support the Department with preparation of the design build package.

I-77 is a north-south interstate route that traveling along the eastern limits of Columbia and provides a direct connection from I-26 in Columbia, SC to Charlotte, NC. Within the project limits, I-77 currently ranges from a four- to six-lane divided freeway with a grassed median and a cable median barrier. The posted speed limit of the interstate varies from 60 mph to 70 mph.

CHE Surveying located and verified sizes of the existing drainage structures. AET Robotic Inspection Services evaluated the conditions of existing cross lines and cleaned drainage pipes. Topographic survey data and USGS quadrangle maps were used to identify drainage patterns. Drainage patterns show that runoff flows generally from northeast to southwest in the project limits. The larger waterbodies crossing I-77 include Crane Creek, Cumbess Creek, Little Jackson Creek, Jackson Creek (Windsor Lake), and an unnamed tributary to Gills Creek. The existing drainage infrastructure will require some field verification during construction. The infrastructure shown in the attached set of plans (See Appendix A) was put together from aerial survey data, topographic field survey, pipe video inventory survey, and field inspection. Some of the pipes shown were not field located but were drawn in by hand referencing asbuilts of past projects involving I-77 & I-20. In the plans, pipes which were drawn in per these asbuilts are assumed to still exist, and are noted as such.

It is assumed that the existing cross-line box culverts will not require extension due to construction of the project.

High-water marks were observed on the walls of the existing box culverts and on the piers of two of the existing bridges. Those structures and the approximate vertical location of their observed high-water marks are listed below:

EX-0308 (inlet of 8' x 6' box culvert EC-0201) – 1.5' above culvert floor, beyond slope and side tapers

EX-0399 (inlet of 6' x 6' box culvert EC-0102) – 2.0' above culvert floor, beyond slope and side tapers

I-77 NB Bridge over Windsor Lake (immediately downstream of dam) -1.0' below the approximate top of the north side bank (1.0' above the normal water surface observed during the field visit)



EX-0998 (inlet of 7.5' x 5' box culvert EC-0906) - 1.5' above culvert floor

I-77 SB Bridge over Little Jackson Creek / Edgewater Drive – at same elevation as the approximate top of the north side bank (3.5' above the normal water surface observed during the field visit)

EX-1491 (inlet of 6' x 5' box culvert EC-) - 2.5' above culvert floor

EX-1597 (inlet of 4' x 4' box culvert EC-) - 1.0' above culvert floor

EX-1796 (inlet of triple 8' x 6' box culvert EC-1702, EC-1703 & EC-1704) – 2.0' above culvert floor

EX-2196 (inlet of triple 12' x 12' box culvert EC-2103, EC-2105 & EC-2106) – 3.0' above culvert floor

Six specific outfalls were studied as part of this project. Each outfall was studied in a Pre-Construction conditions versus Post-Construction conditions analysis. A summary of the pre-versus post flows for the six major basins of the project is shown below.

Basin ID	I-77 Sta. at Outfall	Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase	
		2-Year	472.17	474.75	0.55	
1	1151+00	10-Year	975.83	979.41	0.37	
		50-Year	1587.76	1590.90	0.20	
		2-Year	1147.79	1151.79	0.35	
2	1249+00	10-Year	2178.39	2185.20	0.31	
		50-Year	2671.44	2679.24	0.29	
	1272+60	2-Year	138.88	138.88	0.00	
3		10-Year	258.88	258.88	0.00	
		50-Year	397.43	397.43	0.00	
	1329+00	2-Year	1241.63	1244.30	0.21	
4		10-Year	2329.70	2334.19	0.19	
		50-Year	2834.70	2839.80	0.18	
	1407+00	2-Year	313.11	314.16	0.33	
5		10-Year	547.10	548.16	0.19	
		50-Year	813.19	814.31	0.14	
	_	2-Year	2411.48	2411.48	0.00	
6	1469+50	10-Year	4328.86	4328.88	0.00	
		50-Year	5170.91	5170.92	0.00	

Table 1.1: Summary of Peak Flows at Major Basin Analysis Points



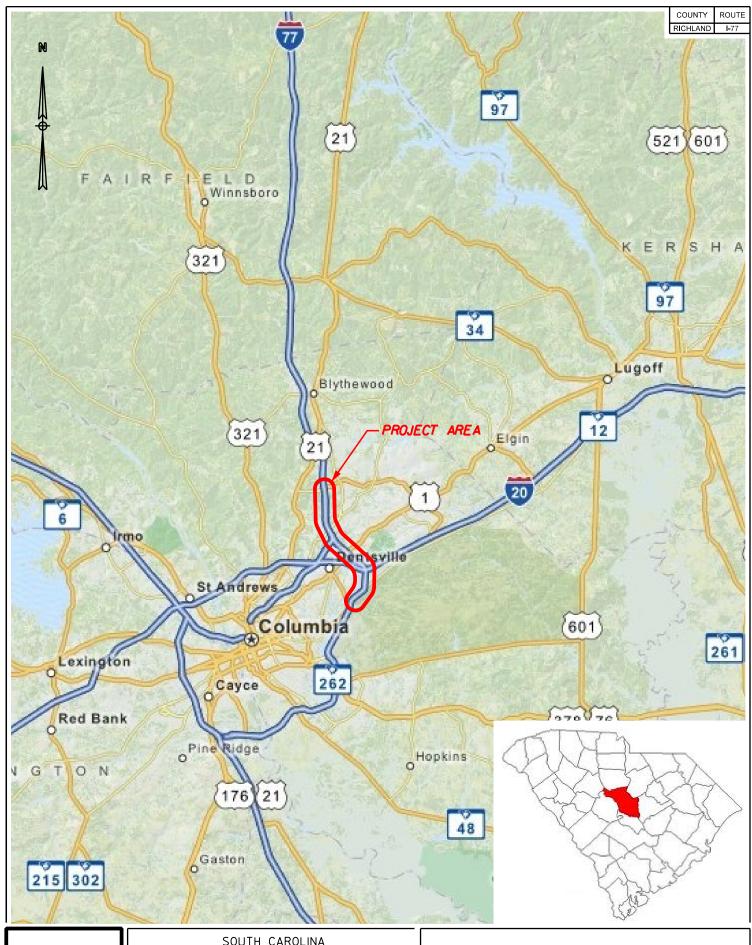
While this project will also entail minor improvements to interchange ramps along the corridor, the main hydrologic effect of the project will be to increase the impervious surface along the median. Due to the relatively minimal new impervious area (relative to the area of the total project basin) to be added by construction of the project, the increase in runoff from pre-construction conditions to post-construction conditions is nearly negligible. No detention should be necessary to mitigate post-construction flows.

Detailed discussion of the stormwater analysis is located in Section 4.0.





Project Location Maps



HDR Engineering, Inc.

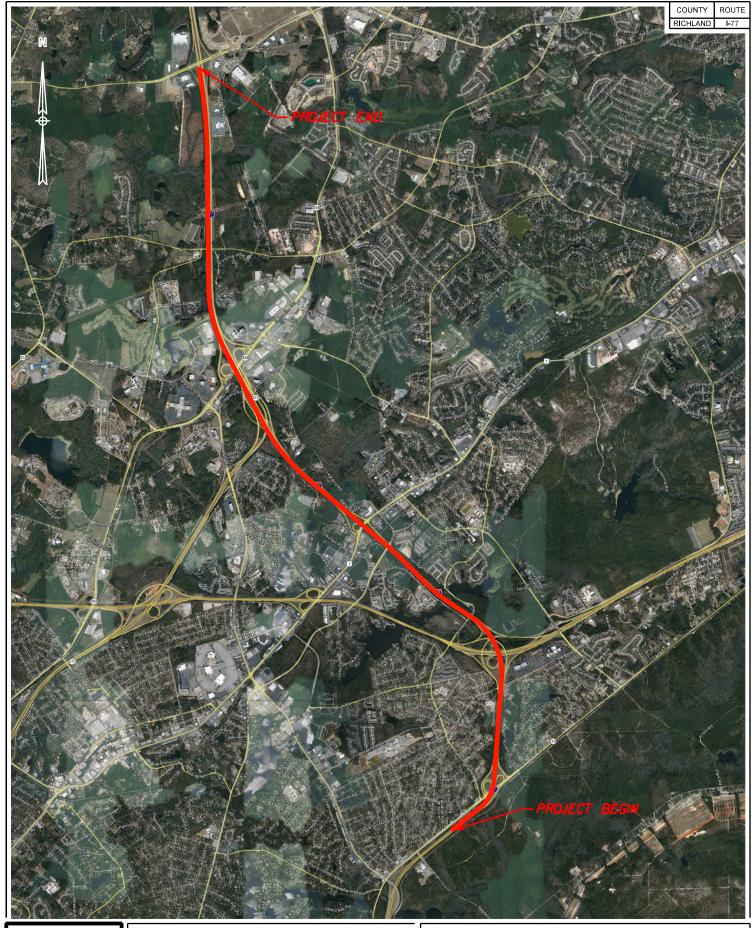
SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

VICINITY MAP

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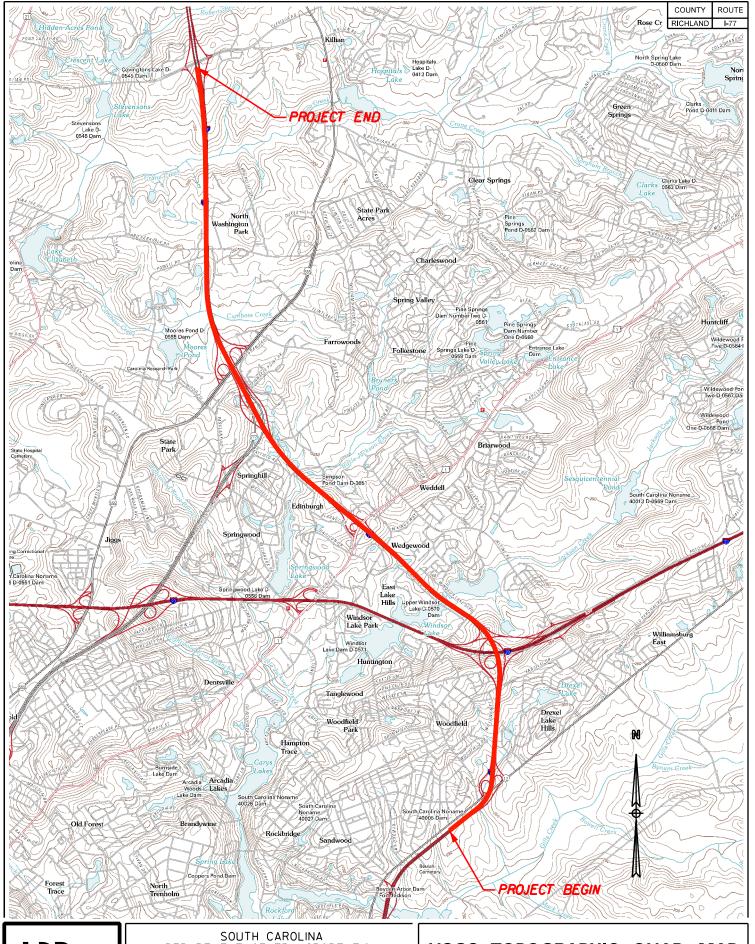
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DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

AERIAL MAP

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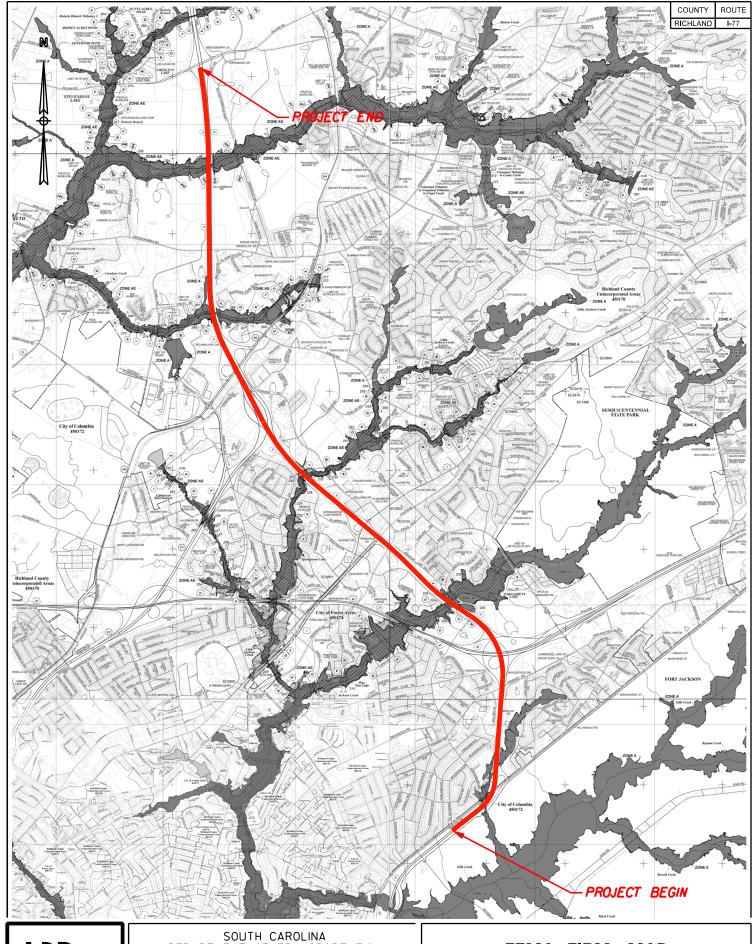
SOUTH CAROLINA
DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

USGS TOPOGRAPHIC QUAD MAP

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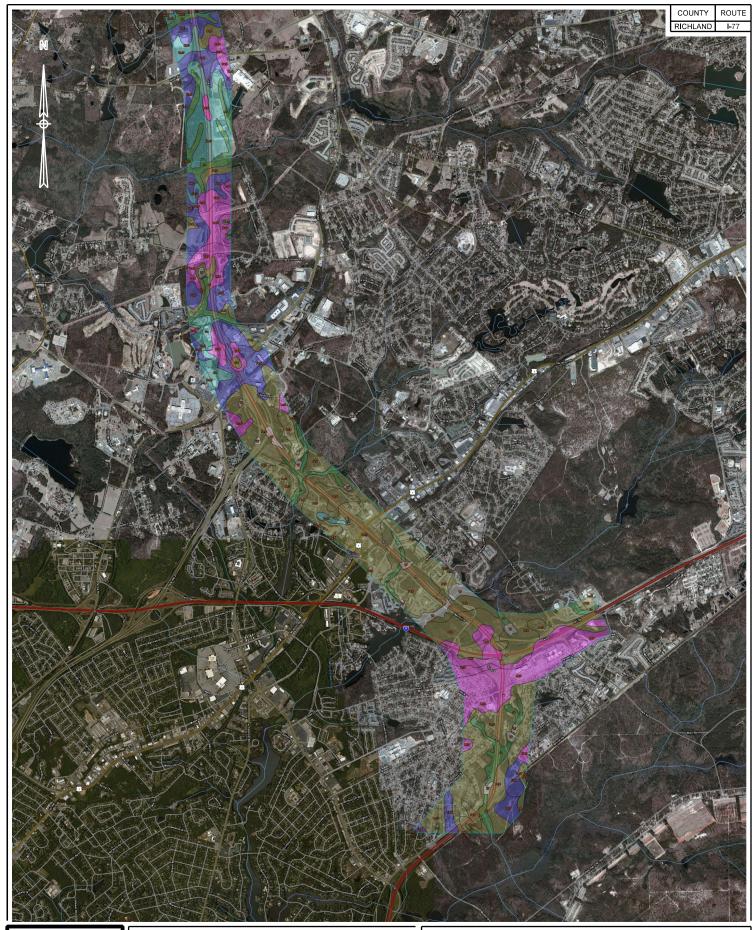
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DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

FEMA FIRM MAP

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DEPARTMENT OF TRANSPORTATION

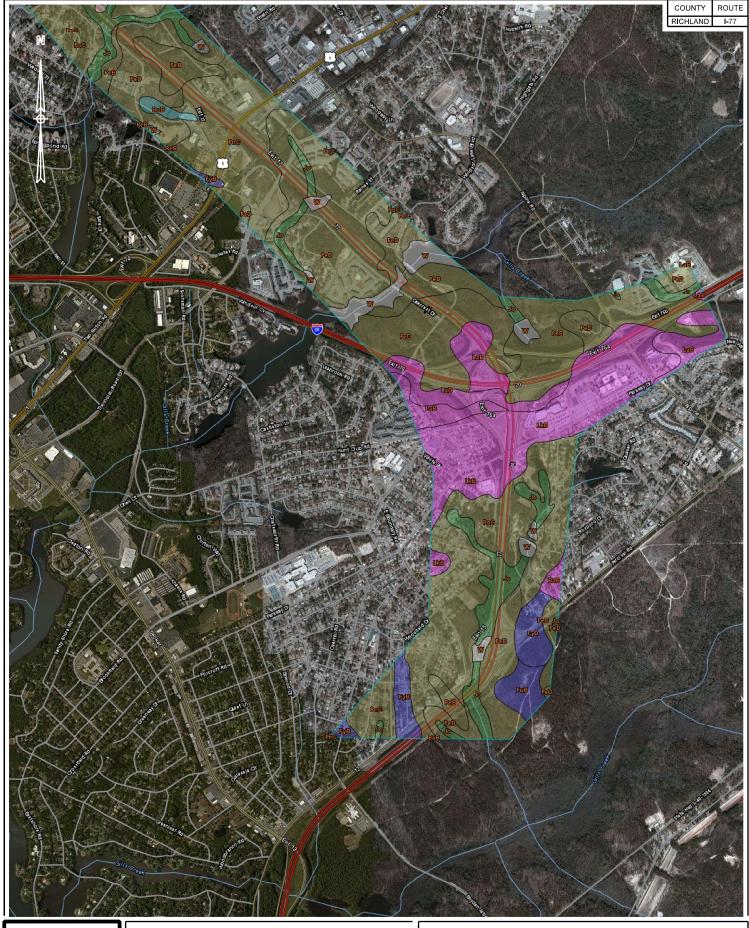
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

NRCS SOILS SURVEY MAP (ENTIRE PROJECT SITE)

0 2,000' 4,000'

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1GURE **2.5**



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SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

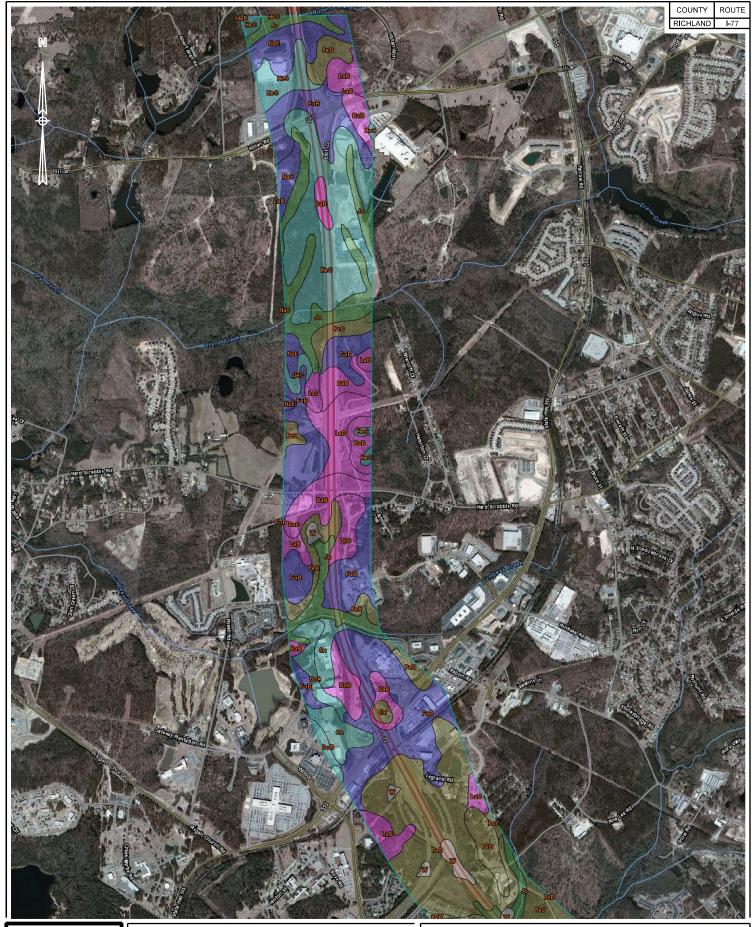
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

NRCS SOILS SURVEY MAP (1 OF 2)

0 1,000' 2,000'

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2.6.1





SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

NRCS SOILS SURVEY MAP (2 OF 2)

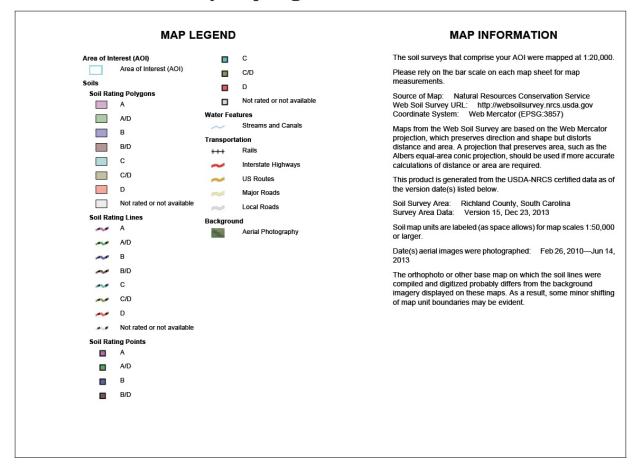
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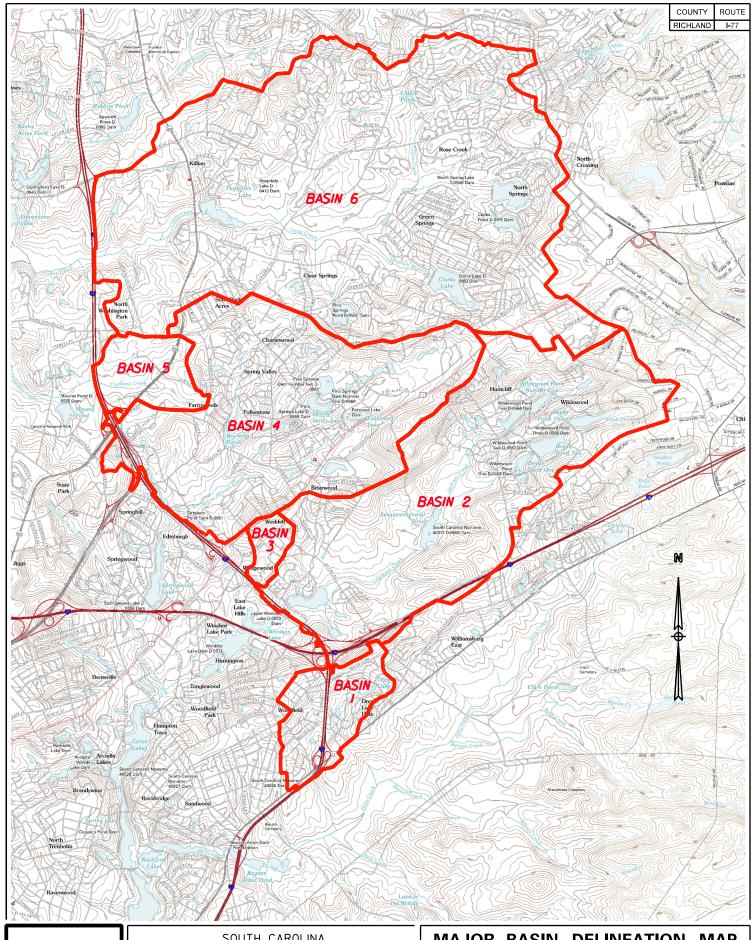
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2.6.2



2.7 NRCS Soils Survey Map Legend





HDR Engineering, Inc.

SOUTH CAROLINA
DEPARTMENT OF TRANSPORTATION
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)

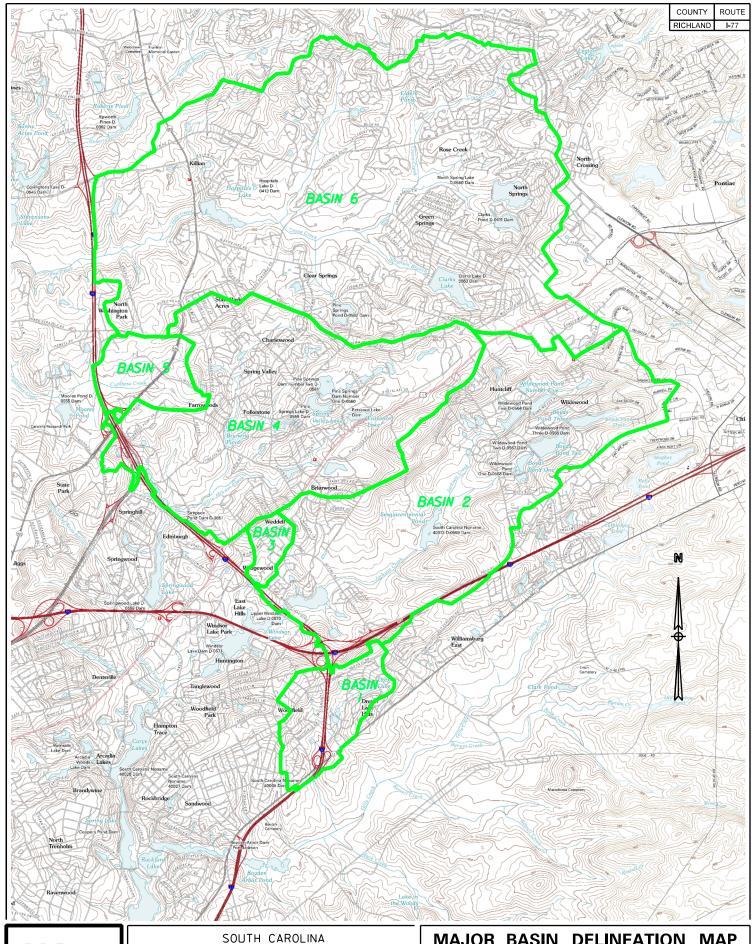
RICHLAND COUNTY, SOUTH CAROLINA

MAJOR BASIN DELINEATION MAP - PRE-DEVELOPED CONDITIONS

0 2,500' 5,000'

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FIGURE **2.8.1**



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I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

MAJOR BASIN DELINEATION MAP - POST-DEVELOPED CONDITIONS

0	2,500'	5,000'	
			-

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FIGURE **2.8.2**





Soils Information



3.1 Soils Information Overview

The I-77 Widening Project is located in the Sand Hills Region of South Carolina. The land use along the project corridor is a mix of residential, commercial, and undeveloped. A soil survey for Richland County was obtained from the Natural Resources Conservation Service (NRCS) database to determine the predominate soil types along the project corridor. The survey indicates Pelion type soils comprise the relative majority, approximately 49%, of the soils throughout the project area. Pelion type soils are classified as hydrologic group C/D, whereas they have a slow to very slow infiltration rate when wet. The NRCS soil survey and soil type descriptions for this project are included in the following sections.

3.2 Hydrologic Soil Group

Table 3.2: Hydrologic Soil Group Summary

Map Unit Symbol	Map Unit Name	Rating	Acres in AOI	Percent of AOI
ВаВ	Blanton sand, 0 to 6 percent slopes	А	92.4	4.0%
Cn	Clarendon sandy loam	С	29.2	1.3%
Cx	Coxville fine sandy loam	C/D	3.5	0.2%
DoA	Dothan loamy sand, 0 to 2 percent slopes	С	14.8	0.6%
DoB	Dothan loamy sand, 2 to 6 percent slopes	С	10.5	0.5%
FuB	Fuquay sand, 2 to 6 percent slopes	В	240.7	10.4%
FyB	Fuquay-Urban land complex, 0 to 6 percent slopes	В	40.1	1.7%
GeC	Georgeville silt loam, 6 to 10 percent slopes	С	5.0	0.2%
HeB	Herndon silt loam, 2 to 6 percent slopes	С	0.1	0.0%
HeC	Herndon silt loam, 6 to 10 percent slopes	С	131.1	5.7%
Jo	Johnston loam	A/D	181.4	7.8%
LaB	Lakeland sand, 2 to 6 percent slopes	А	124.7	5.4%
LaD	Lakeland sand, 10 to 15 percent slopes	А	33.8	1.5%
LkB	Lakeland-Urban land complex, 2 to 6 percent slopes	А	181.4	7.8%
NaC	Nason silt loam, 6 to 10 percent slopes	В	10.2	0.4%
NaE	Nason complex, 10 to 30 percent slopes	В	35.0	1.5%
PeB	Pelion loamy sand, 2 to 6 percent slopes	C/D	353.9	15.3%
PeD	Pelion loamy sand, 6 to 15 percent slopes	C/D	416.2	18.0%
PnC	Pelion-Urban land complex, 2 to 10 percent slopes	C/D	355.1	15.4%
W	Water		53.6	2.3%
Totals for Are	ea of Interest		2,312.7	100.0%

3.3 Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

3.4 Rating Options

Aggregation Method: Dominant Condition **Component Percent Cutoff:** None Specified

Tie-break Rule: Higher

3.5 Map Unit Description (Brief, Generated)

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions in this report, along with the maps, can be used to determine the composition and properties of a unit.



A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

The Map Unit Description (Brief, Generated) report displays a generated description of the major soils that occur in a map unit. Descriptions of non-soil (miscellaneous areas) and minor map unit components are not included. This description is generated from the underlying soil attribute data.

Additional information about the map units described in this report is available in other Soil Data Mart reports, which give properties of the soils and the limitations, capabilities, and potentials for many uses. Also, the narratives that accompany the Soil Data Mart reports define some of the properties included in the map unit descriptions.

Minor map unit components are excluded from this report.

3.6 Report - Map Unit Description (Brief, Generated)

Richland County, South Carolina

Map Unit: BaB—Blanton sand, 0 to 6 percent slopes

Component: Blanton (100%)

The Blanton component makes up 100 percent of the map unit. Slopes are 0 to 6 percent. This component is on marine terraces on sandhills. The parent material consists of sandy and loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 48 inches during January, February, March, and December. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 3s. This soil does not meet hydric criteria.

Map Unit: Cn—Clarendon sandy loam

Component: Clarendon (100%)

The Clarendon component makes up 100 percent of the map unit. Slopes are 0 to 2 percent. This component is on marine terraces on coastal plains. The parent material consists of loamy marine

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deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 24 inches during January, February, March, and December. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 2w. This soil does not meet hydric criteria.

Map Unit: Cx—Coxville fine sandy loam

Component: Coxville (100%)

The Coxville component makes up 100 percent of the map unit. Slopes are 0 to 2 percent. This component is on depressions, flats, marine terraces on coastal plains. The parent material consists of clayey marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is poorly drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is high. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 0 inches during January, February, March, April, November, and December. Organic matter content in the surface horizon is about 3 percent. Nonirrigated land capability classification is 3w. This soil meets hydric criteria.

Map Unit: DoA—Dothan loamy sand, 0 to 2 percent slopes

Component: Dothan (100%)

The Dothan component makes up 100 percent of the map unit. Slopes are 0 to 2 percent. This component is on marine terraces on coastal plains. The parent material consists of plinthic loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 36 inches during January, February, March, April. Organic matter content in the surface horizon is about 0 percent. Nonirrigated land capability classification is 1. This soil does not meet hydric criteria.

Map Unit: DoB—Dothan loamy sand, 2 to 6 percent slopes

Component: Dothan (100%)

The Dothan component makes up 100 percent of the map unit. Slopes are 2 to 6 percent. This component is on marine terraces on coastal plains. The parent material consists of plinthic loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 36 inches during January, February, March, April. Organic matter



content in the surface horizon is about 0 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Map Unit: FuB—Fuquay sand, 2 to 6 percent slopes

Component: Fuquay (100%)

The Fuquay component makes up 100 percent of the map unit. Slopes are 2 to 6 percent. This component is on marine terraces on coastal plains. The parent material consists of plinthic loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 60 inches during January, February, and March. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 2s. This soil does not meet hydric criteria.

Map Unit: FyB—Fuquay-Urban land complex, 0 to 6 percent slopes

Component: Fuquay (60%)

The Fuquay component makes up 60 percent of the map unit. Slopes are 0 to 6 percent. This component is on marine terraces on coastal plains. The parent material consists of plinthic loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately low. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 60 inches during January, February, and March. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 2s. This soil does not meet hydric criteria.

Component: Urban land (40%)

Generated brief soil descriptions are created for major soil components. The Urban land is a miscellaneous area.

Map Unit: GeC—Georgeville silt loam, 6 to 10 percent slopes

Component: Georgeville (100%)

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The Georgeville component makes up 100 percent of the map unit. Slopes are 6 to 10 percent. This component is on hillslopes on uplands. The parent material consists of clayey residuum weathered from slate. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is high. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 3e. This soil does not meet hydric criteria.

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Map Unit: HeB—Herndon silt loam, 2 to 6 percent slopes

Component: Herndon (100%)

The Herndon component makes up 100 percent of the map unit. Slopes are 2 to 6 percent. This component is on hillslopes on uplands. The parent material consists of clayey residuum weathered from slate. Depth to a root restrictive layer 62 inches, bedrock, paralithic. The natural drainage class is well drained. Water movement in the most restrictive layer is very low. Available water to a depth of 60 inches is high. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Map Unit: HeC—Herndon silt loam, 6 to 10 percent slopes

Component: Herndon (100%)

The Herndon component makes up 100 percent of the map unit. Slopes are 6 to 10 percent. This component is on hillslopes on uplands. The parent material consists of clayey residuum weathered from slate. Depth to a root restrictive layer 62 inches, bedrock, paralithic. The natural drainage class is well drained. Water movement in the most restrictive layer is very low. Available water to a depth of 60 inches is high. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 3e. This soil does not meet hydric criteria.

Map Unit: Jo—Johnston loam

Component: Johnston (100%)

The Johnston component makes up 100 percent of the map unit. Slopes are 0 to 2 percent. This component is on flood plains, coastal plains. The parent material consists of loamy alluvium. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is very poorly drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is low. This soil is frequently flooded. It is not ponded. A seasonal zone of water saturation is at 0 inches during January, February, March, April, May, June, July, August, September, October, November, and December. Organic matter content in the surface horizon is about 6 percent. Nonirrigated land capability classification is 7w. This soil meets hydric criteria.

Map Unit: LaB—Lakeland sand, 2 to 6 percent slopes

Component: Lakeland (100%)

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The Lakeland component makes up 100 percent of the map unit. Slopes are 2 to 6 percent. This component is on marine terraces on sandhills. The parent material consists of sandy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is excessively drained. Water movement in the most restrictive layer is high. Available water to a depth of 60 inches is

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low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 4s. This soil does not meet hydric criteria.

Map Unit: LaD—Lakeland sand, 10 to 15 percent slopes

Component: Lakeland (100%)

The Lakeland component makes up 100 percent of the map unit. Slopes are 10 to 15 percent. This component is on marine terraces on sandhills. The parent material consists of sandy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is excessively drained. Water movement in the most restrictive layer is high. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 7s. This soil does not meet hydric criteria.

Map Unit: LkB—Lakeland-Urban land complex, 2 to 6 percent slopes

Component: Lakeland (55%)

The Lakeland component makes up 55 percent of the map unit. Slopes are 2 to 6 percent. This component is on marine terraces on sandhills. The parent material consists of sandy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is excessively drained. Water movement in the most restrictive layer is high. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 4s. This soil does not meet hydric criteria.

Component: Urban land (45%)

Generated brief soil descriptions are created for major soil components. The Urban land is a miscellaneous area.

Map Unit: NaC—Nason silt loam, 6 to 10 percent slopes

Component: Nason (100%)

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The Nason component makes up 100 percent of the map unit. Slopes are 6 to 10 percent. This component is on hillslopes on uplands. The parent material consists of clayey residuum weathered from slate. Depth to a root restrictive layer, bedrock, paralithic, is 40 to 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 3e. This soil does not meet hydric criteria.

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Map Unit: NaE—Nason complex, 10 to 30 percent slopes

Component: Nason (100%)

The Nason component makes up 100 percent of the map unit. Slopes are 10 to 30 percent. This component is on hillslopes on uplands. The parent material consists of clayey residuum weathered from slate. Depth to a root restrictive layer, bedrock, paralithic, is 40 to 60 inches. The natural drainage class is well drained. Water movement in the most restrictive layer is moderately high. Available water to a depth of 60 inches is moderate. Shrink-swell potential is moderate. This soil is not flooded. It is not ponded. There is no zone of water saturation within a depth of 72 inches. Organic matter content in the surface horizon is about 2 percent. Nonirrigated land capability classification is 4e. This soil does not meet hydric criteria.

Map Unit: PeB—Pelion loamy sand, 2 to 6 percent slopes

Component: Pelion (90%)

The Pelion component makes up 90 percent of the map unit. Slopes are 2 to 6 percent. This component is on marine terraces on sandhills. The parent material consists of loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is low. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 12 inches during January, February, March, April, November, and December. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 2e. This soil does not meet hydric criteria.

Component: Rains (2%)

Generated brief soil descriptions are created for major components. The Rains soil is a minor component.

Component: Johnston (1%)

Generated brief soil descriptions are created for major components. The Johnston soil is a minor component.

Map Unit: PeD—Pelion loamy sand, 6 to 15 percent slopes

Component: Pelion (100%)

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The Pelion component makes up 100 percent of the map unit. Slopes are 6 to 15 percent. This component is on marine terraces on sandhills. The parent material consists of loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is low. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water



saturation is at 12 inches during January, February, March, April, November, and December. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 6e. This soil does not meet hydric criteria.

Map Unit: PnC—Pelion-Urban land complex, 2 to 10 percent slopes

Component: Pelion (60%)

The Pelion component makes up 60 percent of the map unit. Slopes are 2 to 10 percent. This component is on marine terraces on sandhills. The parent material consists of loamy marine deposits. Depth to a root restrictive layer is greater than 60 inches. The natural drainage class is moderately well drained. Water movement in the most restrictive layer is low. Available water to a depth of 60 inches is low. Shrink-swell potential is low. This soil is not flooded. It is not ponded. A seasonal zone of water saturation is at 12 inches during January, February, March, April, November, and December. Organic matter content in the surface horizon is about 1 percent. Nonirrigated land capability classification is 4e. This soil does not meet hydric criteria.

Component: Urban land (40%)

Generated brief soil descriptions are created for major soil components. The Urban land is a miscellaneous area.

Map Unit: W—Water

Component: Water (100%)

Generated brief soil descriptions are created for major soil components. The Water is a miscellaneous area.

3.7 Data Source Information

Soil Survey Area: Richland County, South Carolina

Survey Area Data: Version 15, Dec 23, 2013



4.0

Stormwater Analysis



4.1 Stormwater Analysis Overview

The hydrologic analysis for this project was performed in accordance with SCDOT's *Requirements for Hydraulic Design Studies*, dated May 26, 2009. This project includes the widening of existing interstate roadway and minor improvements to associated interchange ramps. The proposed construction will not significantly alter or impact existing drainage patterns.

Drainage areas were delineated using survey data provided by both SCDOT & CHE Surveying, USGS Quad maps for Richland County, Google Earth aerial imaging, and field visits to verify offsite flow patterns. The hydrologic methods used to determine peak discharges were selected upon watershed area and consisted of the Rational Method, the modified NRCS WinTR-55 Method, and USGS Regression Equations. Times of concentrations were computed using the SCS methodology outlined in the WinTR-55 manual, with a minimum time of concentration of 5 minutes. SCDOT-published Rainfall Intensity Value Charts for Columbia, SC were used to determine rainfall intensities (in/hr).

Drainage areas, drainage paths, flow lengths, and watercourse slopes were determined using surveyed contours and the USGS 7.5' Quadrangles for Blythewood, Elgin, Fort Jackson North and Messers Pond. Runoff coefficients and Manning's *n* values were estimated from aerial photography. Table 4.1 outlines the hydrologic methods used.

4.2 Drainage Design Criteria

			SOURCE		
		I-77	Req. For Hydr. Design Studies		
				Section	
		0 - 100 AC	Rational Method	2.2.15.1	
Hydrologic Method	Drainage Area	100 AC - 640 AC	NRCS WinTR-55 Method	2.2.15.2	
Method		> 640 AC	USGS Regression Equations	2.2.15.3	
Freebo	Freeboard For Road Subgrades		1' Above Design High-Water Level	2.2.1	
	Cross-Line Pipes		50 year	2.2.2	
	CI USS-LINE	: ripes	100 year	2.2.2	
Docian	Closed Storm	0 - 40 AC	10 year		
Design Storm	Sewer	40 - 500 AC	25 year	2.2.3	
	Systems	> 500 AC	50 year		
Median Over		rtopping	50 year	2.2.4	
Preferred Headwater to Culvert Barrel Height Ratio		1.2	2.3 (Step 6, Sec. D, pg. 61)		



4.3 Rainfall Depths (SCS Method)

Rainfall depths used in determining peak discharges through the SCS Method were obtained from the South Carolina DHEC Storm Water Management BMP Handbook. The 24-hour storm event rainfall depths for Richland County are shown below.

Table 4.3: 24-Hour Rainfall Depths for Richland County

Return Period	Depth
(years)	(inches)
1	3.1
2	3.6
5	4.5
10	5.3
25	6.4
50	7.3
100	8.3

4.4 Rainfall Intensity Values (Rational Method)

Coefficients used in the rational method for calculating rainfall intensity values were provided by SCDOT. The coefficients for Columbia are shown below.

TABLE 4.3: RAINFALL INTENSITY COEFFICIENTS

Frequency (years)	а	b	С
2	244.34492	34.95806	1.03155
5	258.50572	32.75684	1.01773
10	267.54247	31.39986	1.00904
25	279.77346	29.59043	0.99735
50	288.71309	28.26125	0.98879
100	296.66217	27.04859	0.98111

The coefficients are utilized in the following equation to calculate rainfall intensities.

Rainfall intensity equation: $i = \frac{a}{(b+tc)^c}$

4.5 Zoning Groups of Similar Surface Cover

In determining runoff flows from offsite areas, fully-developed conditions were assumed. A second assumption was that there will be no upstream detention upon development of the offsite areas. This method yields relatively large flows for those basins with substantial offsite areas (Major Basins 1-6).

In order to simulate fully-developed conditions, offsite areas were delineated based on Richland County zoning. Using the Richland County GIS website (http://www.richlandmaps.com/apps/gmap/), zoning maps for the offsite drainage basin areas were developed, scaled and overlaid upon the project survey area in Microstation. For ease of calculations, some zoning districts were assumed to have similar surface cover, and were grouped together accordingly. Those groups are listed below:



Zoning Group 1

Residential, Single Family - Medium Density District Residential, Multi-Family - Medium Density District Manufactured Home District Light Industrial District

Zoning Group 2

Planned Development District
Residential, Single Family - Low Density District

Zoning Group 3

General Commercial District
Heavy Industrial District
Office & Institutional District

Zoning Group 4

Traditional Recreation Open Space District Rural District

Surface cover characteristics were assigned to each of the zoning groups based on district ordinances and on aerial photography of representative developed areas within each group. Areas outside of the zoning groups were assigned surface cover designations based on provided surveys and aerial photography (See Figure 2.2).

4.6 Impervious Area by Zoning Group

Basins 2, 4 & 6 were modeled using the urban regression calculations detailed in USGS Water-Resources Investigations Report 92-4040 "Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds." The equations in Table 9 and Supplement A of said report calculate flows based on two parameters: A (drainage area, in square miles) and TIA (total impervious area, in percent of total drainage area). TIA was calculated based on the assigned percentage of impervious area for each zoning group and from the provided surveys. A breakdown of the impervious area assigned to each zoning group is shown below:

Table 4.6:

Zoning Group #	% Impervious Area
1	27
2	53
3	68
4	15

A breakdown of the composite TIA for Basins 2, 4 & 6 can be found in Appendix B.



4.7 NRCS Method by Zoning Group

Basins 1, 3, 5 (NRCS Basins) were modeled using the modified NRCS WinTR-55 method. In accordance with NRCS methods, curve numbers (*CNs*) were determined for each drainage basin by delineating areas of different soil type and surface cover, for Pre-Construction conditions and Post-Construction conditions alike.

The soil classifications were determined through maps obtained from the Richland County GIS Website, which were overlaid onto the project area. See Appendix C for soil class delineation maps of the NRCS basins.

Composite *CN*s were calculated for each of the NRCS Basins based on surface cover and soil class. For offsite areas, the zoning groups described earlier were assigned *CN*s for each soil type (see Table 4.7 below).

	Curve Number per Hydrologic Soil Group			
Zoning Group #	oup# A B C D			
1	77	85	90	92
2	57	72	81	86
3	89	92	94	95
4	49	69	79	84

Table 4.7: Curve Number per Zoning Group / Hydrologic Soil Group

A breakdown of the composite CNs for Basins 1, 3, & 5 can be found in Appendix C.

4.8 Pre- and Post-Construction Development Analysis

An analysis was performed to evaluate potential peak discharge rate increases of the major basins within the project limits based on Pre- and Post-Construction conditions related to the widening of I-77.

As previously discussed in Section 1, the increase in flow from the project's construction will be nearly negligible. Drainage basin delineation areas and flow paths are not expected to change as a result of this project's construction. The small increases in total impervious area and in composite curve numbers account for the increases in flows from Pre-Construction conditions to Post-Construction conditions.

Basin 1

Analysis Point = Outlet OP-0100 (Downstream end of Box Culverts EC-0102, EC-0103, & EC-0211) I-77 Approx. Sta. 1151+00 RT (See Plan Sheet 01)

Basin 1 is comprised of land on both sides of I-77 from approximate Sta. 1140+75 (immediately south of the I-77 NB off-ramp to Percival Road) to approximate Sta. 1212+00 (near the I-77 / I-20 interchange). Offsite areas to the east of I-77 generally drain to the southwest towards the Percival Road interchange. Likewise, offsite areas to the west of I-77 generally drain to the southeast towards the Percival Road interchange. The outfall point for Basin 1 is at the downstream end of three box culverts which pass



beneath the Percival Road interchange and terminate to the south of I-77 into an unnamed tributary to Gills Creek.

The total drainage area is approximately 571 acres of roadway and off-site areas. The most remote point in the watershed is approximately 7,975 feet from the outfall. Soils in this watershed are primarily classified as being in hydrologic soil groups A & B, with some D soils.

Peak flows in Basin 1 were estimated using the NRCS Method. The addition of new impervious area to the drainage basin by the construction of the proposed project will increase the Composite Curve Number from 74.02 to 74.14. Table 4.8.1 below shows a summary of Pre-Construction flows versus Post-Construction flows for Basin 1.

Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase
2-Year	472.17	474.75	0.55
5-Year	729.70	732.73	0.41
10-Year	975.83	979.41	0.37
25-Year	1303.35	1306.68	0.25
50-Year	1587.76	1590.90	0.20
100-Year	1871.84	1875.14	0.18

Table 4.8.1: Summary of Basin 1 Pre-Construction Flows vs. Post-Construction Flows

See Appendix C for NRCS Delineation by Zoning Group / Surface Cover, Composite Curve Number, and Peak Flow calculations for Pre-Construction conditions vs. Post-Construction conditions, and for Time of Concentration calculations.

Basin 2

Analysis Point = I-77 NB Bridge Over Windsor Lake / Jackson Creek I-77 Approx. Sta. 1249+00 RT (See Plan Sheet 08)

Basin 2 is comprised of land to the northeast of I-20 and of I-77, including a large portion of Sesquicentennial State Park. The limits of the basin along I-77 are from approximate Sta. 1212+00 (prior to the I-77 / I-20 interchange) to approximate Sta. 1270+30. Flows generally drain to the west and south via Jackson Creek and its many connected lakes. The analysis point for this basin is the I-77 NB bridge over Windsor Lake / Jackson Creek.

The total drainage area is approximately 3,703 acres of roadway and off-site areas. The most remote point in the watershed is approximately 23,648 feet from the bridge. Soils in this watershed are generally classified as being in hydrologic soil groups A, B, & D.



Peak flows in Basin 2 were estimated using the USGS Rural Regression Equations. Due to construction of the proposed project, the basin's Total Impervious Area percentage will increase from 32.14% to 32.23%. Table 4.8.2 below shows a summary of Pre-Construction flows versus Post-Construction flows for Basin 2.

Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase
2-Year	1147.79	1151.79	0.35
5-Year	1758.25	1764.03	0.33
10-Year	2178.39	2185.20	0.31
25-Year	2671.44	2679.24	0.29
50-Year	3036.95	3045.36	0.28
100-Year	3409.43	3418.34	0.26

Table 4.8.2: Summary of Basin 2 Pre-Construction Flows vs. Post-Construction Flows

See Appendix B for USGS Regression Equation and associated Total Basin Impervious Area calculations for Pre-Construction conditions vs. Post-Construction conditions.

Basin 3

Analysis Point = EX-0998 (Inlet of Box Culvert EC-0906) I-77 Approx. Sta. 1272+60 RT (See Plan Sheet 09)

Basin 3 is comprised of land to the east of I-77 and to the southeast of Two Notch Road. Runoff from I-77 approximate Sta. 1270+30 to approximate Sta. 1278+50 flows to this basin. Flows generally drain to the southwest in this basin. The analysis point for Basin 3 is the inlet of existing 7.5' x 5' box culvert EC-0906.

The total drainage area is approximately 140 acres of roadway and off-site areas. The most remote point in the watershed is approximately 3,944 feet from the outfall. Soils in this watershed are primarily classified as being in hydrologic soil group B, with some A & D soils.

Peak flows in Basin 3 were estimated using the NRCS Method. The conceptual design files provided by the Department show no new impervious area added within this basin. Thus, the Composite Curve Number remains a constant 80.90, and there is no increase in flows due to construction of the project. Table 4.8.3 below shows a summary of Pre-Construction flows versus Post-Construction flows for Basin 3.

Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase
2-Year	138.88	138.88	0.00

5-Year	200.67	200.67	0.00
10-Year	258.88	258.88	0.00
25-Year	333.50	333.50	0.00
50-Year	397.43	397.43	0.00
100-Year	461.14	461.14	0.00

Table 4.8.3: Summary of Basin 3 Pre-Construction Flows vs. Post-Construction Flows

See Appendix C for NRCS Delineation by Zoning Group / Surface Cover, Composite Curve Number, and Peak Flow calculations for Pre-Construction conditions vs. Post-Construction conditions, and for Time of Concentration calculations.

Basin 4

Analysis Point = I-77 NB Bridge Over Little Jackson Creek I-77 Approx. Sta. 1329+00 (See Plan Sheet 13)

Basin 4 is comprised of land to the northeast of I-77 roughly between Two-Notch Road and Hard Scrabble Road. A not insignificant offsite land area to the west of the I-77 / I-277 interchange flows into this basin. Drainage patterns generally flow to the west and south via Little Jackson Creek and its many connected lakes. The analysis point for this basin is the I-77 NB bridge over Little Jackson Creek.

The total drainage area is approximately 3,302 acres of roadway and off-site areas. The most remote point in the watershed is approximately 18,647 feet from the outfall. Soils in this watershed are generally classified as being in hydrologic soil groups A, B, & D.

Peak flows in Basin 4 were estimated using the USGS Rural Regression Equations. Due to construction of the proposed project, the basin's Total Impervious Area percentage will increase from 36.84% to 36.90%. Table 4.8.4 below shows a summary of Pre-Construction flows versus Post-Construction flows for Basin 4.

Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase
2-Year	1241.63	1244.30	0.21
5-Year	1890.95	1894.77	0.20
10-Year	2329.70	2334.19	0.19
25-Year	2834.70	2839.80	0.18
50-Year	3202.79	3208.25	0.17
100-Year	3573.97	3579.73	0.16

Table 4.8.4: Summary of Basin 4 Pre-Construction Flows vs. Post-Construction Flows

See Appendix B for USGS Regression Equation and associated Total Basin Impervious Area calculations for Pre-Construction conditions vs. Post-Construction conditions.

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Basin 5

Analysis Point = EX-1796 (Inlet of Box Culvert EC-1702) I-77 Approx. Sta. 1407+00 RT (See Plan Sheet 17)

Basin 5 is comprised of land to the east of I-77 beyond its interchange with Farrow Road. Flows generally drain to the west via Cumbess Creek. Runoff from I-77 collects within this basin from approximate Sta. 1390+15 (just beyond the interchange of I-77 with Farrow Road) to approximate Sta. 1415+90. The analysis point for Basin 5 is the inlet of existing triple 8' x 6' box culvert EC-1702, where Cumbess Creek crosses I-77.

The total drainage area is approximately 431 acres of roadway and off-site areas. The most remote point in the watershed is approximately 6,089 feet from the outfall. Soils in this watershed are generally classified as being in hydrologic soil groups A, B, & D.

Peak flows in Basin 5 were estimated using the NRCS Method. The addition of new impervious area to the drainage basin by the construction of the proposed project will increase the Composite Curve Number from 86.04 to 86.15. Table 4.8.5 below shows a summary of Pre-Construction flows versus Post-Construction flows for Basin 5.

Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase
2-Year	313.11	314.16	0.33
5-Year	434.06	435.20	0.26
10-Year	547.10	548.16	0.19
25-Year	690.25	691.44	0.17
50-Year	813.19	814.31	0.14
100-Year	934.74	935.88	0.12

Table 4.8.5: Summary of Basin 5 Pre-Construction Flows vs. Post-Construction Flows

See Appendix C for NRCS Delineation by Zoning Group / Surface Cover, Composite Curve Number, and Peak Flow calculations for Pre-Construction conditions vs. Post-Construction conditions, and for Time of Concentration calculations.

Basin 6

Analysis Point = EX-2196 (Inlet of Box Culvert EC-2105) I-77 Approx. Sta. 1469+50 RT (See Plan Sheet 21)

Basin 6 is comprised of a large swath of land to the east of I-77 within the watershed of Crane Creek. Drainage patterns within this basin generally flow to the west via Crane Creek and its many lakes. Runoff from I-77 flows into Basin 6 from approximate Sta. 1459+20 to approximate Sta. 1475+45. The analysis

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point for Basin 6 is the inlet of existing triple 12' x 12' box culvert EC-2105, where Crane Creek crosses I-77.

The total drainage area is approximately 7,125 acres of roadway and off-site areas. The most remote point in the watershed is approximately 27,915 feet from the outfall. Soils in this watershed are primarily classified as being in hydrologic soil groups A, B, & D, with some type C soils.

Peak flows in Basin 6 were estimated using the USGS Rural Regression Equations. The conceptual design files provided by the Department show a relatively infinitesimal increase in impervious area within this basin, with the basin's Total Impervious Area percentage increasing from 38.4776% to 38.4777%. This results in negligible increases in flow as a result of this project's construction. Table 4.8.6 below shows a summary of Pre-Construction flows versus Post-Construction flows for Basin 6.

Design Storm Event	Pre- Developed Flows (cfs)	Post- Developed Flows (cfs)	% Increase
2-Year	2411.48	2411.48	0.00
5-Year	3567.47	3567.48	0.00
10-Year	4328.86	4328.88	0.00
25-Year	5170.91	5170.92	0.00
50-Year	5780.89	5780.90	0.00
100-Year	6371.93	6371.94	0.00

Table 4.8.6: Summary of Basin 6 Pre-Construction Flows vs. Post-Construction Flows

See Appendix B for USGS Regression Equation and associated Total Basin Impervious Area calculations for Pre-Construction conditions vs. Post-Construction conditions.

4.9 Existing Cross-Line Culvert Analysis

HY-8 version 7.2 (build date January 17, 2012) was used to analyze the headwaters at existing box culverts and open-ended pipe culverts. Culvert analyses were based on a maximum 1.2 headwater to depth (HW/D) ratio for the 50-year rainfall event.

For those box culverts which connect to a downstream structure, tailwater elevations were estimated as the elevation of the normal depth in the pipe immediately prior to the downstream structure. Normal depth of flow in these cases was estimated using the FHWA Hydraulic Toolbox 2.1. For those pipes which connect to a downstream structure, tailwater elevation was assumed as the soffit elevation of the pipe at the downstream structure.

For those culverts that did not connect to a downstream structure but rather outfall into a stream or ditch, the outfall characteristics were used for tailwater elevation estimation. A Manning's "n" value of 0.04 was used for the outfall ditches.



The geometries of the existing inlet openings and head walls were measured in the field and applied accordingly in the HY-8 program.

The analysis results for each culvert opening are listed in Table 4.9 below.

I-77 Station	Pipe ID	Basin	Culvert Shape	Diam- eter (ft)	Span (ft)	Rise (ft)	50-yr HW / D	Over- topping Elev. (ft)	50-yr HW Elev. (ft.)	100-yr HW Elev. (ft.)
1168+32	EC-0102	EX-0399	Box		6	6	1.41	255.00	244.55	247.99
1172+32	EC-0211	EX-0308	Box		8	6	0.79	256.00	249.73	251.31
1272+60	EC-0906	EX-0998 (3)	Box		7.5	5	1.53	254.00	249.52	250.79
1279+53	EP-0908	EX-0993	Circ.	3			1.79	259.00	253.79	254.33
1342+08	EP-1303	EX-1395	Circ.	1.5			1.76	259.00	257.74	258.05
1360+01	EC-1407	EX-1491	Box		6	5	0.58	284.00	271.78	271.94
1369+17	EC-1503	EX-1597	Box		4	4	0.88	303.50	295.47	295.68
1395+12	EP-1799	EX-1798	Circ.	2			0.93	316.80	315.35	315.46
1407+01	EC-1702	EX-1796 (5)	Box		8	6	0.89	308.50	291.32	291.84
1424+12	EP-1804	EX-1894	Circ.	2.5			2.08	312.50	312.53	312.57
1434+60	EP-1999	EX-1997	Circ.	2			0.91	321.50	319.71	319.80
1434+63	EP-1998	EX-1996	Circ.	2.5			1.33	321.50	321.23	321.51
1458+57	EP-2101	EX-2198	Circ.	3			1.45	310.80	308.56	308.94
1469+54	EC-2103	EX-2196 (6)	Box		12	12	1.26	292.50	274.82	276.19

Table 4.9: HY-8 Results for Existing Open-Ended Pipes & Box Culverts

As can be seen in the table above, Culverts EC-0102, EC-0906, EP-0908, EP-1303, EP-1804, EP-1998, EP-2101, & EC-2103 have headwater to depth (HW/D) ratios for the 50-year event that are above the maximum design standard of 1.2. Construction of this project, however, will not affect upstream headwater elevations significantly, and will not have a detrimental effect on upstream properties.

There are potential subgrade issues with freeboard at the inlets of Culverts EP-1303, EP-1709, EP-1999 & EP-1998, and overtopping concerns with EP-1804 & EP-1998. These culverts may need abandoning and replacement.

More detailed HY-8 analysis results can be found in Appendix D.

4.10 Existing Closed Drainage System Analysis

Existing storm drainage systems were analyzed using GEOPak Drainage. The 10-year design storm was used to analyze all storm drainage systems whose elevations were known or which could be reasonably estimated. GEOPak Drainage Area, Link and Node calculations can be found in Appendix E.

Based on preliminary analysis, some of the pipes are flowing at greater than 94% flow depth for the 10-year storm event.

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Median ditch inlets located in sag areas were analyzed for the 50-year storm event to determine if there were hydraulic gradeline blowouts that would overtop the roadway shoulder. Table 4.10 lists the sag inlets, the 10-year and 50-year storm event hydraulic gradeline elevations, and the highway shoulder elevation at the inlet.

Inlet ID	10-Year Upstream Hydraulic Gradeline Elevation (ft)	50-Year Upstream Hydraulic Gradeline Elevation (ft)	Highway Shoulder Elevation (ft)
EX-0302	254.58	259.39	256.50
EX-0601	334.42	334.74	338.50
EX-0605	319.37	320.33	323.00
EX-0701	276.03	277.62	280.00
EX-0702	242.17	243.60	245.75
EX-0801	239.48	242.54	242.90
EX-0803	244.59	245.48	249.00
EX-0901	248.42	249.45	254.50
EX-1202	247.59	248.05	253.90
EX-1301	251.94	252.50	256.00
EX-1501	283.87	284.94	290.50
EX-1801	302.58	304.11	309.00
EX-2102	287.04	291.91	291.70

Table 4.10: Summary of 10-Year & 50-Year Storm Events HGL Elevations at Median Sag Inlet Locations

Inlets EX-0302 and EX-2102 are showing to overtopping of the shoulder during the 50-year storm event. These locations also have possible hydraulic gradeline concerns during the 10-year storm event. These inlets could have potential freeboard issues regarding the subgrade.

While it is expected that water at inlet EX-0801 would overtop the median embankment at the bridge instead of overtopping the shoulder, the hydraulic gradeline during the 50-year storm event is a concern. There could be potential freeboard issues at the subgrade in the vicinity of this inlet.

Existing median inlets were also visually inspected at the surface level for structural integrity. The inlets were all Type 112, and most appeared to be in good condition. Those inlets found to have structural or other concerns are listed below.

Median Inlet EX-0801

I-77 Sta. 1252+01 (See Plan Sheet 8)

The surrounding ground adjacent to EX-0801 appears to have sunken in to roughly 4" to 6" below the rim of the inlet. No evidence of this issue was found in the video survey inspection (detailed in the next section) of connecting existing pipe EP-0801, which was in good condition.

If this inlet is to be retained, the existing ground should be brought up to the rim elevation in order to reduce ponding. A paved apron could also be installed at this inlet.



EX-0801

Median Inlet EX-0901 I-77 Sta. 1270+50 (See Plan Sheet 9)

The paved apron for EX-0901 has some minor cracking. If this inlet is retained, the apron should be repaired or replaced.



EX-0901

Median Inlet EX-1404 I-77 Sta. 1355+38 (See Plan Sheet 14)



A severe collapse of the paved apron and subgrade has occurred at EX-1404, and a large cavity is visible adjacent to the box. No evidence of this issue was found in the video survey inspection of connecting existing pipes EP-1404 & EP-1405, which were in good condition at the inlet. In the video inspection of EP-1406, it appears that the first segment of pipe (upstream from the inlet) is slightly offset from the second segment. This offset appears to be related to the apron collapse.

If this inlet is to be retained, the subgrade adjacent to the box should be excavated and replaced, and the paved apron should be removed and replaced. The pipe video inventory survey detailed in the next section contains recommendations for the connecting pipe.



EX-1404

Median Inlet EX-1501

I-77 Sta. 1362+46 (See Plan Sheet 15)

The paved apron of EX-1501 has a significant hole in its northeast corner, where the subgrade has apparently collapsed. No evidence of this issue was found in the video survey of connecting existing pipe EP-1501, which was in good condition.

If this inlet is to be retained, the subgrade adjacent to the box should be excavated and replaced, and the paved apron should be removed and replaced, at least in the northeast corner where the hole was found.



EX-1501

Median Inlet EX-1607 I-77 Sta. 1383+64 (See Plan Sheet 16)

There has been significant vegetative growth adjacent to EX-1607 that could possibly impede flow to the inlet. In the pipe video survey, major root issues were found in connecting existing pipes EP-1608 & EP-1609 on either side of the box.

If this inlet is to be retained, the area adjacent to the inlet should be cleared and grubbed. A paved apron could also be installed at this inlet. The pipe video inventory survey detailed in the next section contains recommendations for the connecting pipes.



EX-1607

Median Inlet EX-1701 I-77 Sta. 1399+91 (See Plan Sheet 17)



A significant collapse of the paved apron and subgrade has occurred at EX-1404. No evidence of this issue was found in the video survey inspection of connecting existing pipe EP-1701, which was in good condition at the inlet.

If this inlet is to be retained, the subgrade adjacent to the box should be excavated and replaced, and the paved apron should be removed and replaced.



EX-1701

4.11 Video Inventory Survey of Existing Pipes & Culverts

AET Robotic Inspection Services performed a video inventory survey of all accessible pipes which crossed I-77 or crossed any of the associated ramps. Evaluations and recommendations are based on visual review of the videos. A detailed structural inspection has not been performed. The findings of the survey and subsequent evaluations are listed in Table 4.11 below:

See Appendix F for reports of individual pipes.

File No.	Pipe ID	I-77 Station	Pipe / Culvert Type	Measured Footage (ft)	Evaluation of Pipe
	EC-0102	1152+00	6' x 6' Conc. Box Culvert	1660	Patch cracks in culvert (approximately 94 locations).
	EC-0103	1152+00	6' x 6' Conc. Box Culvert	596	Clean sediment buildup throughout. Patch cracks in culvert (approximately 17 locations).
	EC-0211	1152+00	8' x 6' Conc. Box Culvert	2087	Headwall in good condition. Patch cracks in culvert (approximately 140 locations). Clean sediment buildup from 2080' to end.
	EP-0302	1173+55	18" RCP	153.1	If existing pipe is to remain, repair last joint located 8' prior to downstream end. Preliminary hydro analysis indicates that additional capacity may be needed at this location.



EP-030	5 1181+00	18" RCP	124.3	Pipe in good condition.
EP-030		48" RCP	673.3	Pipe in good condition.
EP-040		24" RCP	79.7	Remove root issue at first joint 5' from downstream end.
EP-040	2 1192+50	42" RCP	512.9	Remove medium root issue at joint approx. 323' from downstream end. Remove and replace cracked pipe from approx. 387'-403' from downstream end.
EP-040	3 1187+65	18" RCP	101.9	Pipe in good condition.
EP-040	4 1201+10	42" RCP	376.7	Slip line existing pipe due to joint deficiencies and minor root issues.
EP-040	5 1198+00	18" RCP	133.4	Pipe in good condition.
EP-040	6 1198+60	18" RCP	136.9	Repair first joint of existing pipe 8' from upstream end.
EP-040	7 1192+85	18" RCP	111.4	8' of existing pipe prior to downstream end shall be replaced.
EP-040	8 1197+40	42" RCP	492.6	Slip line existing pipe due to joint deficiencies and minor root issues.
EP-050	3 1204+40	42" RCP	332	Slip line existing pipe due to joint deficiencies.
EP-050	4 1204+25	36" RCP	30.9	Pipe in good condition.
EP-050	5 1204+70	24" RCP	85.3	Slip line existing pipe due to cracks and joint separations.
EP-050	6 1205+05	18" RCP	107	Pipe in good condition.
EP-050	7 1206+10	18" RCP	129.9	Pipe in good condition.
EP-050	8 1205+90	18" RCP	68.5	Pipe in good condition. Clean debris from both ends.
EP-061	3 1229+50	18" RCP	101.2	Replace existing pipe section between approx. 23' and 31' from upstream end and repair separated/broken joint at approx. 88' from upstream end.
EP-061	4 1230+00	18" RCP	69.3	Pipe in good condition.
EP-070	1 1236+00	18" RCP	195.6	Slip line existing pipe due to joint deficiencies and cracks
EP-070	2 1246+60	18" RCP	104.1	76' of existing pipe prior to outlet shall be replaced due to cracks in pipe and major joint separations.
EP-080	1 1252+00	18" RCP	180	Pipe in good condition.
EP-080	2 1253+25	24" RCP	242.8	Slip line existing pipe due to joint separations.
EP-080	3 1261+45	18" RCP	199.2	Pipe in good condition. Clean out.
EP-090	1 1270+50	18" RCP	167.7	8' of existing pipe prior to outlet shall be replaced due to cracked pipe with hole in it.
EC-090	6 1273+15	7.5' x 5' Conc. Box Culvert	240	Patch cracks in culvert (approximately 13 locations).
EP-100	2 1286+60	30" RCP	367.3	Pipe in good condition.
EP-100	3 1286+45	24" RCP	84.9	Pipe in good condition.
EP-100	4 1286+35	24" RCP	84.8	Pipe in good condition.
EP-100	5 1286+40	24" RCP	62.2	Pipe in good condition.



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EP-1006	1289+60	24" RCP	305	Replace first 32' of existing pipe from upstream end due to root issues and broken pipe.
EP-1007	1290+50	18" RCP	108	Pipe in good condition.
EP-1008	1290+05	18" RCP	99.5	Pipe in good condition.
EP-1010	1283+70	30" RCP	384.5	Clean out.
EP-1101	1297+50	18" RCP	97.7	Slip line existing pipe due to joint separations.
EP-1102	1297+50	24" RCP	91.3	Pipe in good condition.
EP-1103	1296+55	18" RCP	16.8	20' of existing pipe from upstream end shall be replaced due to collapse and deficiencies. Clean pipe.
EP-1104	1297+45	24" RCP	111.7	Replace pipe due to cracks, major root issues, and joint separations.
EP-1105	1298+55	24" RCP	65.8	Replace pipe due to joint deficiencies and major root issues.
EP-1106	1299+15	24" RCP	53.9	Pipe in good condition. Clean downstream pipe end and drop inlet.
EP-1111	1304+40	24" RCP	266.9	Pipe in good condition.
EP-1114	1303+60	24" RCP	312.6	Pipe in good condition.
EP-1201	1320+10	24" RCP	108.2	Clean out.
EP-1202	1320+00	18" RCP	110.5	Pipe in good condition.
EP-1203	1323+40	36" RCP	686	Pipe in good condition.
EP-1204	1327+70	36" RCP	93.4	Slip line existing pipe due to joint deficiencies.
EP-1301	1332+20	24" RCP	171.5	Pipe in good condition.
EP-1302	1341+90	24" RCP	116.1	Pipe in good condition.
EP-1303	1342+10	18" RCP	115.8	If retained, clean pipe (large debris item near 75' from upstream end). Preliminary hydro analysis indicates that additional capacity may be needed at this location.
EP-1401	1350+05	18" RCP	264.6	Pipe in good condition.
EP-1402	1352+05	18" RCP	224.7	Pipe in good condition.
EP-1403	1353+85	30" RCP	127.8	Pipe in good condition.
EP-1404	1355+00	30" RCP	101.6	Pipe in good condition.
EP-1405	1355+40	36" RCP	187.8	Clean out.
EP-1406	1357+50	24" RCP	363.4	Replace first section of pipe due to major joint offset. Pipe in good condition otherwise. Clean out.
EC-1407	1358+00	6' x 5' Conc. Box Culvert	440	Headwalls in good condition. Patch cracks in culvert (approximately 35 locations). Clean downstream sediment buildup.
EP-1408	1357+85	18" RCP	180.5	Slip line existing pipe due to joint deficiencies and major root issues.
EP-1409	1358+25	18" RCP	214.8	Slip line existing pipe due to joint separations.
EP-1501	1362+45	18" RCP	136.7	Pipe in good condition.
EC-1503	1366+80	4' x 4' Conc. Box Culvert	680.5	Patch cracks in culvert (approximately 41 locations). Repair connection with EP-1599 at approx. 385' from downstream end.



	P-1506	1372+00	18" RCP	124 E	Ding in good condition
				134.5	Pipe in good condition.
EF	P-1507	1372+00	18" RCP	51.1	Pipe in good condition.
EF	P-1601	1379+80	18" RCP	35.9	Pipe in good condition. Clean debris from pipe and box catch basin at upstream end.
EF	P-1602	1379+80	18" RCP	106.2	Remove root issues at first joint approx. 8' from downstream end and second joint approx. 16' from downstream end. Clean pipe.
EF	P-1603	1379+85	18" RCP	121.9	Pipe in good condition.
EF	P-1604	1379+85	24" RCP	50.1	Pipe in good condition.
EF	P-1606	1383+85	30" RCP	400.6	Pipe in good condition.
EF	P-1607	1383+45	18" RCP	39.8	Slip line existing pipe due to joint separations.
EF	P-1608	1383+50	18" RCP	105.4	Remove and replace pipe 13' from downstream end to repair major root issues in joints. Clean pipe.
EF	P-1609	1383+65	18" RCP	122.1	Remove and replace pipe between 30' from upstream end to repair major root issues in joints. Clean pipe.
EF	P-1610	1383+80	24" RCP	53.5	Pipe in good condition.
EF	P-1701	1399+90	18" RCP	171	10' of existing pipe prior to downstream end shall be replaced due to joint separations and offsets.
EC	C-1702	1407+70	8' x 6' Conc. Box Culvert	280	Headwall in good condition. From upstream end, repair top of culvert where square block of concrete is giving way at approx. 70'. Patch cracks in culvert (approximately 6 locations).
EC	C-1703	1407+70	8' x 6' Conc. Box Culvert	280	Headwalls in good condition. Patch cracks in culvert (approximately 8 locations).
EC	C-1704	1407+70	8' x 6' Conc. Box Culvert	280	Headwalls in good condition. Patch cracks in culvert (approximately 4 locations). Clean sediment buildup at downstream end.
EF	P-1801	1416+20	24" RCP	147.9	Pipe in good condition. Clean pipe. Standing water and sediment in pipe from outfall OP-1896 silting in.
EF	P-1802	1416+50	24" RCP	115.5	Repair joint separations at approx. 16' and 24' from upstream end.
EF	P-1803	1423+70	30" RCP	154.5	Pipe in good condition. Clean pipe. Standing water and sediment in pipe from outfall OP-1895 silting in.
EF	P-1804	1424+10	30" RCP	122.2	Pipe in good condition. Preliminary hydro analysis indicates that additional capacity may be needed at this location.
EF	P-1806	1410+75	24" RCP	130.4	Pipe in good condition.
EF	P-2101	1458+55	36" RCP	133.7	Pipe in good condition. Preliminary hydro analysis indicates that additional capacity may be needed at this location.
EF	P-2102	1459+00	36" RCP	101.4	Pipe in good condition. If retained, clean pipe. Standing water and sediment in pipe from outfall OP-2199 silting in. Preliminary hydro analysis indicates that additional capacity may be needed at this location.



EC	C-2103	1468+95	12' x 12' Conc. Box Culvert	288	Headwall in good condition. Patch cracks in culvert (approximately 6 locations).
EP	P-2104	1471+90	18" RCP	156.2	Repair joint separations at approx. 4' and 12' from downstream end.
EC	C-2105	1468+95	12' x 12' Conc. Box Culvert	288	Headwall in good condition. Patch cracks in culvert (approximately 9 locations).
EC	C-2106	1468+95	12' x 12' Conc. Box Culvert	288	Headwall in good condition. Patch cracks in culvert (approximately 7 locations). Clean sediment buildup throughout culvert.
EP	-2201	1475+65	24" RCP	159.2	Pipe in good condition.
EP	-2202	1476+70	24" RCP	152.2	Pipe in good condition.
EP	-2203	1485+00	36" RCP	145.8	Pipe in good condition.
EP	P-2204	1485+95	36" RCP	138.9	Pipe in good condition. Large HDPE pipe debris inside of drop inlet EX-2204 and upstream end of EP-2204. If retained, clean entire pipe of sediment and debris.
EP	P-2301	1500+90	24" RCP	43.5	Pipe in good condition. Clean pipe. Standing water and sediment in pipe from outfall OP-2399 silting in.
EP	P-2303	1503+95	18" RCP	142.3	8' of existing pipe prior to outlet shall be replaced due to collapse. Slip line existing pipe due to joint separations and deficiencies.
EP	P-2403	1499+95	24" RCP	21.3	Pipe in good condition. Clean pipe. Standing water in pipe.
EP	P-2404	1522+40	18" RCP	61.7	Remove and replace last segment of pipe at downstream end to repair major root issues in joint.

Table 4.11 Pipe Video Inventory Survey Results / Evaluation

4.12 Existing Outfalls

Existing pipe outfalls were inspected in the field. Most of the outfalls were clear of debris.

The following outfalls will some amount of need cleaning / clearing:

OP-0100 (Outfall of existing 8' x 6' concrete box culvert EC-0211) Approx. Sta. 1151+85, Offset 188' RT

The inlet of box culvert EC-0211 is adjacent to the WB Percival Road ramp onto I-77 NB. This culvert and two others (EC-0102 & EC-0103) outfall at OP-0100 into an unnamed tributary of Gill's Creek to the south of the interchange. At the time of a field visit, the existing outfall beyond the culvert was clear, but the box itself had accumulated sediment for approximately 75' into the culvert, and should be cleaned out as noted in the previous section.



OP-0100

OP-0286 (Outfall of existing 18" RCP EP-0289) Approx. Sta. 1167+30, Offset 146' RT

EP-0289 crosses the I-77 NB off-ramp loop to WB Percival Road. At the time of the field visit, the end of the pipe was buried. The outfall should be excavated for several feet to ensure positive drainage from the pipe. There are no concerns associated with right-of-way for this outfall as it is located within the interchange loop.



OP-0286



OP-0394 (Outfall of existing 48" RCP EP-0303) Approx. Sta. 1177+35, Offset 118' LT

EP-0303 parallels I-77 SB and outfalls approximately 200' after the beginning of the taper for the Percival Road exit. At the time of the field visit, standing water was observed. The existing ditch that parallels I-77 SB, into which EP-0303 outfalls, should be cleared of debris as necessary to restore downstream flow. There should not be any right-of-way issues associated with clearing the ditch.



OP-0394

OP-0398 (Outfall of existing 18" RCP EP-0301) Approx. Sta. 1171+30, Offset 200' RT

EP-0301 crosses the WB Percival Road ramp onto I-77 NB and outfalls just to the southeast of EX-0308 (inlet of EC-0211). At the time of the field visit, the pipe was mostly buried and the outfall ditch was covered in vegetation. The outfall should be excavated and cleared of debris for several feet to ensure positive drainage from the pipe. Right-of-way issues may exist downstream of this outfall.



OP-0398



OP-0683 (Outfall of existing 18" RCP EP-0612) Approx. Sta. 1224+45, Offset 115' RT

EP-0612 parallels the loop ramp from I-20 WB / Alpine Road onto I-77 SB beneath the I-77 NB bridge overpass. The outfall was not located in the field and is assumed to be buried. The outfall needs to be located. If the outfall location is where the asbuilts imply, the outfall should be excavated to ensure positive drainage. There are no concerns with right-of-way for this outfall as it is located well within the interchange.



OP-0683 (not located, assumed as buried)

OP-0798 (Outfall of existing 18" RCP EP-0702) Approx. Sta. 1246+60, Offset 155' RT

EP-0702 crosses I-77 NB approximately 50' before the I-77 bridge over Windsor Lake. The pipe has collapsed, and therefore the outfall was not located. The pipe should be replaced due to the collapse and other issues found during the video inspection. There should not be any right-of-way issues associated with the pipe replacement.



OP-0798 (not located, collapsed)

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OP-0897 (Outfall of existing 18" RCP EP-0803) Approx. Sta. 1259+70, Offset 93' RT

EP-0803 crosses I-77 NB approximately 1,000' after the I-77 bridge over Windsor Lake. At the time of the field visit, the existing concrete channel downstream of the outfall had accumulated sediment, causing water to pond. The concrete channel should be cleaned for approximately 470'. There should not be any right-of-way issues associated with clearing the channel.



OP-0897

OP-0995 (Outfall of existing 36" RCP EP-0907) Approx. Sta. 1278+00, Offset 119' LT

EP-0907 crosses I-77 SB approximately 500' before the Trenholm Road bridge over I-77. At the time of the field visit, the pipe outfall and associated ditch were fill with vegetation and debris and will require cleaning. The ditch should be cleared and re-excavated to the edge of the right-of-way.



OP-0995



OP-0999 (Outfall of existing 18" RCP EP-0901) Approx. Sta. 1271+65, Offset 118' LT

EP-0901 crosses I-77 SB approximately 1,250' before the Trenholm Road bridge over I-77. At the time of the field visit, standing water was observed. The pipe outfall and associated ditch were partially silted in and will require cleaning. The ditch should be cleared and re-excavated to the edge of the right-of-way.



OP-0999

OP-1010 (Outfall of existing 30" RCP EP-1010) Approx. Sta. 1281+80, Offset 117' LT

EP-1010 parallels I-77 SB beneath the Trenholm Road bridge overpass and outfalls approximately 75' south of the bridge. At the time of the field visit, the pipe outfall and associated ditch were partially silted in and had accumulated debris. Cleaning will be required and re-excavate the ditch as necessary. Cleaning can take place within the existing right-of-way.



OP-1010



OP-1396 (Outfall of existing 24" RCP EP-1301) Approx. Sta. 1332+10, Offset 166' RT

EP-1301 crosses I-77 NB approximately 50' after the I-77 bridge over Little Jackson Creek / Edgewater Drive. At the time of the field visit, a tree was growing immediately downstream of the outfall. This tree should be removed to prevent future damage and/or blockage to the pipe. There should not be any right-of-way issues associated with removing the tree.



OP-1396

OP-1499 (Outfall of existing 18" RCP EP-1401) Approx. Sta. 1347+75, Offset 134' LT

EP-1401 crosses I-77 SB in the merging lane from the EB Farrow Road onramp. At the time of the field visit, the outfall ditch for the pipe was overgrown with vegetation and will need to be cleared to reestablish positive drainage from the outfall. Clearing can take place within the existing right-of-way.



OP-1499



OP-1598 (Outfall of existing 4' x 4' concrete box culvert EC-1503) Approx. Sta. 1364+80, Offset 129' RT

EC-1503 outfalls between I-77 NB and the ramp from SC-277 EB. At the time of the field visit, debris had collected at the baffles along the concrete outlet apron of this box culvert and could further accumulate to impede flow. The baffles should be cleared of debris. This outfall is within the I-77 / SC-277 interchange area, and all clearing can take place within the existing right-of-way.



OP-1598

OP-1699 (Outfall of existing 36" RCP EP-1605) Approx. Sta. 1379+05, Offset 263' RT

There is some uncertainty regarding the location of this outfall. EP-1605 is assumed to cross under Legrand Road approximately 100' after the railroad bridge over I-77. It was not located in the field and is assumed to be buried. The outfall needs to be located. If the outfall location is where assumed, the outfall should be excavated to ensure positive drainage. Right-of-way issues may exist near this outfall.



OP-1698 (not located, assumed as buried)



OP-1799 (Outfall of existing 24" RCP EP-1799) Approx. Sta. 1396+05, Offset 201' RT

EP-1799 crosses the ramp from Farrow Road to I-77 NB. At the time of the field visit, the end of the pipe was partially buried and the outfall ditch was silted in. The ditch should be excavated for approximately 25' between the pipe end and the concrete lined channel downstream. There should not be any right-of-way issues associated with clearing the ditch.



OP-1799

OP-1895 (Outfall of existing 30" RCP EP-1803) Approx. Sta. 1423+20, Offset 148' LT

EP-1803 crosses I-77 SB approximately 1,050' before the Hard Scrabble Road bridge over I-77. At the time of the field visit, the outfall was silted in and retaining water at the pipe end. The outfall ditch should be excavated for approximately 20' between the pipe end and the receiving ditch downstream. There should not be any right-of-way issues associated with clearing the ditch.



OP-1895



OP-1896 (Outfall of existing 24" RCP EP-1801) Approx. Sta. 1451+70, Offset 170' LT

EP-1801 crosses I-77 SB approximately 1,800' before the Hard Scrabble Road bridge over I-77. At the time of the field visit, the outfall ditch was silted in and retaining water at the pipe end. The outfall should be excavated to ensure positive drainage from the pipe. The pipe outfall is located only a couple of feet from right-of-way and permissions may be needed for excavation.



OP-1896

OP-1998 (Outfall of existing 30" RCP EP-1998) Approx. Sta. 1433+15, Offset 123' RT

EP-1998 parallels I-77 NB beneath the Hard Scrabble Road overpass and outfalls approximately 75' south of the bridge. At the time of the field visit, the outfall ditch was filled with debris and needs to be cleaned to promote positive drainage. Right-of-way is not an issue in this area.



OP-1998



OP-1999 (Outfall of existing 24" RCP EP-1999) Approx. Sta. 1433+10, Offset 121' LT

EP-1999 parallels I-77 SB beneath the Hard Scrabble Road overpass and outfalls approximately 75' south of the bridge. At the time of the field visit, the pipe was partially buried and the outfall ditch was covered in vegetation and debris. The outfall should be excavated and cleared of debris to ensure positive drainage from the pipe. Right-of-way is not an issue in this area.



OP-1999

OP-2199 (Outfall of existing 36" RCP EP-2102) Approx. Sta. 1459+55, Offset 162' LT

EP-2102 crosses I-77 SB approximately 2,550' after the Hard Scrabble Road bridge over I-77. At the time of the field visit, standing water was observed. The outfall was partially buried and the downstream channel was filled with silt. The outfall needs to be excavated to ensure positive drainage. Right-of-way issues may exist downstream of this outfall.



OP-2199



OP-2298 (Outfall of existing 36" RCP EP-2203) Approx. Sta. 1459+55, Offset 162' LT

EP-2203 crosses I-77 SB approximately 2,650' before the Killian Road bridge over I-77. At the time of the field visit, the outfall was slightly silted in with minimal positive drainage. The outfall ditch should be excavated as necessary to restore downstream flow. Right-of-way may be a concern if excavation occurs for several feet.



OP-2298

OP-2299 (Outfall of existing 24" RCP EP-2201) Approx. Sta. 1474+60, Offset 114' LT

EP-2201 crosses I-77 SB approximately 3,600' before the Killian Road bridge over I-77. A fallen tree limb is currently draped over the outfall ditch of this pipe. While it is not currently impeding flow, it should be cleared from the outfall. Clearing can take place within the existing right-of-way.



OP-2299

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OP-2396 (Outfall of existing 18" RCP EP-2305) Approx. Sta. 1505+50, Offset 310' LT

EP-2305 crosses the onramp from Killian Road to I-77 SB, approximately 570' before Killian Road bridge. At the time of the field visit, standing water was observed and the downstream channel was silted and established with grass. The channel should be excavated for several feet to restore downstream flow. There are no concerns associated with right-of-way in this area.



OP-2396

OP-2397 (Outfall of existing 18" RCP EP-2303) Approx. Sta. 1503+15, Offset 239' LT

EP-2303 crosses the onramp from Killian Road to I-77 SB, approximately 800' before Killian Road bridge. At the time of the field visit, standing water was observed and the downstream channel was silted and filled with vegetation. In addition, the pipe end is slightly damaged. The outfall should be excavated for to restore positive drainage and the last pipe joint may need to be repaired or replaced. There are no concerns associated with right-of-way at this outfall.



OP-2397



OP-2398 (Outfall of existing 18" RCP EP-2304) Approx. Sta. 1499+65, Offset 175' LT

EP-2304 crosses the onramp from Killian Road to I-77 SB, approximately 1,150' before the Killian Road bridge. The outfall was not located in the field and is assumed to be buried. The outfall needs to be located. If the outfall location is where assumed, the pipe end should be excavated to ensure positive drainage. There are no concerns associated with right-of-way at this outfall.



OP-2398 (not located, assumed as buried)

OP-2399 (Outfall of existing 24" RCP EP-2301) Approx. Sta. 1500+30, Offset 147' RT

EP-2301 crosses the Killian Road exit off-ramp from I-77 NB, approximately 1,090' before Killian Road bridge. At the time of the field visit, the outfall was partially buried and the downstream ditch was silted had accumulated sediment, causing water to pond. The outfall and ditch should be excavated and cleaned to promote positive drainage. There should be no right-of-way issues in this area.



OP-2399



4.13 HEC-RAS Analysis

Four Federal Emergency Management Agency (FEMA) regulated streams cross I-77 within the project limits. These four streams have detailed studies. The streams lie in the Upper Coastal Plains physiographic province.

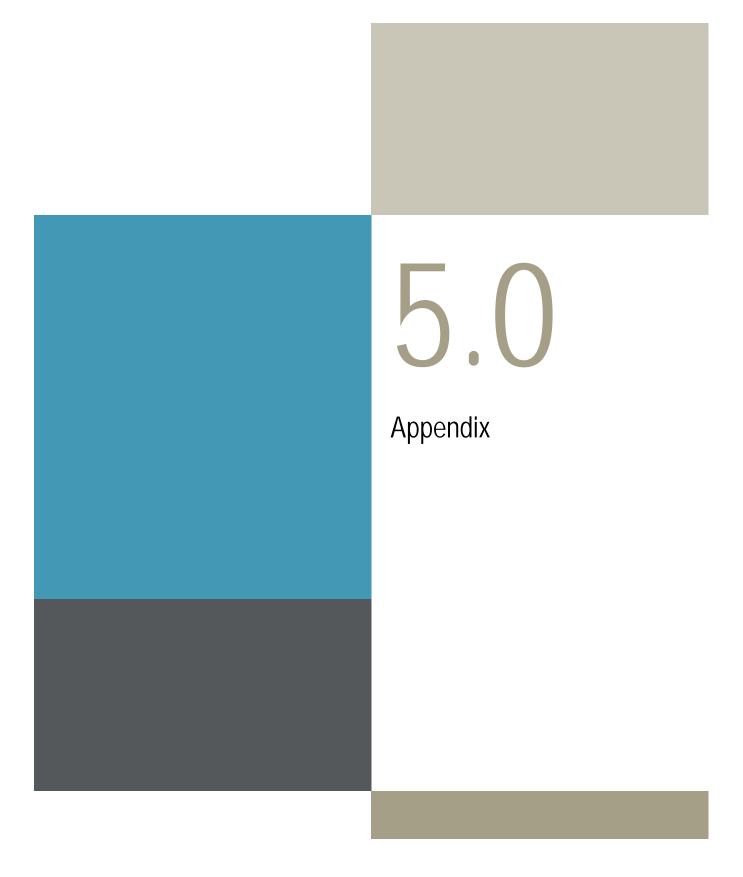
A Flood Insurance Study (FIS) for Richland County, South Carolina and incorporated areas, dated September 29, 2010, was obtained from FEMA. Volume 1 of the FIS is numbered 45079CV001A and volume 2 is numbered 45079CV002A. Pages of the FEMA FIS that pertain to each of the streams crossing I-77 within the project limits can be found in the individual stream sections below. The corresponding Flood Insurance Rate Maps (FIRM) are numbered 45079C0140K, 45079C0255K, 45079C0260K, and 45079C0270K. The maps have been merged into one map that can be seen in Figure 2.4.

FEMA provided HEC-2 models for each of the streams. These models were produced in the NGVD 29 datum and included input data for the existing conditions and output data for the flood plains runs and encroachment runs. The HEC-2 data was input into HEC-RAS Version 4.1.0 and then analyzed to compare pre- and post- construction conditions.

It was determined that the modeling of I-77 at the triple barrel culvert crossings located at Crane Creek and Cumbess Creek will not need to be extended since no widening will occur to the outside of I-77. Also, the bridge widenings at Jackson Creek and Little Jackson Creek will not impact the water surface elevations because the widening is only occurring on the inside towards the median.

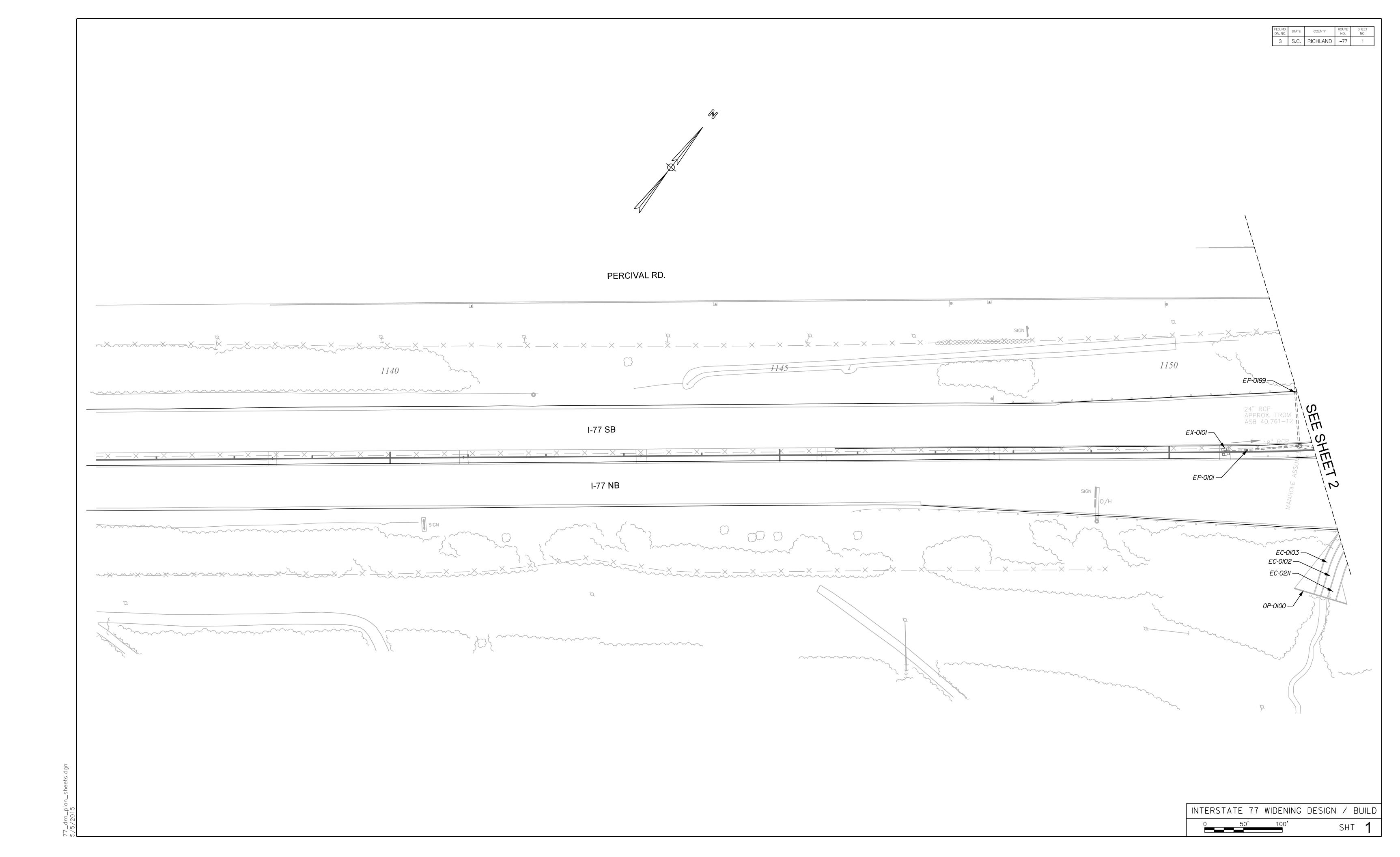
The stream crossings at Crane Creek, Cumbess Creek, Jackson Creek, and Little Jackson Creek are all located in special flood hazard zone AE. Each of these crossings should be eligible for "No-Rise" certifications since there will be no anticipated change in the 100-year flood elevations.

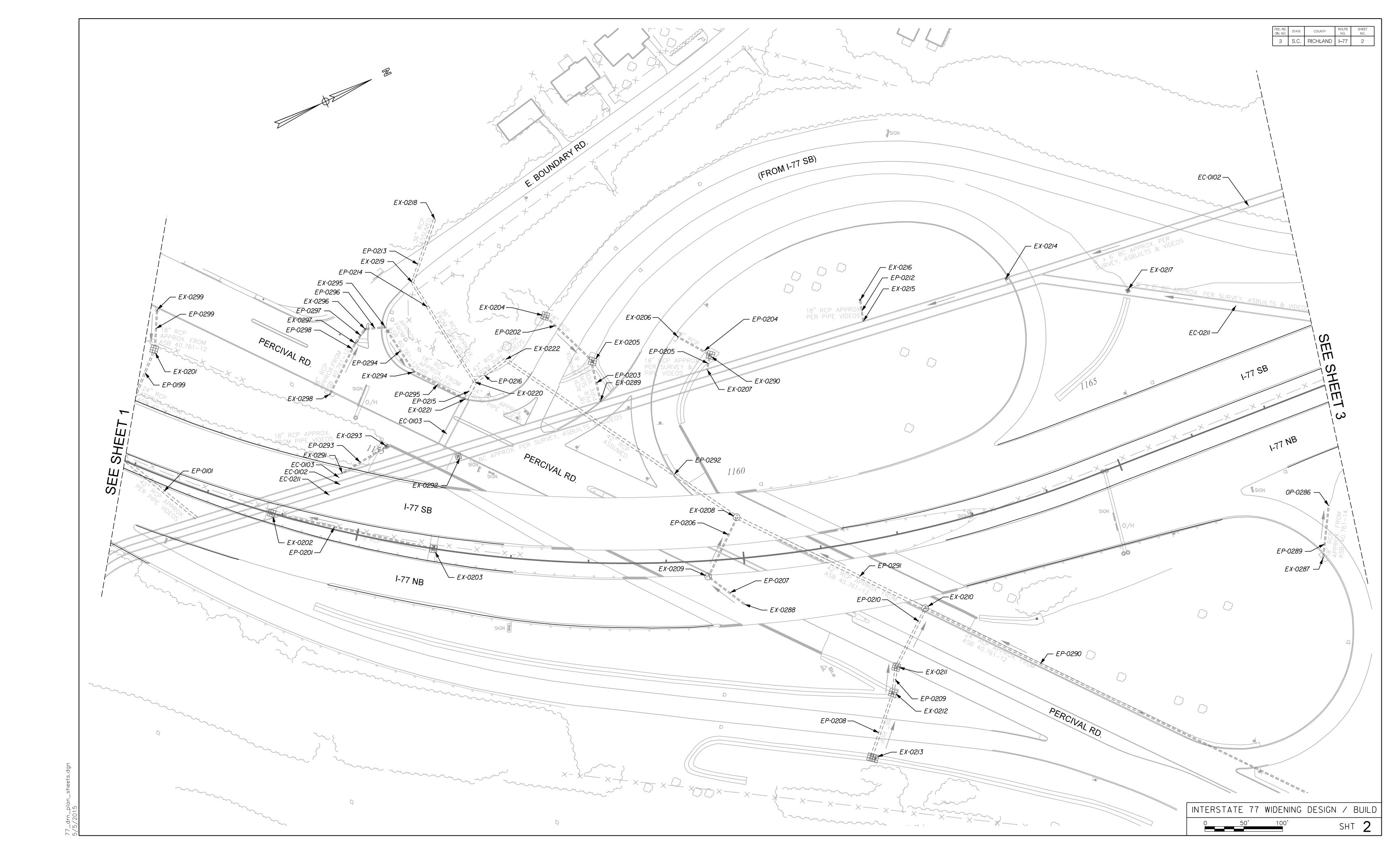


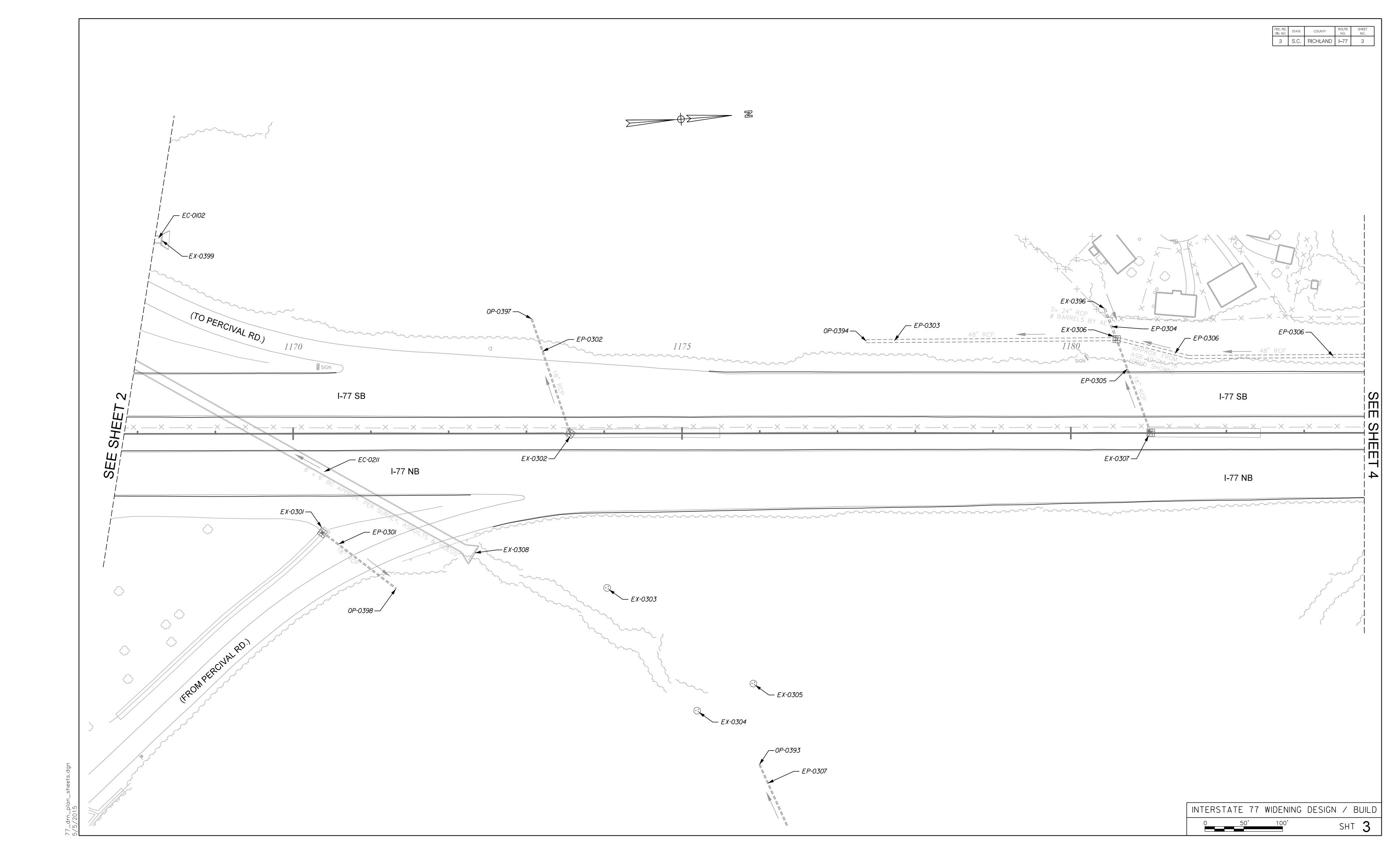


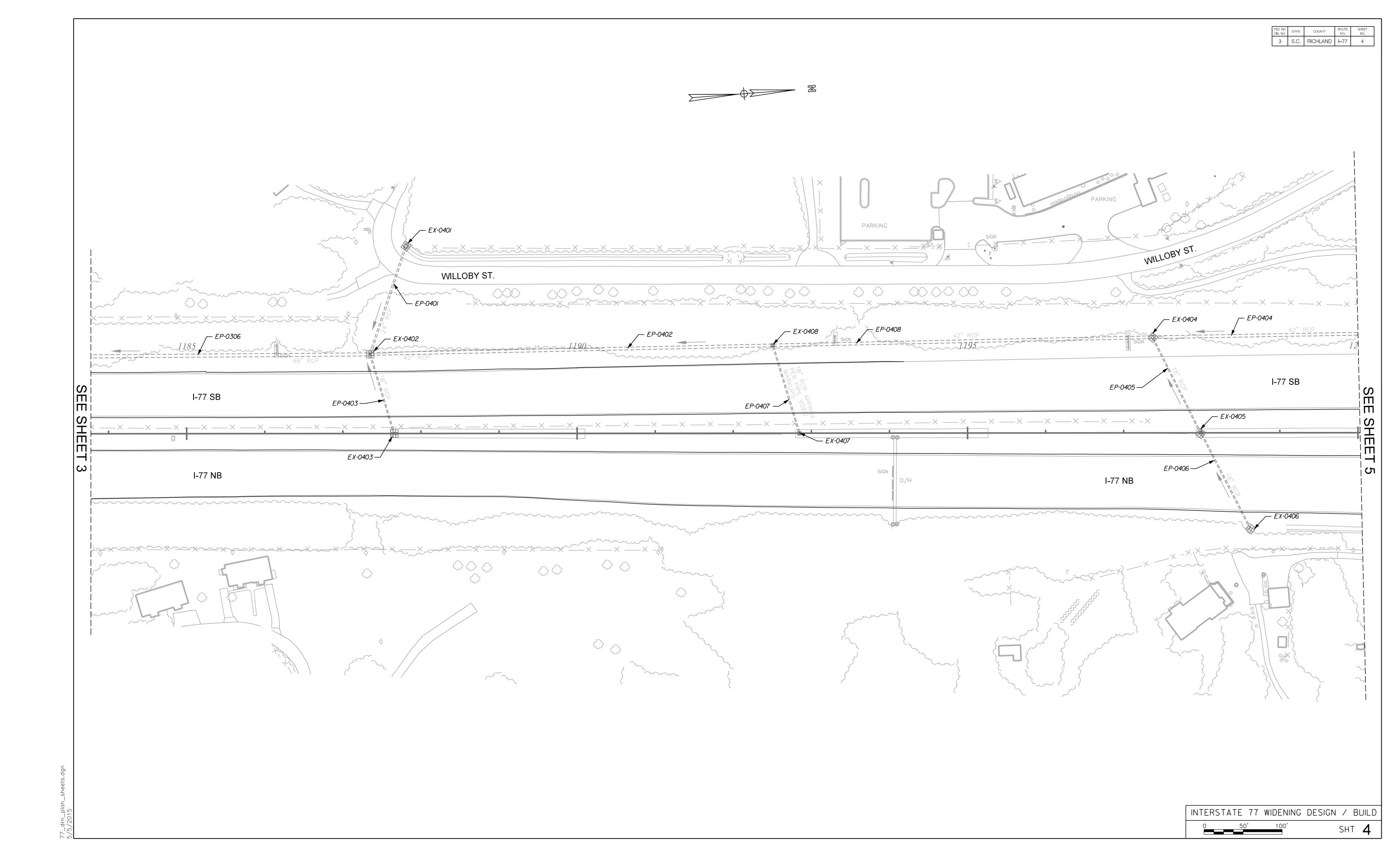
Appendix A

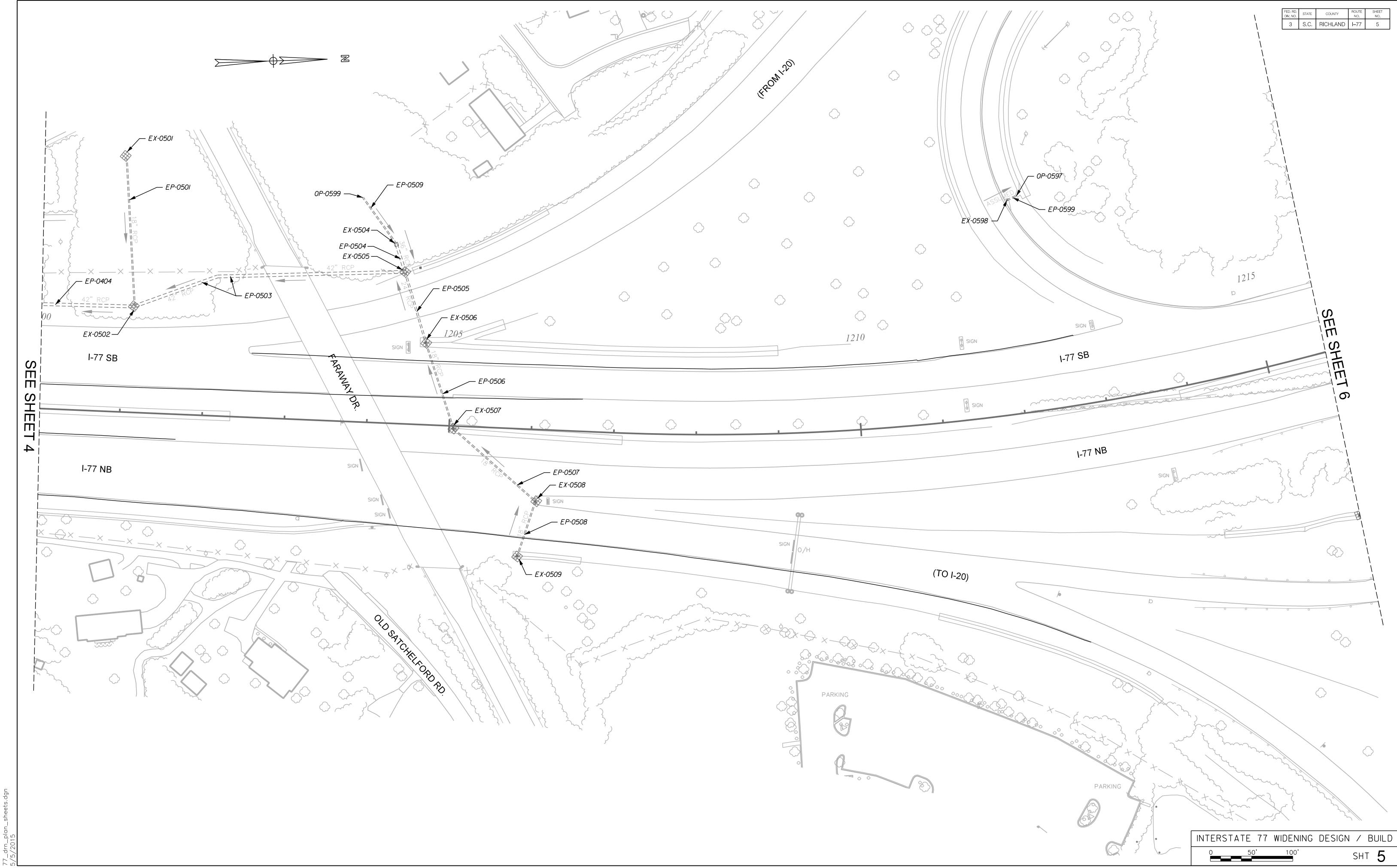
Plan Sheets

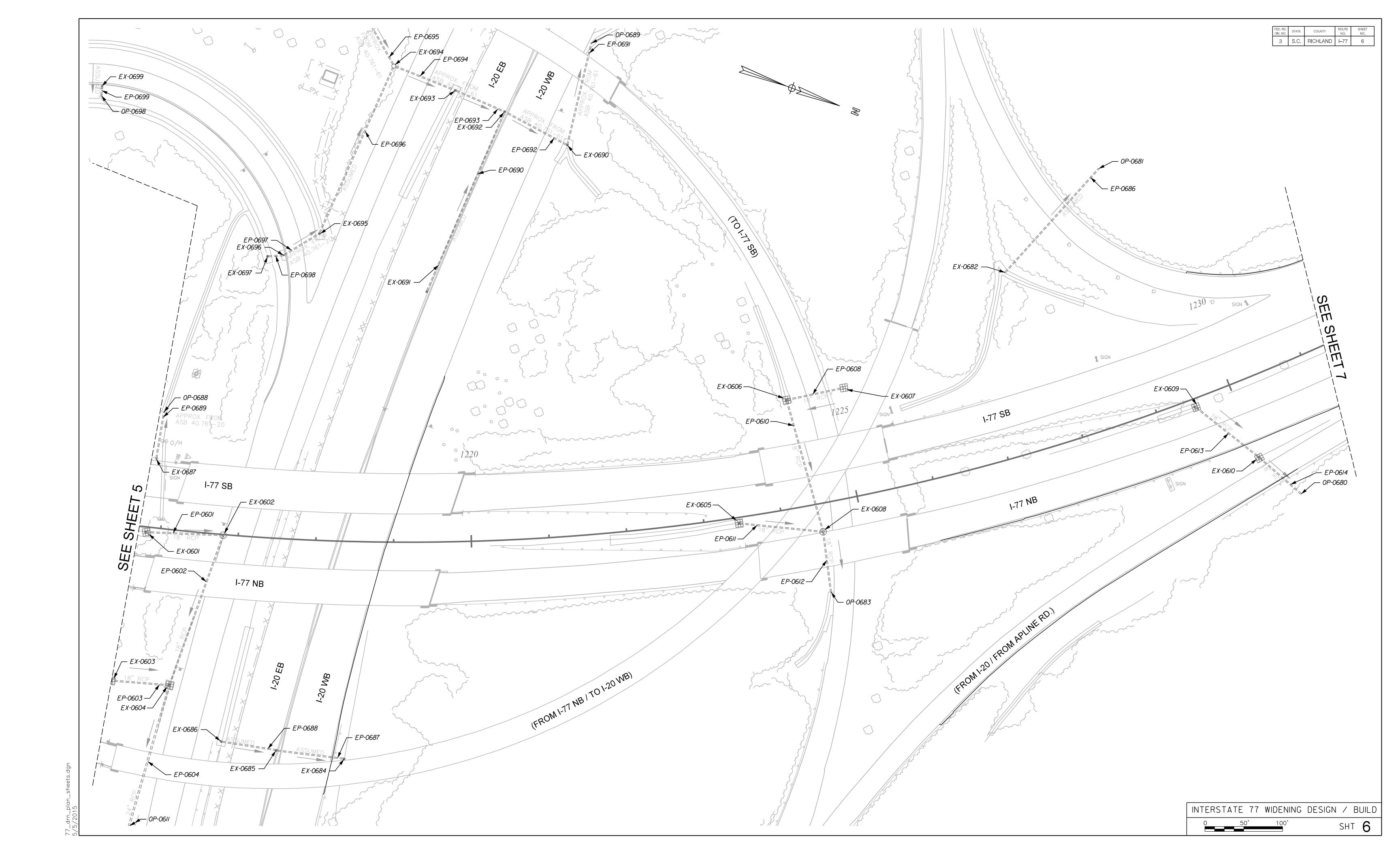


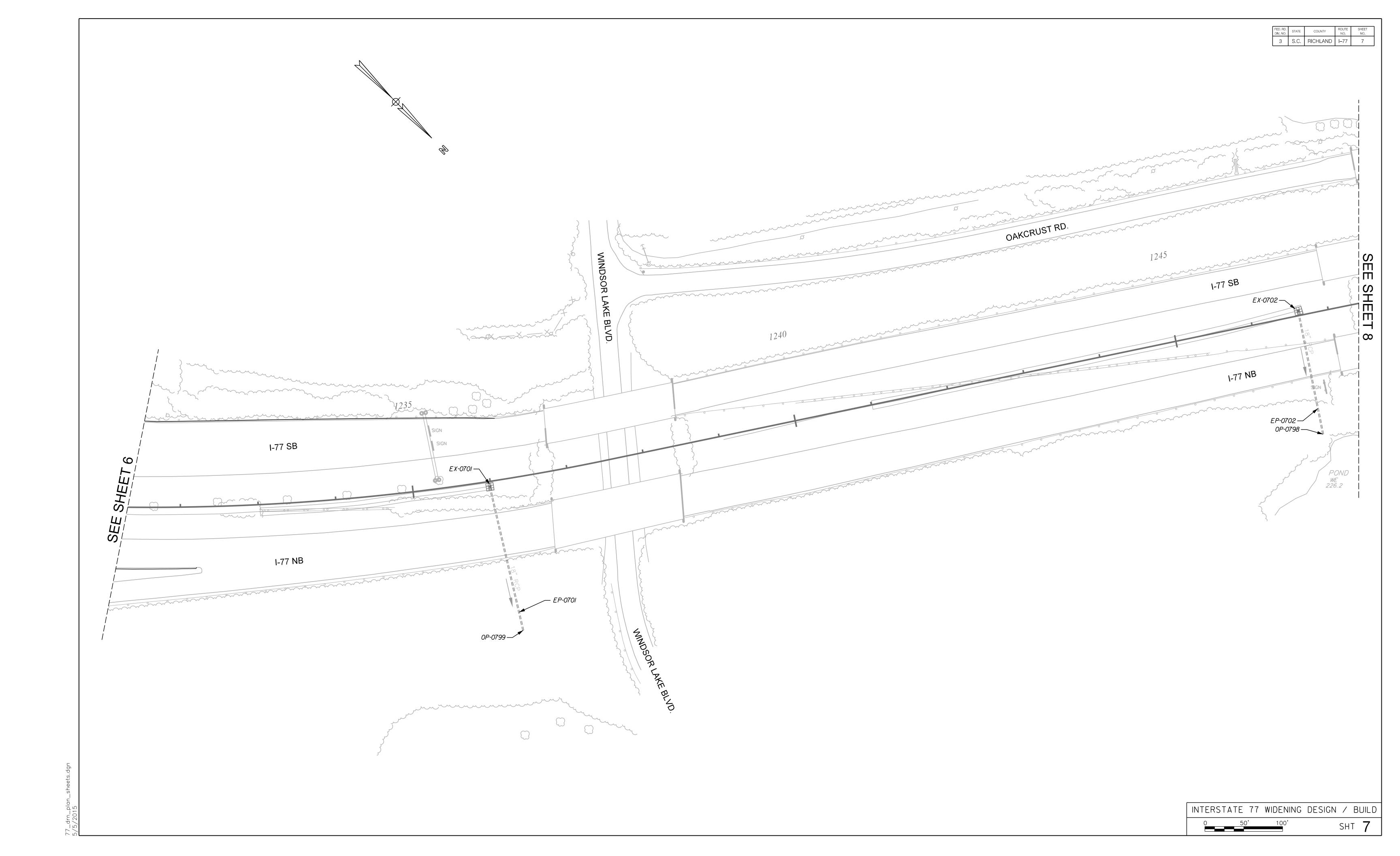


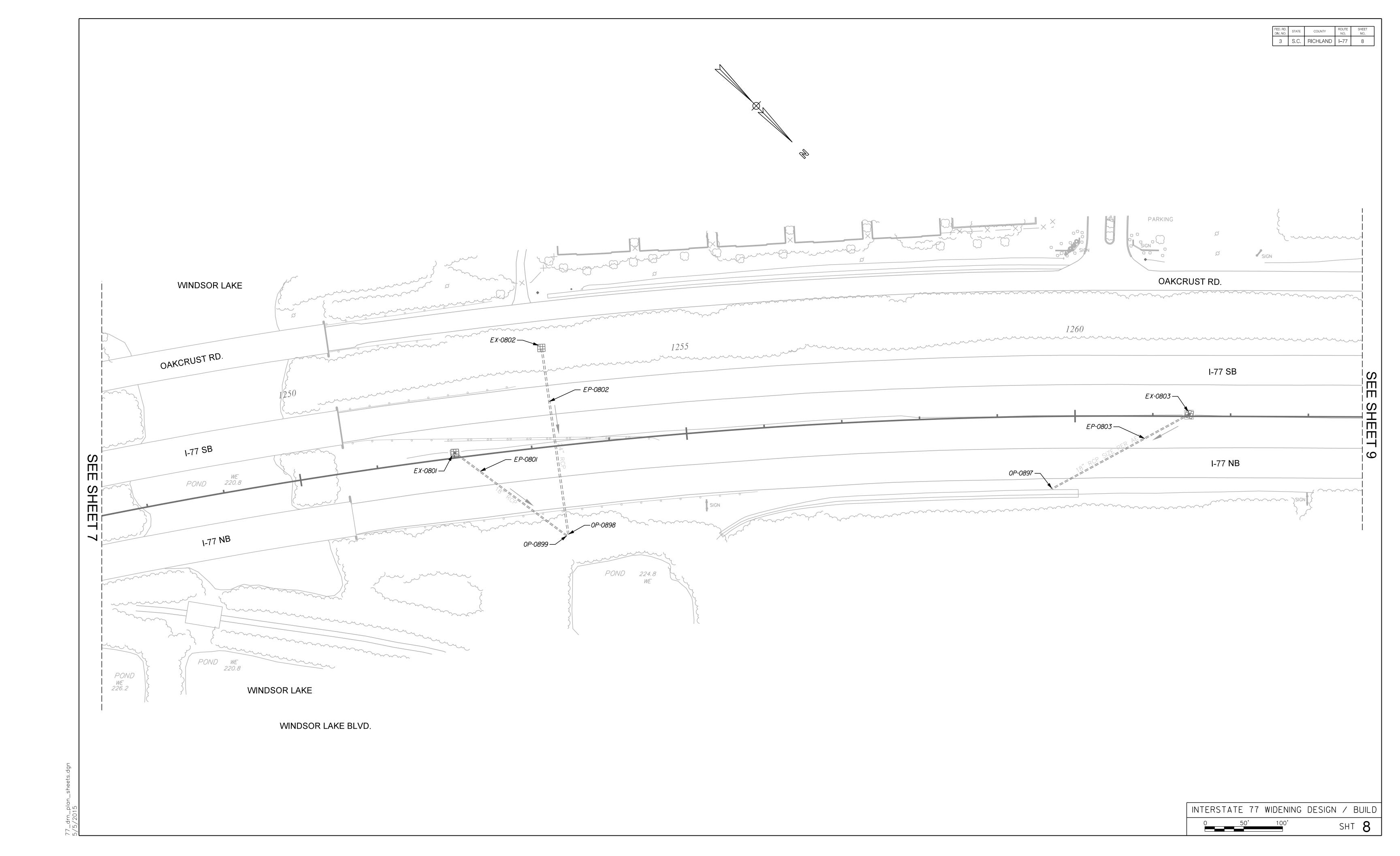


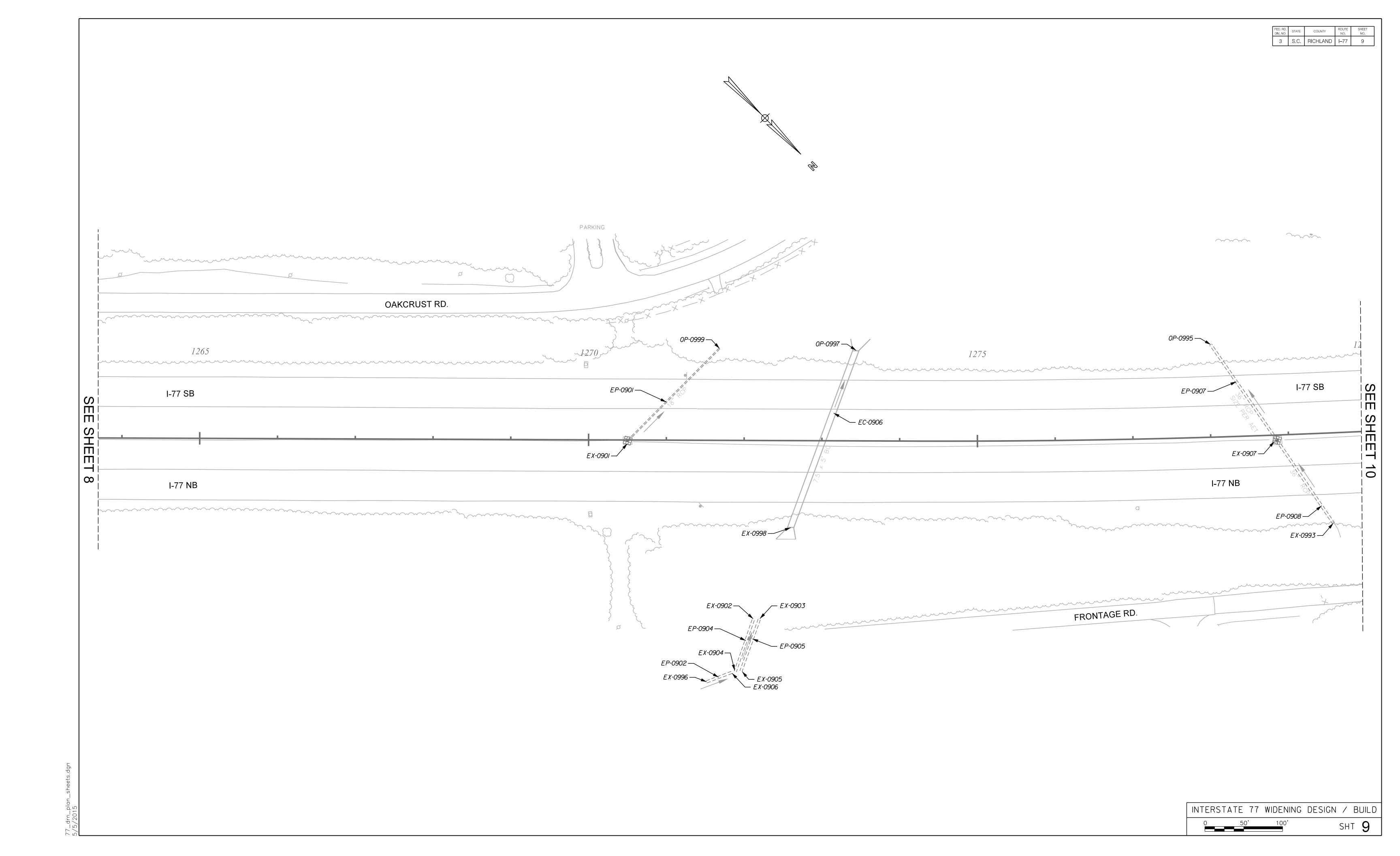


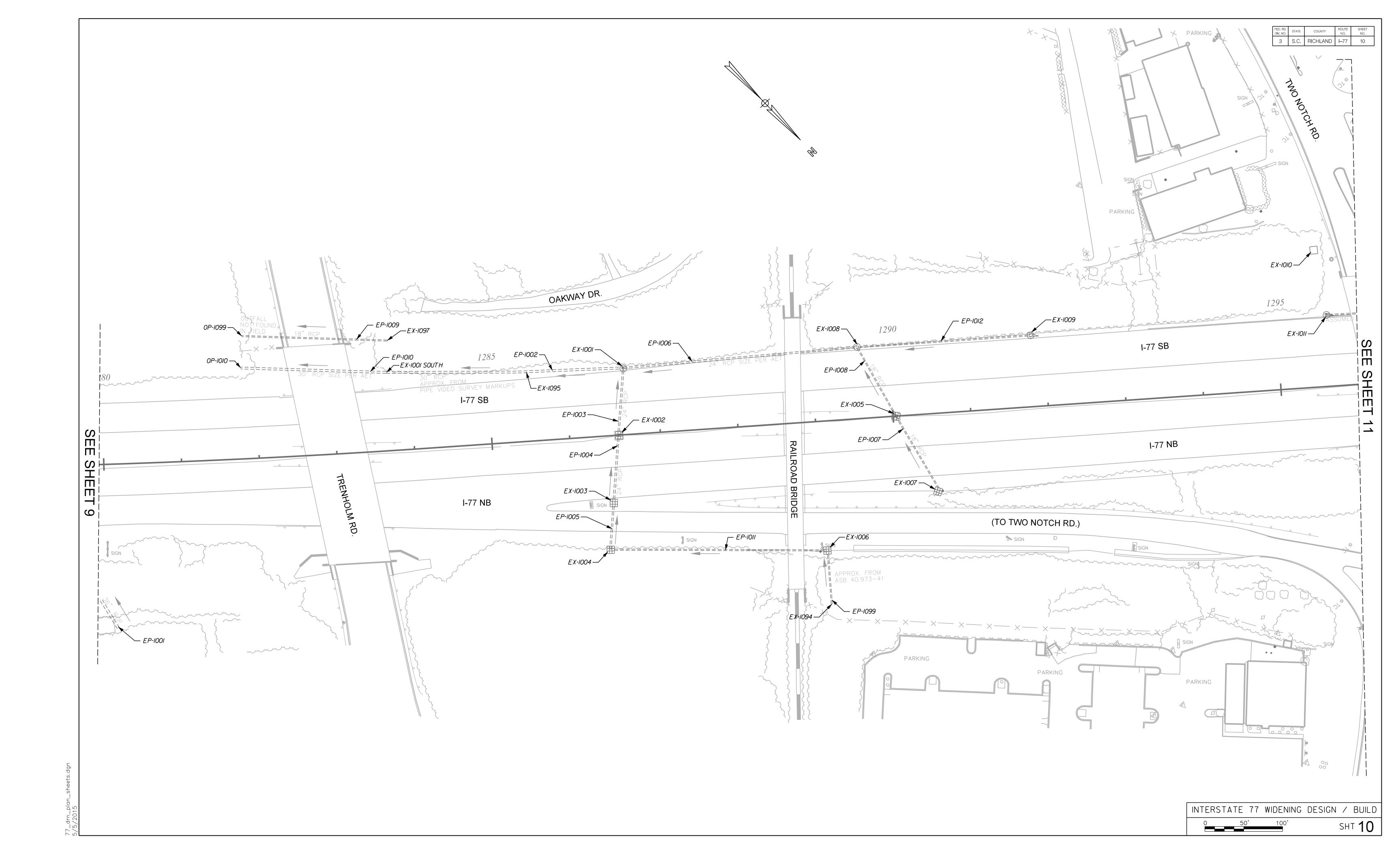


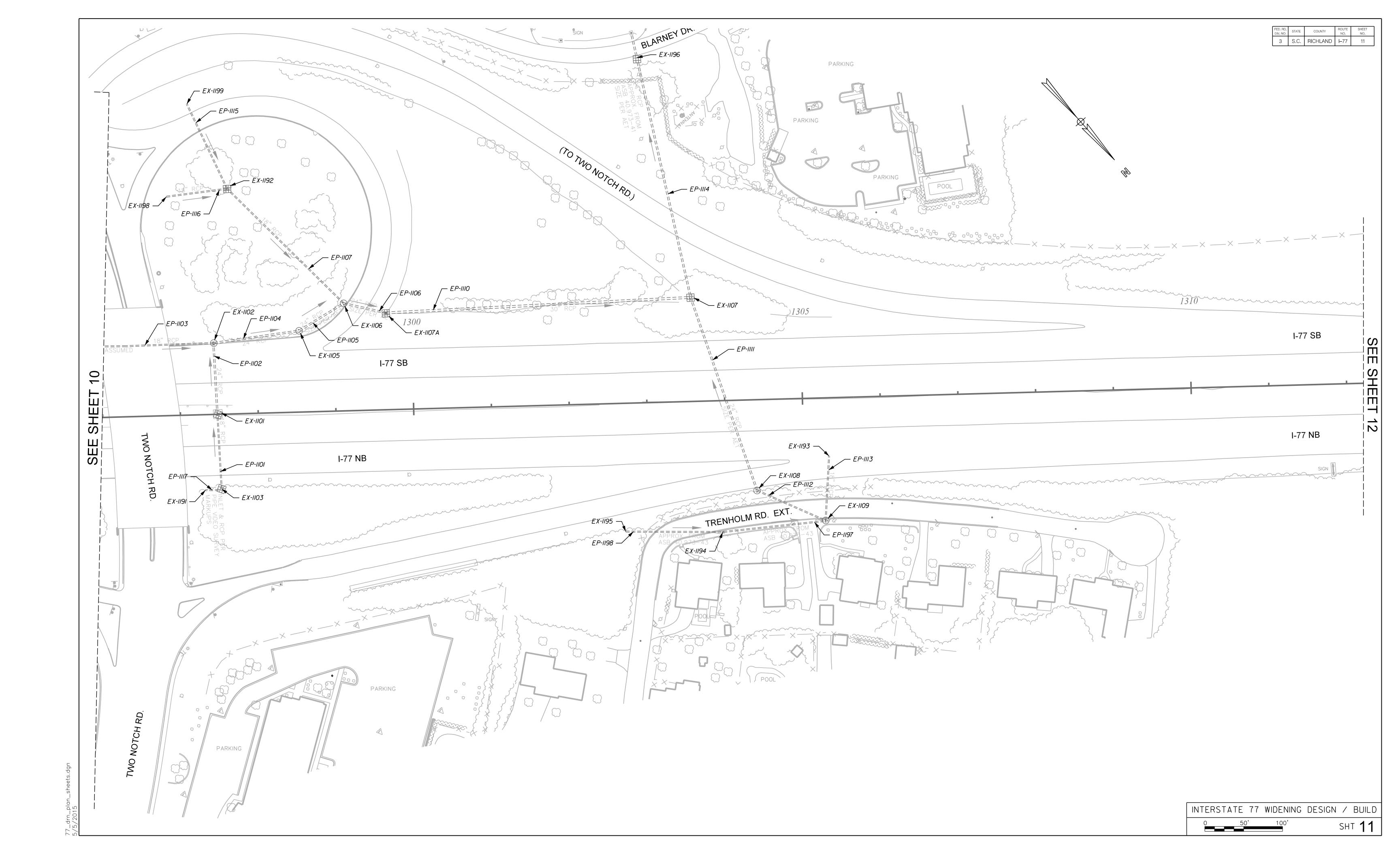


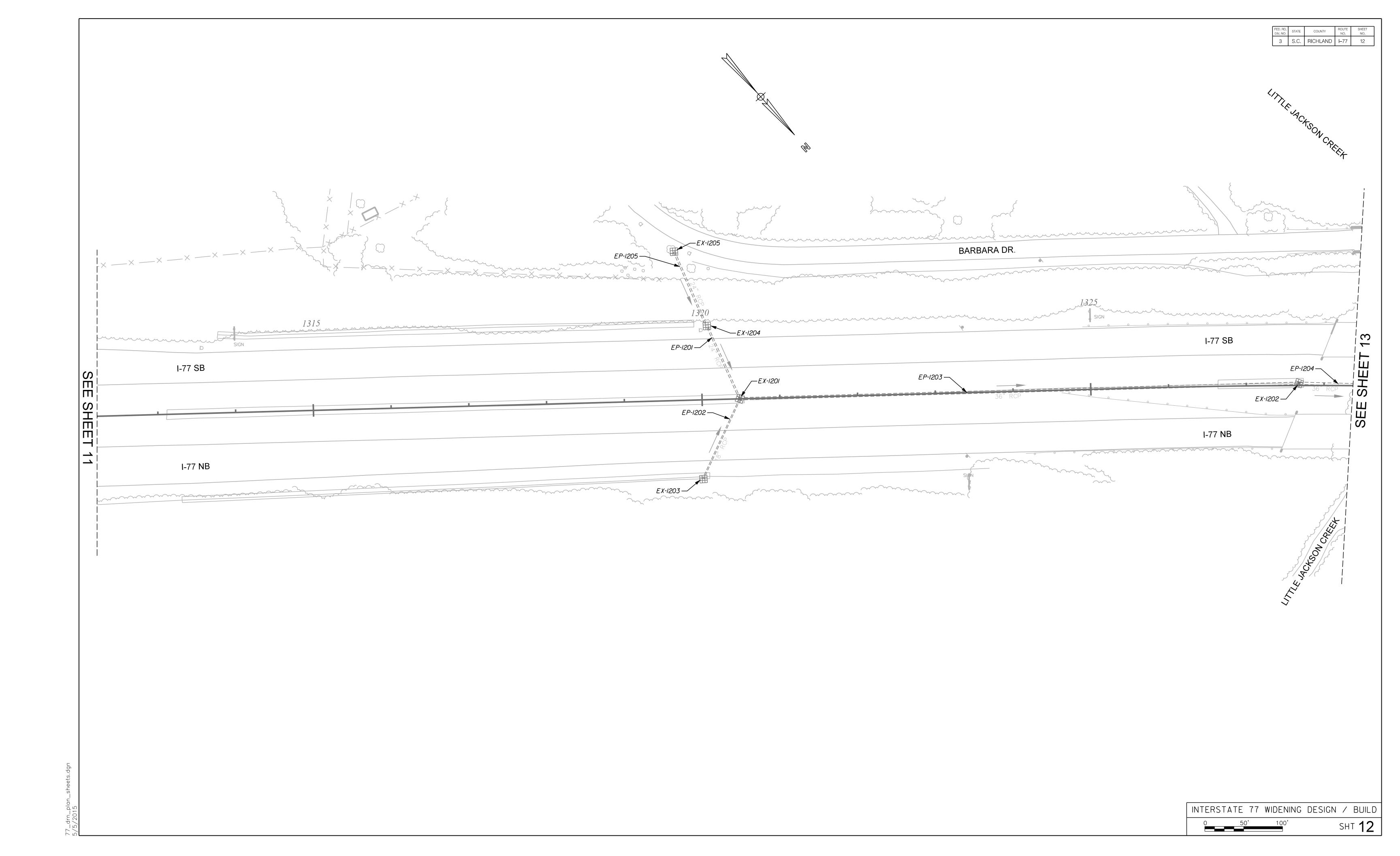


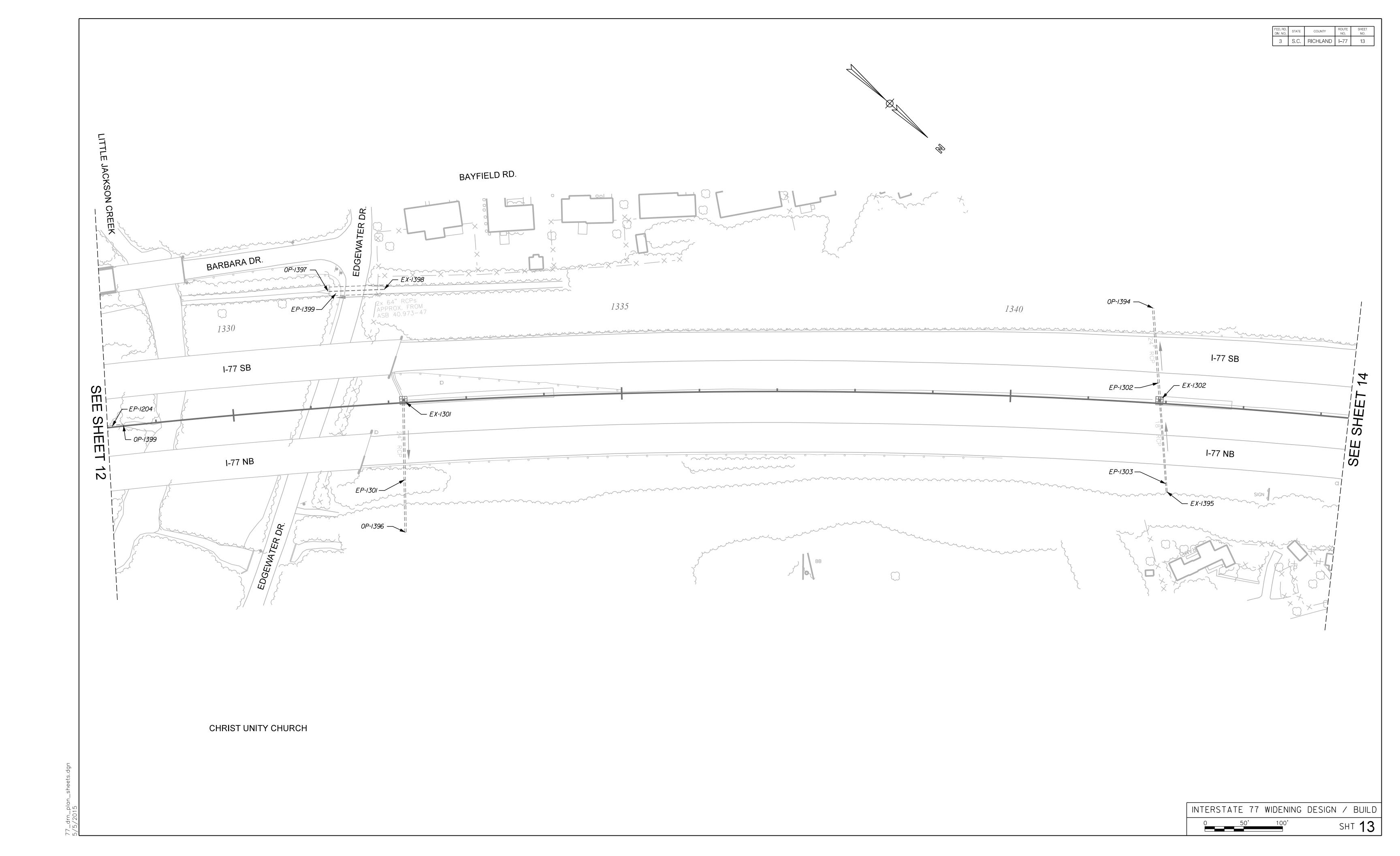


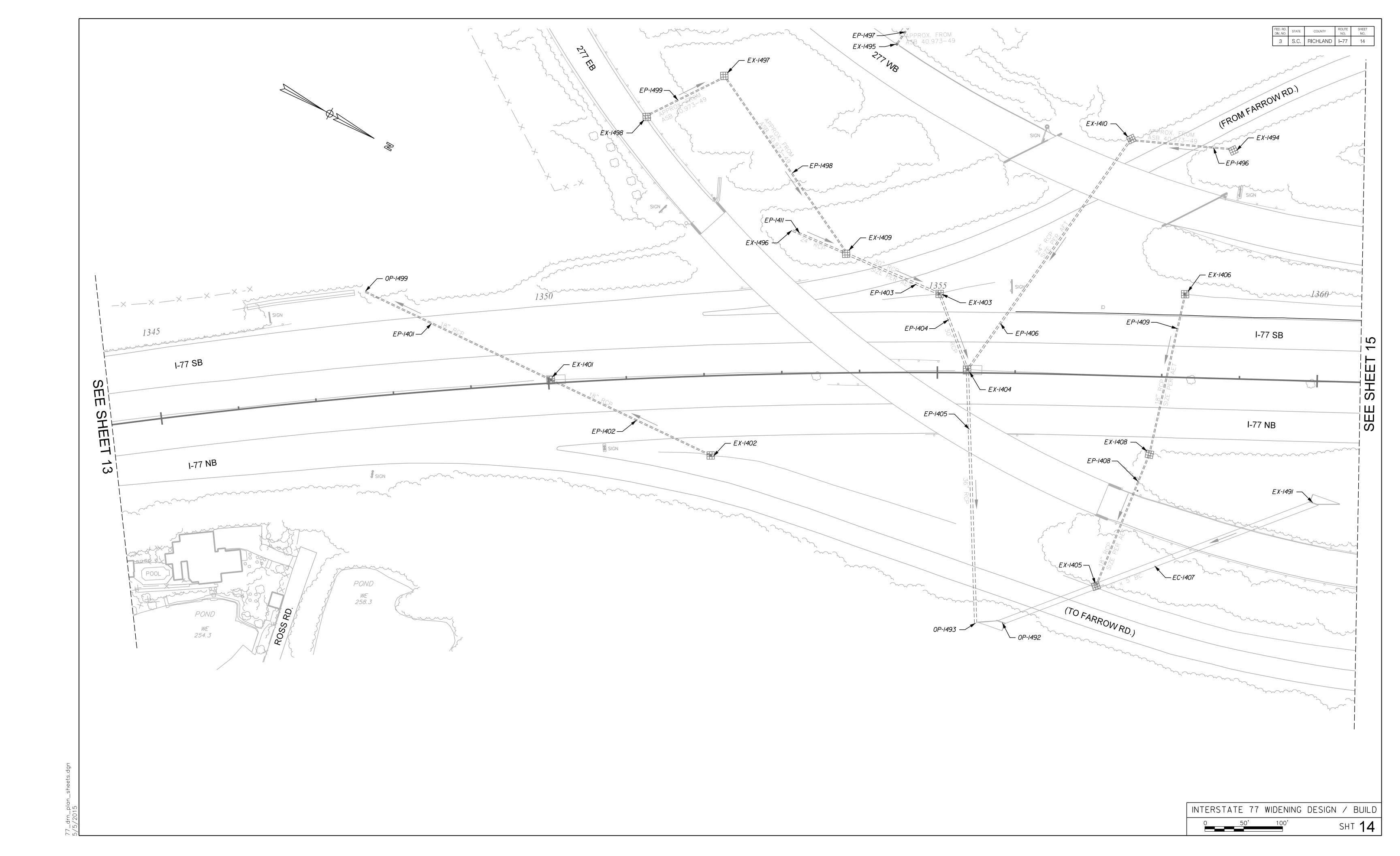


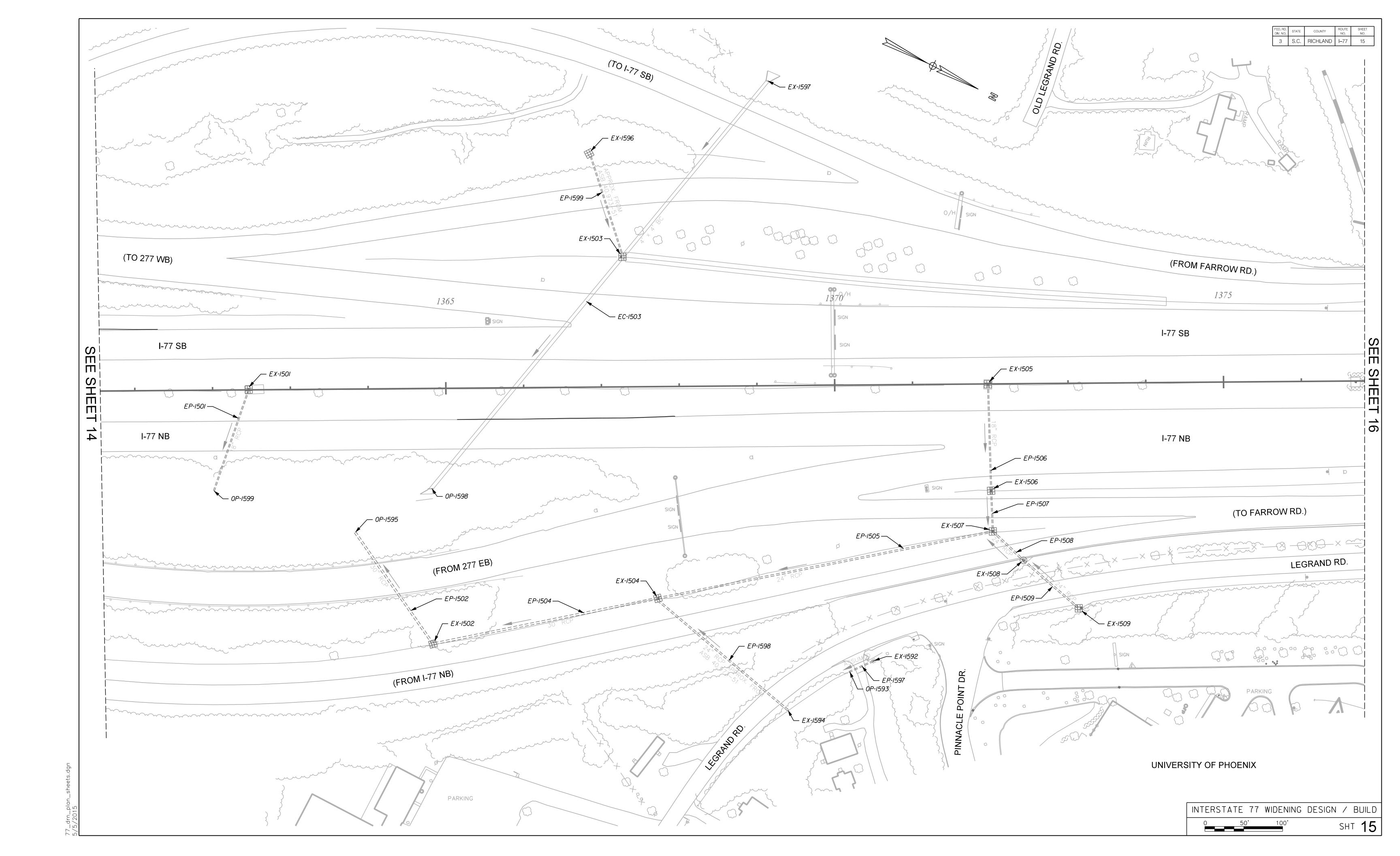


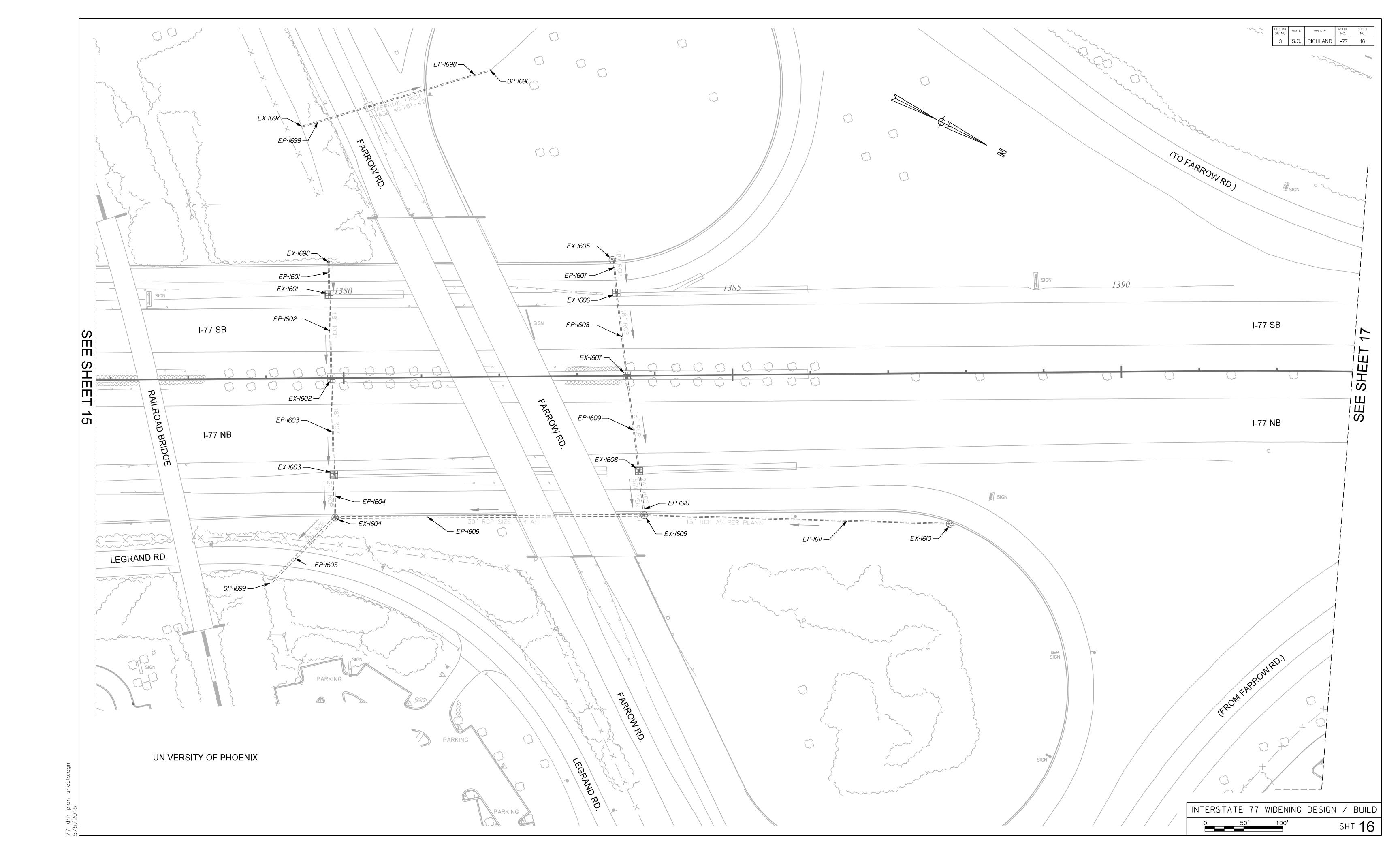


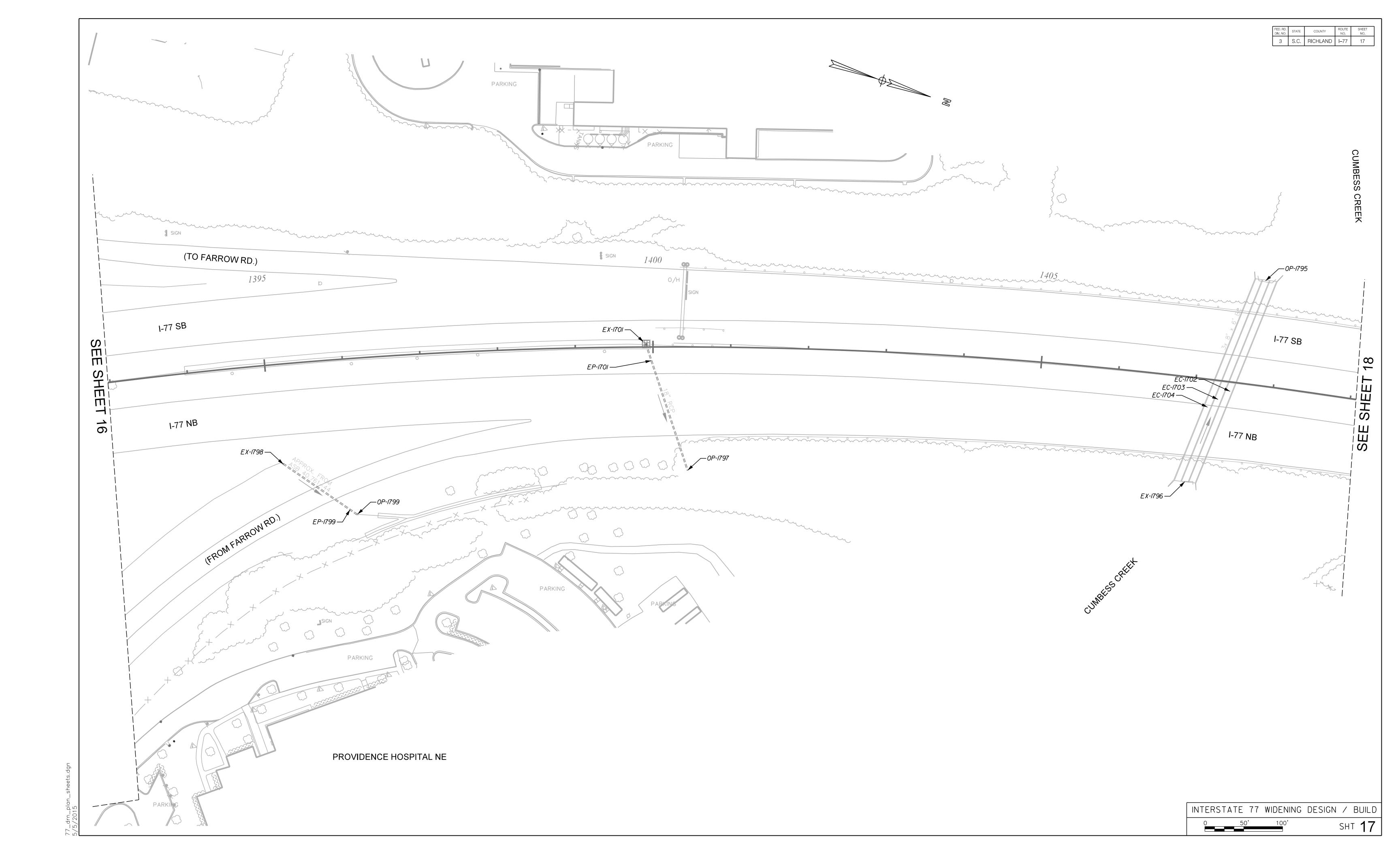


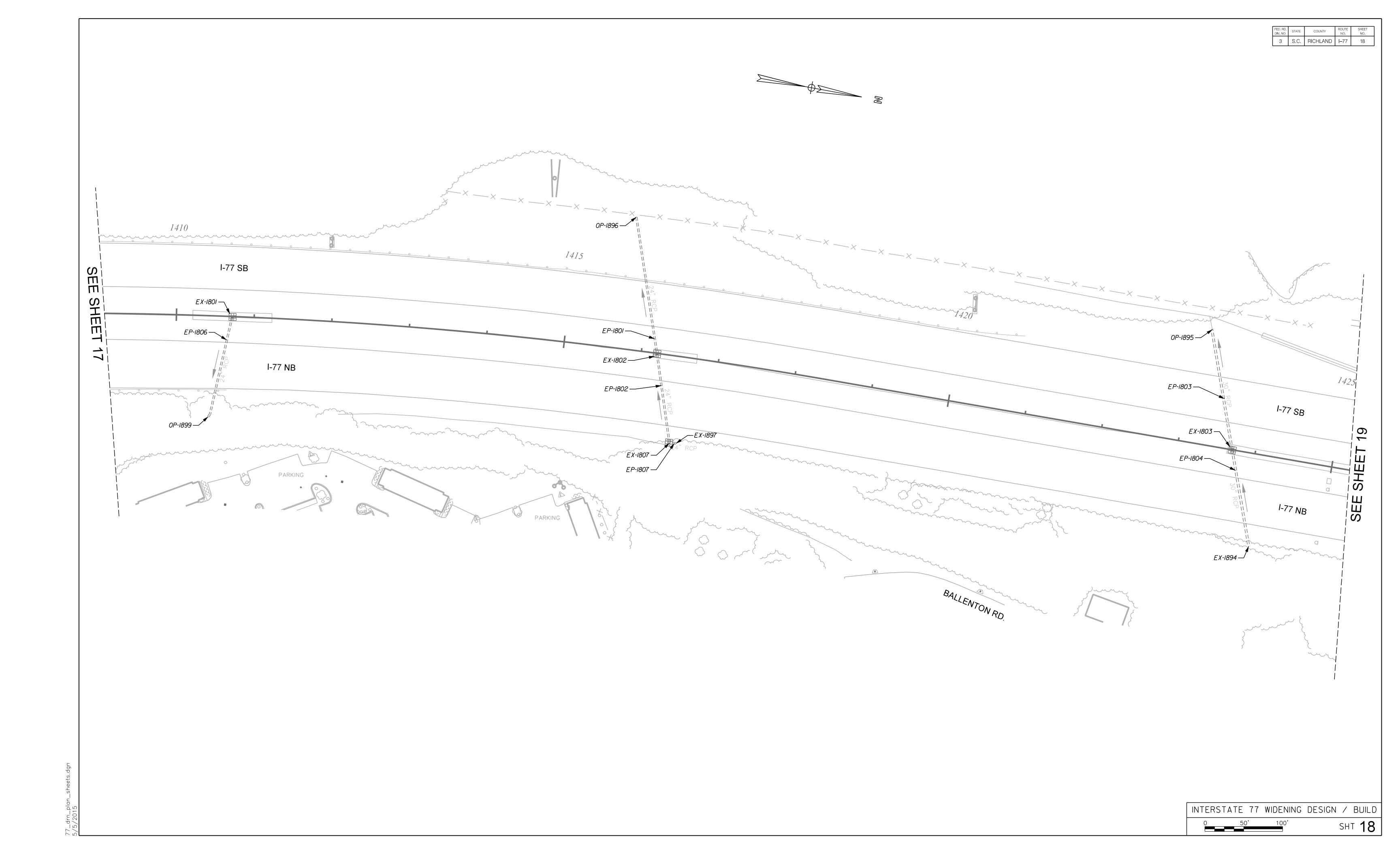


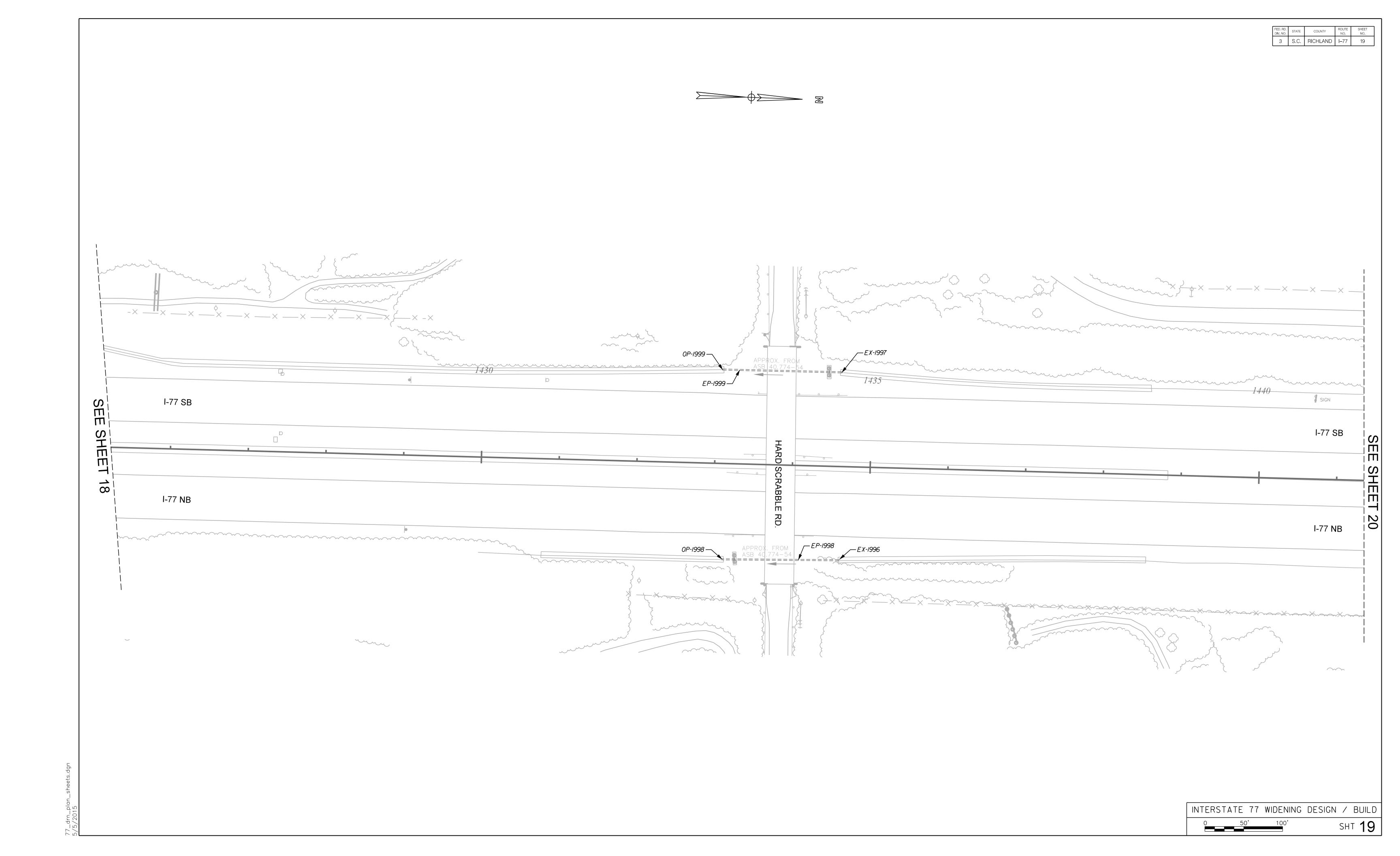


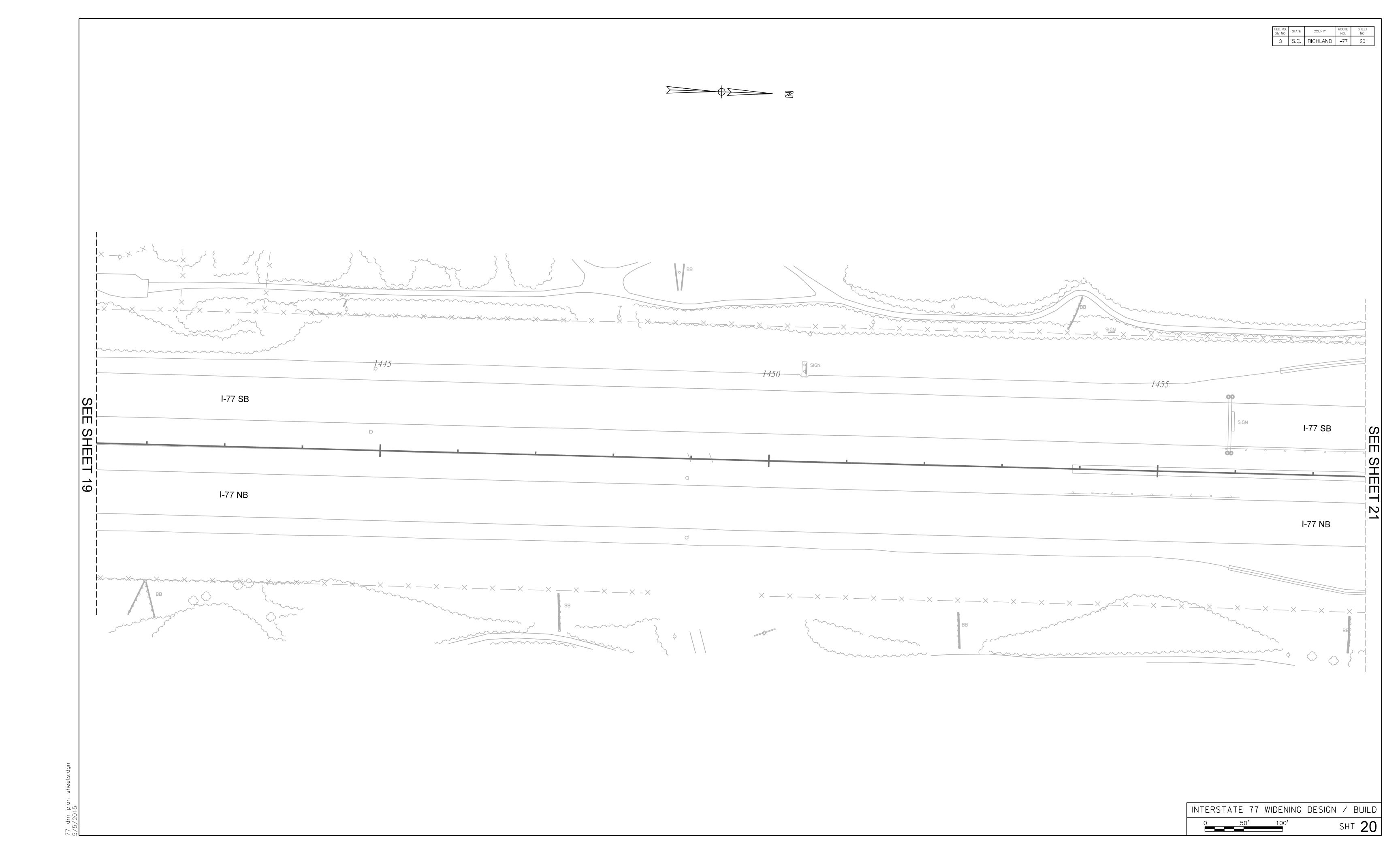


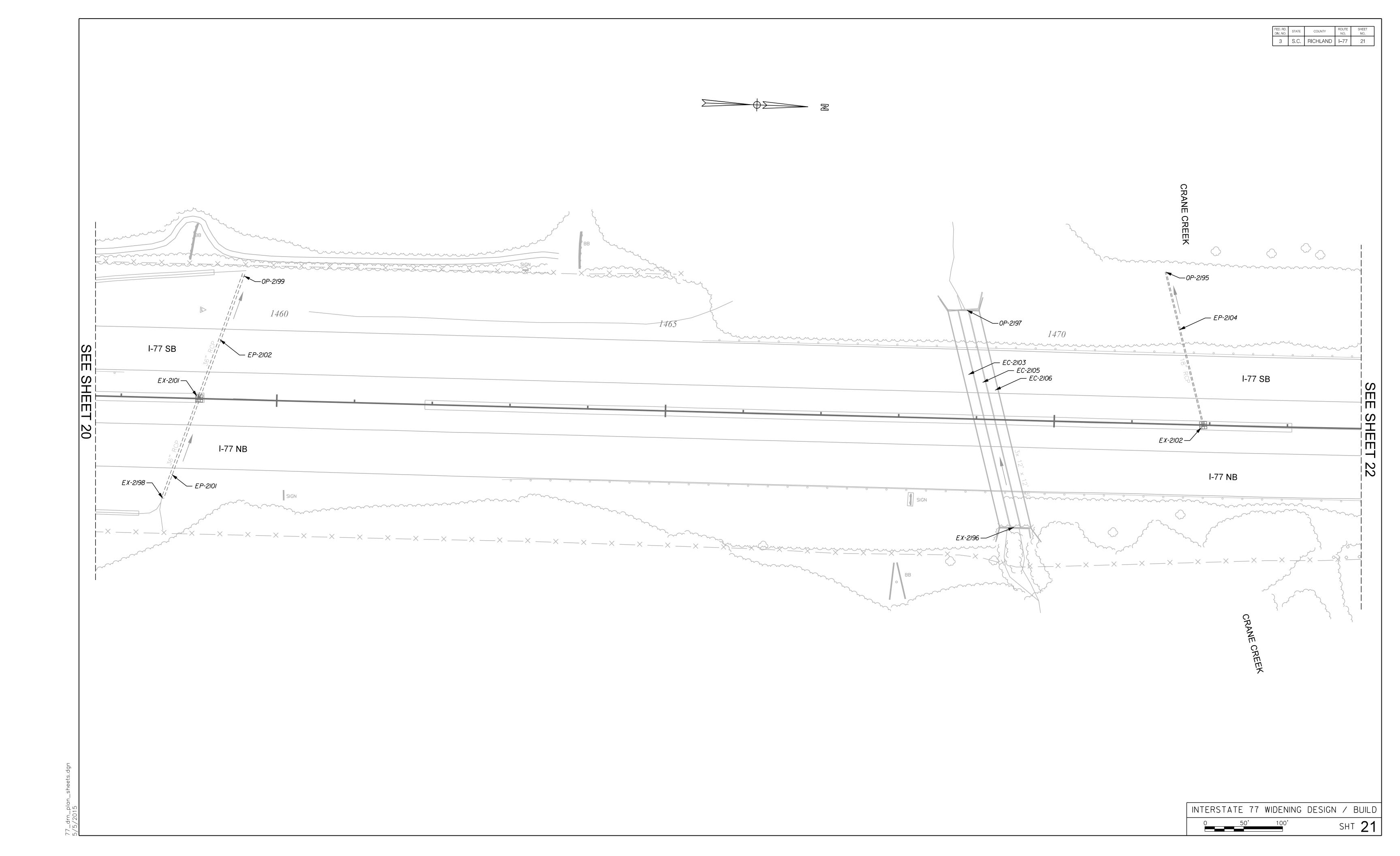


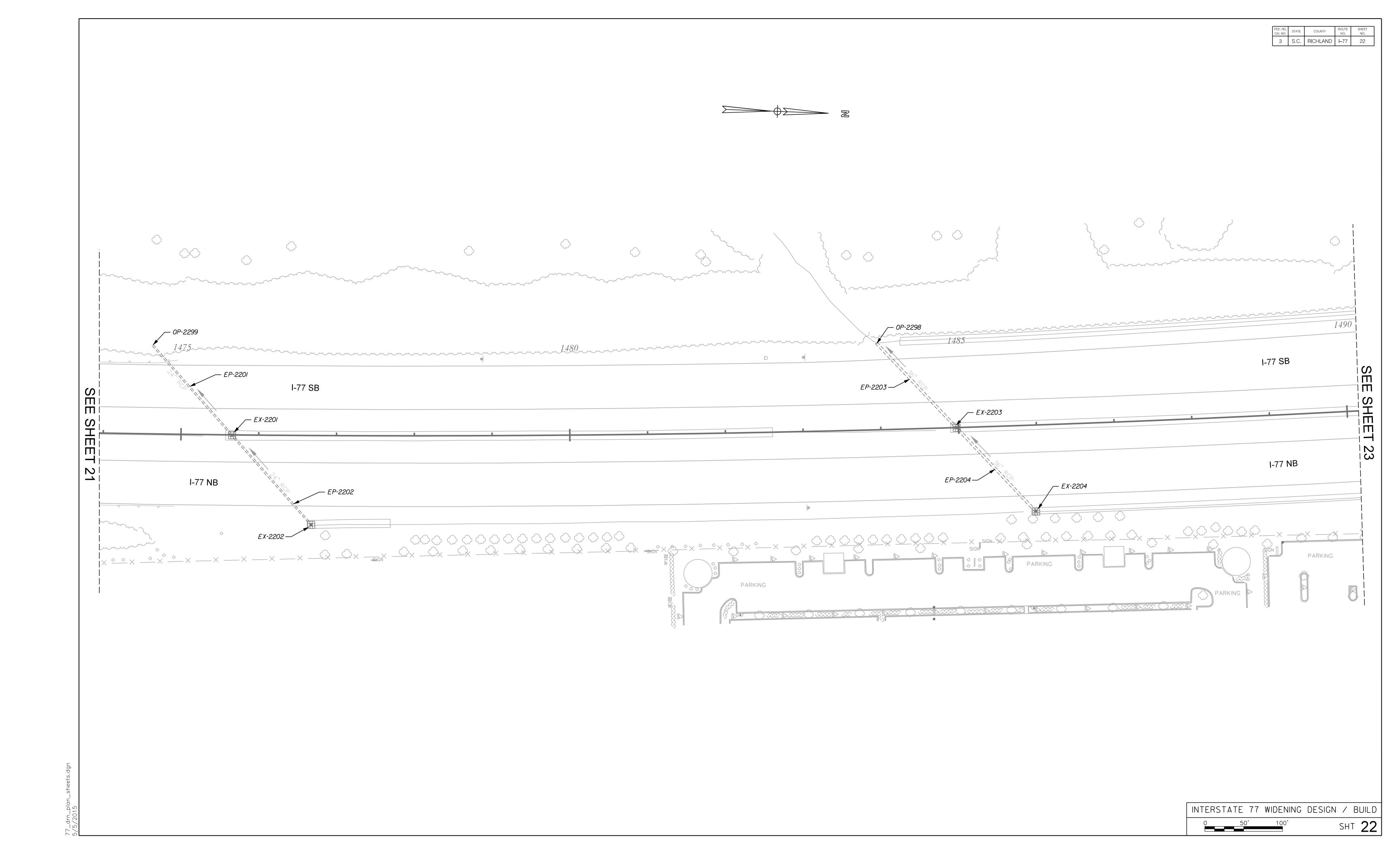


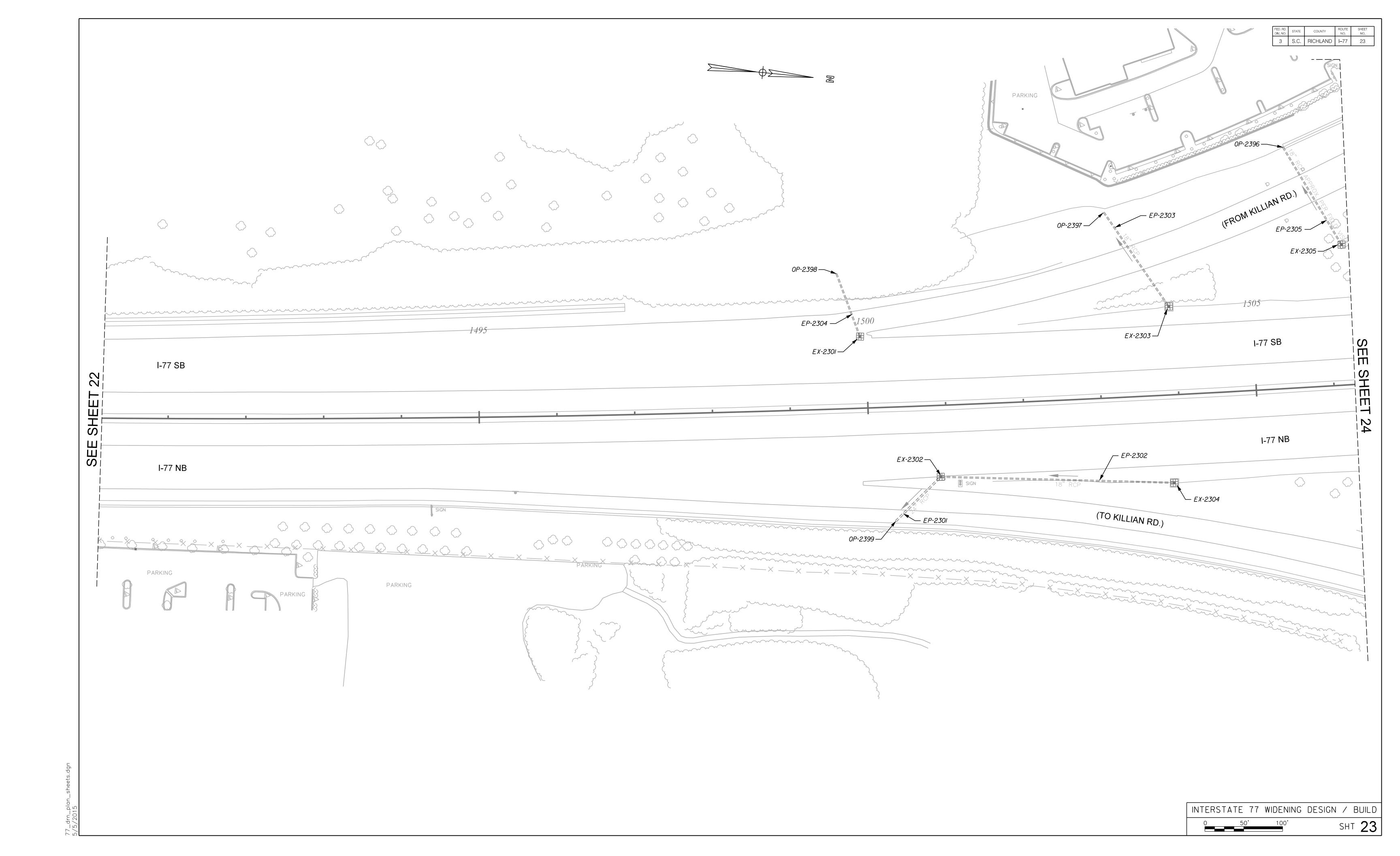


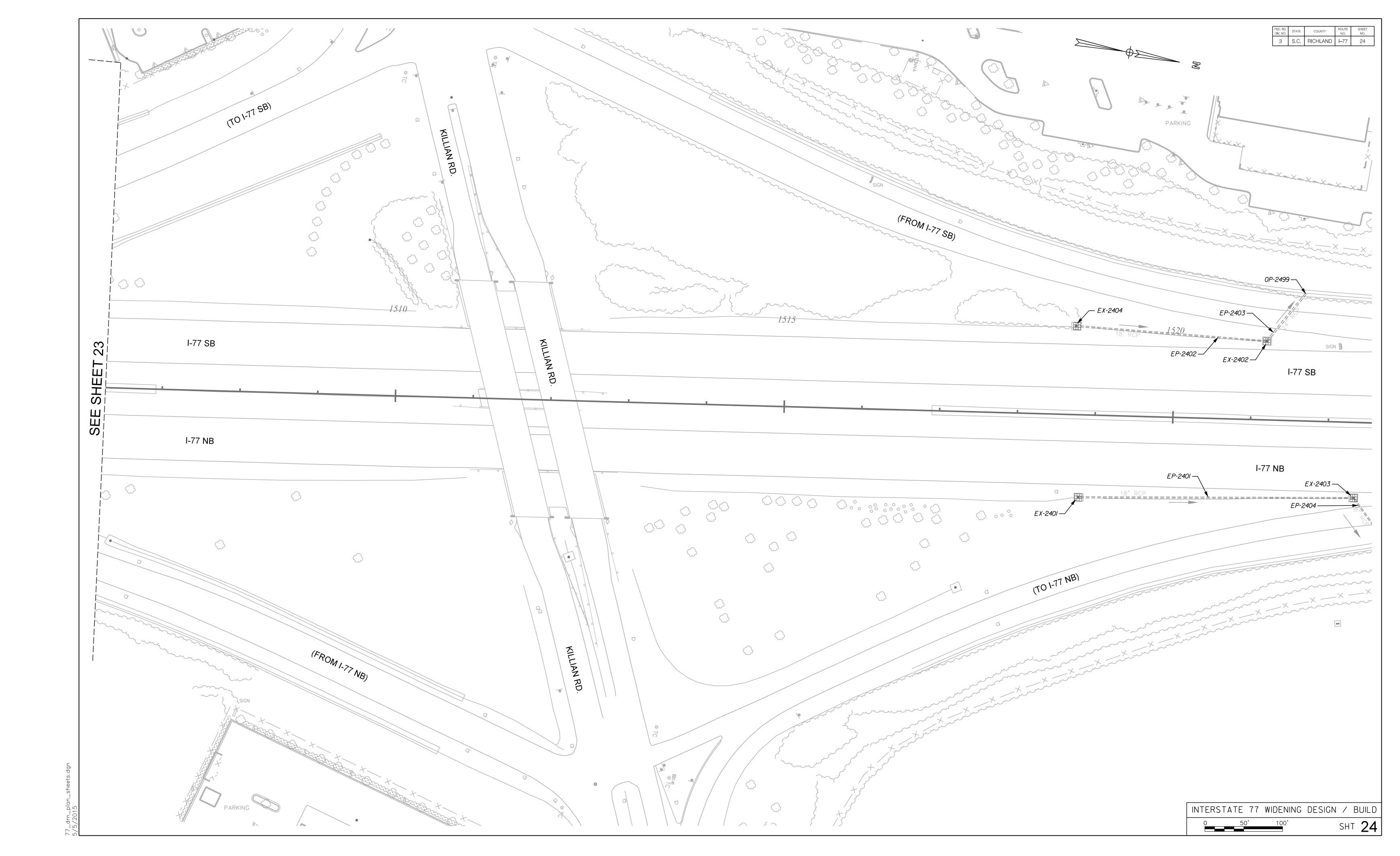






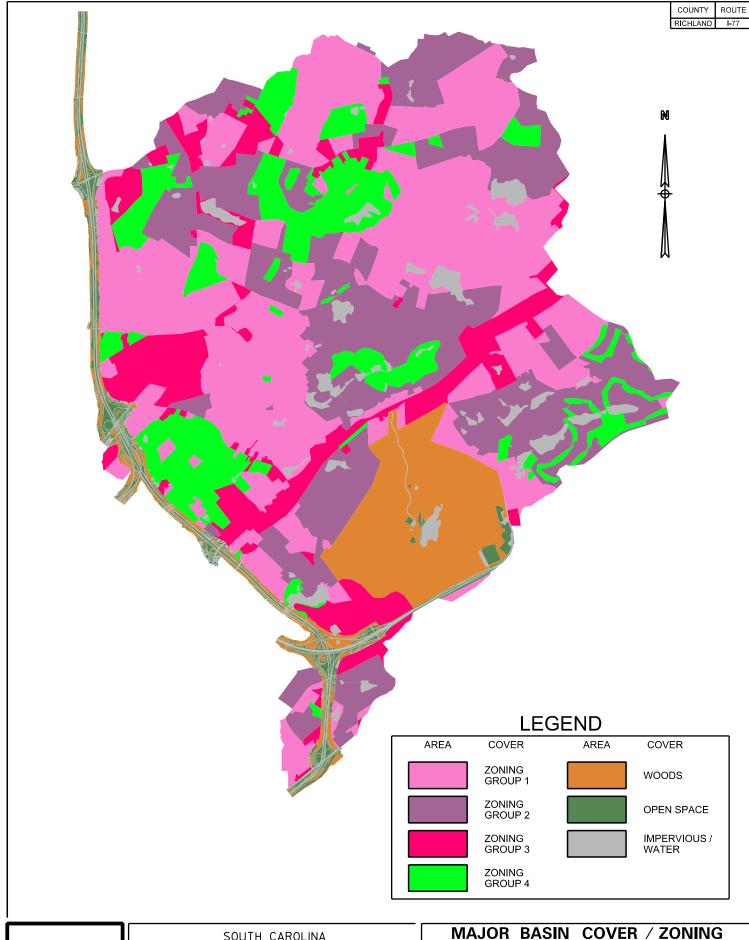






Appendix B

USGS Regression Calculations



SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

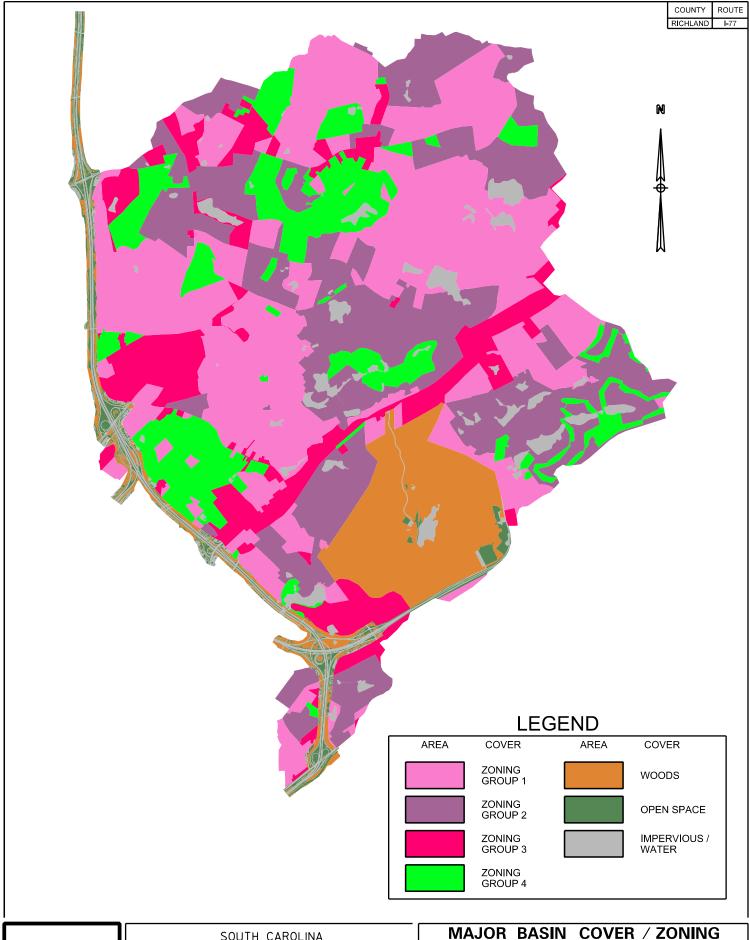
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

MAJOR BASIN COVER / ZONING GROUP DELINEATION MAP – PRE-DEVELOPED CONDITIONS

0 2,500' 5,000'

FILENAME	77_drn_exhibits
SCALE	r - 5,000°

B1



SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

MAJOR BASIN	COVER / ZONING
GROUP DELIN	IEATION MAP –
PRE-DEVELOP	ED CONDITIONS

0 2,500' 5,000'

FILENAME	77_drn_exhibits
SCALE	r • 5,000°

FIGURE **B2**

Project: I-77 Widening D/B	By : D	PH Date :	5-May-15
Location: Richland County, SC	Checked:	Date:	

Pre-Developed Conditions, Total Basin Impervious Area %

	Basin 2	Basin 4	Basin 6
Zone Group 1 Area (ac)	519.78	1,279.86	3,207.07
Zone Group 1 % Impervious	27%	27%	27%
Zone Group 1 Impervious Area (ac)	140.34	345.56	865.91
Zone Group 2 Area (ac)	1,028.75	613.01	2,105.51
Zone Group 2 % Impervious	53%	53%	53%
Zone Group 2 Impervious Area (ac)	545.24	324.90	1,115.92
Zone Group 3 Area (ac)	319.42	489.87	564.26
Zone Group 3 % Impervious	68%	68%	68%
Zone Group 3 Impervious Area (ac)	217.20	333.11	383.70
Zone Group 4 Area (ac)	215.64	630.41	1,010.40
Zone Group 4 % Impervious	15%	15%	15%
Zone Group 4 Impervious Area (ac)	32.35	94.56	151.56
Non-Zone Impervious / Water Area (ac)	254.99	118.37	224.35
Total Basin Impervious Area (ac)	1,190.12	1,216.51	2,741.43
Total Basin Area (ac)	3,703.42	3,302.14	7,124.75
Total Basin Area (sq. mi)	5.79	5.16	11.13
Total Basin % Impervious	32.14%	36.84%	38.4776%

Post-Developed Conditions, Total Basin Impervious Area %

	Basin 2	Basin 4	Basin 6
Zone Group 1 Area (ac)	519.78	1,279.86	3,207.07
Zone Group 1 % Impervious	27%	27%	27%
Zone Group 1 Impervious Area (ac)	140.34	345.56	865.91
Zone Group 2 Area (ac)	1,028.75	613.01	2,105.51
Zone Group 2 % Impervious	53%	53%	53%
Zone Group 2 Impervious Area (ac)	545.24	324.90	1,115.92
Zone Group 3 Area (ac)	319.42	489.87	564.26
Zone Group 3 % Impervious	68%	68%	68%
Zone Group 3 Impervious Area (ac)	217.20	333.11	383.70
Zone Group 4 Area (ac)	215.64	630.41	1,010.40
Zone Group 4 % Impervious	15%	15%	15%
Zone Group 4 Impervious Area (ac)	32.35	94.56	151.56
Non-Zone Impervious / Water Area (ac)	258.33	120.47	224.35
Total Basin Impervious Area (ac)	1,193.46	1,218.61	2,741.44
Total Basin Area (ac)	3,703.42	3,302.14	7,124.75
Total Basin Area (sq. mi)	5.79	5.16	11.13
Total Basin % Impervious	32.23%	36.90%	38.4777%

Project:	I-77 Widening D/B	Ву:	DPH	Date:	5-May-15
Location:	Richland County, SC	Checked:		Date:	

Physiographic Region: Upper Coastal Plain Conditions: Pre-Developed

Drainage Area = 3,703 Acres > 640 Acres, use USGS Urban Regression Equations

USGS Regression Equations

Refer to USGS Water-Resources Investigations Report 92-4040 titled Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds

See Supplement A for Estimation of Equivalent Rural Drainage Basin Peak Discharge, RQ

Drainage Area, $A = \boxed{3,703}$ Acres = $\boxed{5.79}$ Square Miles

Event		RQ (cfs)	
2-year	25 A ^{0.74}	=	91.65
5-year	44 A ^{0.72}	=	155.74
10-year	59 A ^{0.71}	=	205.20
25-year	80 A ^{0.70}	=	273.39
50-year	97 A ^{0.70}	=	331.49
100-year	116 A ^{0.69}	=	389.52
500-year	179 A ^{0.66}	=	570.23

See Table 9 for Estimation of Peak Discharges in Urban Streams, Q

Total Impervious Area, TIA, in % = 32.14 %

Event	Q (cfs)		
2-year	1.36 A ^{0.554} TIA ^{1.241} RQ ₂ ^{0.323}	=	1,148
5-year	2.58 A ^{0.544} TIA ^{1.170} RQ ₅ ^{0.299}	=	1,758
10-year	3.77 A ^{0.536} TIA ^{1.115} RQ ₁₀ ^{0.291}	=	2,178
25-year	5.84 A ^{0.524} TIA ^{1.041} RQ ₂₅ ^{0.284}	=	2,671
50-year	$7.76 \text{ A}^{0.514} \text{ TIA}^{0.987} \text{ RQ}_{50}^{0.283}$	=	3,037
100-year	10.4 A ^{0.506} TIA ^{0.932} RQ ₁₀₀ ^{0.280}	=	3,409
500-year	18.8 A ^{0.484} TIA ^{0.800} RQ ₅₀₀ ^{0.281}	=	4,200

Project:	I-77 Widening D/B	Ву:	DPH	Date:	5-May-15
Location:	Richland County, SC	Checked:		Date:	

Physiographic Region: Upper Coastal Plain Conditions: Post-Developed

Drainage Area = 3,703 Acres > 640 Acres, use USGS Urban Regression Equations

USGS Regression Equations

Refer to USGS Water-Resources Investigations Report 92-4040 titled Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds

See Supplement A for Estimation of Equivalent Rural Drainage Basin Peak Discharge, RQ

Drainage Area, A = 3,703 Acres = 5.79 Square Miles

Event	RQ (cfs)		
2-year	25 A ^{0.74}	=	91.65
5-year	44 A ^{0.72}	=	155.74
10-year	59 A ^{0.71}	=	205.20
25-year	80 A ^{0.70}	=	273.39
50-year	97 A ^{0.70}	=	331.49
100-year	116 A ^{0.69}	=	389.52
500-year	179 A ^{0.66}	=	570.23

See Table 9 for Estimation of Peak Discharges in Urban Streams, Q

Total Impervious Area, TIA, in % = 32.23 %

Event	Q (cfs)		
2-year	1.36 A ^{0.554} TIA ^{1.241} RQ ₂ ^{0.323}	=	1,152
5-year	2.58 A ^{0.544} TIA ^{1.170} RQ ₅ ^{0.299}	=	1,764
10-year	$3.77 A^{0.536} TIA^{1.115} RQ_{10}^{0.291}$	=	2,185
25-year	5.84 A ^{0.524} TIA ^{1.041} RQ ₂₅ ^{0.284}	=	2,679
50-year	$7.76 \text{ A}^{0.514} \text{ TIA}^{0.987} \text{ RQ}_{50}^{0.283}$	=	3,045
100-year	10.4 A ^{0.506} TIA ^{0.932} RQ ₁₀₀ ^{0.280}	=	3,418
500-year	18.8 A ^{0.484} TIA ^{0.800} RQ ₅₀₀ ^{0.281}	=	4,209

Project:	I-77 Widening D/B	Ву:	DPH	Date:	5-May-15
Location:	Richland County, SC	Checked:		Date:	

Physiographic Region: Upper Coastal Plain Conditions: Pre-Developed

Drainage Area = 3,302 Acres > 640 Acres, use USGS Urban Regression Equations

USGS Regression Equations

Refer to USGS Water-Resources Investigations Report 92-4040 titled Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds

See Supplement A for Estimation of Equivalent Rural Drainage Basin Peak Discharge, RQ

Drainage Area, A = 3,302 Acres = 5.16 Square Miles

Event		RQ (cfs)	
2-year	25 A ^{0.74}	=	84.19
5-year	44 A ^{0.72}	=	143.40
10-year	59 A ^{0.71}	=	189.15
25-year	80 A ^{0.70}	=	252.30
50-year	97 A ^{0.70}	=	305.92
100-year	116 A ^{0.69}	=	359.88
500-year	179 A ^{0.66}	=	528.66

See Table 9 for Estimation of Peak Discharges in Urban Streams, Q

Total Impervious Area, TIA, in % = 36.84 %

Event	Q (cfs)		
2-year	1.36 A ^{0.554} TIA ^{1.241} RQ ₂ ^{0.323}	=	1,242
5-year	2.58 A ^{0.544} TIA ^{1.170} RQ ₅ ^{0.299}	=	1,891
10-year	$3.77 \text{ A}^{0.536} \text{ TIA}^{1.115} \text{ RQ}_{10}^{0.291}$	=	2,330
25-year	5.84 A ^{0.524} TIA ^{1.041} RQ ₂₅ ^{0.284}	=	2,835
50-year	$7.76 \text{ A}^{0.514} \text{ TIA}^{0.987} \text{ RQ}_{50}^{0.283}$	=	3,203
100-year	10.4 A ^{0.506} TIA ^{0.932} RQ ₁₀₀ ^{0.280}	=	3,574
500-year	18.8 A ^{0.484} TIA ^{0.800} RQ ₅₀₀ ^{0.281}	=	4,338

Project:	I-77 Widening D/B	By:	DPH	Date:	5-May-15
Location:	Richland County, SC	Checked:		Date:	

Physiographic Region: Upper Coastal Plain Conditions: Post-Developed

Drainage Area = 3,302 Acres > 640 Acres, use USGS Urban Regression Equations

USGS Regression Equations

Refer to USGS Water-Resources Investigations Report 92-4040 titled Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds

See Supplement A for Estimation of Equivalent Rural Drainage Basin Peak Discharge, RQ

Drainage Area, $A = \boxed{3,302}$ Acres = $\boxed{5.16}$ Square Miles

Event		RQ (cfs)	
2-year	25 A ^{0.74}	=	84.19
5-year	44 A ^{0.72}	=	143.40
10-year	59 A ^{0.71}	=	189.15
25-year	80 A ^{0.70}	=	252.30
50-year	97 A ^{0.70}	=	305.92
100-year	116 A ^{0.69}	=	359.88
500-year	179 A ^{0.66}	=	528.66

See Table 9 for Estimation of Peak Discharges in Urban Streams, Q

Total Impervious Area, TIA, in % = 36.90 %

Event	Q (cfs)		
2-year	1.36 A ^{0.554} TIA ^{1.241} RQ ₂ ^{0.323}	=	1,244
5-year	2.58 A ^{0.544} TIA ^{1.170} RQ ₅ ^{0.299}	=	1,895
10-year	$3.77 A^{0.536} TIA^{1.115} RQ_{10}^{0.291}$	=	2,334
25-year	5.84 A ^{0.524} TIA ^{1.041} RQ ₂₅ ^{0.284}	=	2,840
50-year	7.76 A ^{0.514} TIA ^{0.987} RQ ₅₀ ^{0.283}	=	3,208
100-year	10.4 A ^{0.506} TIA ^{0.932} RQ ₁₀₀ ^{0.280}	=	3,580
500-year	18.8 A ^{0.484} TIA ^{0.800} RQ ₅₀₀ ^{0.281}	=	4,344

Project:	I-77 Widening D/B	By:	DPH	Date:	5-May-15
Location:	Richland County, SC	Checked:		Date:	

Physiographic Region: Upper Coastal Plain Conditions: Pre-Developed

Drainage Area = 7,125 Acres > 640 Acres, use USGS Urban Regression Equations

USGS Regression Equations

Refer to USGS Water-Resources Investigations Report 92-4040 titled Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds

See Supplement A for Estimation of Equivalent Rural Drainage Basin Peak Discharge, RQ

Drainage Area, $A = \boxed{7,125}$ Acres = $\boxed{11.13}$ Square Miles

Event		RQ (cfs)	
2-year	25 A ^{0.74}	=	148.74
5-year	44 A ^{0.72}	=	249.46
10-year	59 A ^{0.71}	=	326.54
25-year	80 A ^{0.70}	=	432.22
50-year	97 A ^{0.70}	=	524.06
100-year	116 A ^{0.69}	=	611.79
500-year	179 A ^{0.66}	=	878.22

See Table 9 for Estimation of Peak Discharges in Urban Streams, Q

Total Impervious Area, TIA, in % = 38.48 %

Event	Q (cfs)		
2-year	1.36 A ^{0.554} TIA ^{1.241} RQ ₂ ^{0.323}	=	2,411
5-year	2.58 A ^{0.544} TIA ^{1.170} RQ ₅ ^{0.299}	=	3,567
10-year	$3.77 A^{0.536} TIA^{1.115} RQ_{10}^{0.291}$	=	4,329
25-year	5.84 A ^{0.524} TIA ^{1.041} RQ ₂₅ ^{0.284}	=	5,171
50-year	7.76 A ^{0.514} TIA ^{0.987} RQ ₅₀ ^{0.283}	=	5,781
100-year	10.4 A ^{0.506} TIA ^{0.932} RQ ₁₀₀ ^{0.280}	=	6,372
500-year	18.8 A ^{0.484} TIA ^{0.800} RQ ₅₀₀ ^{0.281}	=	7,517

Project:	I-77 Widening D/B	By:	DPH	Date:	5-May-15
Location:	Richland County, SC	Checked:		Date:	

Physiographic Region: Upper Coastal Plain Conditions: Post-Developed

Drainage Area = 7,125 Acres > 640 Acres, use USGS Urban Regression Equations

USGS Regression Equations

Refer to USGS Water-Resources Investigations Report 92-4040 titled Determination Of Flood Hydrographs For Streams In South Carolina: Volume 2 Estimation Of Peak-Discharge Frequency, Runoff Volumes, And Flood Hydrographs For Urban Watersheds

See Supplement A for Estimation of Equivalent Rural Drainage Basin Peak Discharge, RQ

Drainage Area, A = 7,125 Acres = 11.13 Square Miles

Event		RQ (cfs)	
2-year	25 A ^{0.74}	=	148.74
5-year	44 A ^{0.72}	=	249.46
10-year	59 A ^{0.71}	=	326.54
25-year	80 A ^{0.70}	=	432.22
50-year	97 A ^{0.70}	=	524.06
100-year	116 A ^{0.69}	=	611.79
500-year	179 A ^{0.66}	=	878.22

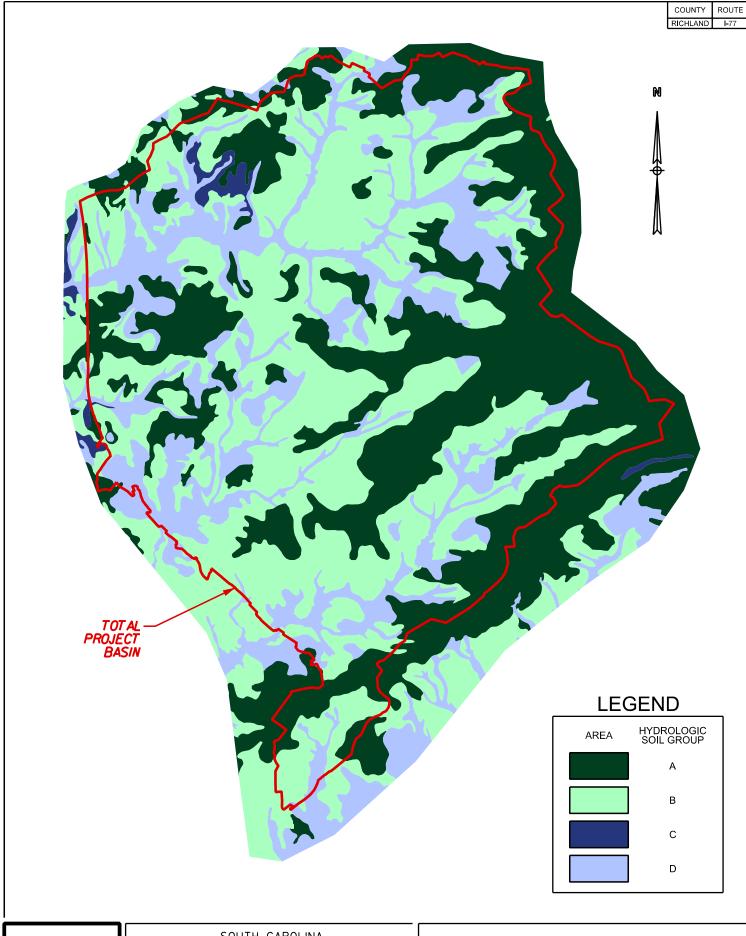
See Table 9 for Estimation of Peak Discharges in Urban Streams, Q

Total Impervious Area, TIA, in % = 38.48 %

Event	Q (cfs)		
2-year	1.36 A ^{0.554} TIA ^{1.241} RQ ₂ ^{0.323}	=	2,411
5-year	2.58 A ^{0.544} TIA ^{1.170} RQ ₅ ^{0.299}	=	3,567
10-year	3.77 A ^{0.536} TIA ^{1.115} RQ ₁₀ ^{0.291}	=	4,329
25-year	5.84 A ^{0.524} TIA ^{1.041} RQ ₂₅ ^{0.284}	=	5,171
50-year	7.76 A ^{0.514} TIA ^{0.987} RQ ₅₀ ^{0.283}	=	5,781
100-year	10.4 A ^{0.506} TIA ^{0.932} RQ ₁₀₀ ^{0.280}	=	6,372
500-year	18.8 A ^{0.484} TIA ^{0.800} RQ ₅₀₀ ^{0.281}	=	7,517

Appendix C

NRCS Calculations



SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

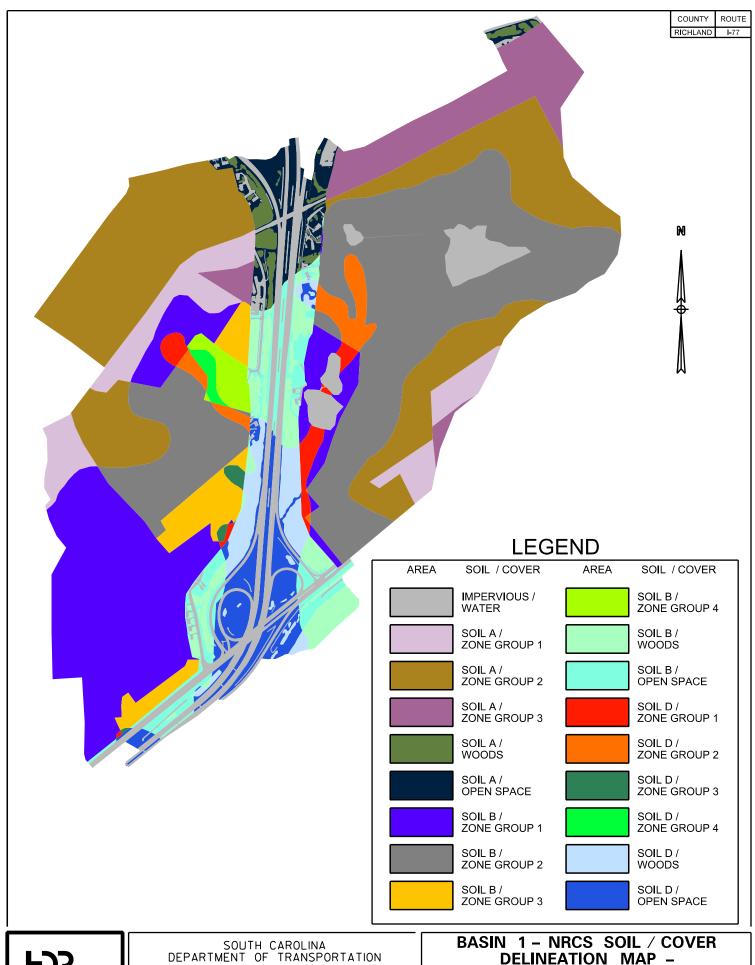
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

SOIL GROUP DELINEATION MAP

0 2,500 5,000

FILENAME	77_drn_exhibits
SCALE	r* • 5,000°

FIGURE C1

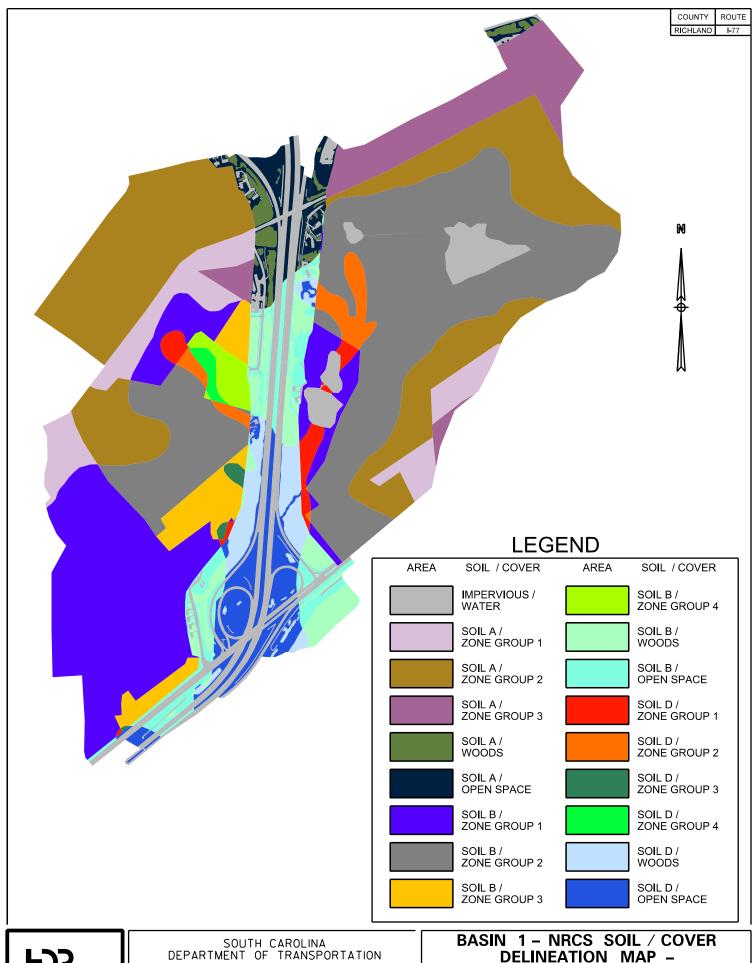


I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.) RICHLAND COUNTY, SOUTH CAROLINA

DELINEATION MAP -PRE-DEVELOPED CONDITIONS

FILENAME	77_drn_exhibits
SCALE	l" + 1,000°

C2



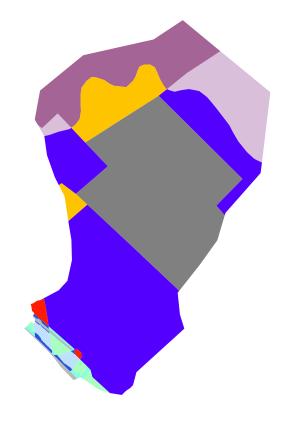
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.) RICHLAND COUNTY, SOUTH CAROLINA

DELINEATION MAP -POST-DEVELOPED CONDITIONS

FILENAME	77_drn_exhibits
SCALE	r - 1,000°

C3

COUNTY ROUTE
RICHLAND I-77



LEGEND





SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

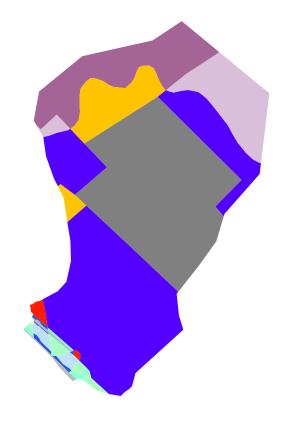
BASIN 3 - NRCS SOIL / COVER
DELINEATION MAP -
PRE-DEVELOPED CONDITIONS

0 500' 1,000'

FILENAME	77_drn_exhibits
SCALE	l" - 1,000°

FIGURE C4

COUNTY ROUTE
RICHLAND I-77





LEGEND





SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

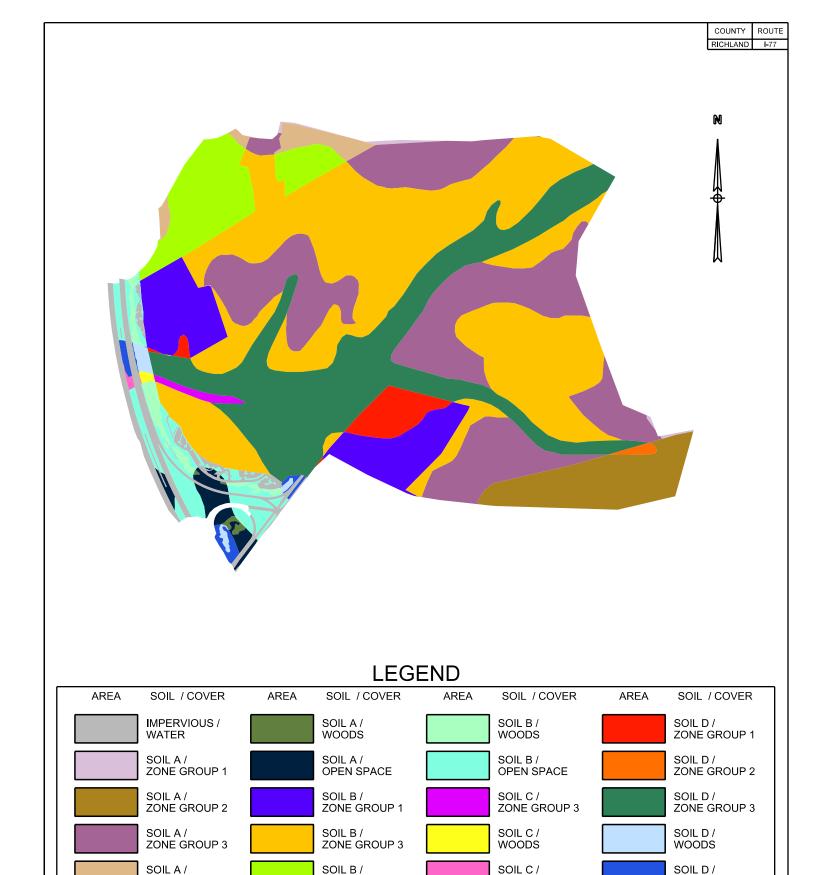
I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

BASIN 3 - NRCS SOIL / COVER
DELINEATION MAP -
POST-DEVELOPED CONDITIONS

0	500'	1,000'
=		

FILENAME	77_drn_exhibits
SCALE	r - 1,000°

FIGURE C5





ZONE GROUP 4

SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION

ZONE GROUP 4

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

BASIN 5 - NRCS SOIL / COVER
DELINEATION MAP -
PRE-DEVELOPED CONDITIONS

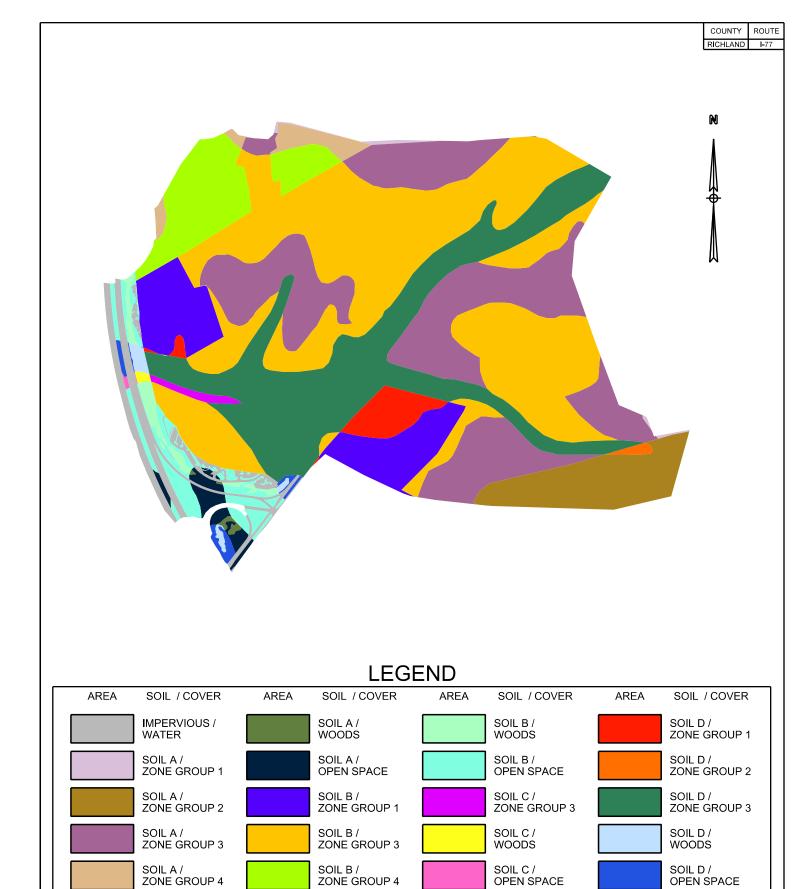
0	500'	1,000'

OPEN SPACE

FILENAME	77_drn_exhibits
SCALE	I" - 1,000°

FIGURE **C6**

OPEN SPACE





SOUTH CAROLINA
DEPARTMENT OF TRANSPORTATION

I-77 WIDENING (PERCIVAL RD. TO KILLIAN RD.)
RICHLAND COUNTY, SOUTH CAROLINA

BASIN 5 - NRCS SOIL / COVER DELINEATION MAP - POST-DEVELOPED CONDITIONS

0	500'	1,000'

FILENAME	77_drn_exhibits
SCALE	1" - 1,000

FIGURE **C7**

Project: I-77 Widening D/B	By:	DPH	Date:	5-May-15
Location: Richland County, SC	Checked:		Date:	

Pre-Developed Conditions, NRCS Basins - Composite Curve Numbers

	Basin 1	Basin 3	Basin 5
Impervious / Water Area (ac)	47.57	0.63	12.89
Impervious / Water CN	98	98	98
Soil Class A / Zone Group 1 Area (ac)	30.77	12.70	0.98
Soil Class A / Zone Group 1 CN	77	77	77
Soil Class A / Zone Group 2 Area (ac)	117.66	0.00	22.00
Soil Class A / Zone Group 2 CN	57	57	57
Soil Class A / Zone Group 3 Area (ac)	33.97	17.01	89.42
Soil Class A / Zone Group 3 CN	89	89	89
Soil Class A / Zone Group 4 Area (ac)	0.00	0.00	5.51
Soil Class A / Zone Group 4 CN	49	49	49
Soil Class A / Woods Area (ac)	5.71	0.00	0.53
Soil Class A / Woods CN	36	36	36
Soil Class A / Open Space Area (ac)	13.12	0.00	4.41
Soil Class A / Open Space CN	39	39	39
Soil Class B / Zone Group 1 Area (ac)	86.34	52.34	28.09
Soil Class B / Zone Group 1 CN	85	85	85
Soil Class B / Zone Group 2 Area (ac)	132.75	43.83	0.00
Soil Class B / Zone Group 2 CN	72	72	72
Soil Class B / Zone Group 3 Area (ac)	16.56	9.74	138.91
Soil Class B / Zone Group 3 CN	92	92	92
Soil Class B / Zone Group 4 Area (ac)	5.01	0.00	24.86
Soil Class B / Zone Group 4 CN	69	69	69
Soil Class B / Woods Area (ac)	16.85	1.34	3.45
Soil Class B / Woods CN	60	60	60
Soil Class B / Open Space Area (ac)	15.92	0.51	12.12
Soil Class B / Open Space CN	61	61	61
Soil Class C / Zone Group 1 Area (ac)	0.00	0.00	0.00
Soil Class C / Zone Group 1 CN	90	90	90
Soil Class C / Zone Group 2 Area (ac)	0.00	0.00	0.00
Soil Class C / Zone Group 2 CN	81	81	81
Soil Class C / Zone Group 3 Area (ac)	0.00	0.00	1.57
Soil Class C / Zone Group 3 CN	94	94	94
Soil Class C / Zone Group 4 Area (ac)	0.00	0.00	0.00
Soil Class C / Zone Group 4 CN	79	79	79
Soil Class C / Woods Area (ac)	0.00	0.00	0.34
Soil Class C / Woods CN	73	73	73
Soil Class C / Open Space Area (ac)	0.00	0.00	0.26
Soil Class C / Open Space CN	74	74	74
Soil Class D / Zone Group 1 Area (ac)	5.65	0.74	8.17
Soil Class D / Zone Group 1 CN	92	92	92
Soil Class D / Zone Group 2 Area (ac)	8.51	0.00	0.78
Soil Class D / Zone Group 2 CN	86	86	86
Soil Class D / Zone Group 3 Area (ac)	1.24	0.00	73.17
Soil Class D / Zone Group 3 CN	95	95	95
Soil Class D / Zone Group 4 Area (ac)	1.80	0.00	0.00
Soil Class D / Zone Group 4 CN	84	84	84
Soil Class D / Woods Area (ac)	12.69	1.07	1.55
Soil Class D / Woods CN	79	79	79
Soil Class D / Open Space Area (ac)	19.30	0.43	2.00
Soil Class D / Open Space CN	80	80	80
Total Basin Area (ac)	571.42	140.34	431.01
Basin Composite CN	74.02	80.90	86.04

Project: I-77 Widening D/B	By:	DPH	Date:	5-May-15
Location: Richland County, SC	Checked:		Date:	

Post-Developed Conditions, NRCS Basins - Composite Curve Numbers

i ost-beveloped Conditions, Mixos basing	13 Composite	Carve Namb	CIS
	Basin 1	Basin 3	Basin 5
Impervious / Water Area (ac)	49.16	0.63	14.13
Impervious / Water CN	98	98	98
Soil Class A / Zone Group 1 Area (ac)	30.77	12.70	0.98
Soil Class A / Zone Group 1 CN	77	77	77
Soil Class A / Zone Group 2 Area (ac)	117.66	0.00	22.00
Soil Class A / Zone Group 2 CN	57	57	57
Soil Class A / Zone Group 3 Area (ac)	33.97	17.01	89.42
Soil Class A / Zone Group 3 CN	89	89	89
Soil Class A / Zone Group 4 Area (ac)	0.00	0.00	5.51
Soil Class A / Zone Group 4 CN	49	49	49
Soil Class A / Woods Area (ac)	5.71	0.00	0.53
Soil Class A / Woods CN	36	36	36
Soil Class A / Open Space Area (ac)	12.30	0.00	4.24
Soil Class A / Open Space CN	39	39	39
Soil Class B / Zone Group 1 Area (ac)	86.34	52.34	28.09
Soil Class B / Zone Group 1 CN	85	85	85
Soil Class B / Zone Group 2 Area (ac)	132.75	43.83	0.00
Soil Class B / Zone Group 2 CN	72	72	72
Soil Class B / Zone Group 3 Area (ac)	16.56	9.74	138.91
Soil Class B / Zone Group 3 CN	92	92	92
Soil Class B / Zone Group 4 Area (ac)	5.01	0.00	24.86
Soil Class B / Zone Group 4 CN	69	69	69
Soil Class B / Woods Area (ac)	16.85	1.34	3.45
Soil Class B / Woods CN	60	60	60
Soil Class B / Open Space Area (ac)	15.35	0.51	11.33
Soil Class B / Open Space CN	61	61	61
Soil Class C / Zone Group 1 Area (ac)	0.00	0.00	0.00
Soil Class C / Zone Group 1 CN	90	90	90
Soil Class C / Zone Group 2 Area (ac)	0.00	0.00	0.00
Soil Class C / Zone Group 2 CN	81	81	81
Soil Class C / Zone Group 3 Area (ac)	0.00	0.00	1.57
Soil Class C / Zone Group 3 CN	94	94	94
Soil Class C / Zone Group 4 Area (ac)	0.00	0.00	0.00
Soil Class C / Zone Group 4 Area (ac)	79	79	79
Soil Class C / Zone Group 4 CN Soil Class C / Woods Area (ac)	0.00	0.00	0.34
Soil Class C / Woods Area (ac) Soil Class C / Woods CN	73	73	73
Soil Class C / Woods CN Soil Class C / Open Space Area (ac)	0.00	0.00	0.16
Soil Class C / Open Space CN Soil Class D / Zone Group 1 Area (ac)	74 5.65	74 0.74	74 8.17
, , ,			
Soil Class D / Zone Group 1 CN	92 8 51	92	92
Soil Class D / Zone Group 2 Area (ac)	8.51	0.00	0.78
Soil Class D / Zone Group 2 CN	86	86	86 72.17
Soil Class D / Zone Group 3 Area (ac)	1.24	0.00	73.17
Soil Class D / Zone Group 3 CN	95	95	95
Soil Class D / Zone Group 4 Area (ac)	1.80	0.00	0.00
Soil Class D / Zone Group 4 CN	84	84	84
Soil Class D / Woods Area (ac)	12.69	1.07	1.55
Soil Class D / Woods CN	79	79	79
Soil Class D / Open Space Area (ac)	19.11	0.43	1.83
Soil Class D / Open Space CN	80	80	80
Total Basin Area (ac)	571.42	140.34	431.01
Basin Composite CN	74.14	80.90	86.15

Project: I-77 Widening D/B	By:	DPH	Date:	5-May-15
Location: Richland County, SC	Checked:		Date:	

NRCS Peak Flow Calculations

	Area	Time of	Design	Pre-	Post-
Basin ID	Basin ID (ac)	Concentration	Storm	Developed	Developed
(ac)	(min)	Event	Flows (cfs)	Flows (cfs)	
			2-Year	472.17	474.75
			5-Year	729.70	732.73
1	571.42	63.06	10-Year	975.83	979.41
1	371.42	03.00	25-Year	1303.35	1306.68
			50-Year	1587.76	1590.90
			100-Year	1871.84	1875.14
		74.11	2-Year	138.88	138.88
			5-Year	200.67	200.67
3	3 140.34		10-Year	258.88	258.88
3	140.54		25-Year	333.50	333.50
			50-Year	397.43	397.43
			100-Year	461.14	461.14
			2-Year	313.11	314.16
			5-Year	434.06	435.20
5 431.01	152.25	10-Year	547.10	548.16	
	431.01	1.01 153.35	25-Year	690.25	691.44
			50-Year	813.19	814.31
		100-Year	934.74	935.88	

Project: I-77 Widening D/B	By:	DPH	Date:	5-May-15	
Location: Richland County, SC	By: Checked:	DEII	Date:	3-iviay-13	
Location: Internal County, Co	Onconcu.		Duto.		
Time of Concentration		ID:	Basin 1		
Sheet Flow (Applicable to Tc only)					
 Manning's roughness coefficient, n (table 3-1) Flow length, L (total L + 300 ft) Two-year 24-hour rainfall, P₂ Land slope, s Travel time, T_t = 0.007(nL)^{0.8}/((P₂^{0.5})(s^{0.4})) 		ft in ft/ft hr	0.24 217 3.8 0.037 0.32	19.20	min
Shallow concentrated flow					
 6. Surface description (paved or unpaved) 7. Flow length, L 8. Watercourse slope, s 9. Average velocity, V (Figure 3-1) 10. Travel time, T_t = L / 60*V*60 	Compute T_t	ft ft / ft ft/s hr	Unpaved 227 0.044 3.39 0.02	1.12	min
<u>Channel flow</u>					
 11. Cross sectional flow area, a 12. Wetted perimeter, p_w 13. Hydraulic radius, r = a/p_w 14. Channel slope, s 15. Manning's roughness coefficient, n 16. V=1.49 r^{2/3} s^{1/2} / n 	Compute V	ft ² ft ft ft ft/ft	18 13 1.385 0.018 0.1 2.47		
17. Flow length, L 18. Travel time, T _t = L / 3600* V	Compute T _t	ft	6334	10 7E	min
10. Haverume, I _t = L / 3000 V	Compute 1 _t	hr	0.71	42.75	min
19. Watershed or subarea T_c or T_t	Total	hr	1.05	63.06	min

Project: I-77 Widening D/B	By:	DPH	Date: 5-May-15	
Location: Richland County, SC	Checked:	<u> </u>	Date:	
Time of Concentration		ID:	Basin 3	
Sheet Flow (Applicable to Tc only) 1. Manning's roughness coefficient, n (table 3-1) 2. Flow length, L (total L + 300 ft) 3. Two-year 24-hour rainfall, P ₂ 4. Land slope, s 5. Travel time, T _t = 0.007(nL) ^{0.8} /((P ₂ ^{0.5})(s ^{0.4}))		ft in ft/ft hr	0.4 300 3.8 0.067 0.49 29.40 mir	n
Shallow concentrated flow 6. Surface description (paved or unpaved) 7. Flow length, L 8. Watercourse slope, s 9. Average velocity, V (Figure 3-1) 10. Travel time, T _t = L / 60*V*60	Compute T _t	ft ft / ft ft/s hr	Unpaved 360 0.033 2.94 0.03 2.04 min	n
Channel flow 11. Cross sectional flow area, a 12. Wetted perimeter, p _w 13. Hydraulic radius, r = a/p _w 14. Channel slope, s 15. Manning's roughness coefficient, n 16. V=1.49 r ^{2/3} s ^{1/2} / n 17. Flow length, L 18. Travel time, T _t = L / 3600* V	$\begin{array}{c} Compute \ V \\ Compute \ T_t \end{array}$	ft ² ft ft ft/ft ft/s ft/s ft	16 12 1.333 0.017 0.14 1.68 4304 0.71 42.67 mir	n
19. Watershed or subarea T_c or T_t	Total	hr	1.24 74.11 mi	in

Project:	I-77 Widening D/B	Ву:	DPH	Date:	5-May-15	
Location:	Richland County, SC	Checked:		Date:		
Time of Co			ID:	Basin 5		
Sheet Flow	v (Applicable to Tc only)					
 Flow length Two-year Land slop 	g's roughness coefficient, n (table 3-1) gth, L (total L + 300 ft) ar 24-hour rainfall, P ₂ pe, s me, $T_t = 0.007(nL)^{0.8}/((P_2^{0.5})(s^{0.4}))$		ft in ft/ft hr	0.4 300 3.8 0.017 0.85	51.00	min
Shallow co	oncentrated flow					
7. Flow leng 8. Watercou 9. Average	urse slope, s velocity, V (Figure 3-1)	Compute T _t	ft ft / ft ft/s hr	Unpaved 793 0.015 1.98 0.11	6.67	min
Channel flo	<u>ow</u>					
11. Cross s 12. Wetted 13. Hydraul 14. Channe 15. Mannin 16. V=1.49 17. Flow ler	sectional flow area, a perimeter, p_w lic radius, $r = a/p_w$ el slope, s ag's roughness coefficient, $n = r^{2/3} s^{1/2} / n$ ngth, L	Compute V Compute T_t	ft ² ft ft ft ft/ft ft/s ft hr	12.5 11 1.136 0.006 0.14 0.90 5154 1.59	95.69	min
19. Watersl	hed or subarea T_c or T_t	Total	hr	2.56	153.35	min

Appendix D

HY-8 Analysis Results

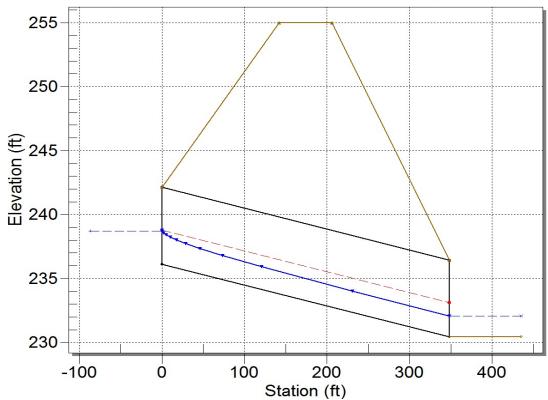
Culvert Crossing: 1168+32

Culvert Summary Table - EC-0102 (Inlet EX-0399)

	dirining y				,						
Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
148.71	148.71	238.73	2.61	0.0*	1-S2n	1.62	2.68	1.63	1.65	15.23	15.05
200.15	200.15	239.30	3.18	0.0*	1-S2n	2.01	3.26	2.01	2.03	16.57	16.43
251.59	251.59	239.82	3.70	0.0*	1-S2n	2.37	3.80	2.43	2.40	17.23	17.50
303.03	303.03	240.31	4.19	0.0*	1-S2n	2.72	4.30	2.73	2.75	18.52	18.38
354.47	354.47	240.77	4.65	0.0*	1-S2n	3.06	4.78	3.16	3.09	18.68	19.13
405.91	405.91	241.21	5.09	0.0*	1-S2n	3.39	5.23	3.52	3.42	19.24	19.77
457.35	457.35	241.63	5.51	0.0*	1-S2n	3.71	5.66	3.86	3.75	19.74	20.33
508.79	508.79	243.23	5.92	7.11	6-FFc	4.03	6.00	6.00	4.07	14.13	20.81
555.91	555.91	244.55	6.28	8.43	6-FFc	4.32	6.00	6.00	4.37	15.44	21.22
611.67	611.67	246.26	6.71	10.14	6-FFc	4.66	6.00	6.00	4.71	16.99	21.65
663.11	663.11	247.99	7.15	11.87	6-FFc	4.97	6.00	6.00	5.02	18.42	22.01

Water Surface Profile Plot

Crossing - 1168+32, Design Discharge - 555.9 cfs Culvert - EC-0102 (Inlet EX-0399), Culvert Discharge - 148.7 cfs

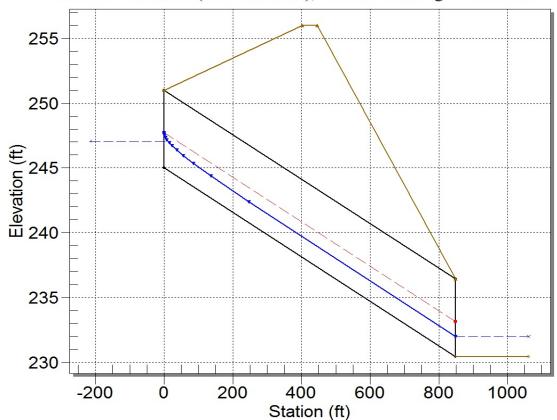


Culvert Crossing: 1172+32

Culvert Summary Table - EC-0211 (Inlet EX-0308)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
202.62	202.62	247.07	2.07	0.0*	1-S2n	1.55	2.72	1.56	1.57	16.27	16.13
265.07	265.07	247.48	2.48	0.0*	1-S2n	1.87	3.25	1.88	1.88	17.64	17.58
327.51	327.51	247.86	2.86	0.0*	1-S2n	2.17	3.74	2.17	2.18	18.83	18.76
389.96	389.96	248.21	3.21	0.0*	1-S2n	2.45	4.20	2.46	2.47	19.80	19.76
452.40	452.40	248.54	3.54	0.0*	1-S2n	2.72	4.64	2.73	2.74	20.71	20.62
514.85	514.85	248.86	3.86	0.0*	1-S2n	3.00	5.06	3.00	3.01	21.47	21.38
577.30	577.30	249.17	4.17	0.0*	1-S2n	3.25	5.46	3.28	3.27	21.97	22.05
639.74	639.74	249.46	4.46	0.0*	1-S2n	3.51	5.85	3.52	3.53	22.75	22.67
698.42	698.42	249.73	4.73	2.04	6-FFc	3.75	6.00	3.75	3.77	23.27	23.18
764.63	764.63	250.03	5.03	4.15	6-FFc	4.01	6.00	4.02	4.03	23.80	23.72
827.08	827.08	251.31	5.30	6.31	6-FFc	4.25	6.00	6.00	4.27	17.23	24.18

Crossing - 1172+32, Design Discharge - 698.4 cfs Culvert - EC-0211 (Inlet EX-0308), Culvert Discharge - 202.6 cfs

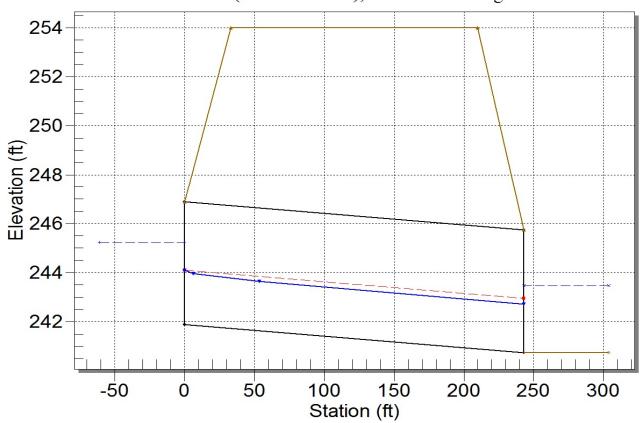


Culvert Crossing: 1272+60

Culvert Summary Table - EC-0906 (Inlet EX-0998)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
139.40	139.40	245.23	3.34	0.0*	1-S2n	1.98	2.21	1.98	2.73	9.37	1.38
171.64	171.64	245.77	3.88	0.0*	1-S2n	2.29	2.54	2.30	2.97	9.96	1.45
203.88	203.88	246.28	4.39	0.0*	1-S2n	2.59	2.85	2.60	3.19	10.47	1.52
236.13	236.13	246.79	4.90	0.0*	1-S2n	2.88	3.14	2.89	3.39	10.89	1.57
268.37	268.37	247.30	5.41	0.0*	5-S2n	3.17	3.42	3.17	3.57	11.29	1.62
300.61	300.61	247.82	5.93	0.0*	5-S2n	3.44	3.69	3.45	3.74	11.62	1.67
332.85	332.85	248.35	6.46	0.0*	5-S2n	3.71	3.95	3.72	3.89	11.94	1.72
365.09	365.09	248.91	7.02	0.0*	5-S2n	3.98	4.20	3.99	4.04	12.20	1.76
397.34	397.34	249.51	7.62	3.03	5-S2n	4.25	4.44	4.25	4.18	12.46	1.79
398.04	398.04	249.52	7.63	3.03	5-S2n	4.25	4.45	4.26	4.18	12.47	1.79
461.82	461.82	250.79	8.90	7.84	2-M2c	5.00	4.91	4.90	4.44	12.57	1.86

Crossing - 1272+60, Design Discharge - 398.0 cfs Culvert - EC-0906 (Inlet EX-0998), Culvert Discharge - 139.4 cfs

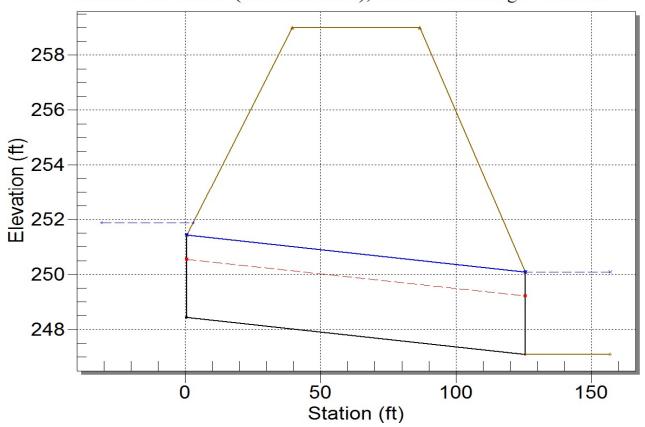


Culvert Crossing: 1279+53

Culvert Summary Table - EP-0908 (Inlet EX-0993)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
42.48	42.48	251.88	3.45	3.01	4-FFf	1.70	2.12	1.70	3.00	10.29	0.00
45.27	45.27	252.07	3.64	3.19	4-FFf	1.77	2.18	1.77	3.00	10.44	0.00
48.05	48.05	252.27	3.84	3.38	4-FFf	1.84	2.25	1.84	3.00	10.56	0.00
50.84	50.84	252.48	4.05	3.59	4-FFf	1.91	2.31	1.91	3.00	10.69	0.00
53.62	53.62	252.70	4.27	3.80	4-FFf	1.99	2.38	1.99	3.00	10.81	0.00
56.41	56.41	252.94	4.51	4.03	4-FFf	2.06	2.43	2.06	3.00	10.91	0.00
59.20	59.20	253.19	4.76	4.27	4-FFf	2.14	2.47	2.14	3.00	10.97	0.00
61.98	61.98	253.46	5.03	4.52	4-FFf	2.23	2.52	2.23	3.00	11.05	0.00
64.77	64.77	253.73	5.30	4.79	4-FFf	2.31	2.56	2.31	3.00	11.10	0.00
65.30	65.30	253.79	5.36	4.84	4-FFf	2.33	2.57	2.33	3.00	11.12	0.00
70.34	70.34	254.33	5.90	5.35	4-FFf	2.53	2.65	2.53	3.00	11.09	0.00

Crossing - 1279+53, Design Discharge - 65.3 cfs Culvert - EP-0908 (Inlet EX-0993), Culvert Discharge - 42.5 cfs

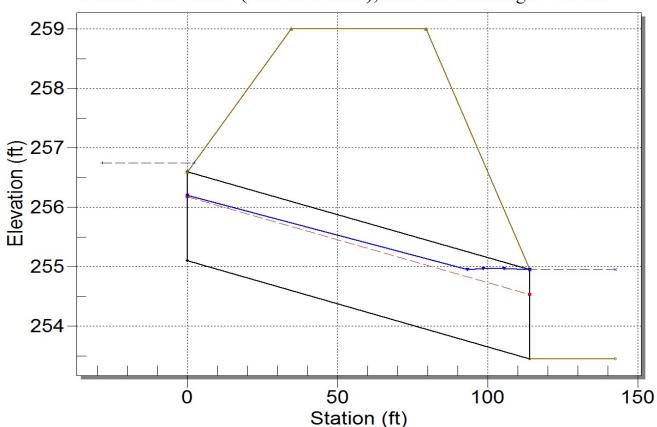


Culvert Crossing: 1342+08

Culvert Summary Table - EP-1303 (Inlet EX-1395)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
7.79	7.79	256.74	1.64	1.64	5-S1f	0.85	1.08	1.08	1.50	5.72	0.00
8.39	8.39	256.84	1.74	1.74	5-S1f	0.89	1.12	1.12	1.50	5.96	0.00
8.99	8.99	256.95	1.85	1.85	5-S1f	0.93	1.16	1.16	1.50	6.16	0.00
9.59	9.59	257.06	1.96	1.96	5-S1f	0.98	1.20	1.20	1.50	6.35	0.00
10.19	10.19	257.17	2.07	1.54	4-FFf	1.02	1.22	1.02	1.50	7.97	0.00
10.79	10.79	257.30	2.20	1.74	4-FFf	1.07	1.25	1.07	1.50	8.03	0.00
11.39	11.39	257.44	2.34	1.96	4-FFf	1.12	1.28	1.12	1.50	8.09	0.00
11.99	11.99	257.58	2.48	2.18	4-FFf	1.17	1.31	1.17	1.50	8.14	0.00
12.59	12.59	257.73	2.63	2.42	4-FFf	1.23	1.33	1.23	1.50	8.11	0.00
12.65	12.65	257.74	2.64	2.45	4-FFf	1.24	1.34	1.24	1.50	8.09	0.00
13.79	13.79	258.05	2.95	2.94	4-FFf	1.50	1.39	1.50	1.50	7.80	0.00

Crossing - 1342+08, Design Discharge - 12.7 cfs
Culvert - EP-1303 (Inlet EX-1395), Culvert Discharge - 7.8 cfs

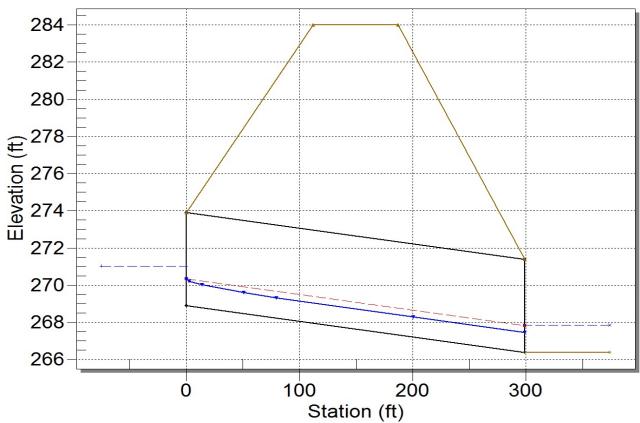


Culvert Crossing: 1360+01

Culvert Summary Table - EC-1407 (Inlet EX-1491)

	dirining y										
Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
58.59	58.59	271.03	2.13	0.0*	1-S2n	1.08	1.44	1.08	1.46	9.02	0.00
62.56	62.56	271.12	2.22	0.0*	1-S2n	1.12	1.50	1.12	1.46	9.28	0.00
66.52	66.52	271.22	2.32	0.0*	1-S2n	1.17	1.57	1.18	1.46	9.40	0.00
70.48	70.48	271.31	2.41	0.0*	1-S2n	1.22	1.63	1.22	1.46	9.65	0.00
74.45	74.45	271.40	2.50	0.0*	1-S2n	1.26	1.69	1.28	1.46	9.71	0.00
78.41	78.41	271.49	2.59	0.0*	1-S2n	1.31	1.75	1.32	1.46	9.93	0.00
82.38	82.38	271.58	2.68	0.0*	1-S2n	1.36	1.81	1.36	1.46	10.12	0.00
86.34	86.34	271.68	2.78	0.0*	1-S2n	1.40	1.86	1.40	1.46	10.25	0.00
90.31	90.31	271.77	2.87	0.0*	1-S2n	1.45	1.92	1.45	1.46	10.36	0.00
91.01	91.01	271.78	2.88	0.0*	1-S2n	1.46	1.93	1.46	1.46	10.39	0.00
98.24	98.24	271.94	3.04	0.0*	1-S2n	1.54	2.03	1.54	1.46	10.63	0.00

Crossing - 1360+01, Design Discharge - 91.0 cfs Culvert - EC-1407 (Inlet EX-1491), Culvert Discharge - 58.6 cfs

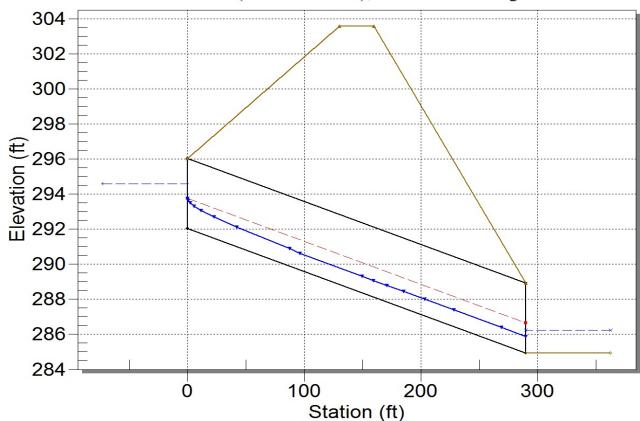


Culvert Crossing: 1369+17

Culvert Summary Table - EC-1503 (Inlet EX-1597)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
51.03	51.03	294.50	2.56	0.0*	1-S2n	0.95	1.72	0.95	1.30	13.37	0.00
54.52	54.52	294.62	2.68	0.0*	1-S2n	0.99	1.80	1.00	1.30	13.68	0.00
58.00	58.00	294.74	2.80	0.0*	1-S2n	1.03	1.87	1.04	1.30	13.91	0.00
61.49	61.49	294.86	2.92	0.0*	1-S2n	1.08	1.95	1.09	1.30	14.15	0.00
64.97	64.97	294.98	3.04	0.0*	1-S2n	1.12	2.02	1.13	1.30	14.38	0.00
68.46	68.46	295.10	3.16	0.0*	1-S2n	1.17	2.09	1.17	1.30	14.62	0.00
71.95	71.95	295.22	3.28	0.0*	1-S2n	1.21	2.16	1.21	1.30	14.85	0.00
75.43	75.43	295.34	3.40	0.0*	1-S2n	1.25	2.23	1.25	1.30	15.04	0.00
78.92	78.92	295.45	3.51	0.0*	1-S2n	1.29	2.30	1.30	1.30	15.19	0.00
79.51	79.51	295.47	3.53	0.0*	1-S2n	1.30	2.31	1.30	1.30	15.27	0.00
85.89	85.89	295.68	3.74	0.0*	1-S2n	1.37	2.43	1.38	1.30	15.61	0.00

Crossing - 1369+17, Design Discharge - 79.5 cfs Culvert - EC-1503 (Inlet EX-1597), Culvert Discharge - 51.0 cfs

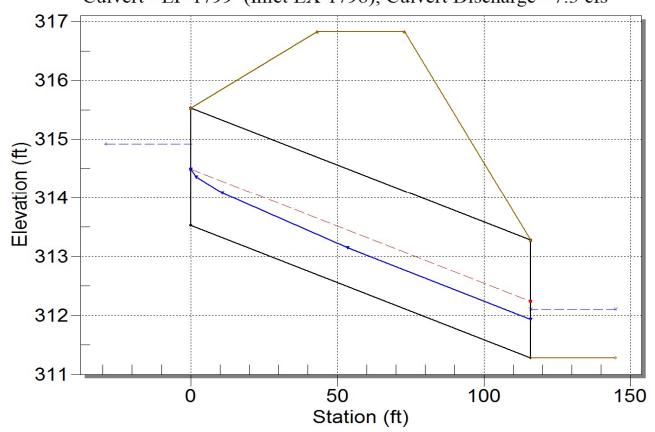


Culvert Crossing: 1395+12

Culvert Summary Table - EP-1799 (Inlet EX-1798)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
7.33	7.33	314.89	1.39	0.0*	1-S2n	0.65	0.96	0.66	0.83	8.12	2.15
7.85	7.85	314.95	1.45	0.0*	1-S2n	0.67	1.00	0.68	0.85	8.38	2.19
8.37	8.37	315.01	1.51	0.0*	1-S2n	0.70	1.03	0.70	0.87	8.47	2.22
8.89	8.89	315.07	1.57	0.0*	1-S2n	0.72	1.06	0.73	0.89	8.56	2.26
9.41	9.41	315.12	1.62	0.0*	1-S2n	0.75	1.09	0.75	0.91	8.72	2.29
9.93	9.93	315.18	1.68	0.0*	1-S2n	0.77	1.12	0.77	0.93	8.86	2.32
10.46	10.46	315.24	1.74	0.0*	1-S2n	0.79	1.16	0.79	0.94	8.99	2.35
10.98	10.98	315.29	1.79	0.0*	1-S2n	0.81	1.19	0.81	0.96	9.15	2.38
11.50	11.50	315.35	1.85	0.0*	1-S2n	0.83	1.22	0.83	0.98	9.28	2.41
11.57	11.57	315.35	1.85	0.0*	1-S2n	0.84	1.22	0.84	0.98	9.28	2.41
12.54	12.54	315.46	1.96	0.0*	1-S2n	0.87	1.27	0.88	1.01	9.44	2.46

Crossing - 1395+12, Design Discharge - 11.6 cfs Culvert - EP-1799 (Inlet EX-1798), Culvert Discharge - 7.3 cfs

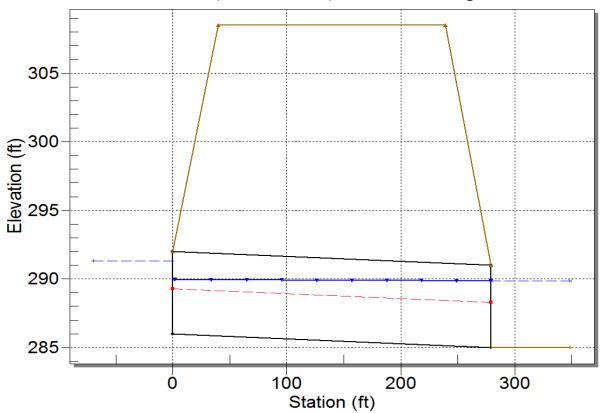


Culvert Crossing: 1407+01

Culvert Summary Table - EC-1702 (Inlet EX-1796)

	dirining y				,						
Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
312.74	312.74	288.79	2.59	2.79	1-S1t	1.69	1.74	3.00	3.00	4.34	4.34
374.90	374.90	289.16	2.93	3.16	1-S1t	1.91	1.97	3.30	3.30	4.73	4.56
437.06	437.06	289.51	3.26	3.51	1-S1t	2.12	2.18	3.57	3.57	5.10	4.76
499.23	499.23	289.84	3.58	3.84	1-S1t	2.34	2.38	3.82	3.82	5.45	4.94
561.39	561.39	290.16	3.89	4.16	1-S1t	2.54	2.58	4.05	4.05	5.77	5.10
623.55	623.55	290.46	4.19	4.46	1-S1t	2.74	2.76	4.27	4.27	6.08	5.25
685.71	685.71	290.76	4.49	4.76	1-S1t	2.93	2.94	4.48	4.48	6.38	5.39
747.87	747.87	291.04	4.78	5.04	3-M1t	3.12	3.12	4.67	4.67	6.67	5.52
810.04	810.04	291.31	5.06	5.31	3-M1t	3.31	3.29	4.86	4.86	6.95	5.64
812.81	812.81	291.32	5.07	5.32	3-M1t	3.32	3.30	4.87	4.87	6.96	5.64
934.36	934.36	291.84	5.62	5.84	3-M1t	3.68	3.62	5.21	5.21	7.47	5.85

Crossing - 1407+01, Design Discharge - 812.8 cfs Culvert - EC-1702 (Inlet EX-1796), Culvert Discharge - 812.8 cfs

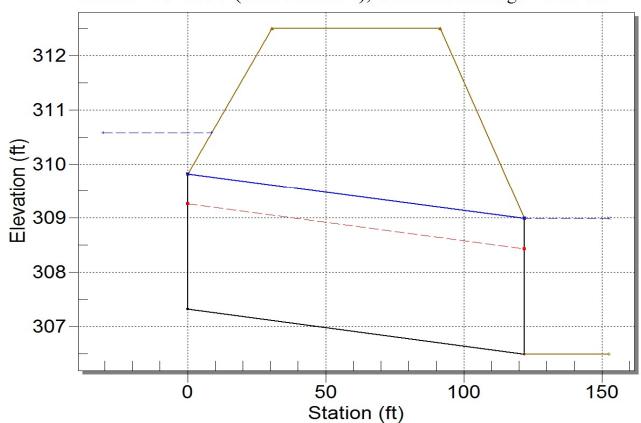


Culvert Crossing: 1424+12

Culvert Summary Table - EP-1804 (Inlet EX-1894)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
32.63	32.63	310.58	3.12	3.26	4-FFf	1.97	1.94	2.50	2.50	6.65	0.00
34.77	34.77	310.80	3.30	3.48	4-FFf	2.14	2.00	2.50	2.50	7.08	0.00
36.91	36.91	311.03	3.50	3.71	4-FFf	2.50	2.05	2.50	2.50	7.52	0.00
39.05	39.05	311.27	3.71	3.95	4-FFf	2.50	2.09	2.50	2.50	7.95	0.00
41.19	41.19	311.52	3.94	4.20	4-FFf	2.50	2.14	2.50	2.50	8.39	0.00
43.33	43.33	311.80	4.18	4.48	4-FFf	2.50	2.18	2.50	2.50	8.83	0.00
45.46	45.46	312.08	4.43	4.76	4-FFf	2.50	2.23	2.50	2.50	9.26	0.00
47.60	47.60	312.38	4.69	5.06	4-FFf	2.50	2.27	2.50	2.50	9.70	0.00
49.74	48.62	312.52	4.82	5.20	4-FFf	2.50	2.29	2.50	2.50	9.90	0.00
50.15	48.66	312.53	4.83	5.21	4-FFf	2.50	2.30	2.50	2.50	9.91	0.00
54.02	48.92	312.57	4.86	5.25	4-FFf	2.50	2.30	2.50	2.50	9.97	0.00

Crossing - 1424+12, Design Discharge - 50.1 cfs Culvert - EP-1804 (Inlet EX-1894), Culvert Discharge - 32.6 cfs

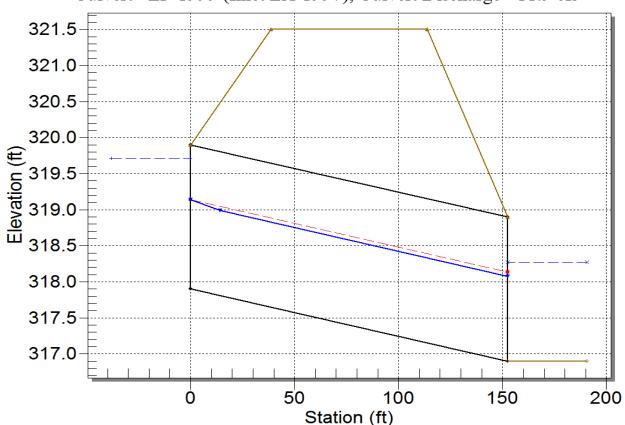


Culvert Crossing: 1434+60

Culvert Summary Table - EP-1999 (Inlet EX-1997)

					,						
Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
7.57	7.57	319.29	1.39	0.0*	1-S2n	0.89	0.98	0.90	1.15	5.56	1.14
8.11	8.11	319.35	1.45	0.0*	1-S2n	0.93	1.01	0.93	1.18	5.65	1.16
8.65	8.65	319.40	1.50	0.0*	1-S2n	0.96	1.05	0.97	1.21	5.76	1.18
9.18	9.18	319.45	1.55	0.0*	1-S2n	1.00	1.08	1.00	1.24	5.84	1.19
9.72	9.72	319.50	1.60	0.0*	1-S2n	1.03	1.11	1.03	1.27	5.93	1.21
10.26	10.26	319.55	1.65	0.0*	1-S2n	1.07	1.14	1.07	1.29	5.98	1.23
10.80	10.80	319.60	1.70	0.0*	1-S2n	1.10	1.18	1.11	1.32	6.07	1.24
11.34	11.34	319.65	1.75	0.0*	1-S2n	1.14	1.21	1.14	1.34	6.15	1.26
11.87	11.87	319.70	1.80	0.0*	1-S2n	1.17	1.23	1.17	1.37	6.20	1.27
11.95	11.95	319.71	1.81	0.0*	1-S2n	1.18	1.24	1.18	1.37	6.21	1.27
12.95	12.95	319.80	1.90	0.0*	1-S2n	1.24	1.29	1.24	1.41	6.32	1.30

Crossing - 1434+60, Design Discharge - 11.9 cfs Culvert - EP-1999 (Inlet EX-1997), Culvert Discharge - 11.9 cfs



Culvert Crossing: 1434+63

Culvert Summary Table - EP-1998 (Inlet EX-1996)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
22.65	22.65	320.28	2.38	0.53	1-S2n	1.56	1.61	1.56	1.43	7.02	3.17
24.14	24.14	320.38	2.48	0.56	1-S2n	1.63	1.67	1.64	1.46	7.10	3.22
25.64	25.64	320.49	2.59	0.60	5-S2n	1.70	1.72	1.70	1.50	7.21	3.27
27.13	27.13	320.66	2.69	2.76	2-M2c	1.78	1.77	1.78	1.53	7.28	3.31
28.63	28.63	320.76	2.80	2.86	2-M2c	1.86	1.82	1.82	1.56	7.46	3.36
30.12	30.12	320.85	2.92	2.95	2-M2c	1.94	1.86	1.87	1.59	7.65	3.40
31.61	31.61	320.95	3.03	3.05	2-M2c	2.04	1.91	1.92	1.62	7.83	3.44
33.11	33.11	321.07	3.16	3.17	2-M2c	2.18	1.95	1.96	1.65	8.03	3.48
34.60	34.60	321.19	3.29	3.28	2-M2c	2.50	2.00	2.00	1.68	8.22	3.52
34.88	34.88	321.23	3.32	3.33	2-M2c	2.50	2.00	2.01	1.68	8.26	3.53
37.59	36.98	321.51	3.51	3.61	7-M2c	2.50	2.05	2.06	1.73	8.55	3.60

Water Surface Profile Plot

Crossing - 1434+63, Design Discharge - 34.9 cfs Culvert - EP-1998 (Inlet EX-1996), Culvert Discharge - 22.6 cfs

321.5 321.0 320.0 319.5 319.0 318.5 318.0 317.5 317.0 0 50 100 150 Station (ft)

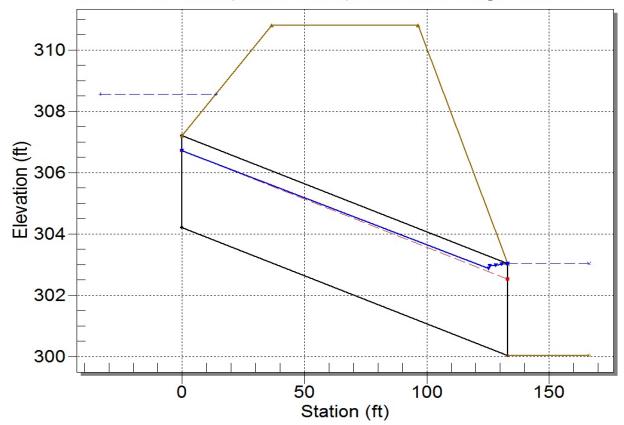
Culvert Crossing: 1458+57

Culvert Summary Table - EP-2101 (Inlet EX-2198)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
39.24	39.24	307.21	3.00	3.00	5-S1f	1.19	2.03	2.03	3.00	7.70	0.00
41.86	41.86	307.35	3.14	3.14	5-S1f	1.23	2.11	2.11	3.00	7.89	0.00
44.49	44.49	307.50	3.29	3.29	5-S1f	1.27	2.17	2.17	3.00	8.15	0.00
47.11	47.11	307.65	3.44	3.44	5-S1f	1.31	2.23	2.23	3.00	8.39	0.00
49.74	49.74	307.81	3.60	3.60	5-S1f	1.35	2.29	2.29	3.00	8.62	0.00
52.36	52.36	307.97	3.76	3.76	5-S1f	1.39	2.35	2.35	3.00	8.84	0.00
54.98	54.98	308.15	3.94	3.94	5-S1f	1.43	2.41	2.41	3.00	9.05	0.00
57.61	57.61	308.33	4.12	4.12	5-S1f	1.47	2.45	2.45	3.00	9.32	0.00
60.23	60.23	308.53	4.32	4.32	5-S1f	1.51	2.49	2.49	3.00	9.63	0.00
60.71	60.71	308.56	4.35	4.35	5-S1f	1.52	2.50	2.50	3.00	9.69	0.00
65.48	65.48	308.94	4.73	4.73	5-S1f	1.59	2.57	2.57	3.00	10.18	0.00

Water Surface Profile Plot

Crossing - 1458+57, Design Discharge - 60.7 cfs Culvert - EP-2101 (Inlet EX-2198), Culvert Discharge - 60.7 cfs

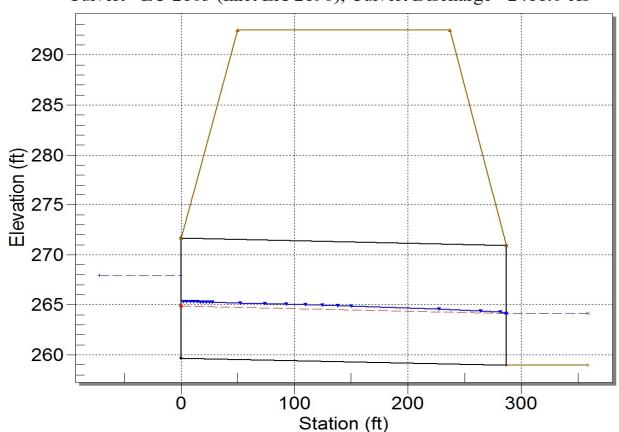


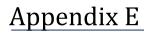
Culvert Crossing: 1469+54

Culvert Summary Table - EC-2103 (Inlet EX-2196)

Total Discharge (cfs)	Culvert Discharg e (cfs)	Head- water Elev. (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tail- water Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
2411.00	2411.00	267.93	7.85	8.27	3-M2t	5.73	5.19	5.20	5.20	12.88	9.16
2807.10	2807.10	268.82	8.75	9.16	2-M2c	6.43	5.75	5.74	5.64	13.59	9.59
3203.20	3203.20	269.66	9.62	10.00	2-M2c	7.11	6.28	6.26	6.05	14.21	9.96
3599.30	3599.30	270.48	10.47	10.82	2-M2c	7.78	6.79	6.77	6.43	14.77	10.30
3995.40	3995.40	271.26	11.31	11.60	2-M2c	8.44	7.27	7.26	6.80	15.29	10.61
4391.50	4391.50	272.02	12.15	12.36	2-M2c	9.09	7.75	7.73	7.14	15.78	10.90
4787.60	4787.60	272.76	12.99	13.10	2-M2c	9.74	8.21	8.19	7.47	16.24	11.17
5183.70	5183.70	273.50	13.84	13.82	2-M2c	10.37	8.65	8.63	7.78	16.68	11.42
5579.80	5579.80	274.37	14.71	14.50	2-M2c	12.00	9.09	9.07	8.08	17.09	11.66
5781.00	5781.00	274.82	15.16	14.87	2-M2c	12.00	9.31	9.28	8.23	17.30	11.77
6372.00	6372.00	276.19	16.53	15.87	2-M2c	12.00	9.93	9.91	8.64	17.87	12.10

Crossing - 1469+54, Design Discharge - 5781.0 cfs Culvert - EC-2103 (Inlet EX-2196), Culvert Discharge - 2411.0 cfs





GEOPak Drainage Area, Link & Node Tables

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - AREA (1 OF 3)

Area		Discharge		C	•	Node
ID	T _C	Discharge (efs)	Intensity		Area (20)	
	(min)	(cfs)	(in/hr)	Value	(ac)	Station
EX-0301	7	8.56	6.74	0.42	3.05	1170+37.50
EX-0302	15	16.81	5.57	0.64	4.69	1173+56.42
EX-0307	7	5.41	6.74	0.60	1.33	1181+03.11
EX-0402	9	8.83	6.41	0.45	3.07	1187+35.02
EX-0403	5	4.52	7.12	0.61	1.04	1187+66.04
EX-0404	7	4.15	6.74	0.55	1.12	1197+37.35
EX-0405	9	5.79	6.41	0.66	1.37	1197+98.53
EX-0406	6	6.27	6.92	0.58	1.55	1198+62.42
EX-0407	5	4.40	7.12	0.61	1.02	1192+84.72
EX-0501	12	2.86	5.96	0.32	1.50	1200+94.07
EX-0502	7	3.78	6.74	0.60	0.94	1201+11.72
EX-0504	5	0.83	7.12	0.30	0.39	1204+26.73
EX-0505	7	7.21	6.74	0.53	2.04	1204+39.03
EX-0506	18	6.06	5.23	0.39	2.99	1204+68.07
EX-0507	8	8.73	6.57	0.66	2.00	1205+05.31
EX-0508	5	0.55	7.12	0.39	0.20	1206+09.46
EX-0509	5	2.32	7.12	0.55	0.60	1205+89.05
EX-0601	5	3.65	7.12	0.66	0.78	1215+82.20
EX-0603	9	5.06	6.41	0.34	2.34	1215+61.01
EX-0604	5	0.77	7.12	0.35	0.31	1216+31.17
EX-0605	7	7.68	6.74	0.73	1.56	1223+44.71
EX-0606	5	1.29	7.12	0.30	0.61	1224+32.81
EX-0607	5	0.79	7.12	0.30	0.37	1225+12.54
EX-0608	5	1.76	7.12	0.30	0.83	1224+48.58
EX-0609	6	4.57	6.92	0.64	1.03	1229+50.07
EX-0610	5	0.19	7.12	0.32	0.09	1230+01.54
EX-0701	6	9.77	6.92	0.68	2.09	1235+98.59
EX-0702	8	9.36	6.57	0.66	2.17	1246+61.97
EX-0801	11	13.44	6.10	0.70	3.15	1252+00.86
EX-0802	9	9.47	6.41	0.43	3.41	1253+26.13
EX-0803	10	6.91	6.25	0.60	1.84	1261+46.66
EX-0901	10	8.02	6.25	0.65	1.97	1270+50.25
EX-0907	9	7.76	6.41	0.65	1.87	1278+84.95
EX-0993	75	51.23	2.41	0.60	35.67	1279+52.96
EX-1001	8	2.31	6.57	0.49	0.71	1286+58.13
EX-1002	5	3.14	7.12	0.60	0.74	1286+46.91
EX-1003	5	1.80	7.12	0.57	0.44	1286+34.09
EX-1004	50	31.03	3.16	0.53	18.49	1286+42.80
EX-1005	8	5.94	6.57	0.60	1.51	1290+03.73
EX-1007	12	5.25	5.96	0.46	1.94	1290+51.16
EX-1008	8	2.21	6.57	0.40	0.85	1289+59.80
EX-1009	8	2.94	6.57	0.44	1.02	1291+83.45

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - AREA (2 OF 3)

Δ	т	Discharge		STORIVI EVE	<u> </u>	•
Area	T _C	Discharge	Intensity	C	Area	Node
ID	(min)	(cfs)	(in/hr)	Value	(ac)	Station
EX-1101	7	5.24	6.74	0.60	1.30	1297+48.27
EX-1103	10	6.70	6.25	0.57	1.87	1297+50.24
EX-1107	12	8.45	5.96	0.36	3.93	1303+59.76
EX-1107A	5	1.35	7.12	0.41	0.46	1299+67.58
EX-1108	25	22.27	4.58	0.51	9.49	1304+38.27
EX-1192	8	6.42	6.57	0.41	2.41	1297+67.75
EX-1201	10	12.75	6.25	0.60	3.39	1320+48.58
EX-1202	11	5.92	6.10	0.62	1.56	1327+67.60
EX-1203	13	9.15	5.82	0.44	3.54	1319+99.01
EX-1204	13	8.30	5.82	0.44	3.23	1320+08.68
EX-1205	17	5.09	5.34	0.30	3.18	1319+68.98
EX-1301	12	9.96	5.96	0.65	2.56	1332+19.25
EX-1302	14	9.09	5.69	0.69	2.32	1341+91.59
EX-1395	12	10.99	6.02	0.57	3.19	1342+08.30
EX-1401	8	6.10	6.57	0.67	1.38	1350+03.10
EX-1402	17	5.33	5.34	0.42	2.36	1352+05.10
EX-1403	5	1.63	7.12	0.31	0.75	1355+02.43
EX-1404	7	6.78	6.74	0.63	1.60	1355+38.26
OP-1405	8	13.39	6.57	0.85	2.39	1357+22.79
EX-1406	10	5.96	6.25	0.54	1.77	1358+27.31
EX-1407	5	0.87	7.12	0.90	0.14	n/a
EX-1409	8	3.43	6.57	0.41	1.28	1353+84.32
EX-1410	10	3.69	6.25	0.31	1.93	1357+52.70
EX-1494	12	6.31	5.96	0.38	2.76	1358+84.23
EX-1497	10	9.25	6.25	0.34	4.34	1352+37.32
EX-1501	5	8.10	7.12	0.60	1.89	1362+46.47
EX-1502	6	2.32	6.92	0.41	0.81	1364+80.78
EX-1503	13	17.61	5.82	0.59	5.17	n/a
EX-1504	10	6.88	6.25	0.59	1.87	1367+70.48
EX-1505	7	6.46	6.74	0.60	1.59	1371+97.00
EX-1506	10	8.08	6.25	0.69	1.86	1371+99.99
EX-1507	5	1.27	7.12	0.66	0.27	1372+01.82
EX-1508	5	4.33	7.12	0.65	0.94	1372+40.91
EX-1509	40	25.19	3.61	0.40	17.60	1373+11.59
EX-1594	50	26.69	3.16	0.54	15.60	1369+36.36
EX-1601	7	3.21	6.74	0.66	0.72	1379+82.20
EX-1602	5	3.21	7.12	0.60	0.75	1379+84.09
EX-1603	9	3.77	6.41	0.64	0.93	1379+86.51
EX-1604	5	2.46	7.12	0.55	0.63	1379+87.52
EX-1605	5	3.64	7.12	0.57	0.90	1383+46.69
EX-1606	17	5.16	5.34	0.41	2.38	1383+51.50
EX-1607	8	5.46	6.57	0.61	1.37	1383+64.04

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - AREA (3 OF 3)

Area	T _C	Discharge	Intensity	C	Area	Node
ID	(min)	(cfs)	(in/hr)	Value	(ac)	Station
EX-1608	12	5.56	5.96	0.58	1.60	1383+78.82
EX-1609	5	2.63	7.12	0.61	0.61	1383+84.97
EX-1610	6	1.83	6.92	0.30	0.88	1387+77.60
EX-1698	5	1.40	7.12	0.50	0.39	1379+81.57
EX-1701	11	11.32	6.10	0.68	2.73	1399+90.93
EX-1801	14	17.53	5.69	0.68	4.50	1410+72.23
EX-1802	10	8.43	6.25	0.67	2.00	1416+20.50
EX-1803	15	23.05	5.57	0.67	6.21	1423+70.33
EX-1894	76	40.86	2.39	0.42	40.69	1424+12.27
EX-1897	22	8.51	4.84	0.30	5.91	1416+60.60
EX-2101	11	11.68	6.10	0.66	2.88	1459+00.30
EX-2102	12	16.80	5.96	0.67	4.22	1471+91.64
EX-2198	46	54.49	3.31	0.52	31.63	1458+56.65
EX-2201	9	9.69	6.41	0.64	2.38	1475+65.71
EX-2202	10	6.06	6.25	0.51	1.90	1476+68.11
EX-2203	20	19.54	5.02	0.57	6.86	1484+97.49
EX-2204	20	26.00	5.02	0.45	11.49	1485+95.89
EX-2301	6	2.61	6.92	0.60	0.63	1499+93.01
EX-2302	5	2.01	7.12	0.58	0.49	1500+90.21
EX-2303	14	6.28	5.69	0.36	3.10	1503+93.13
EX-2304	15	6.11	5.57	0.35	3.12	1503+88.57

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - LINK (1 OF 3)

				, ,,			3131E		INAGE IIV	117131110									•	, , , , , , , , , , , , , , , , , , ,			
Link	US	DS			# of	Dia-	Length	Slope	Discharge	Capacity	Uniform	Uniform	Soffit	HGL	Soffit	HGL	Invert	Invert	Actual	Actual	Actual	Actual	Node
ID	Node	Node	Shape	Material	Barrels	meter	(ft)	(%)	(cfs)	(cfs)	Depth	Velocity	US Elev.	US Elev.	DS Elev.	DS Elev.	US Elev.	DS Elev.	US Vel.	DS Vel.	US Depth	DS Depth	Station
וט	Noac	Noac			Darreis	(ft)	(10)	(70)	(613)	(013)	(ft)	(fps)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(fps)	(fps)	(ft)	(ft)	Station
EP-0301	EX-0301	OP-0398	Circ.	Conc.	1	1.5	118.96	2.25	8.56	18.37	0.75	9.67	250.53	250.97	247.85	247.11	249.03	246.35	4.84	9.57	1.50	0.76	1170+37.50
EP-0302	EX-0302	OP-0397	Circ.	Conc.	1	1.5	155.57	2.73	16.81	20.23	1.11	12.04	251.47	254.58	247.22	246.84	249.97	245.72	9.51	11.93	1.50	1.12	1173+56.42
EP-0303	EX-0306	OP-0394	Circ.	Conc.	1	4.0	321.78	0.92	128.46	160.67	2.86	13.36	256.62	257.62	253.66	252.54	252.62	249.66	10.22	13.27	4.00	2.88	1180+59.01
EP-0304	EX-0396	EX-0306	Circ.	Conc.	2	2.0	42.00	6.58	50.00	135.26	0.88	18.87	259.39	261.22	256.62	255.66	257.39	254.62	7.96	15.15	2.00	1.04	1180+43.59
EP-0305	EX-0307	EX-0306	Circ.	Conc.	1	1.5	128.08	3.71	5.41	23.56	0.51	10.27	261.37	261.24	256.62	255.63	259.87	255.12	3.20	10.22	1.37	0.51	1181+03.11
EP-0306	EX-0402	DN-0306	Circ.	Conc.	1	4.0	583.61	1.82	75.36	225.49	1.66	15.32	267.50	266.92	256.91	254.57	263.50	252.91	6.58	15.28	3.42	1.66	1187+35.02
EP-0306DS	DN-0306	EX-0306	Circ.	Conc.	1	4.0	95.19	0.30	75.36	91.69	2.93	7.63	256.91	257.94	256.62	257.62	252.91	252.62	6.00	6.00	4.00	4.00	1181+51.42
EP-0401	EX-0401	EX-0402	Circ.	Conc.	1	2.0	145.80	5.89	25.00	63.97	0.91	18.11	278.74	280.58	270.16	269.09	276.74	268.16	7.96	17.52	2.00	0.93	1187+81.00
EP-0402	EX-0408	EX-0402	Circ.	Conc.	1	3.5	515.78	6.10	40.73	289.47	0.92	20.18	300.00	298.61	268.56	265.98	296.50	265.06	6.74	20.15	2.11	0.92	1192+50.69
EP-0403	EX-0403	EX-0402	Circ.	Conc.	1	1.5	106.19	1.77	4.52	16.29	0.56	7.47	269.38	269.10	267.50	266.56	267.88	266.00	2.93	7.44	1.22	0.56	1187+66.04
EP-0404	EX-0502	EX-0404	Circ.	Conc.	1	3.5	374.43	3.42	26.39	216.78	0.86	14.50	312.92	311.37	300.12	297.48	309.42	296.62	4.79	14.46	1.95	0.86	1201+11.72
EP-0405	EX-0405	EX-0404	Circ.	Conc.	1	1.5	137.27	2.15	11.59	17.95	0.92	10.18	302.62	302.49	299.67	299.10	301.12	298.17	6.87	10.11	1.37	0.93	1197+98.53
EP-0405	EX-0403	EX-0404	Circ.	Conc.	1	1.5	137.74	1.82	6.27	16.52	0.92	8.24	305.19	305.20	302.68	301.85	303.69	301.18	3.55	8.22	1.50	0.93	1197+98.33
EP-0400	EX-0400	EX-0403 EX-0408	Circ.		1		117.74		4.40	6.71	0.07	3.85	300.92	300.62	302.08		299.42		2.90		1.20	0.81	1198+02.42
				Conc.	_	1.5		0.30								299.87		299.07		4.56			
EP-0408	EX-0404	EX-0408	Circ.	Conc.	1	3.5	486.76	0.30	38.57	64.22	2.05	6.57	301.46	300.39	300.00	298.61	297.96	296.50	5.42	6.38	2.43	2.11	1197+37.35
EP-0501	EX-0501	EX-0502	Circ.	Conc.	1	1.5	182.62	1.00	2.86	12.24	0.51	5.38	313.50	312.92	311.67	310.68	312.00	310.17	2.51	5.38	0.92	0.51	1200+94.07
EP-0503	EX-0505	EX-0503	Circ.	Conc.	1	3.5	228.01	1.29	21.27	133.17	0.98	9.62	317.63	315.91	314.69	312.18	314.13	311.19	4.32	9.56	1.78	0.99	1204+39.03
EP-0503DS	EX-0503	EX-0502	Circ.	Conc.	1	3.5	107.65	1.29	21.27	133.17	0.98	9.62	314.69	312.72	313.30	310.81	311.19	309.80	5.25	9.26	1.53	1.01	1202+11.51
EP-0504	EX-0504	EX-0505	Circ.	Conc.	1	3.0	35.24	1.00	0.83	77.73	0.23	3.43	317.70	315.91	317.35	315.91	314.70	314.35	0.31	0.22	1.21	1.56	1204+26.73
EP-0505	EX-0506	EX-0505	Circ.	Conc.	1	2.0	89.74	6.53	15.11	67.37	0.67	16.41	322.07	321.49	316.21	314.91	320.07	314.21	6.34	15.53	1.42	0.70	1204+68.07
EP-0506	EX-0507	EX-0506	Circ.	Conc.	1	1.5	108.90	4.23	11.37	25.19	0.74	13.16	326.18	326.32	321.57	320.82	324.68	320.07	6.43	12.84	1.50	0.75	1205+05.31
EP-0507	EX-0508	EX-0507	Circ.	Conc.	1	1.5	133.72	1.62	2.84	15.56	0.45	6.37	328.60	327.89	326.44	325.39	327.10	324.94	2.99	6.34	0.79	0.45	1206+09.46
EP-0508	EX-0509	EX-0508	Circ.	Conc.	1	1.5	70.98	0.30	2.32	6.66	0.64	3.23	328.94	328.25	328.73	327.89	327.44	327.23	2.39	3.07	0.81	0.66	1205+89.05
EP-0601	EX-0601	EX-0602	Circ.	Conc.	1	1.5	98.78	10.00	3.65	38.71	0.32	13.07	314.67	314.24	304.79	303.61	313.17	303.29	2.70	13.01	1.07	0.32	1215+82.20
EP-0602	EX-0602	EX-0604	Circ.	Conc.	1	1.5	204.75	0.82	3.65	11.06	0.62	5.34	304.58	304.01	302.91	302.03	303.08	301.41	3.18	5.34	0.93	0.62	1216+80.67
EP-0603	EX-0603	EX-0604	Circ.	Conc.	1	1.5	75.14	9.58	5.06	37.89	0.38	14.15	310.83	310.64	303.63	302.52	309.33	302.13	3.09	13.93	1.31	0.39	1215+61.01
EP-0604	EX-0604	OP-0611	Circ.	Conc.	1	2.0	354.13	2.18	9.01	38.95	0.68	9.56	303.37	302.79	295.64	294.32	301.37	293.64	3.78	9.56	1.42	0.68	1216+31.17
EP-0608	EX-0607	EX-0606	Circ.	Conc.	1	1.5	75.86	0.13	0.79	4.44	0.44	1.83	288.03	287.15	287.93	287.13	286.53	286.43	1.16	0.99	0.62	0.70	1225+12.54
EP-0610	EX-0606	EX-0608	Circ.	Conc.	1	1.5	175.67	2.98	2.05	21.12	0.33	7.19	287.93	287.13	282.70	281.53	286.43	281.20	2.55	7.19	0.70	0.33	1224+32.81
EP-0611	EX-0605	EX-0608	Circ.	Conc.	1	1.5	108.48	32.50	7.68	69.79	0.35	24.66	319.11	319.37	283.85	282.70	317.61	282.35	4.35	24.53	1.50	0.35	1223+44.71
EP-0612	EX-0608	OP-0683	Circ.	Conc.	1	1.5	78.99	2.60	11.31	19.72	0.85	10.91	282.55	283.02	280.50	279.88	281.05	279.00	6.40	10.50	1.50	0.88	1224+48.58
EP-0613	EX-0609	EX-0610	Circ.	Conc.	1	1.5	104.20	2.19	4.57	18.11	0.53	8.11	297.50	297.23	295.22	294.26		293.72	2.95	8.05	1.23	0.54	1229+50.07
EP-0614	EX-0610	OP-0680	Circ.	Conc.	1	1.5	70.96	2.40	4.73	18.95	0.53	8.46	295.22	294.57	293.52		293.72	292.02	4.60	8.27	0.85	0.54	1230+01.54
EP-0701	EX-0701	OP-0799	Circ.	Conc.	1	1.5	190.38	17.05	9.77	50.55	0.46	20.99	275.32	276.03	242.86	241.82	273.82	241.36	5.53	20.98	1.50	0.46	1235+98.59
EP-0702	EX-0702	OP-0798	Circ.	Conc.	1	1.5	161.54	9.77	9.36	38.26	0.53	16.98	241.55	242.17	225.77	224.80	240.05	224.27	5.30	16.91	1.50	0.53	1246+61.97
EP-0801	EX-0801	OP-0899	Circ.	Conc.	1	1.5	179.22	6.02	13.44	30.04	0.73	15.65	237.69	239.48	226.90	226.14		225.40	7.61	15.55	1.50	0.74	1252+00.86
EP-0802	EX-0802	OP-0898	Circ.	Conc.	1	2.0	242.12		9.47	33.20	0.76	8.66	231.38	231.03	227.54			225.54	3.42	8.63	1.65	0.76	1253+26.13
EP-0803	EX-0803	OP-0897	Circ.	Conc.	1	1.5	200.40	0.33	6.91	7.08	1.32	4.19	244.47	244.59	243.80		242.97	242.30	3.91	3.91	1.50	1.50	1261+46.66
EP-0901	EX-0803	OP-0897	Circ.	Conc.	1	1.5	167.35	1.00	8.02	12.24	0.93	6.97	248.09	248.42	246.42		246.59	244.92	4.54	6.97	1.50	0.93	1270+50.25
EP-0901	EX-0901	OP-0995	Circ.	Conc.	1	3.0	149.49		54.06	65.14	2.20	9.73	250.09	249.50	249.04	243.83		246.04	8.88	9.67	2.41	2.21	1278+84.95
EP-0907 EP-0908	EX-0907			_	1		129.21		51.23					252.53		248.23		247.09		10.93	3.00		
		EX-0907	Circ.	Conc.	1	3.0		1.04		79.15	1.85	11.21	251.43		250.09				7.25			1.89	1279+52.96
EP-1002	FX-1001	EX-1001S	Circ.	Conc.	1	2.5	304.96	0.45	42.25	31.92	2.50	8.83	257.86	260.86	256.50	257.48	255.36	254.00	8.61	8.61	2.50	2.50	1286+58.13

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - LINK (2 OF 3)

			1 4	,	11110 C		SISILI	71 010 (INAGE IN	110 151110									VIN (2 O	,			
Link	US	DS			# of	Dia-	Length	Slope	Discharge	Canacity	Uniform	Uniform	Soffit	HGL	Soffit	HGL	Invert	Invert	Actual	Actual	Actual	Actual	Node
			Shape	Material		meter		•	_		Depth	Velocity	US Elev.	US Elev.	DS Elev.	DS Elev.	US Elev.	DS Elev.	US Vel.	DS Vel.	US Depth	DS Depth	
ID	Node	Node			Barrels	(ft)	(ft)	(%)	(cfs)	(cfs)	(ft)	(fps)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(fps)	(fps)	(ft)	(ft)	Station
EP-1003	EX-1002	EX-1001	Circ.	Conc.	1	2.0	86.13	1.02	33.12	26.65	2.00	10.82	258.33	262.46	257.45	260.86	256.33	255.45	10.54	10.54	2.00	2.00	1286+46.91
EP-1004	EX-1003	EX-1002	Circ.	Conc.	1	2.0	87.25	0.67	31.79	21.50	2.00	10.38	258.91	263.94	258.33	262.46	256.91	256.33	10.12	10.12	2.00	2.00	1286+34.09
EP-1005	EX-1004	EX-1003	Circ.	Conc.	1	2.0	60.25	1.10	31.03	27.59	2.00	10.13	259.57	264.91	258.91	263.94	257.57	256.91	9.88	9.88	2.00	2.00	1286+42.80
EP-1006	EX-1008	EX-1001	Circ.	Conc.	1	2.0	301.74	1.00	15.12	26.36	1.14	8.20	260.45	262.20	257.43	260.86	258.45	255.43	4.81	4.81	2.00	2.00	1289+59.80
EP-1007	EX-1007	EX-1005	Circ.	Conc.	1	1.5	110.80	0.92	5.25	11.75	0.73	6.13	263.44	263.32	262.42	263.10	261.94	260.92	3.09	2.97	1.38	1.50	1290+51.16
EP-1008	EX-1007	EX-1008	Circ.	Conc.	1	1.5	102.13	0.95	10.56	11.75	1.17	7.16	262.42	263.10	261.45	262.20	260.92	259.95	5.98	5.98	1.50	1.50	1290+03.73
EP-1010	EX-1003	OP-1010	Circ.	Conc.	1	2.5	187.06	0.40	42.25	30.27	2.50	8.83	256.50	257.48	255.75	255.75	254.00	253.25	8.61	8.61	2.50	2.50	1283+69.91
EP-1012	EX-1009	EX-1008	Circ.	Conc.	1	1.5	223.65	1.00	2.94	12.24	0.52	5.41	263.68	263.12	261.45	260.47	262.18	259.95	2.53	5.40	0.94	0.52	1291+83.45
EP-1101	EX-1103	EX-1101	Circ.	Conc.	1	1.5	95.87	0.34	6.70	7.18	1.23	4.31	269.81	269.89	269.48	268.98	268.31	267.98	3.79	5.34	1.50	1.00	1297+50.24
EP-1102	EX-1101	EX-1101	Circ.	Conc.	1	2.0	91.95	1.13	11.45	28.04	0.93	8.03	269.42	268.66	268.38	267.32	267.42	266.38	5.60	7.89	1.24	0.94	1297+48.27
EP-1104	EX-1101	EX-1102	Circ.	Conc.	1	2.0	110.79	0.30	11.45	14.44	1.41	4.85	268.38	268.58	268.05	268.20	266.38	266.05	3.65	3.65	2.00	2.00	1297+45.42
EP-1104	EX-1102	EX-1105	Circ.	Conc.	1	2.0	67.25	2.17	11.45	38.81	0.77	10.21	268.05	268.20	266.59	268.00	266.05	264.59	3.65	3.65	2.00	2.00	1298+55.47
EP-1105 EP-1106	EX-1105 EX-1106	EX-1106 EX-1107A	Circ.		1	2.0	55.80	1.60	17.22	33.29	1.07	10.21	266.56	268.00	265.67	267.37	264.56	263.67	5.48	5.48	2.00	2.00	1299+13.66
				Conc.	_															1			
EP-1107	EX-1192	EX-1106	Circ.	Conc.	1	1.5	209.98	3.47	6.42	22.79	0.57	10.51	276.03	276.07	268.75	267.82	274.53	267.25	3.63	10.50	1.50	0.57	1297+67.75
EP-1110	EX-1107A	EX-1107	Circ.	Conc.	1	2.5	392.30	0.56	18.34	35.88	1.32	6.97	265.67	267.37	263.46	266.65	263.17	260.96	3.74	3.74	2.50	2.50	1299+67.58
EP-1111	EX-1108	EX-1107	Circ.	Conc.	1	2.0	262.23	4.70	22.27	57.15	0.90	16.17	275.78	277.10	263.46	262.37	273.78	261.46	7.09	16.11	2.00	0.91	1304+38.27
EP-1114	EX-1107	OP-1196	Circ.	Conc.	1	2.0	313.98	2.58	42.35	42.35	1.88	13.83	262.96	266.65	254.86	254.86	260.96	252.86	13.48	13.48	2.00	2.00	1303+59.76
EP-1201	EX-1204	EX-1201	Circ.	Conc.	1	2.0	102.75	0.91	12.62	25.12	1.05	7.57	255.63	254.96	254.70	253.76	253.63	252.70	5.70	7.51	1.33	1.06	1320+08.68
EP-1202	EX-1203	EX-1201	Circ.	Conc.	1	1.5	113.29	1.00	9.15	12.24	1.02	7.17	255.10	255.67	253.97	254.91	253.60	252.47	5.18	5.18	1.50	1.50	1319+99.01
EP-1203	EX-1201	EX-1202	Circ.	Conc.	1	3.0	719.07	0.90	31.63	73.73	1.43	9.52	255.40	254.91	248.93	247.36	252.40	245.93	5.01	9.50	2.51	1.43	1320+48.58
EP-1204	EX-1202	OP-1399	Circ.	Conc.	1	3.0	90.21	21.85	35.83	363.33	0.66	31.08	248.59	247.59	228.88	226.58	245.59	225.88	7.16	28.46	2.00	0.70	1327+67.60
EP-1205	EX-1205	EX-1204	Circ.	Conc.	1	2.0	104.39	0.91	5.09	25.11	0.63	5.95	256.58	255.71	255.63	254.27	254.58	253.63	2.80	5.93	1.13	0.64	1319+68.98
EP-1301	EX-1301	OP-1396	Circ.	Conc.	1	2.0	169.84	12.81	9.96	94.36	0.46	18.53	252.24	251.94	230.48	228.94	250.24	228.48	3.50	18.46	1.70	0.46	1332+19.25
EP-1302	EX-1302	OP-1394	Circ.	Conc.	1	2.0	119.17	0.82	19.49	23.91	1.45	7.98	255.42	255.02	254.44	253.89	253.42	252.44	7.26	7.97	1.60	1.45	1341+91.59
EP-1303	EX-1395	EX-1302	Circ.	Conc.	1	1.5	117.38	1.41	10.99	14.51	1.03	8.51	256.60	257.63	254.95	254.48	255.10	253.45	6.22	8.46	1.50	1.03	1342+08.30
EP-1401	EX-1401	OP-1499	Circ.	Conc.	1	1.5	264.12	0.59	10.09	9.41	1.50	5.86	270.28	270.81	268.72	268.72	268.78	267.22	5.71	5.71	1.50	1.50	1350+03.10
EP-1402	EX-1402	EX-1401	Circ.	Conc.	1	1.5	227.15	0.33	5.33	6.99	1.03	4.10	271.10	271.31	270.36	270.81	269.60	268.86	3.02	3.02	1.50	1.50	1352+05.10
EP-1403	EX-1409	EX-1403	Circ.	Conc.	1	2.5	130.79	1.10	12.32	50.16	0.88	8.04	273.14	272.17	271.70	270.08	270.64	269.20	3.93	7.95	1.53	0.88	1353+84.32
EP-1404	EX-1403	EX-1404	Circ.	Conc.	1	2.5	103.71	0.78	13.65	42.24	1.02	7.27	271.38	270.47	270.57	269.09	268.88	268.07	4.13	7.21	1.59	1.02	1355+02.43
EP-1405	EX-1404	OP-1493	Circ.	Conc.	1	3.0	325.67	0.84	28.32	71.16	1.38	8.96	270.80	270.11	268.07	266.45	267.80	265.07	4.85	8.96	2.31	1.38	1355+38.26
EP-1406	EX-1410	EX-1404	Circ.	Conc.	1	2.0	363.89	0.94	9.75	25.52	0.90	7.16	276.08	275.47	272.67	271.57	274.08	270.67	4.19	7.16	1.39	0.90	1357+52.70
EP-1408	EX-1408	OP-1405	Circ.	Conc.	1	1.5	182.70	0.44	5.96	8.14	1.00	4.75	274.60	274.20	273.79	273.79	273.10	272.29	4.27	3.37	1.11	1.50	1357+86.96
EP-1409	EX-1406	EX-1408	Circ.	Conc.	1	1.5	210.72	0.44	5.96	8.14	1.00	4.75	275.53	275.49	274.60	274.20	274.03	273.10	3.40	4.27	1.46	1.11	1358+27.31
EP-1496	EX-1494	EX-1410	Circ.	Conc.	1	1.5	131.99	0.80	6.31	10.91	0.86	6.04	277.13	277.15	276.08	275.44	275.63	274.58	3.57	6.04	1.50	0.86	1358+84.23
EP-1498	EX-1497	EX-1409	Circ.	Conc.	1	1.5	277.13	1.00	9.25	12.24	1.03	7.17	275.34	275.93	272.57			271.07	5.24	7.17	1.50	1.03	1352+37.32
EP-1501	EX-1501	OP-1599	Circ.	Conc.	1	1.5	137.55	4.65	8.10	26.41	0.59	12.45	283.52	283.87	277.12		282.02	275.62	4.58	12.36	1.50	0.60	1362+46.47
EP-1502	EX-1502	OP-1595		Conc.	1	2.5	175.01	2.14	62.36	69.88	1.96	15.09	281.78	283.22	278.04			275.54	12.70	14.74	2.50	2.01	1364+80.78
EP-1504	EX-1504	EX-1502	Circ.	Conc.	1	2.5	_	1.59	61.59	60.19	2.50	12.87	286.46	290.05	281.78			279.28	12.55	12.55	2.50	2.50	1367+70.48
EP-1505	EX-1507	EX-1504	Circ.	Conc.	1	2.0	439.17	1.62	36.06	33.59	2.00	11.78	294.48	301.05	287.35			285.35	11.48	11.48	2.00	2.00	1372+01.82
EP-1506	EX-1505	EX-1506	Circ.	Conc.	1	1.5	136.88	0.17	6.46	5.02	1.50	3.75	295.53	302.33	295.30		294.03	293.80	3.66	3.66	1.50	1.50	1371+97.00
EP-1507	EX-1506		Circ.	Conc.	1	1.5	52.38	2.27	14.07	18.45	1.03	10.83	295.22	301.89	294.03	301.05		292.53	7.96	7.96	1.50	1.50	1371+99.99
EP-1508	EX-1508		Circ.	Conc.	1	2.0	54.17	2.23	27.35	39.40	1.29	12.80	297.39	301.76				294.18	8.70	8.70	2.00	2.00	1372+40.91
LI 1300	LV 1300	LN 1307	CII C.	COIIC.	T	۷.0	J7.11	۷.۷	27.33	33.40	1.43	12.00	231.33	301.70	250.10	301.03	233.33	234.10	0.70	0.70	2.00	2.00	13/2:40.31

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - LINK (3 OF 3)

Link	US	DS			# of	Dia-	Length	Slope	Discharge	Capacity	Uniform	Uniform	Soffit	HGL	Soffit	HGL	Invert	Invert	Actual	Actual	Actual	Actual	Node
ID	Node	Node	Shape	Material	Barrels	meter			(cfs)	(cfs)	Depth	Velocity	US Elev.	US Elev.	DS Elev.	DS Elev.	US Elev.	DS Elev.	US Vel.	DS Vel.	US Depth	DS Depth	Station
ID	Noue	Noue			Darreis	(ft)	(ft)	(%)	(CI3)	(CIS)	(ft)	(fps)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(fps)	(fps)	(ft)	(ft)	Station
EP-1509	EX-1509	EX-1508	Circ.	Conc.	1	2.0	94.16	3.68	25.19	50.54	1.04	15.20	300.85	302.76	297.39	301.76	298.85	295.39	8.02	8.02	2.00	2.00	1373+11.59
EP-1598	EX-1594	EX-1504	Circ.	Conc.	1	2.5	220.51	1.70	26.69	62.33	1.19	11.56	291.60	291.97	287.85	286.55	289.10	285.35	5.44	11.45	2.50	1.20	1369+36.36
EP-1601	EX-1698	EX-1601	Circ.	Conc.	1	1.5	41.26	5.11	1.40	27.68	0.24	7.78	312.58	311.67	310.47	309.21	311.08	308.97	2.20	7.67	0.59	0.24	1379+81.57
EP-1602	EX-1601	EX-1602	Circ.	Conc.	1	1.5	107.88	0.30	4.54	6.71	0.95	3.86	310.27	310.10	309.95	309.95	308.77	308.45	2.74	2.57	1.33	1.50	1379+82.20
EP-1603	EX-1602	EX-1603	Circ.	Conc.	1	1.5	123.98	0.15	7.48	4.66	1.50	4.35	309.85	309.95	309.67	309.23	308.35	308.17	4.24	5.61	1.50	1.06	1379+84.09
EP-1604	EX-1603	EX-1604	Circ.	Conc.	1	2.0	54.92	0.87	10.97	24.65	0.98	7.21	310.03	309.23	309.55	308.54	308.03	307.55	5.58	7.04	1.20	0.99	1379+86.51
EP-1605	EX-1604	OP-1699	Circ.	Conc.	1	3.0	118.99	10.00	30.41	245.79	0.74	22.47	289.94	289.27	278.04	275.81	286.94	275.04	5.16	21.28	2.33	0.77	1379+87.52
EP-1606	EX-1609	EX-1604	Circ.	Conc.	1	2.5	397.46	0.30	20.35	26.18	1.76	5.51	313.05	312.64	311.86	310.89	310.55	309.36	4.64	6.45	2.09	1.53	1383+84.97
EP-1607	EX-1605	EX-1606	Circ.	Conc.	1	1.5	42.62	0.30	3.64	6.71	0.83	3.66	315.04	315.14	314.91	315.10	313.54	313.41	2.06	2.06	1.50	1.50	1383+46.69
EP-1608	EX-1606	EX-1607	Circ.	Conc.	1	1.5	107.50	0.55	7.89	9.07	1.14	5.46	314.94	315.10	314.35	314.57	313.44	312.85	4.47	4.47	1.50	1.50	1383+51.50
EP-1609	EX-1607	EX-1608	Circ.	Conc.	1	1.5	123.87	0.72	12.24	10.38	1.50	7.11	314.11	314.57	313.22	313.04	312.61	311.72	6.93	7.42	1.50	1.32	1383+64.04
EP-1610	EX-1608	EX-1609	Circ.	Conc.	1	2.0	57.05	0.30	17.09	14.44	2.00	5.58	313.22	313.03	313.05	312.64	311.22	311.05	5.72	6.38	1.81	1.59	1383+78.82
EP-1611	EX-1610	EX-1609	Circ.	Conc.	1	1.5	392.88	1.24	1.83	13.62	0.39	5.10	324.13	323.32	319.27	318.16	322.63	317.77	2.28	5.10	0.69	0.39	1387+77.60
EP-1701	EX-1701	OP-1797	Circ.	Conc.	1	1.5	170.91	6.59	11.32	31.42	0.65	15.48	310.16	311.28	298.90	298.05	308.66	297.40	6.41	15.39	1.50	0.65	1399+90.93
EP-1801	EX-1802	OP-1896	Circ.	Conc.	1	2.0	177.77	2.85	14.98	44.52	0.83	12.11	302.39	301.84	297.32	296.16	300.39	295.32	6.16	12.00	1.45	0.84	1416+20.50
EP-1802	EX-1807	EX-1802	Circ.	Conc.	1	2.0	115.83	2.58	8.51	42.38	0.63	10.01	305.46	304.77	302.47	301.11	303.46	300.47	3.90	9.86	1.31	0.64	1416+52.08
EP-1803	EX-1803	OP-1895	Circ.	Conc.	1	2.5	156.84	0.69	50.64	39.67	2.50	10.58	308.92	309.89	307.84	307.84	306.42	305.34	10.32	10.32	2.50	2.50	1423+70.33
EP-1804	EX-1894	EX-1803	Circ.	Conc.	1	2.5	125.52	0.66	40.86	38.87	2.50	8.54	309.82	311.73	308.99	309.89	307.32	306.49	8.32	8.32	2.50	2.50	1424+12.27
EP-1806	EX-1801	OP-1899	Circ.	Conc.	1	2.0	131.81	2.09	17.53	38.08	0.99	11.25	302.00	302.58	299.25	298.26	300.00	297.25	5.58	10.99	2.00	1.01	1410+72.23
EP-1807	EX-1897	EX-1807	Circ.	Conc.	1	2.0	8.39	2.59	8.51	42.40	0.63	10.01	305.68	305.22	305.46	304.27	303.68	303.46	3.27	7.14	1.54	0.81	1416+60.60
EP-2101	EX-2198	EX-2101	Circ.	Conc.	1	3.0	136.61	3.07	54.49	136.11	1.38	17.25	307.21	308.57	303.02	301.49	304.21	300.02	7.71	15.88	3.00	1.47	1458+56.65
EP-2102	EX-2101	OP-2199	Circ.	Conc.	1	3.0	169.92	1.19	60.73	84.77	1.98	12.27	303.02	302.57	301.00	300.02	300.02	298.00	9.48	12.03	2.55	2.02	1459+00.30
EP-2104	EX-2102	OP-2195	Circ.	Conc.	1	1.5	202.74	6.98	16.80	32.34	0.80	17.48	283.94	287.04	269.79	269.10	282.44	268.29	9.51	17.39	1.50	0.81	1471+91.64
EP-2201	EX-2201	OP-2299	Circ.	Conc.	1	2.0	154.33	1.35	15.39	30.68	1.05	9.26	288.06	287.53	285.97	285.02	286.06	283.97	6.22	9.17	1.47	1.05	1475+65.71
EP-2202	EX-2202	EX-2201	Circ.	Conc.	1	2.0	153.69	1.41	6.06	31.33	0.62	7.32	290.14	289.40	287.97	286.59	288.14	285.97	2.92	7.31	1.26	0.62	1476+68.11
EP-2203	EX-2203	OP-2298	Circ.	Conc.	1	3.0	150.53	0.59	45.29	59.43	2.07	8.71	304.66	304.18	303.78	303.78	301.66	300.78	7.14	6.41	2.52	3.00	1484+97.49
EP-2204	EX-2204	EX-2203	Circ.	Conc.	1	3.0	148.46	0.89	26.00	73.29	1.29	8.97	306.01	305.48	304.69	302.99	303.01	301.69	4.18	8.87	2.47	1.30	1485+95.89
EP-2301	EX-2302	OP-2399	Circ.	Conc.	1	2.0	81.18	1.08	7.54	27.45	0.74	7.08	335.63	334.79	334.75	333.50	333.63	332.75	3.99	6.97	1.16	0.75	1500+90.21
EP-2302	EX-2304	EX-2302	Circ.	Conc.	1	1.5	300.45	0.78	6.11	10.78	0.85	5.94	337.34	337.32	335.01	334.36	335.84	333.51	3.47	5.94	1.48	0.85	1503+88.57
EP-2303	EX-2303	OP-2397	Circ.	Conc.	1	1.5	147.18	1.55	6.28	15.24	0.70	7.79	337.00	337.01	334.72	333.92	335.50	333.22	3.55	7.75	1.50	0.70	1503+93.13
EP-2304	EX-2301	OP-2398	Circ.	Conc.	1	1.5	87.03	0.37	2.61	7.42	0.64	3.65	334.82	334.53	334.50	334.50	333.32	333.00	1.71	1.48	1.21	1.50	1499+93.01

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - NODE (1 OF 4)

		Library								Cumulative	Cumulative	Cumulative	Cumulative	Cumulative
Node	Type	Library Item	Reference	Station	Offset	Elevation	Depth	Junction	T _C	T _C	Discharge	Area	Cumulative	Intensity
ID	Type	Name	Chain	Station	(ft)	(ft)	(ft)	Loss	Used	(min)	(cfs)	(ac)	Value	(in/hr)
EV 0204			1775)	4470 07.50	427.40	254.00	2.06	0.00	7.00	-				
EX-0301	Inlet	Ex. D.I.	I77EX	1170+37.50	127.18	251.99	2.96	0.00	7.00	7.00	8.56	3.05	0.42	6.74
EX-0302	Inlet	Ex. D.I.	I77EX	1173+56.42	0.04	253.45	3.48	0.00	15.00	15.00	16.81	4.69	0.64	5.57
DN-0306	Inlet	Dummy Joint	I77EX	1181+51.42	-97.86	257.91	5.00	0.10	20.76	21.40	75.36	19.81	0.51	4.95
EX-0306	Inlet	Ex. D.I.	I77EX	1180+59.01	-120.68	257.91	5.29	1.60	21.61	21.61	128.46	21.14	0.52	4.87
EX-0307	Inlet	Ex. D.I.	I77EX	1181+03.11	-0.43	262.37	2.50	0.00	7.00	7.00	5.41	1.33	0.60	6.74
OP-0394	Outlet	Ex. Outlet	I77EX	1177+37.24	-118.33	249.66	0.00	0.00	0.00	22.01	0.00	21.14	0.52	0.00
EX-0396	Inlet	Ex. D.I.	I77EX	1180+43.59	-159.75	260.39	3.00	0.00	0.00	0.00	50.00	0.00	0.00	0.00
OP-0397	Outlet	Ex. Outlet	I77EX	1173+06.85	-147.42	245.72	0.00	0.00	0.00	15.22	0.00	4.69	0.64	0.00
OP-0398	Outlet	Ex. Outlet	I77EX	1171+32.32	199.03	246.35	0.00	0.00	0.00	7.21	0.00	3.05	0.42	0.00
EX-0401	Inlet	Ex. D.I.	I77EX	1187+81.00	-239.87	279.74	3.00	0.00	0.00	0.00	25.00	0.00	0.00	0.00
EX-0402	Inlet	Ex. D.I.	177EX	1187+35.02	-101.51	271.16	7.66	0.80	20.76	20.76	75.36	19.81	0.51	4.95
EX-0403	Inlet	Ex. D.I.	177EX	1187+66.04	0.06	272.82	4.94	0.00	5.00	5.00	4.52	1.04	0.61	7.12
EX-0404	Inlet	Ex. D.I.	177EX	1197+37.35	-122.02	301.92	5.30	0.37	19.10	19.10	38.57	14.68	0.51	5.11
EX-0405	Inlet	Ex. D.I.	177EX	1197+98.53	0.86	305.78	4.66	0.07	9.00	9.00	11.59	2.92	0.62	6.41
EX-0406	Inlet	Ex. D.I.	177EX	1198+62.42	122.89	306.52	2.83	0.00	6.00	6.00	6.27	1.55	0.58	6.92
EX-0407	Inlet	Ex. D.I.	177EX	1192+84.72	0.20	301.92	2.50	0.00	5.00	5.00	4.40	1.02	0.61	7.12
EX-0408	Inlet	Ex. D.I.	177EX	1192+50.69	-112.51	301.92	5.42	0.12	20.34	20.34	40.73	15.70	0.52	4.99
EX-0501	Inlet	Ex. D.I.	177EX	1200+94.07	-310.57	317.24	5.24	0.00	12.00	12.00	2.86	1.50	0.32	5.96
EX-0502	Inlet	Ex. D.I.	177EX	1201+11.72	-128.80	315.01	5.59	0.37	18.67	18.67	26.39	10.64	0.48	5.16
EX-0503	Junction	Ex. M.H.	177EX	1202+11.51	-169.16	322.27	11.08	0.12	18.09	18.49	21.27	8.21	0.50	5.22
EX-0504	Inlet	Ex. D.I.	177EX	1204+26.73	-217.15	323.45	8.75	0.00	5.00	5.00	0.83	0.39	0.30	7.12
EX-0505	Inlet	Ex. D.I.	177EX	1204+39.03	-184.12	322.21	8.08	0.37	18.09	18.09	21.27	8.21	0.50	5.22
EX-0506	Inlet	Ex. D.I.	177EX	1204+68.07	-99.21	325.62	5.55	0.02	18.00	18.00	15.11	5.78	0.50	5.23
EX-0507	Inlet	Ex. D.I.	I77EX	1205+05.31	3.13	328.32	3.64	0.35	8.00	8.00	11.37	2.79	0.62	6.57
EX-0508	Inlet	Ex. D.I.	177EX	1206+09.46	86.99	333.96	6.86	0.15	5.37	5.37	2.84	0.79	0.51	7.05
EX-0509	Inlet	Ex. D.I.	I77EX	1205+89.05	154.97	329.74	2.30	0.00	5.00	5.00	2.32	0.60	0.55	7.12
EX-0601	Inlet	Ex. D.I.	I77EX	1215+82.20	6.78	335.84	22.67	0.00	5.00	5.00	3.65	0.78	0.66	7.12
EX-0602	Junction	Ex. M.H.	I77EX	1216+80.67	0.89	307.44	4.36	0.20	5.00	5.13	3.65	0.78	0.66	7.12
EX-0603	Inlet	Ex. D.I.	I77EX	1215+61.01	201.62	312.20	2.87	0.00	9.00	9.00	5.06	2.34	0.34	6.41
EX-0604	Inlet	Ex. D.I.	I77EX	1216+31.17	199.13	305.62	4.25	0.35	9.09	9.09	9.01	3.42	0.41	6.39
EX-0605	Inlet	Ex. D.I.	I77EX	1223+44.71	7.63	322.16	4.55	0.00	7.00	7.00	7.68	1.56	0.73	6.74
EX-0606	Inlet	Ex. D.I.	177EX	1224+32.81	-138.78	290.73	4.30	0.16	5.69	5.69	2.05	0.98	0.30	6.98
EX-0607	Inlet	Ex. D.I.	177EX	1225+12.54	-140.25	294.35	7.82	0.00	5.00	5.00	0.79	0.37	0.30	7.12
EX-0608	Inlet	Ex. D.I.	177EX	1224+48.58	36.20	287.65	6.60	0.69	7.07	7.07	11.31	3.37	0.50	6.73
EX-0609	Inlet	Ex. D.I.	177EX	1229+50.07	11.29	300.19	4.19	0.00	6.00	6.00	4.57	1.03	0.64	6.92
EX-0610	Inlet	Ex. D.I.	I77EX	1230+01.54	101.30	299.47	5.75	0.01	6.21	6.21	4.73	1.11	0.62	6.88
OP-0611	Outlet	Ex. Outlet	177EX	1215+78.21	548.18	293.64	0.00	0.00	0.00	9.71	0.00	3.42	0.41	0.00
OP-0680	Outlet	Ex. Outlet	I77EX	1230+32.55	164.41	292.02	0.00	0.00	0.00	6.35	0.00	1.11	0.62	0.00
OP-0683	Outlet	Ex. Outlet	I77EX	1224+44.47	115.07	282.35	3.35	0.00	0.00	7.19	0.00	3.37	0.50	0.00
EX-0701	Inlet	Ex. D.I.	I77EX	1235+98.59	7.12	277.55	3.73	0.00	6.00	6.00	9.77	2.09	0.68	6.92
EX-0702	Inlet	Ex. D.I.	I77EX	1246+61.97	-6.00	244.76	4.71	0.00	8.00	8.00	9.36	2.17	0.66	6.57
OP-0798	Outlet	Ex. Outlet	I77EX	1246+60.90	155.54	224.27	0.00	0.00	0.00	8.16	0.00	2.17	0.66	0.00

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - NODE (2 OF 4)

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Node		Library	Reference		Offset	Elevation	Depth	Junction	T _C	Cumulative	Cumulative	Cumulative	Cumulative	Cumulative
ID	Type	Item	Chain	Station	(ft)	(ft)	(ft)	Loss		T _C	Discharge	Area	С	Intensity
10		Name	Chain		(11)	(11)	(11)	LU33	Used	(min)	(cfs)	(ac)	Value	(in/hr)
OP-0799	Outlet	Ex. Outlet	I77EX	1236+12.14	196.98	241.36	0.00	0.00	0.00	6.15	0.00	2.09	0.68	0.00
EX-0801	Inlet	Ex. D.I.	177EX	1252+00.86	-6.56	240.19	4.00	0.00	11.00	11.00	13.44	3.15	0.70	6.10
EX-0802	Inlet	Ex. D.I.	I77EX	1253+26.13	-127.40	233.84	4.46	0.00	9.00	9.00	9.47	3.41	0.43	6.41
EX-0803	Inlet	Ex. D.I.	I77EX	1261+46.66	-2.56	245.37	2.40	0.00	10.00	10.00	6.91	1.84	0.60	6.25
OP-0897	Outlet	Ex. Outlet	I77EX	1259+69.51	93.03	242.30	0.00	0.00	0.00	10.80	0.00	1.84	0.60	0.00
OP-0898	Outlet	Ex. Outlet	I77EX	1253+34.79	114.57	225.54	0.00	0.00	0.00	9.47	0.00	3.41	0.43	0.00
OP-0899	Outlet	Ex. Outlet	I77EX	1253+32.63	116.10	225.40	0.00	0.00	0.00	11.19	0.00	3.15	0.70	0.00
EX-0901	Inlet	Ex. D.I.	I77EX	1270+50.25	0.89	250.82	4.23	0.00	10.00	10.00	8.02	1.97	0.65	6.25
EX-0907	Inlet	Ex. D.I.	I77EX	1278+84.95	6.47	256.69	9.60	0.02	75.19	75.19	54.06	37.54	0.60	2.41
EX-0993	Inlet	Ex. C.B.	I77EX	1279+52.96	116.11	248.43	0.00	0.00	75.00	75.00	51.23	35.67	0.60	2.41
OP-0995	Outlet	Ex. Outlet	I77EX	1278+03.43	-119.09	246.04	0.00	0.00	0.00	75.45	0.00	37.54	0.60	0.00
OP-0999	Outlet	Ex. Outlet	I77EX	1271+67.92	-118.10	244.92	0.00	0.00	0.00	10.40	0.00	1.97	0.65	0.00
EX-1001	Inlet	Ex. C.B.	I77EX	1286+58.13	-85.13	264.17	8.81	0.62	50.37	50.37	42.25	25.69	0.52	3.15
EX-1001S	Inlet	Dummy Joint	I77EX	1283+69.91	-100.81	260.00	6.00	0.04	50.37	50.95	42.25	25.69	0.52	3.15
EX-1002	Inlet	Ex. D.I.	I77EX	1286+46.91	0.26	262.00	5.67	0.02	50.24	50.24	33.12	19.67	0.54	3.15
EX-1003	Inlet	Ex. D.I.	I77EX	1286+34.09	86.56	263.15	6.24	0.02	50.10	50.10	31.79	18.94	0.53	3.16
EX-1004	Inlet	Ex. D.I.	I77EX	1286+42.80	146.26	261.00	3.43	0.00	50.00	50.00	31.03	18.49	0.53	3.16
EX-1005	Inlet	Ex. D.I.	I77EX	1290+03.73	0.06	265.00	4.08	0.02	12.30	12.30	10.56	3.44	0.52	5.92
EX-1007	Inlet	Ex. D.I.	I77EX	1290+51.16	100.20	264.44	2.50	0.00	12.00	12.00	5.25	1.94	0.46	5.96
EX-1008	Inlet	Ex. C.B.	I77EX	1289+59.80	-92.14	267.47	9.02	0.19	12.54	12.54	15.12	5.31	0.48	5.89
EX-1009	Inlet	Ex. C.B.	I77EX	1291+83.45	-91.79	269.34	7.16	0.00	8.00	8.00	2.94	1.02	0.44	6.57
OP-1010	Outlet	Ex. Outlet	I77EX	1281+81.78	-116.88	253.25	0.00	0.00	0.00	51.30	0.00	25.69	0.52	0.00
EX-1101	Inlet	Ex. D.I.	I77EX	1297+48.27	0.25	270.81	3.39	0.03	10.37	10.37	11.45	3.17	0.58	6.19
EX-1102	Junction	Ex. M.H.	I77EX	1297+45.42	-91.66	273.83	7.45	0.14	10.37	10.56	11.45	3.17	0.58	6.19
EX-1103	Inlet	Ex. D.I.	I77EX	1297+50.24	96.10	270.81	2.50	0.00	10.00	10.00	6.70	1.87	0.57	6.25
EX-1105	Junction	Ex. M.H.	I77EX	1298+55.47	-104.45	276.80	10.75	0.06	10.37	10.94	11.45	3.17	0.58	6.19
EX-1106	Junction	Ex. M.H.	I77EX	1299+13.66	-138.16	273.75	9.19	0.35	11.05	11.05	17.22	5.58	0.51	6.09
EX-1107	Inlet	Ex. D.I.	I77EX	1303+59.76	-133.40	264.46	3.50	2.42	25.27	25.27	42.35	19.46	0.48	4.55
EX-1107A	Inlet	Ex. D.I.	I77EX	1299+67.58	-123.79	272.28	9.11	0.05	11.15	11.15	18.34	6.04	0.50	6.08
EX-1108	Inlet	Ex. D.I.	I77EX	1304+38.27	116.81	276.78	3.00	0.00	25.00	25.00	22.27	9.49	0.51	4.58
EX-1192	Inlet	Ex. D.I.	I77EX	1297+67.75	-289.17	279.23	4.70	0.00	8.00	8.00	6.42	2.41	0.41	6.57
OP-1196	Outlet	Ex. Outlet	I77EX	1302+99.11	-441.46	261.84	8.98	0.00	0.00	25.65	0.00	19.46	0.48	0.00
EX-1201	Inlet	Ex. D.I.	I77EX	1320+48.58	-0.08	258.49	6.09	0.69	17.52	17.52	31.63	13.33	0.45	5.28
EX-1202	Inlet	Ex. D.I.	I77EX	1327+67.60	-2.97	251.09	5.50	0.05	18.78	18.78	35.83	14.89	0.47	5.15
EX-1203	Inlet	Ex. D.I.	I77EX	1319+99.01	101.79	258.86	5.26	0.00	13.00	13.00	9.15	3.54	0.44	5.82
EX-1204	Inlet	Ex. D.I.	I77EX	1320+08.68	-94.77	259.32	5.69	0.05	17.29	17.29	12.62	6.40	0.37	5.31
EX-1205	Inlet	Ex. D.I.	I77EX	1319+68.98	-191.32	258.89	4.31	0.00	17.00	17.00	5.09	3.18	0.30	5.34
EX-1301	Inlet	Ex. D.I.	I77EX	1332+19.25	-3.29	253.35	3.11	0.00	12.00	12.00	9.96	2.56	0.65	5.96
EX-1302	Inlet	Ex. D.I.	I77EX	1341+91.59	-3.59	263.17	9.75	0.01	14.00	14.00	19.49	5.51	0.62	5.69
OP-1394	Outlet	Ex. Outlet	I77EX	1341+76.28	-121.75	252.44	0.00	0.00	0.00	14.25	0.00	5.51	0.62	0.00
EX-1395	Inlet	Dummy Joint	I77EX	1342+08.30	112.61	255.10	0.00	0.00	11.55	11.55	10.99	3.19	0.57	6.02
OP-1396	Outlet	Ex. Outlet	I77EX	1332+11.55	166.38	228.48	0.00	0.00	0.00	12.15	0.00	2.56	0.65	0.00

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - NODE (3 OF 4)

		Library								Cumulative	Cumulative	Cumulative	Cumulative	Cumulative
Node	Туре	Item	Reference	Station	Offset	Elevation	Depth	Junction	T _C	T_C	Discharge	Area	С	Intensity
ID	.,,,,	Name	Chain	Station.	(ft)	(ft)	(ft)	Loss	Used	(min)	(cfs)	(ac)	Value	(in/hr)
OP-1399	Outlet	Ex. Outlet	I77EX	1328+57.76	-0.77	225.88	0.00	0.00	0.00	18.83	0.00	14.89	0.47	0.00
EX-1401	Inlet	Ex. D.I.	I77EX	1350+03.10	-4.06	271.72	2.94	0.01	17.92	17.92	10.09	3.75	0.52	5.24
EX-1402	Inlet	Ex. D.I.	I77EX	1352+05.10	102.24	271.45	1.85	0.00	17.00	17.00	5.33	2.36	0.42	5.34
EX-1403	Inlet	Ex. D.I.	I77EX	1355+02.43	-101.05	277.20	8.32	0.35	10.92	10.92	13.65	6.37	0.35	6.11
EX-1404	Inlet	Ex. D.I.	I77EX	1355+38.26	-3.81	277.69	9.89	0.59	13.21	13.21	28.32	12.67	0.39	5.80
OP-1405	Outlet	Ex. Outlet	I77EX	1357+22.79	270.84	272.29	0.00	0.00	0.00	11.38	0.00	1.77	0.54	0.00
EX-1406	Inlet	Ex. D.I.	I77EX	1358+27.31	-107.05	280.00	5.97	0.00	10.00	10.00	5.96	1.77	0.54	6.25
EX-1408	Inlet	Ex. D.I.	I77EX	1357+86.96	99.78	279.96	6.86	0.04	10.00	10.74	5.96	1.77	0.54	6.25
EX-1409	Inlet	Ex. D.I.	I77EX	1353+84.32	-153.05	274.55	3.91	0.35	10.65	10.65	12.32	5.62	0.36	6.15
EX-1410	Inlet	Ex. D.I.	I77EX	1357+52.70	-304.47	277.08	3.00	0.27	12.36	12.36	9.75	4.70	0.35	5.91
OP-1493	Outlet	Ex. Outlet	I77EX	1355+52.53	321.56	265.07	0.00	0.00	0.00	13.82	0.00	12.67	0.39	0.00
EX-1494	Inlet	Ex. D.I.	I77EX	1358+84.23	-293.47	278.39	2.76	0.00	12.00	12.00	6.31	2.76	0.38	5.96
EX-1497	Inlet	Ex. D.I.	I77EX	1352+37.32	-384.82	297.89	24.05	0.00	10.00	10.00	9.25	4.34	0.34	6.25
OP-1499	Outlet	Ex. Outlet	I77EX	1347+75.41	-134.51	267.22	0.00	0.00	0.00	18.67	0.00	3.75	0.52	0.00
EX-1501	Inlet	Ex. D.I.	I77EX	1362+46.47	-0.35	287.07	5.05	0.00	5.00	5.00	8.10	1.89	0.60	7.12
EX-1502	Inlet	Ex. D.I.	I77EX	1364+80.78	328.41	283.85	4.57	1.53	50.70	50.70	62.36	40.55	0.49	3.13
EX-1504	Inlet	Ex. D.I.	I77EX	1367+70.48	272.14	289.10	5.14	1.15	50.32	50.32	61.59	39.73	0.49	3.15
EX-1505	Inlet	Ex. D.I.	I77EX	1371+97.00	0.37	300.61	6.58	0.00	7.00	7.00	6.46	1.59	0.60	6.74
EX-1506	Inlet	Ex. D.I.	I77EX	1371+99.99	137.22	299.02	5.30	0.04	10.00	10.00	14.07	3.45	0.65	6.25
EX-1507	Inlet	Ex. D.I.	I77EX	1372+01.82	189.57	297.82	5.34	1.50	40.17	40.17	36.06	22.26	0.45	3.60
EX-1508	Inlet	Ex. C.B.	I77EX	1372+40.91	227.07	300.09	4.70	0.04	40.10	40.10	27.35	18.54	0.41	3.60
EX-1509	Inlet	Ex. D.I.	I77EX	1373+11.59	289.27	302.44	3.59	0.00	40.00	40.00	25.19	17.60	0.40	3.61
EX-1594	Inlet	Ex. D.I.	I77EX	1369+36.36	417.43	289.10	0.00	0.00	50.00	50.00	26.69	15.60	0.54	3.16
OP-1595	Outlet	Ex. Outlet	I77EX	1363+81.78	184.10	275.54	0.00	0.00	0.00	50.89	0.00	40.55	0.49	0.00
OP-1599	Outlet	Ex. Outlet	I77EX	1362+00.72	129.36	275.62	0.00	0.00	0.00	5.18	0.00	1.89	0.60	0.00
EX-1601	Inlet	Ex. D.I.	I77EX	1379+82.20	-107.25	311.76	2.99	0.01	7.00	7.00	4.54	1.12	0.60	6.74
EX-1602	Inlet	Ex. D.I.	I77EX	1379+84.09	0.61	311.81	3.46	0.01	7.47	7.47	7.48	1.86	0.60	6.66
EX-1603	Inlet	Ex. D.I.	I77EX	1379+86.51	124.57	311.50	3.47	0.01	9.00	9.00	10.97	2.79	0.61	6.41
EX-1604	Inlet	Ex. C.B.	I77EX	1379+87.52	179.48	315.69	28.75	0.54	18.99	18.99	30.41	11.15	0.53	5.13
EX-1605	Inlet	Ex. C.B.	I77EX	1383+46.69	-149.27	318.30	4.76	0.00	5.00	5.00	3.64	0.90	0.57	7.12
EX-1606	Inlet	Ex. D.I.	I77EX	1383+51.50	-106.92	317.23	3.82	0.01	17.00	17.00	7.89	3.28	0.45	5.34
EX-1607	Inlet	Ex. D.I.	I77EX	1383+64.04	-0.16	317.54	4.93	0.01	17.33	17.33	12.24	4.65	0.50	5.30
EX-1608	Inlet	Ex. D.I.	I77EX	1383+78.82	122.82	316.58	5.36	0.01	17.62	17.62	17.09	6.25	0.52	5.27
EX-1609	Inlet	Ex. C.B.	I77EX	1383+84.97	179.53	321.27	10.72	0.34	17.79	17.79	20.35	7.73	0.50	5.25
EX-1610	Inlet	Ex. C.B.	I77EX	1387+77.60	193.67	326.13	3.50	0.00	6.00	6.00	1.83	0.88	0.30	6.92
EX-1698	Inlet	Dummy Joint	I77EX	1379+81.57	-148.51	311.08	0.00	0.00	5.00	5.00	1.40	0.39	0.50	7.12
OP-1699	Outlet	Ex. Outlet	I77EX	1379+02.96	263.21	275.54	0.50	0.00	0.00	19.08	0.00	11.15	0.53	0.00
EX-1701	Inlet	Ex. D.I.	I77EX	1399+90.93	-3.77	313.10	4.44	0.00	11.00	11.00	11.32	2.73	0.68	6.10
OP-1797	Outlet	Ex. Outlet	177EX	1400+45.07	158.58	297.40	0.00	0.00	0.00	11.18	0.00	2.73	0.68	0.00
EX-1801	Inlet	Ex. D.I.	I77EX	1410+72.23	0.61	305.12	5.12	0.00	14.00	14.00	17.53	4.50	0.68	5.69
EX-1802	Inlet	Ex. D.I.	I77EX	1416+20.50	0.53	306.68	6.29	0.05	22.10	22.10	14.98	7.91	0.39	4.83
EX-1803	Inlet	Ex. D.I.	I77EX	1423+70.33	0.19	311.38	4.96	0.01	76.13	76.13	50.64	46.90	0.45	2.39

I-77 - EXISTING CLOSED SYSTEM DRAINAGE INFRASTRUCTURE GEOPAK DRAINAGE REPORTS - 10-YR STORM EVENT - NODE (4 OF 4)

Node ID	Туре	Library Item Name	Reference Chain	Station	Offset (ft)	Elevation (ft)	Depth (ft)	Junction Loss	T _c Used	Cumulative T _C (min)	Cumulative Discharge (cfs)	Cumulative Area (ac)	Cumulative C Value	Cumulative Intensity (in/hr)
EX-1807	Inlet	Dummy Joint	I77EX	1416+52.08	112.06	308.00	4.54	0.27	21.89	21.90	8.51	5.91	0.30	4.84
EX-1894	Inlet	Dummy Joint	I77EX	1424+12.27	118.49	307.32	0.00	0.00	75.88	75.88	40.86	40.69	0.42	2.39
OP-1895	Outlet	Ex. Outlet	I77EX	1423+18.17	-147.72	305.34	0.00	0.00	0.00	76.37	0.00	46.90	0.45	0.00
OP-1896	Outlet	Ex. Outlet	I77EX	1415+71.96	-170.28	295.32	0.00	0.00	0.00	22.34	0.00	7.91	0.39	0.00
EX-1897	Inlet	Dummy Joint	I77EX	1416+60.60	111.36	303.68	0.00	0.00	21.89	21.89	8.51	5.91	0.30	4.84
OP-1899	Outlet	Ex. Outlet	I77EX	1410+45.30	129.71	297.25	0.00	0.04	0.00	14.20	0.00	4.50	0.68	0.00
EX-2101	Inlet	Ex. D.I.	I77EX	1459+00.30	-0.37	309.92	9.90	0.04	46.38	46.38	60.73	34.51	0.53	3.31
EX-2102	Inlet	Ex. D.I.	I77EX	1471+91.64	0.23	287.64	5.20	0.00	12.00	12.00	16.80	4.22	0.67	5.96
OP-2195	Outlet	Ex. Outlet	I77EX	1471+38.32	-195.38	268.29	0.00	0.00	0.00	12.19	0.00	4.22	0.67	0.00
EX-2198	Inlet	Dummy Joint	I77EX	1458+56.65	129.08	304.21	0.00	0.00	46.25	46.25	54.49	31.63	0.52	3.31
OP-2199	Outlet	Ex. Outlet	I77EX	1459+53.18	-161.85	298.00	0.00	0.00	0.00	46.61	0.00	34.51	0.53	0.00
EX-2201	Inlet	Ex. D.I.	I77EX	1475+65.71	0.49	290.51	4.54	0.06	10.35	10.35	15.39	4.28	0.58	6.20
EX-2202	Inlet	Ex. D.I.	I77EX	1476+68.11	114.86	291.55	3.41	0.00	10.00	10.00	6.06	1.90	0.51	6.25
EX-2203	Inlet	Ex. D.I.	I77EX	1484+97.49	0.25	305.89	4.23	0.06	20.28	20.28	45.29	18.35	0.49	5.00
EX-2204	Inlet	Ex. D.I.	I77EX	1485+95.89	111.22	307.34	4.33	0.00	20.00	20.00	26.00	11.49	0.45	5.02
OP-2298	Outlet	Ex. Outlet	I77EX	1483+96.76	-111.82	300.78	0.00	0.00	0.00	20.56	0.00	18.35	0.49	0.00
OP-2299	Outlet	Ex. Outlet	I77EX	1474+61.90	-113.95	283.97	0.00	0.00	0.00	10.63	0.00	4.28	0.58	0.00
EX-2301	Inlet	Ex. D.I.	I77EX	1499+93.01	-92.13	335.92	2.60	0.00	6.00	6.00	2.61	0.63	0.60	6.92
EX-2302	Inlet	Ex. D.I.	I77EX	1500+90.21	91.87	337.63	4.12	0.18	15.84	15.84	7.54	3.61	0.38	5.47
EX-2303	Inlet	Ex. D.I.	I77EX	1503+93.13	-113.65	339.85	4.35	0.00	14.00	14.00	6.28	3.10	0.36	5.69
EX-2304	Inlet	Ex. D.I.	I77EX	1503+88.57	113.11	338.86	3.02	0.00	15.00	15.00	6.11	3.12	0.35	5.57
OP-2397	Outlet	Ex. Outlet	I77EX	1503+14.97	-238.73	333.22	0.00	0.00	0.00	14.32	0.00	3.10	0.36	0.00
OP-2398	Outlet	Ex. Outlet	I77EX	1499+64.98	-174.57	333.00	0.00	0.00	0.00	6.40	0.00	0.63	0.60	0.00
OP-2399	Outlet	Ex. Outlet	177EX	1500+30.57	146.62	332.75	0.00	0.00	0.00	16.03	0.00	3.61	0.38	0.00

Appendix F

Video Inspection Reports of Existing Pipes



AET Services

Robotic Inspections

EC0102

EC0102 6' Culvert 1660'

Cracking Throughout

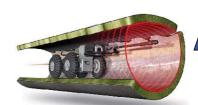
18" Lateral 736'

18" Lateral 878'

Infiltration 1002'

18" Lateral 1089'

Manhole 1270'



AET Services

Robotic Inspections

EC0103

EC0103 6' Culvert 596'

Cracking Throughout

42" Lateral 200'

18" Lateral 395"

6" x2 Lateral 395'

45degree turn to left 527'

Turns into 54" and 18" 596

Then 42" straight ahead

Then 30" goes left



AET Services

Robotic Inspections

EC0211

EC0211 6' Culvert 2165'

Cracking Throughout

Manhole 735'

45 degree left 896"

Manhole 1631'

Manhole 1883'

AET Services 623 Rices Creek Road Liberty,SC 29657 1-800-990-8406 www.aettopgun.com

www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0302 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/15/2015 Dry 18 18 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 153.1 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous At 6.8 ft Grouted - Joint grouted Category: Inventory EX0302 Crack - Crack in the pipe 10.0 ft Category: Structural 20.0 ft 30.0 ft 40.0 ft 138.3 ft

Omitted: 70.1 ft

50.0 ft

60.0 ft

70.0 ft

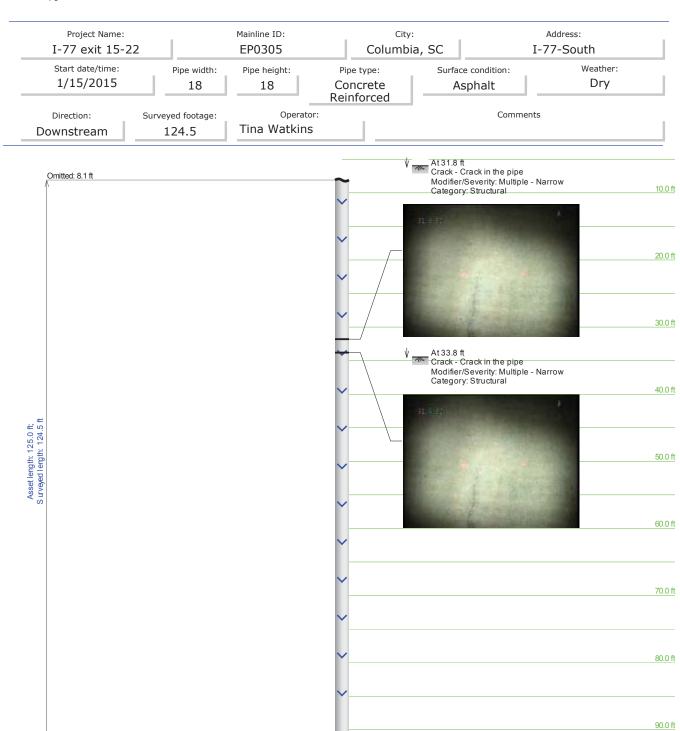
80.0 ft

AET Services 623 Rices Creek Road Liberty,SC 29657 1-800-990-8406 www.aettopgun.com

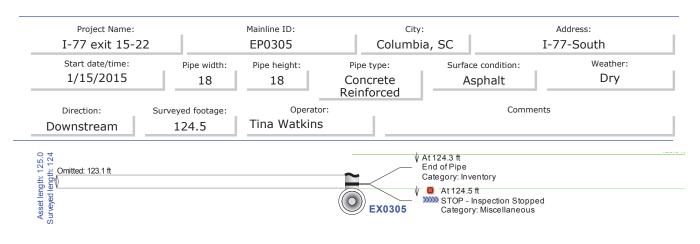


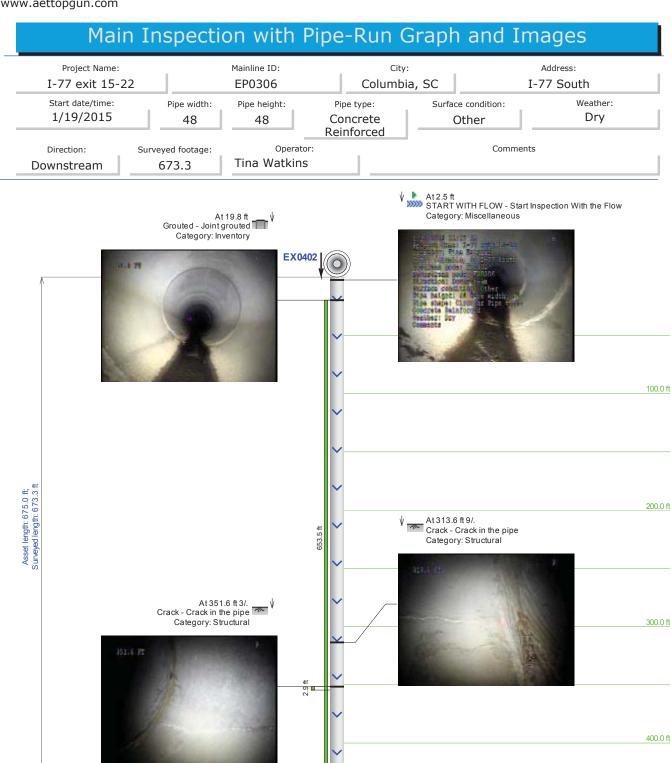
Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0305 Columbia, SC I-77-South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/15/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 124.5 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous At 2.8 ft Grouted - Joint grouted Category: Inventory EX0307 0.0 ft At 15.0 ft Crack - Crack in the pipe Category: Structural 10.0 ft 20.0 ft 30.0 ft 121.7 ft At 31.1 ft Joint - Separated - Joint separated in the Pipe Category: Structural 40.0 ft 50.0 ft 60.0 ft

Omitted: 56.0 ft

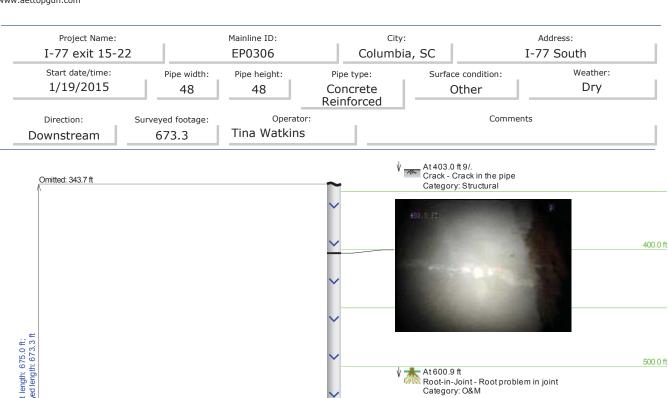


Omitted: 33.2 ft





Omitted: 246.5 ft



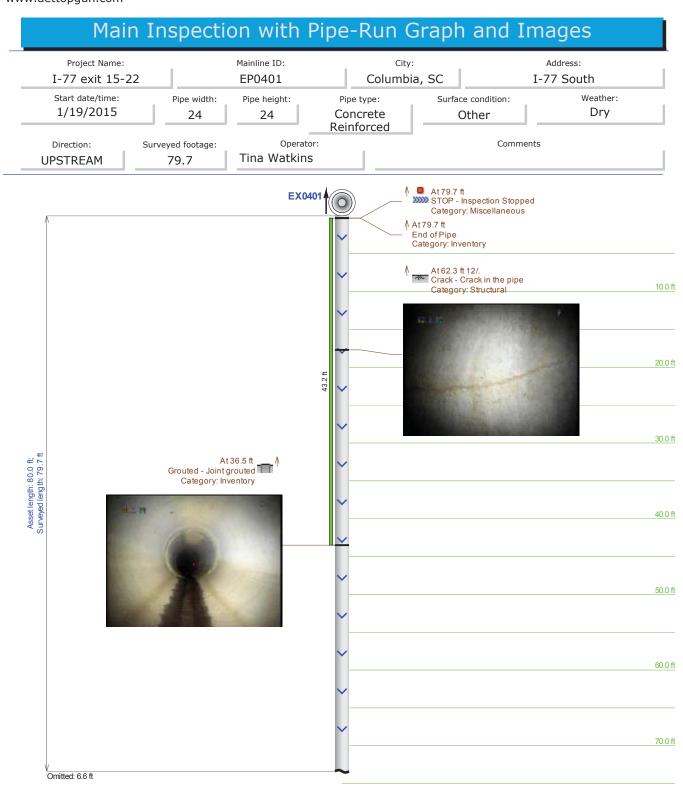
VAt 673.3 ft
End of Pipe
Category: Inventory

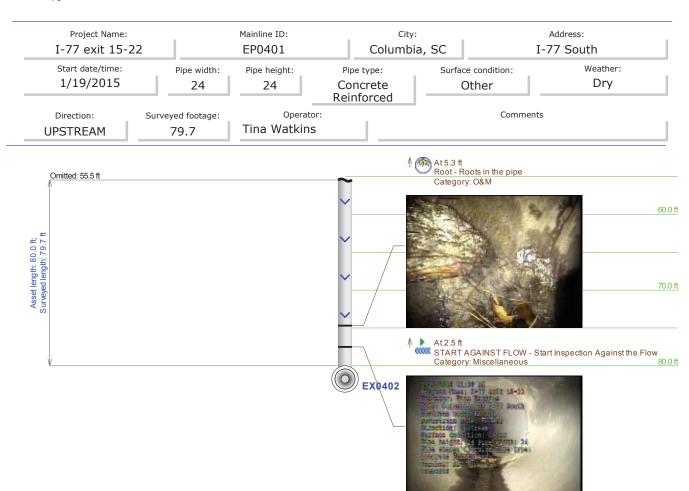
★ At 673.3 ft

▼ At 673.3 ft

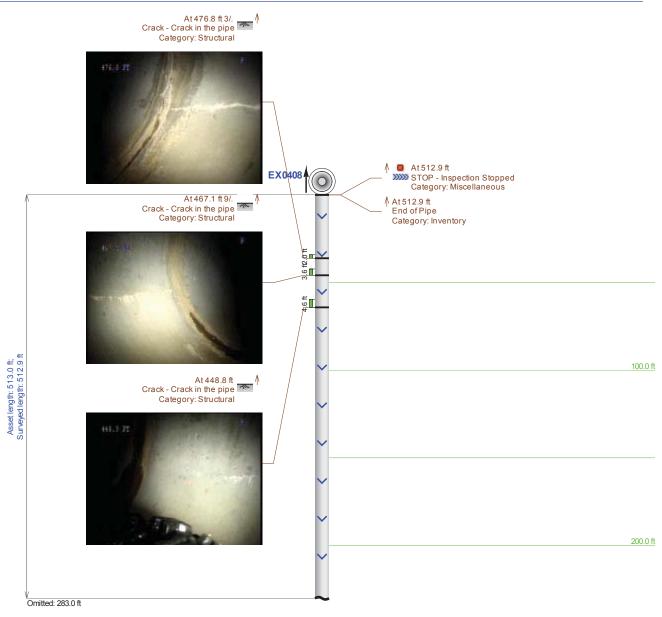
▼ STOP - Inspection Stopped
Category: Miscellaneous

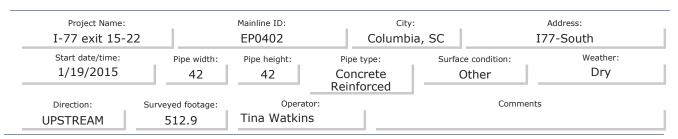
600.0 ft



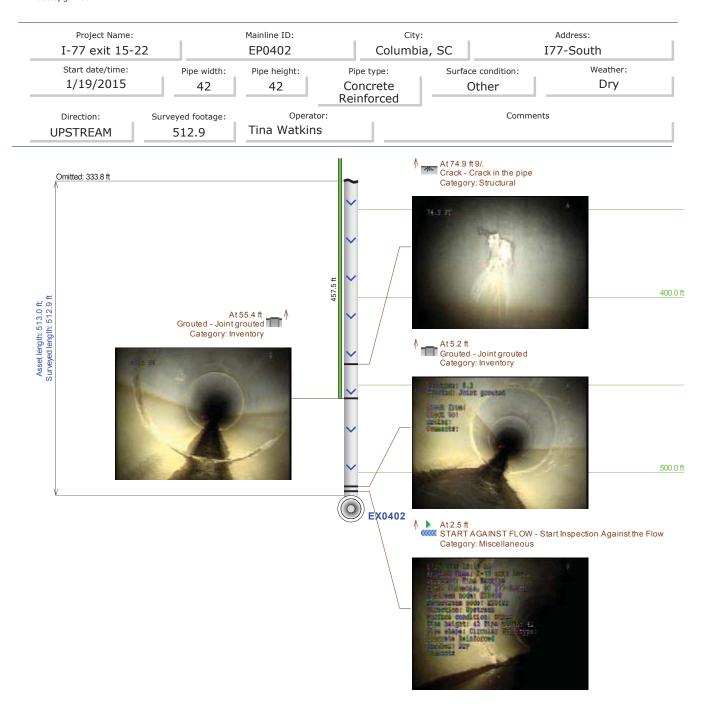


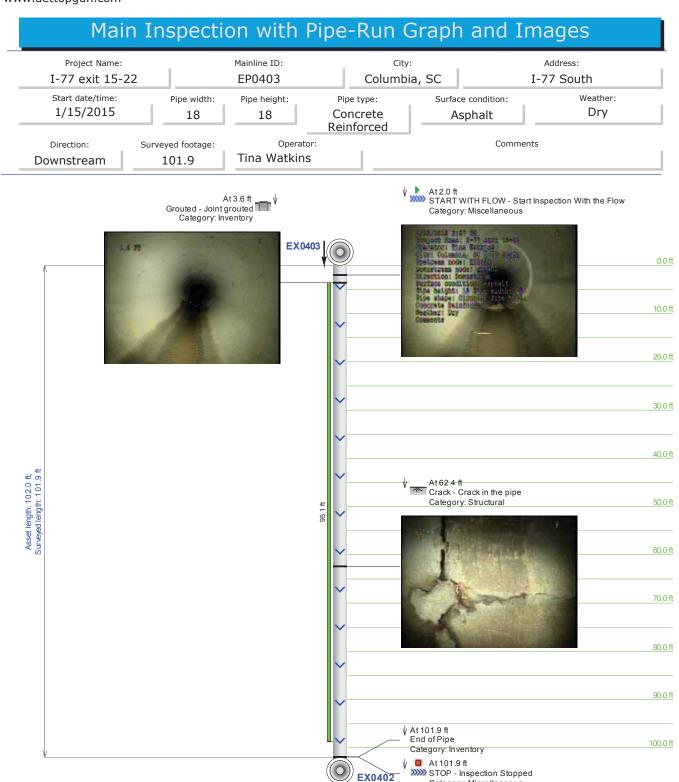
Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0402 Columbia, SC I77-South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/19/2015 Dry 42 42 Concrete Other Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins **UPSTREAM** 512.9



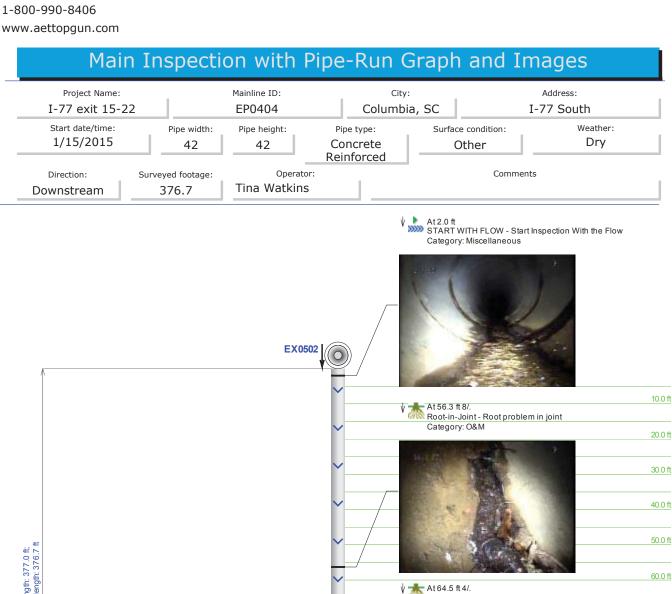








Category: Miscellaneous



Omitted: 254.2 ft

70.0 ft

80.0 ft

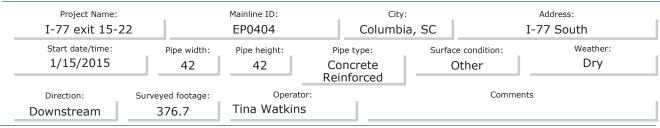
90.0 ft

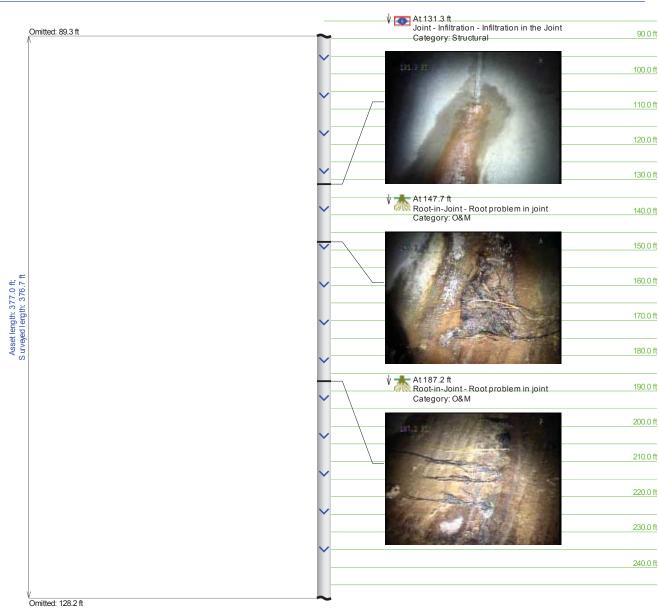
100.0 ft

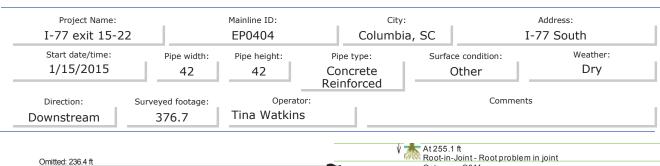
110.0 ft

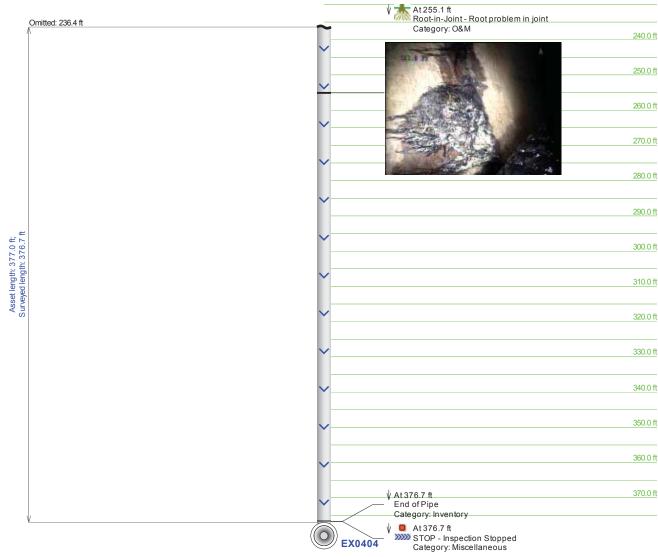
120.0 ft

Root-in-Joint - Root problem in joint Category: O&M

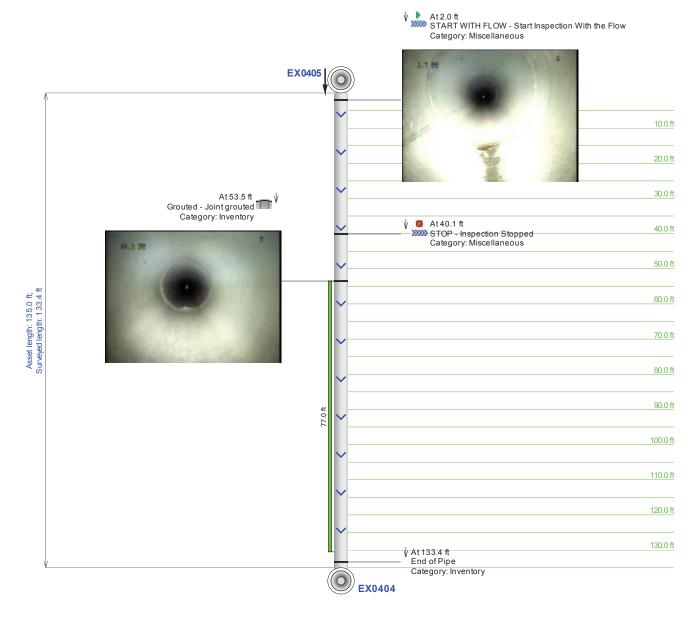


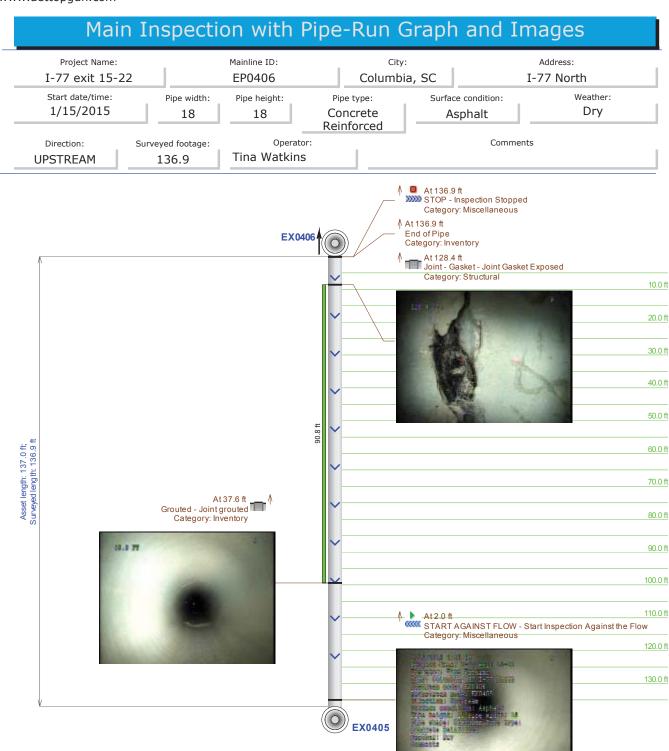




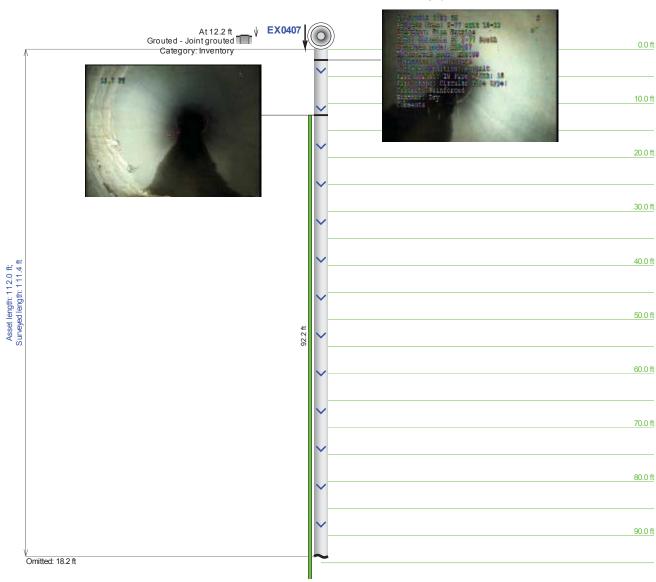


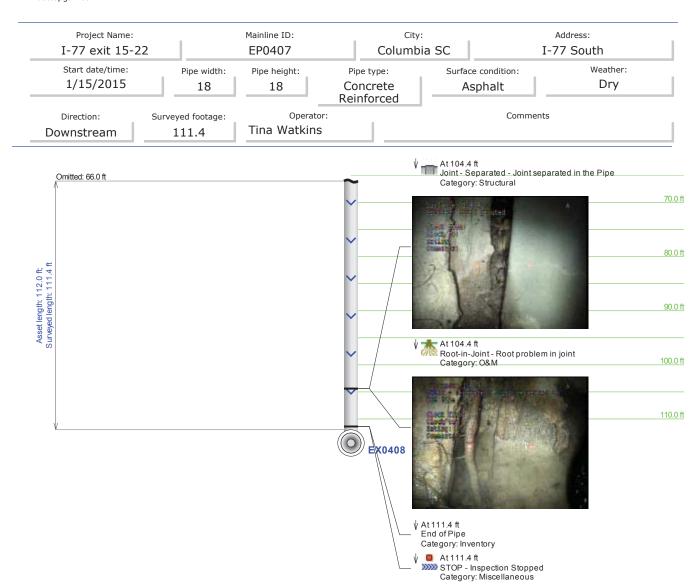
Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0405 Columbia, SC I-77South Weather: Start date/time: Pipe width: Pipe height: Surface condition: Pipe type: 1/15/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 133.4

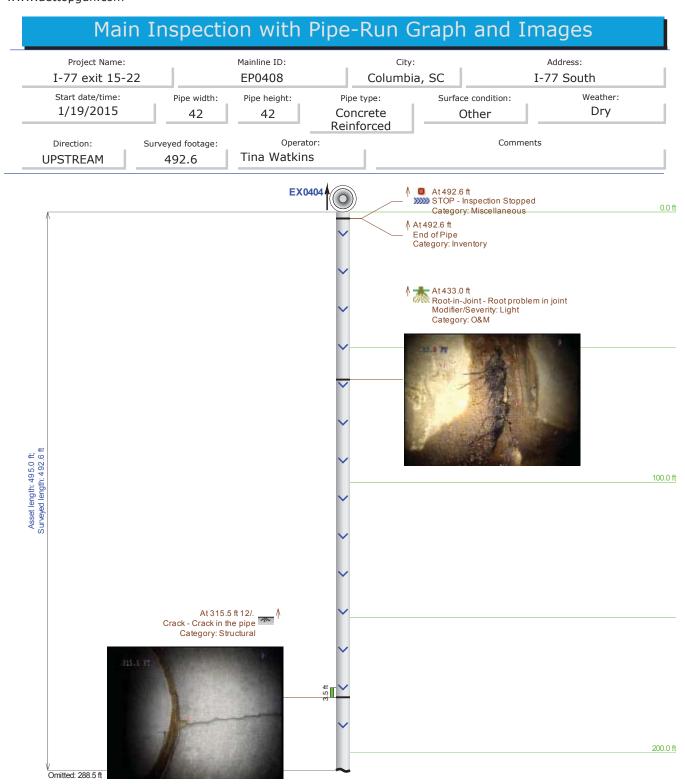


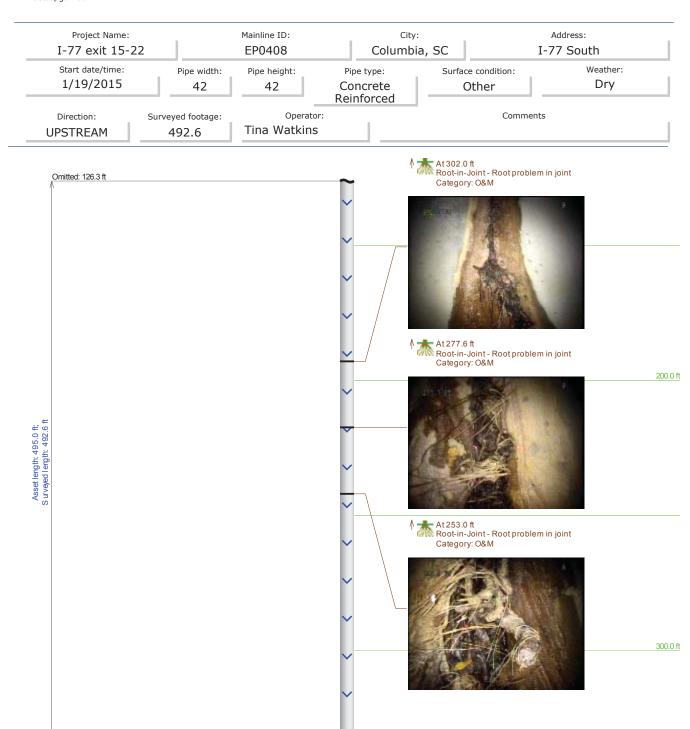


www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0407 Columbia SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/15/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 111.4 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous At 12.2 ft Grouted - Joint grouted Category: Inventory

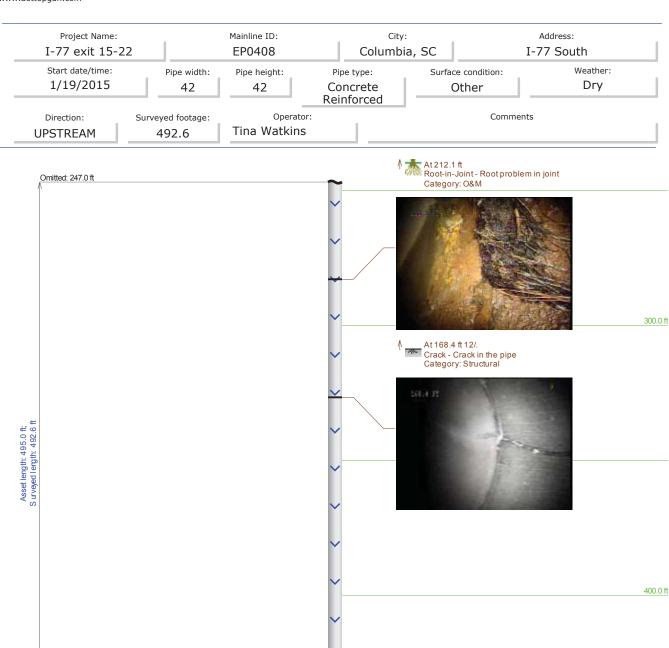




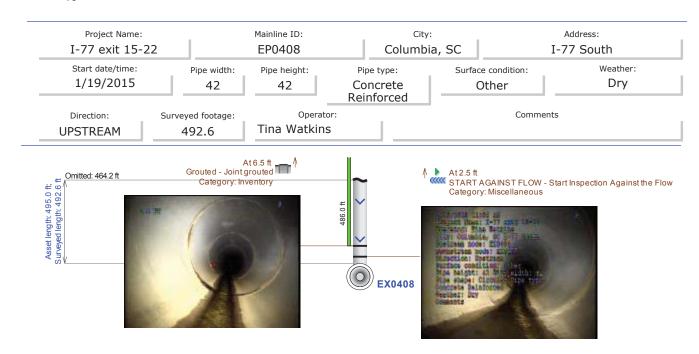


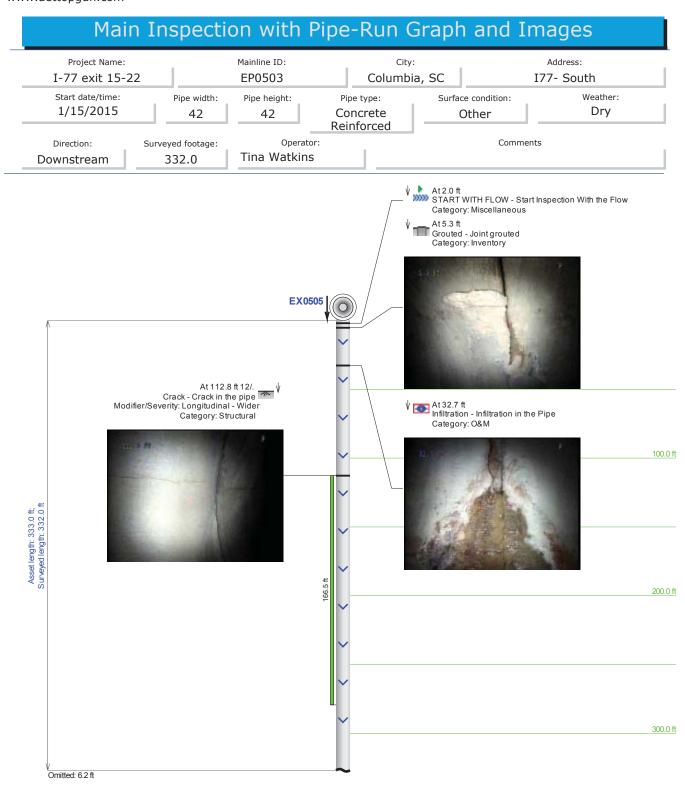


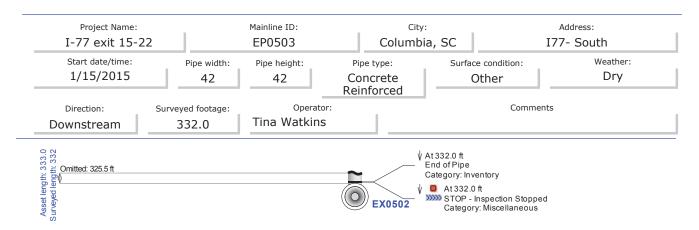
Omitted: 160.8 ft

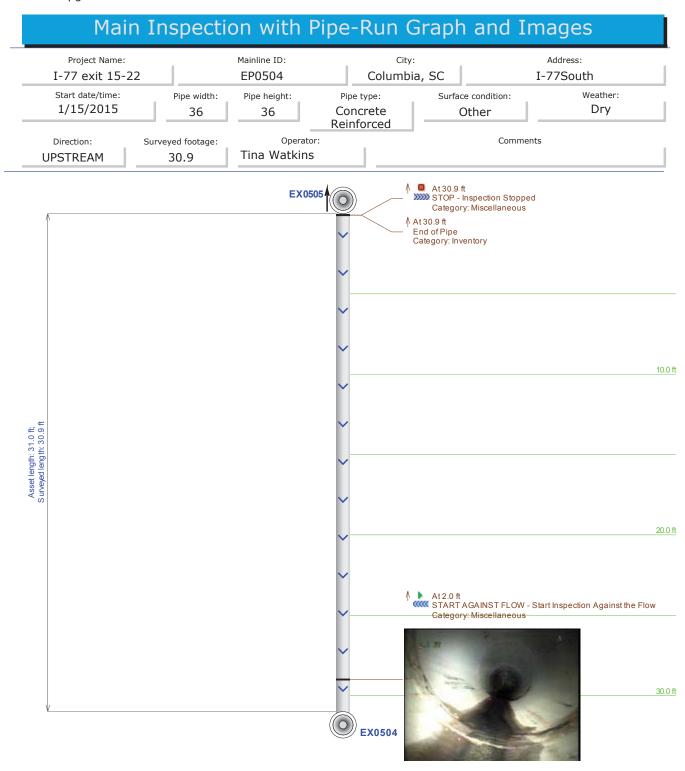


Omitted: 40.1 ft

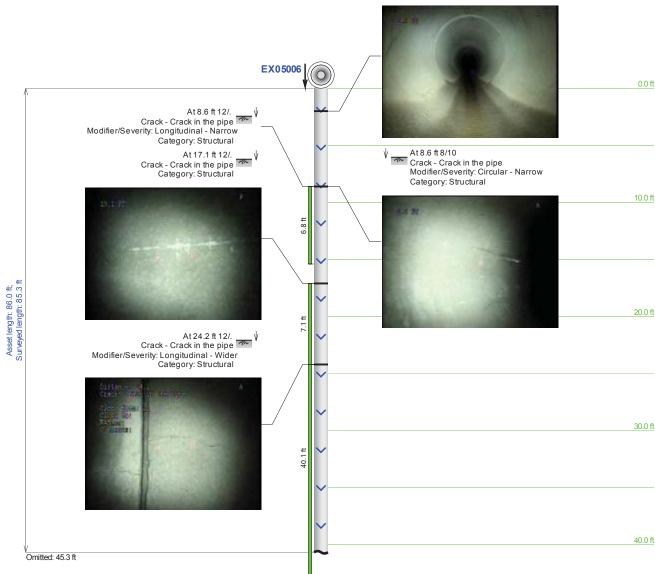


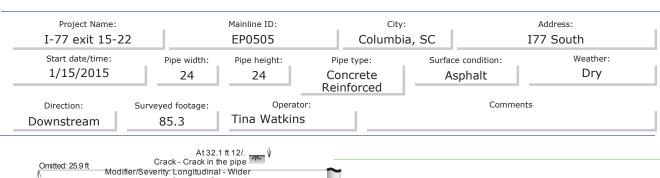


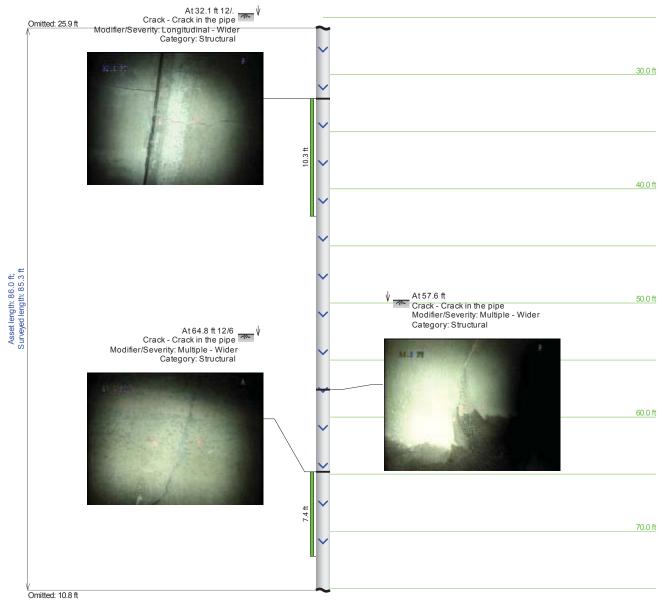


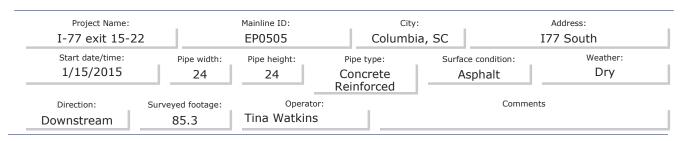


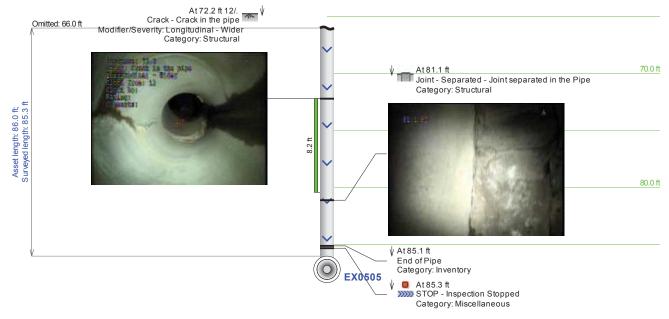
www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0505 Columbia, SC I77 South Start date/time: Pipe width: Pipe height: Surface condition: Weather: Pipe type: 1/15/2015 Dry 24 24 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 85.3 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous EX05006

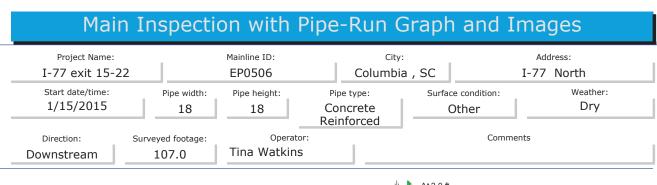


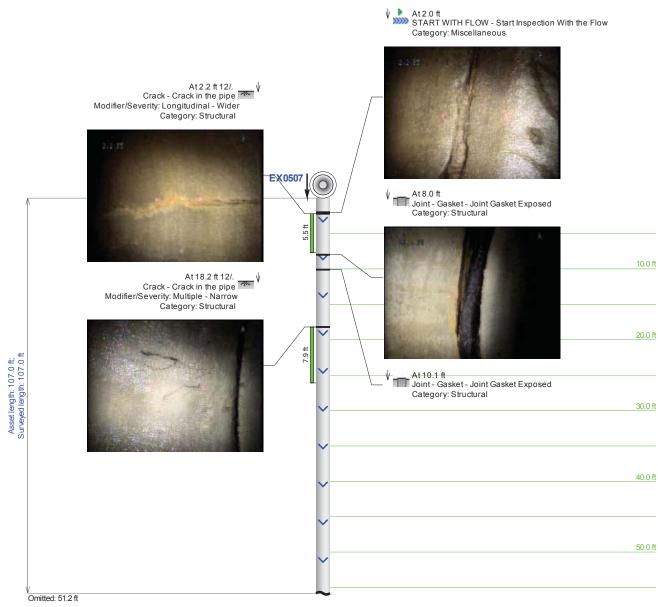


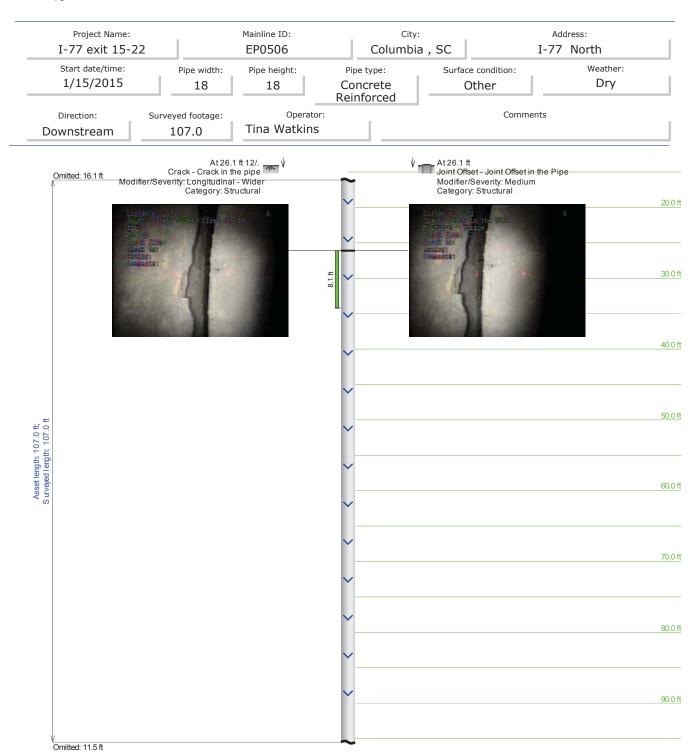


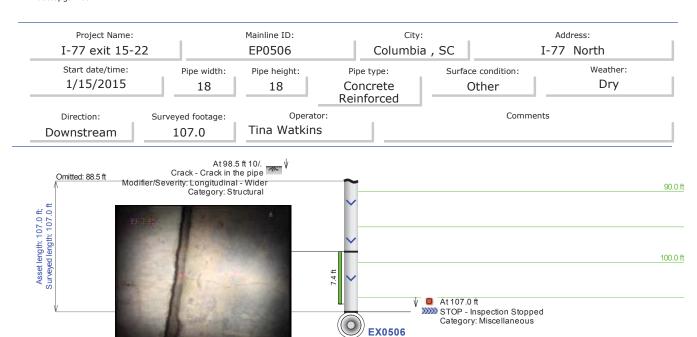


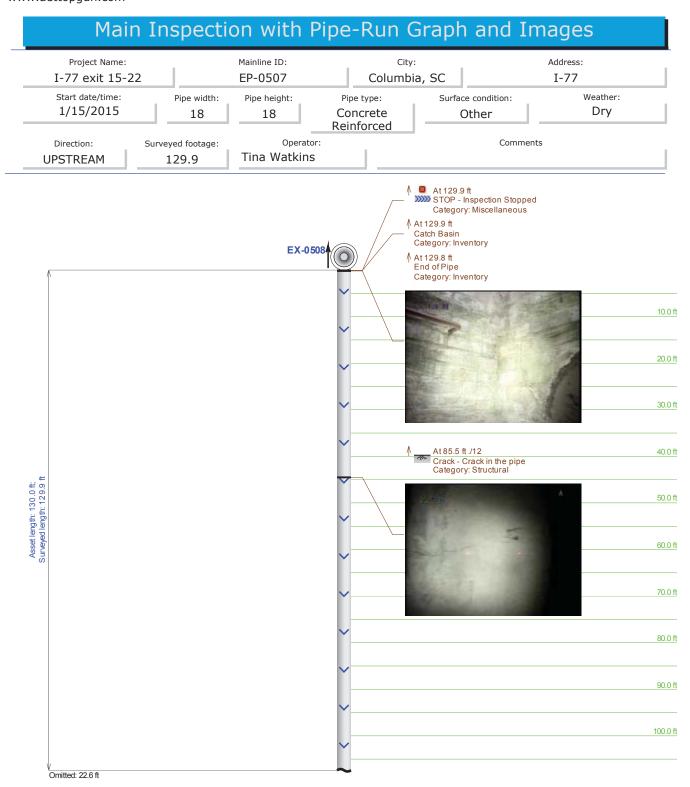


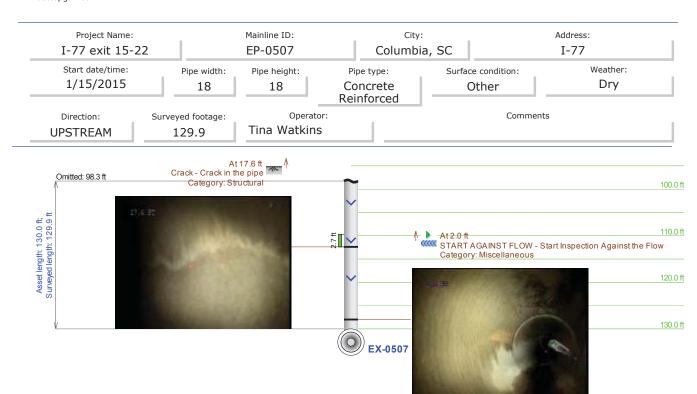


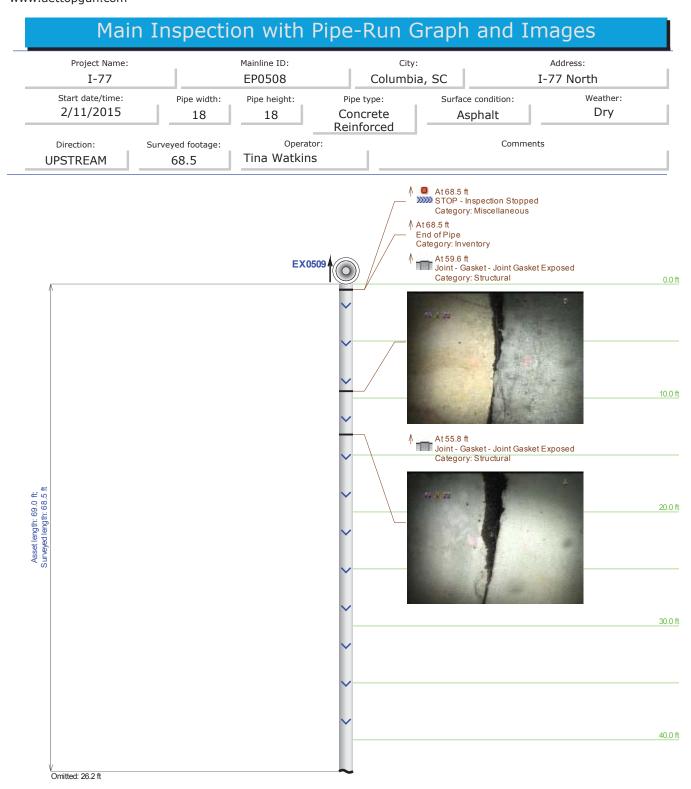


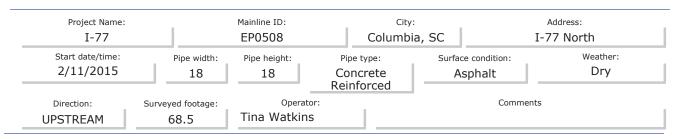


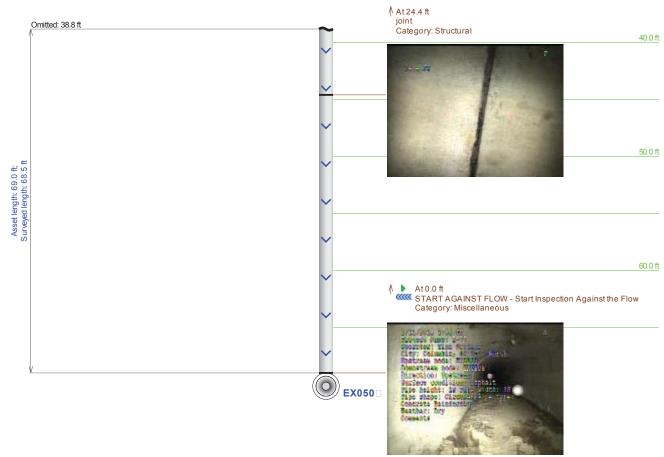


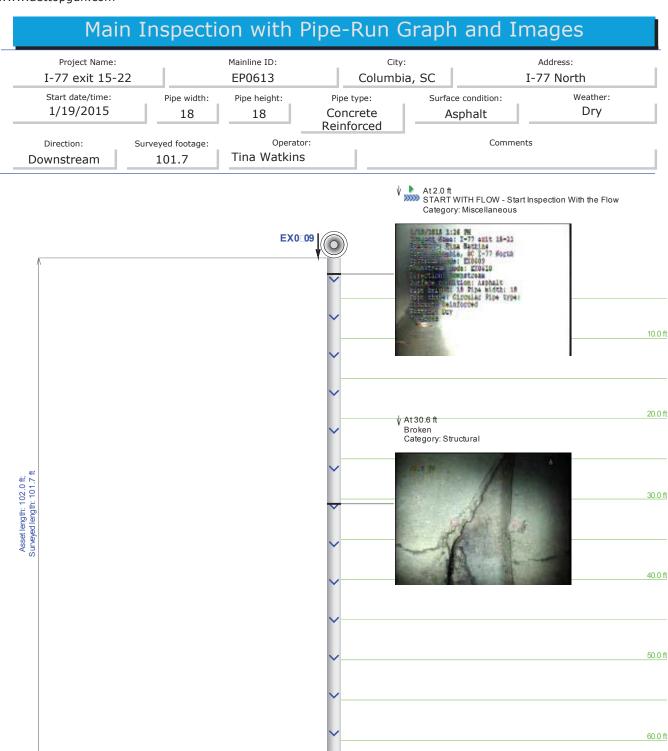




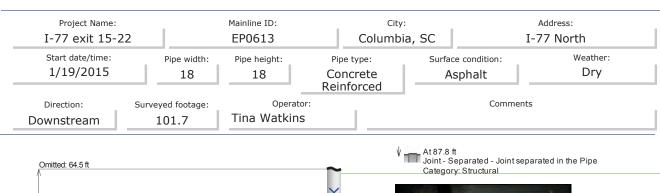


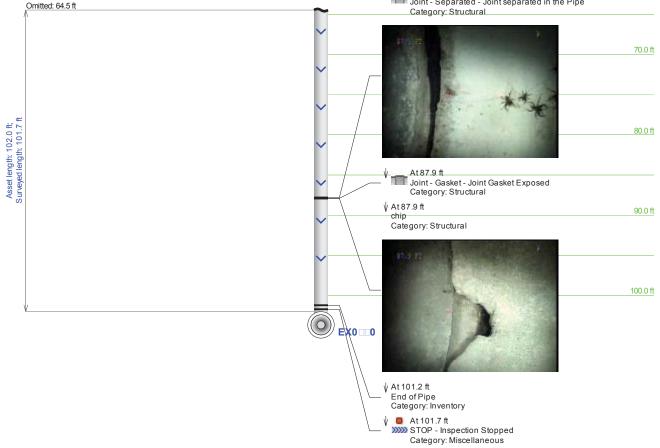




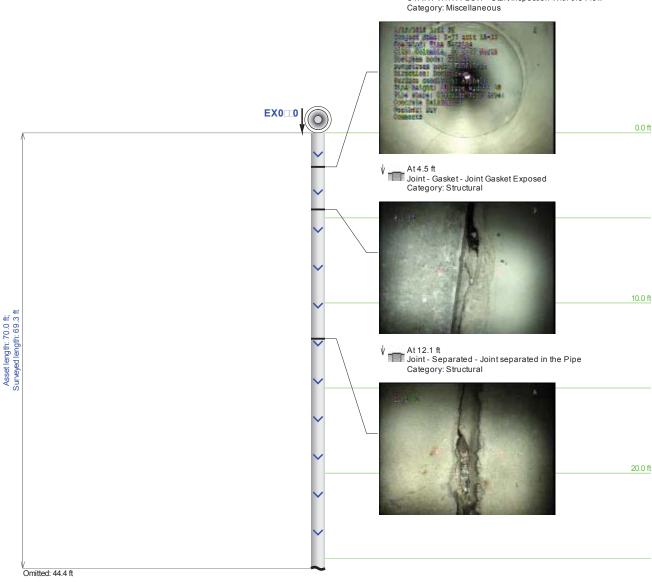


Omitted: 38.3 ft

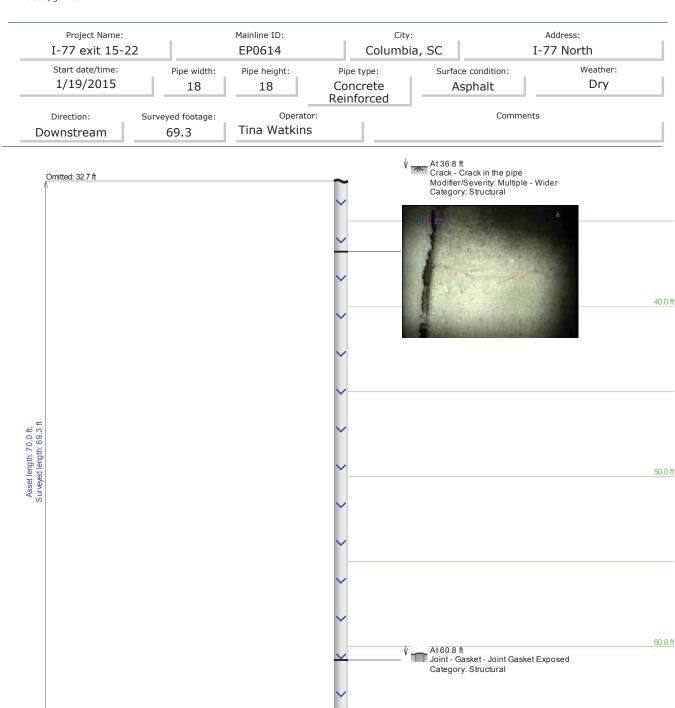




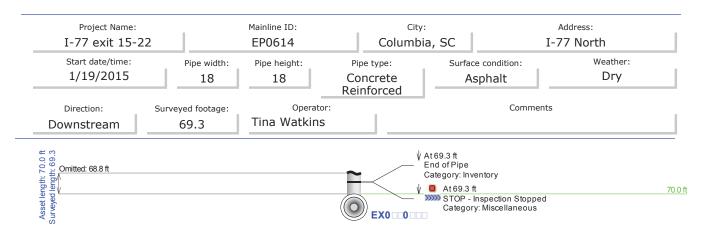
www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP0614 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/19/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 69.3 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous

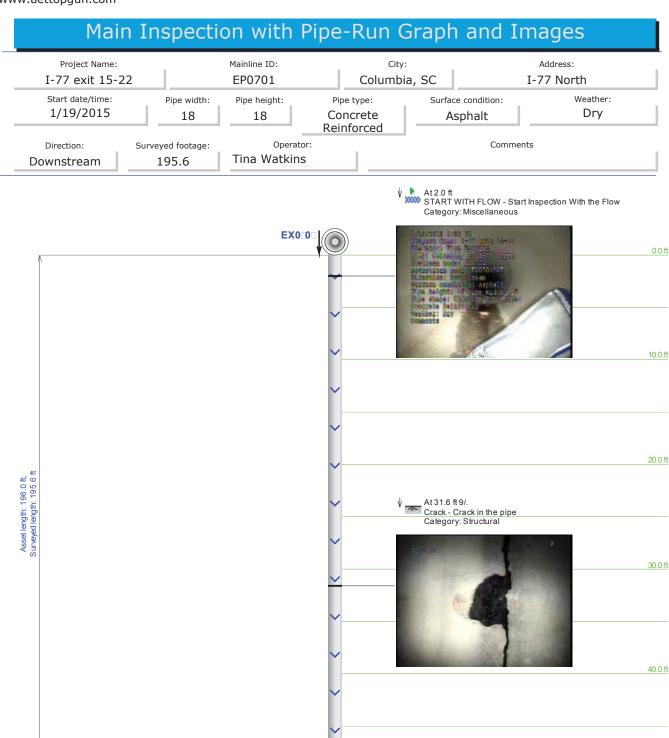




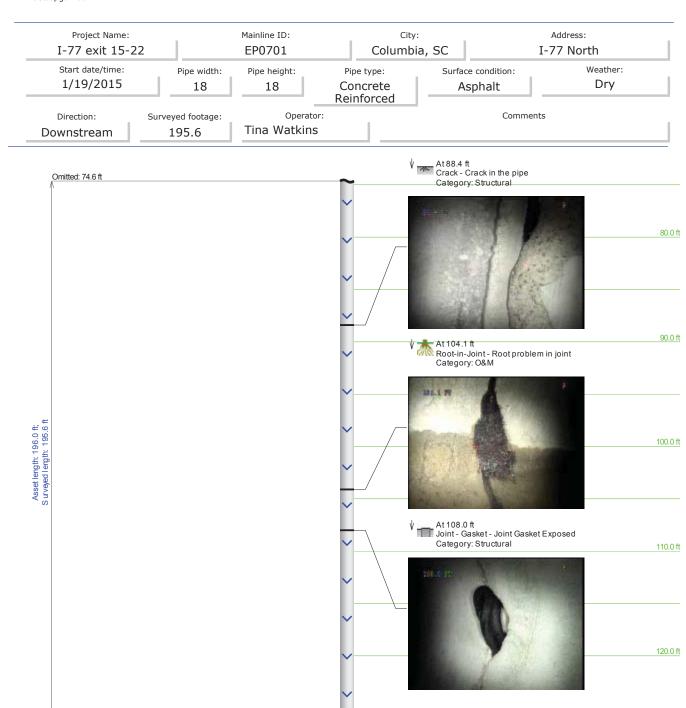


Omitted: 4.3 ft

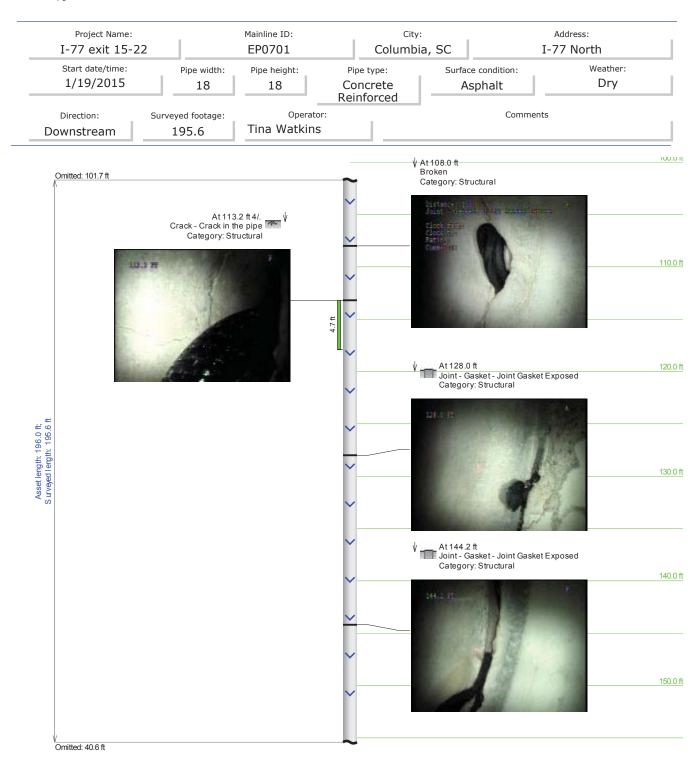


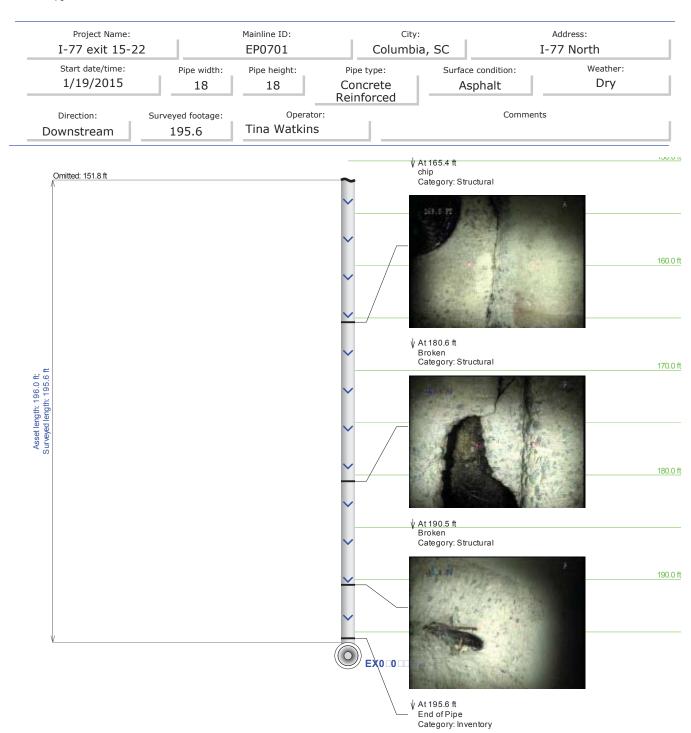


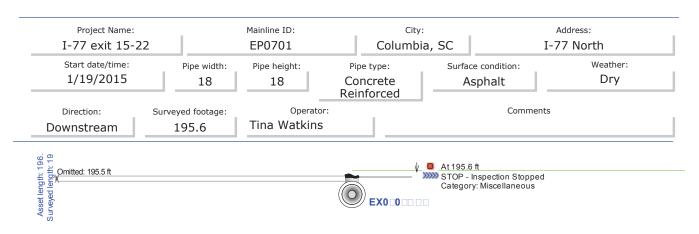
Omitted: 146.6 ft

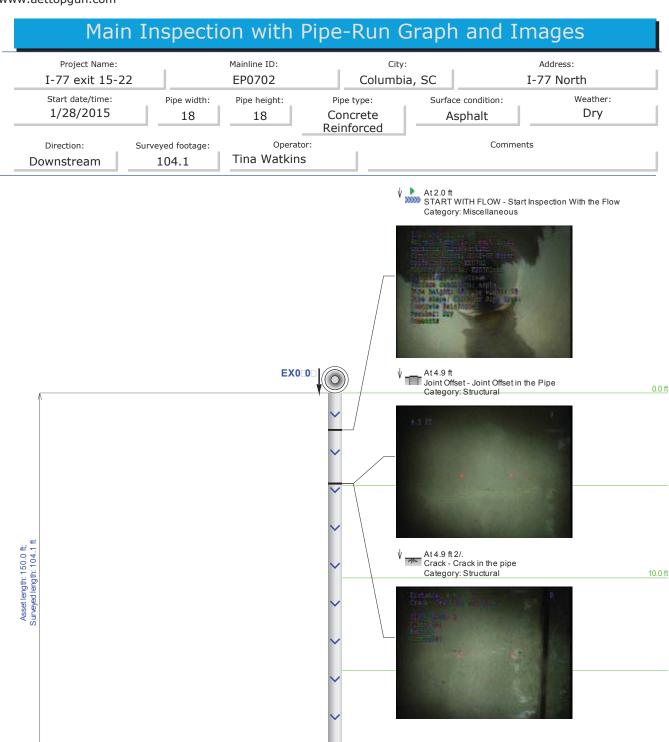


Omitted: 67.7 ft



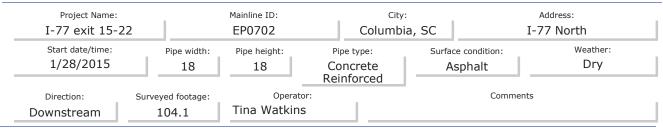


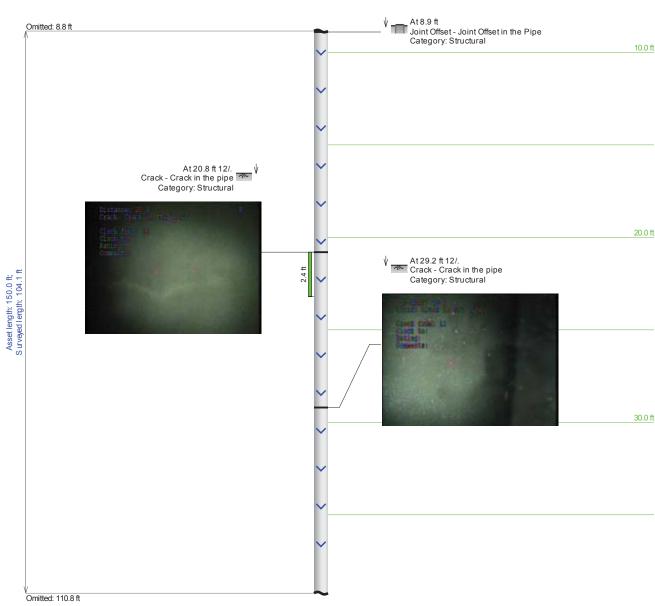


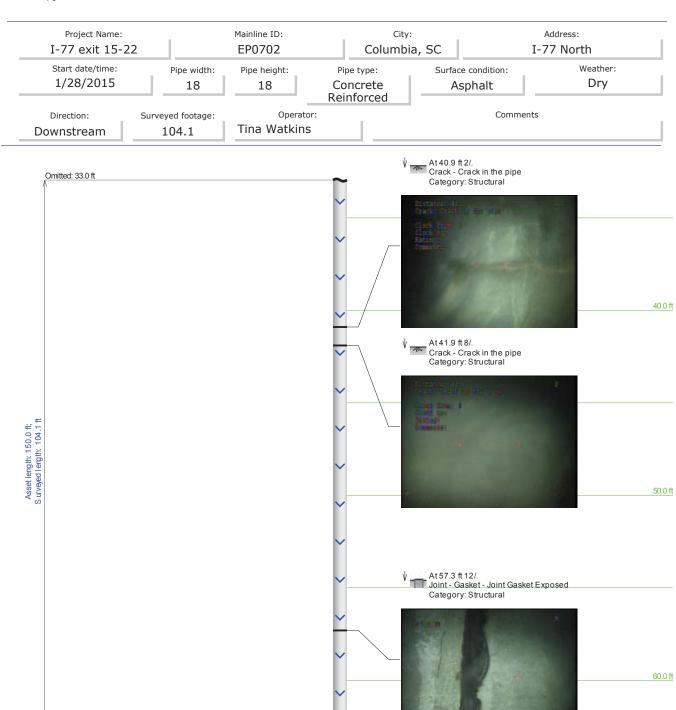


Omitted: 129.5 ft

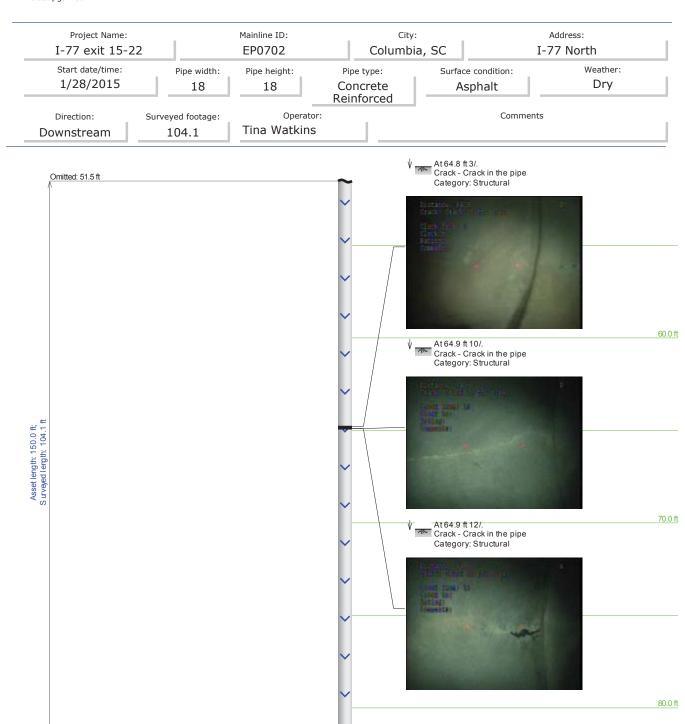
20.0 ft





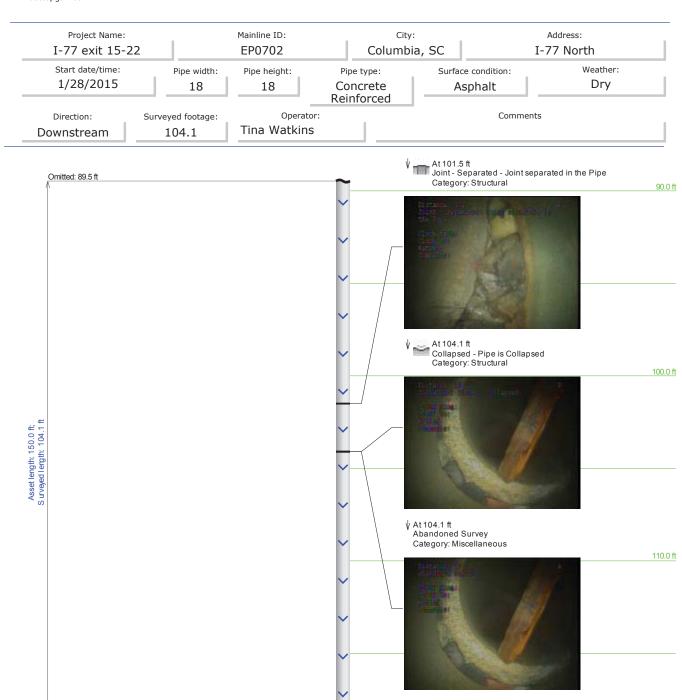


Omitted: 86.7 ft

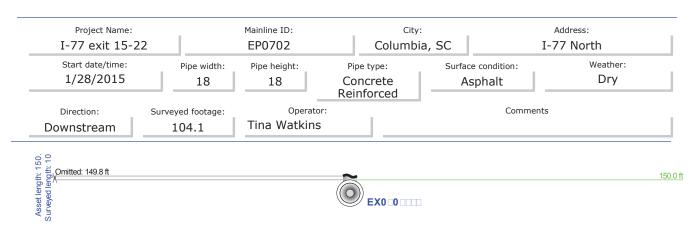


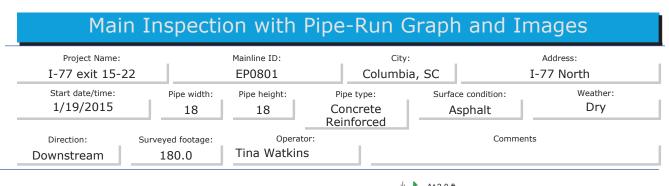
Omitted: 68.1 ft

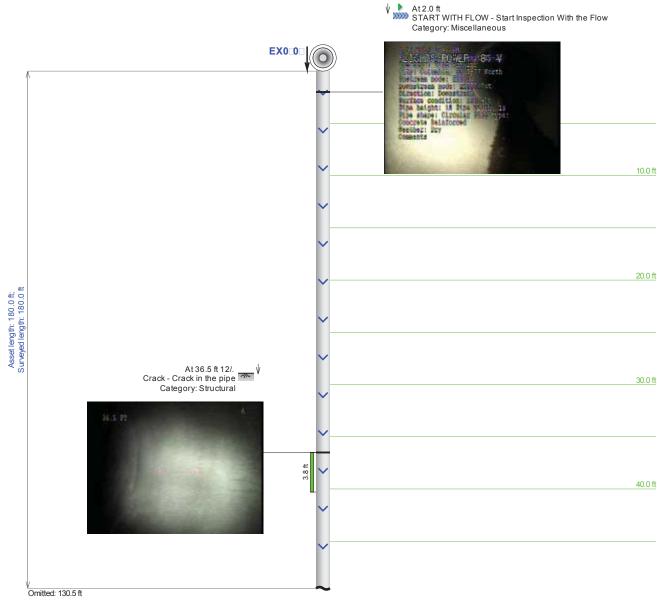


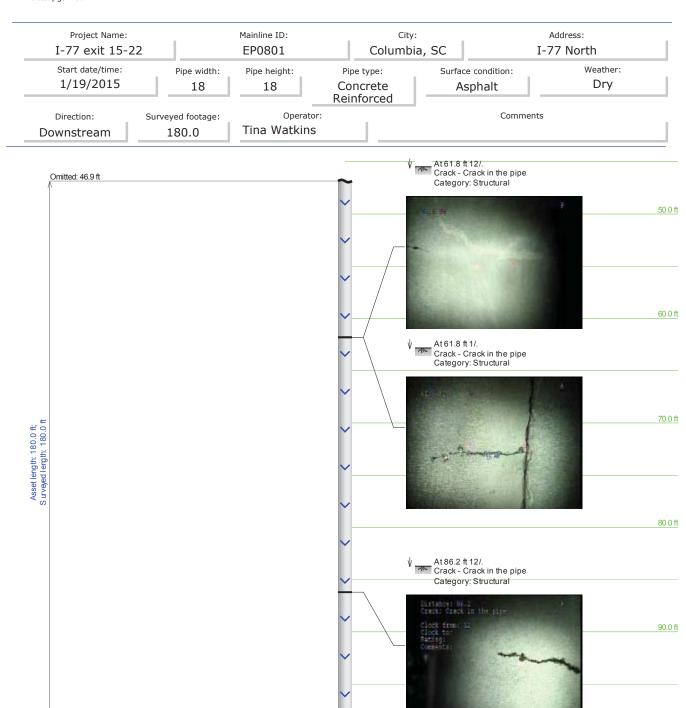


Omitted: 30.1 ft



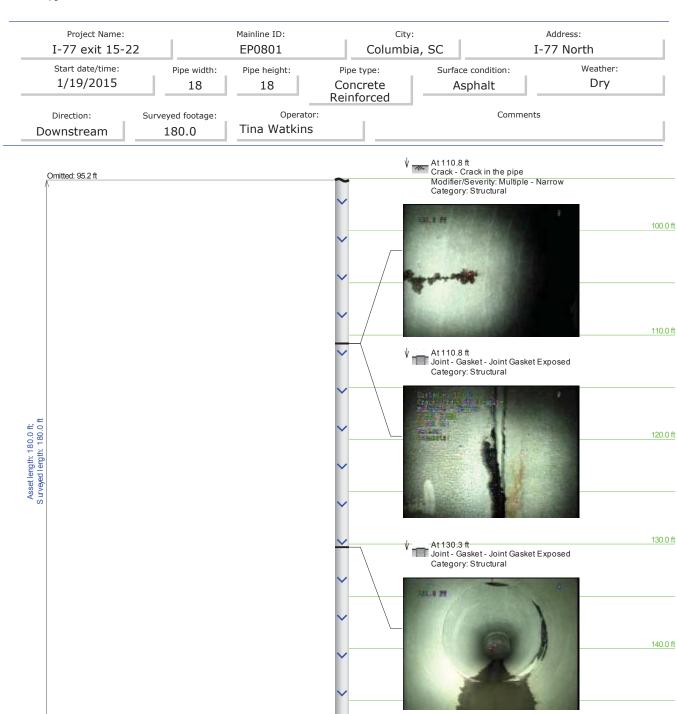




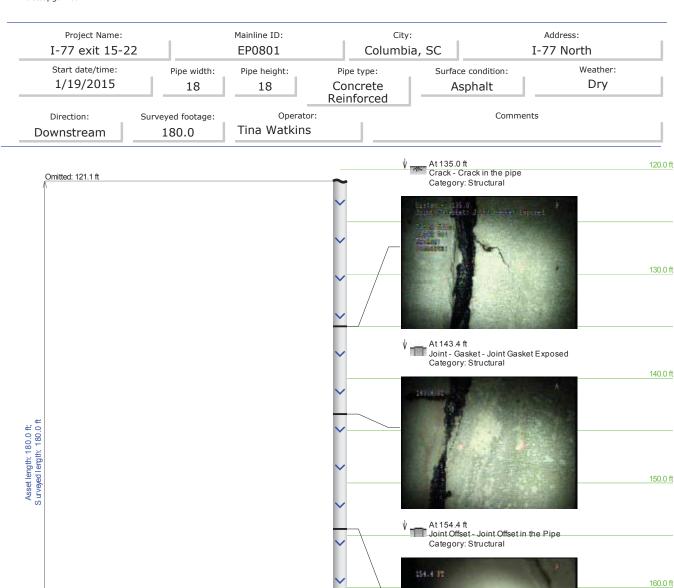


Omitted: 79.3 ft

100.0 ft

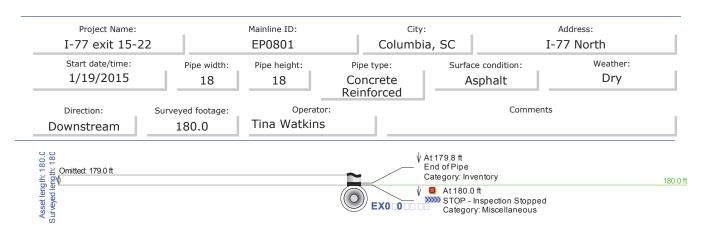


Omitted: 31.0 ft

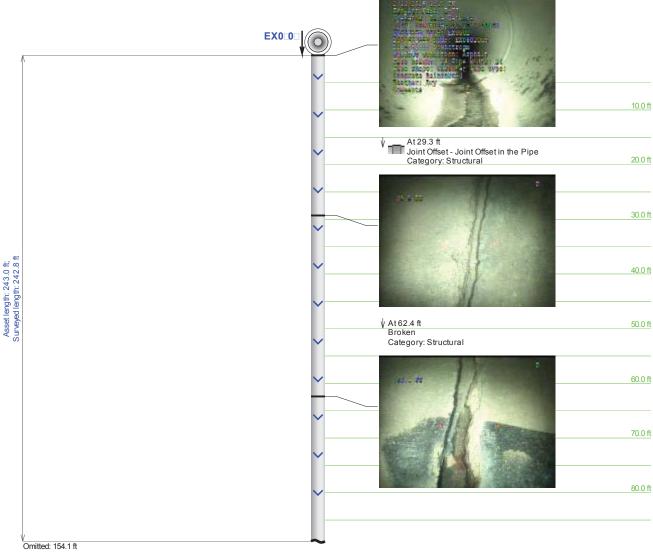


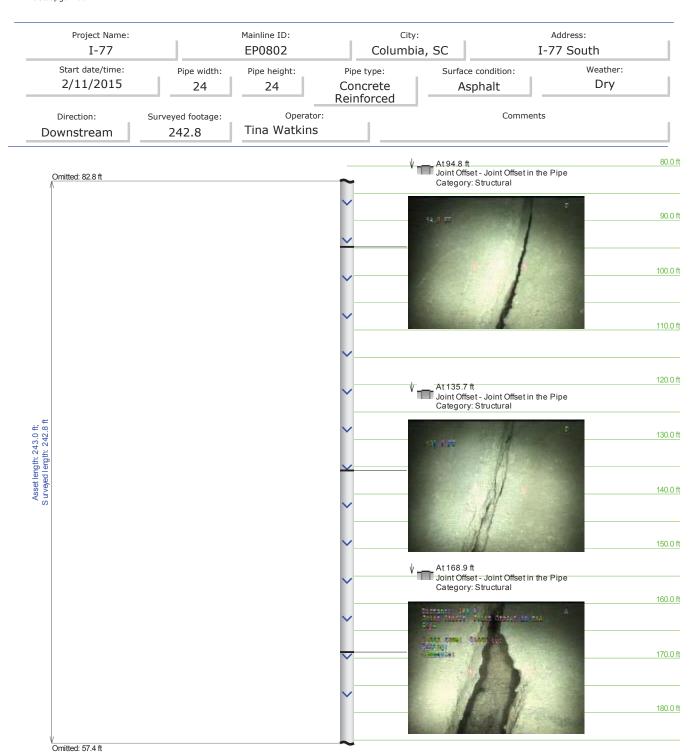
Omitted: 5.1 ft

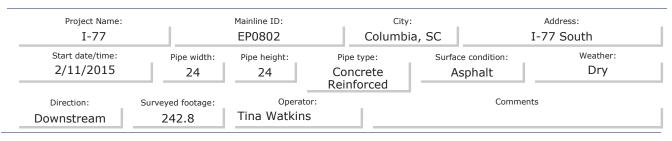
170.0 ft

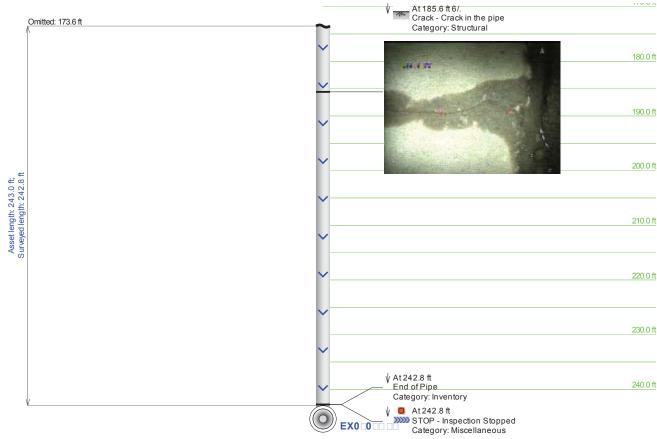


www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 EP0802 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 2/11/2015 Dry 24 24 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 242.8 At 0.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous EX0₀









www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP0803 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/19/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 199.2 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous 10.0 ft 20.0 ft 30.0 ft 40.0 ft 50.0 ft At 102.7 ft Joint Offset - Joint Offset in the Pipe Category: Structural 60.0 ft 70.0 ft

Omitted: 44.7 ft

80.0 ft

90.0 ft

100.0 ft

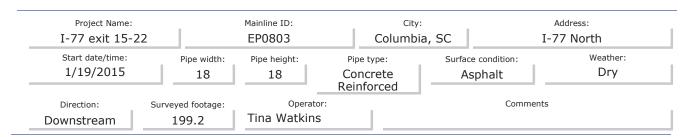
110.0 ft

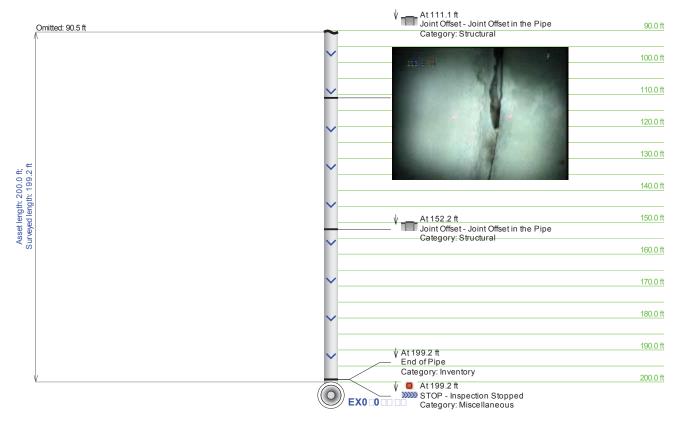
120.0 ft

130.0 ft

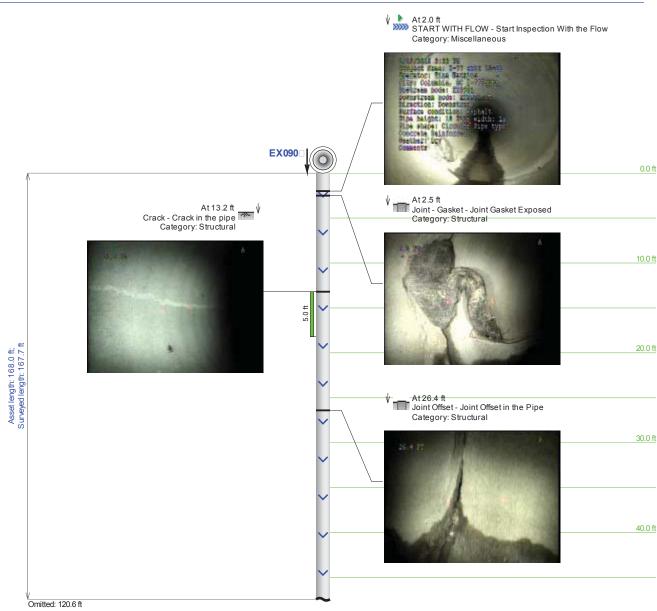
140.0 ft

150.0 ft



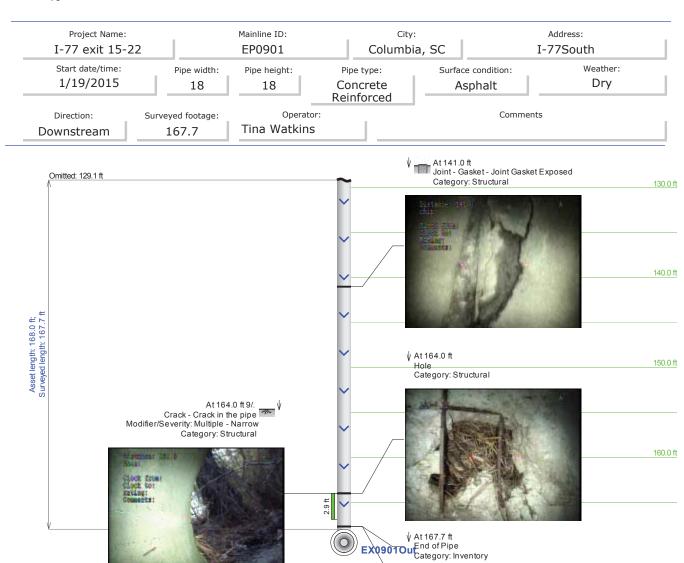


Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP0901 Columbia, SC I-77South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/19/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 167.7





EX0901Out



At 167.7 ft
STOP - Inspection Stopped
Category: Miscellaneous



AET Services

Robotic Inspections

EC0906

EC0906 Culvert 240'

Cracking Throughout

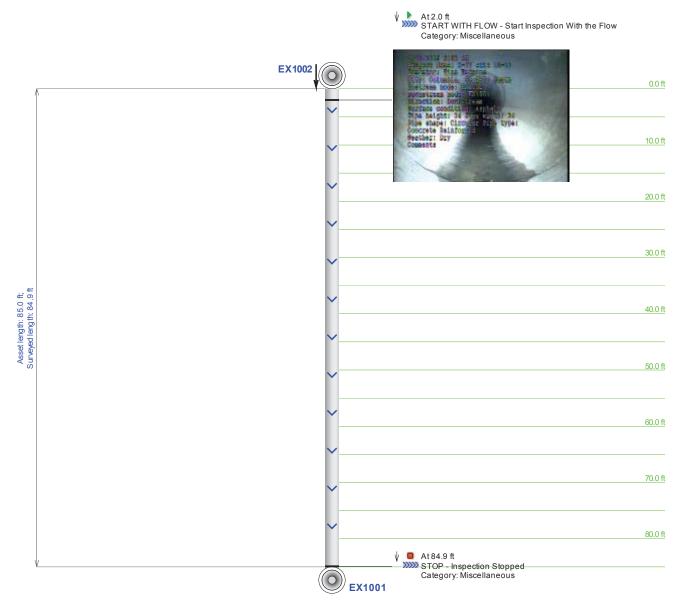
www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1002 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/20/2015 Dry 30 30 Concrete Other Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 367.3 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous At 56.1 ft 2/. Crack - Crack in the pipe Category: Structural 100.0 ft 200.0 ft 300.0 ft

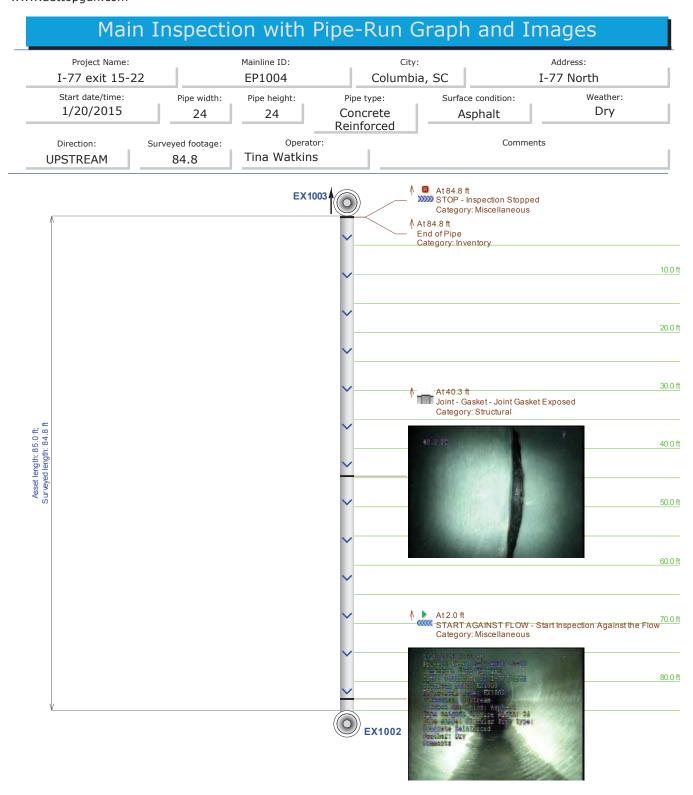
√ At 367.3 ft End of Pipe Category: Inventory

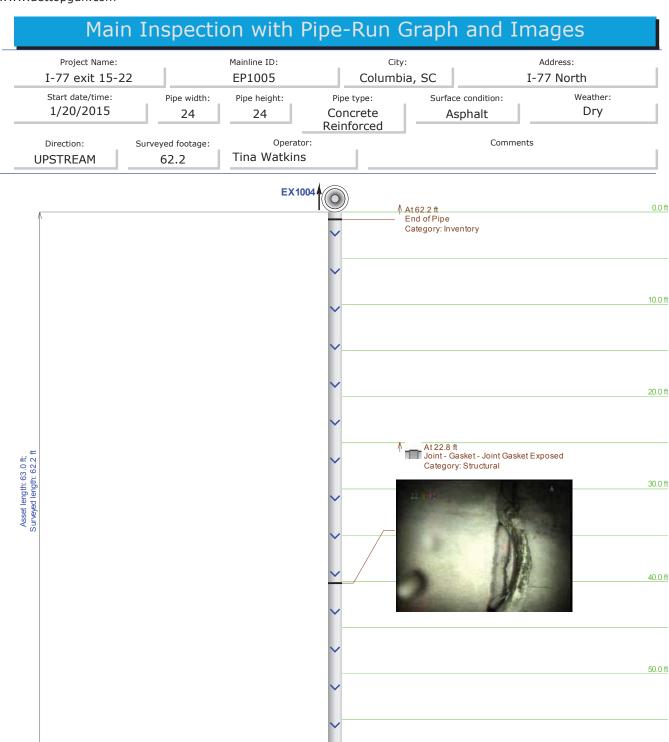
At 367.3 ft

STOP - Inspection Stopped
Category: Miscellaneous

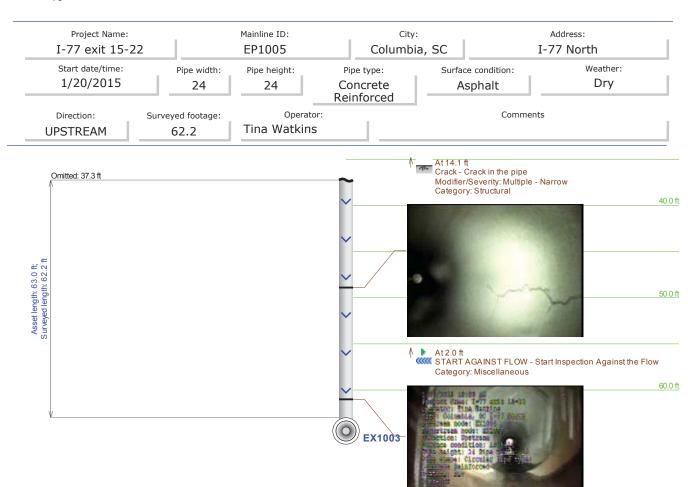
Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1003 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/20/2015 Dry 24 24 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 84.9

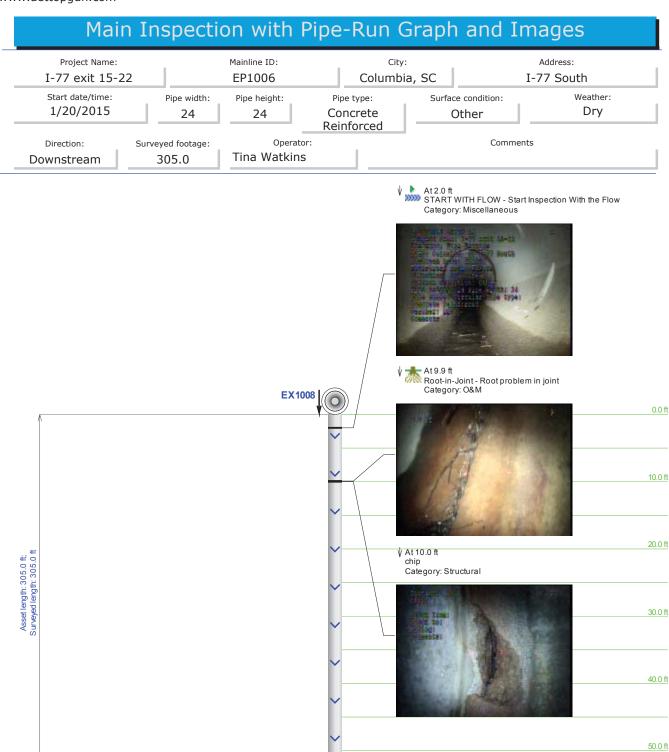




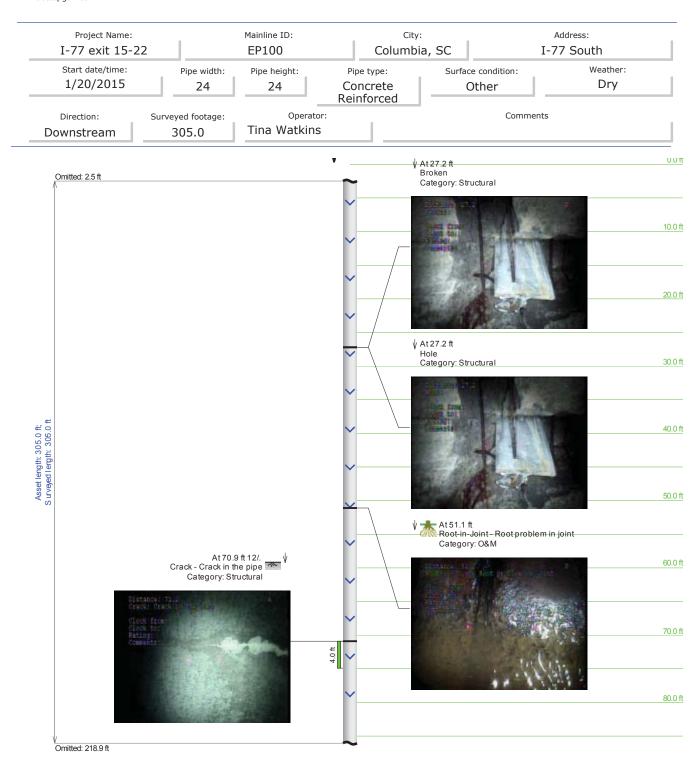


Omitted: 2.5 ft



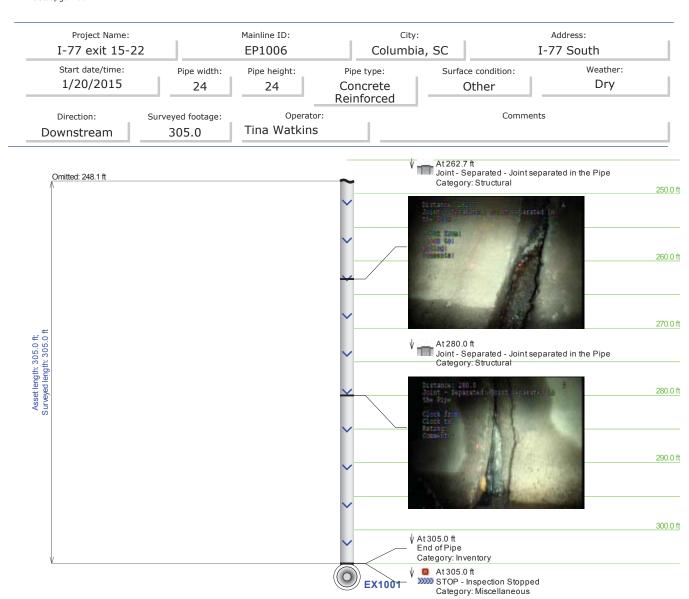


Omitted: 252.1 ft

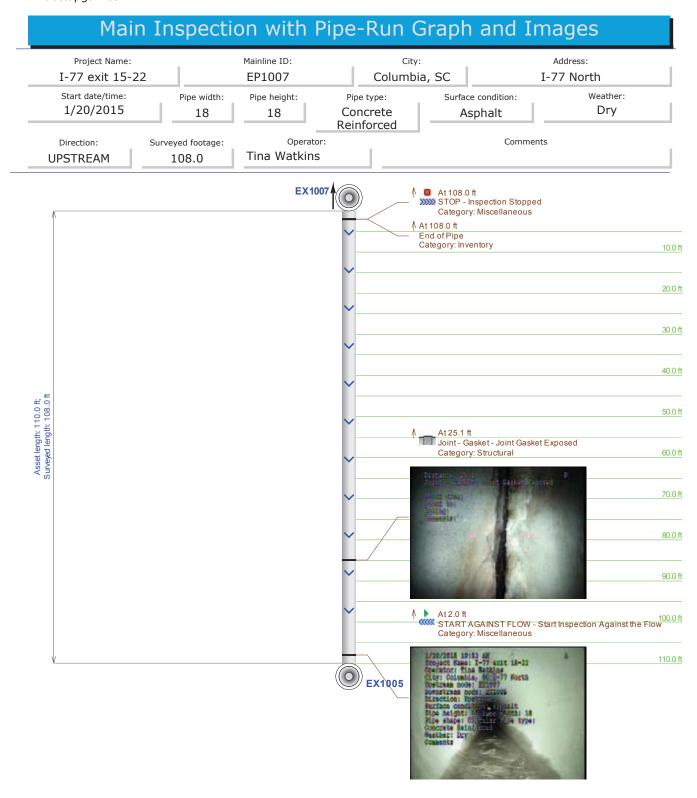




Omitted: 136.2 ft



EX1001



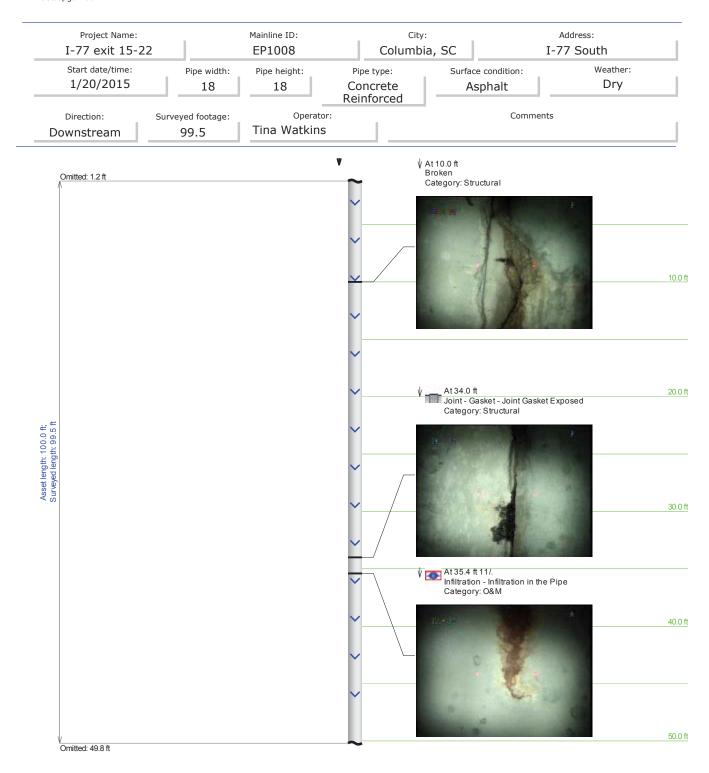
www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1008 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/20/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 99.5 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous At 6.1 ft 3/. Crack - Crack in the pipe EX1005 Category: Structural 10.0 ft

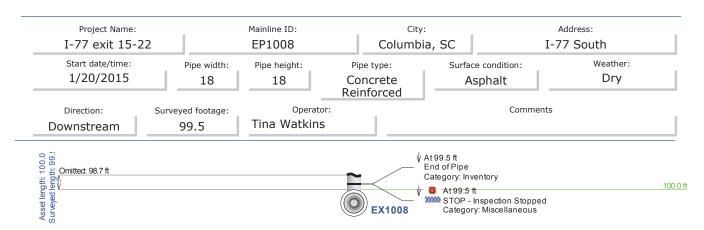
Omitted: 68.5 ft

At 6.1 ft 9/.
Crack - Crack in the pipe
Category: Structural

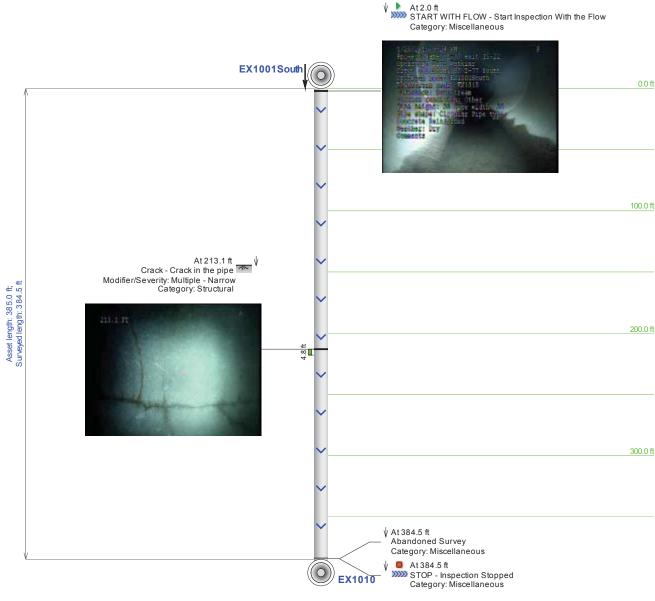
20.0 ft

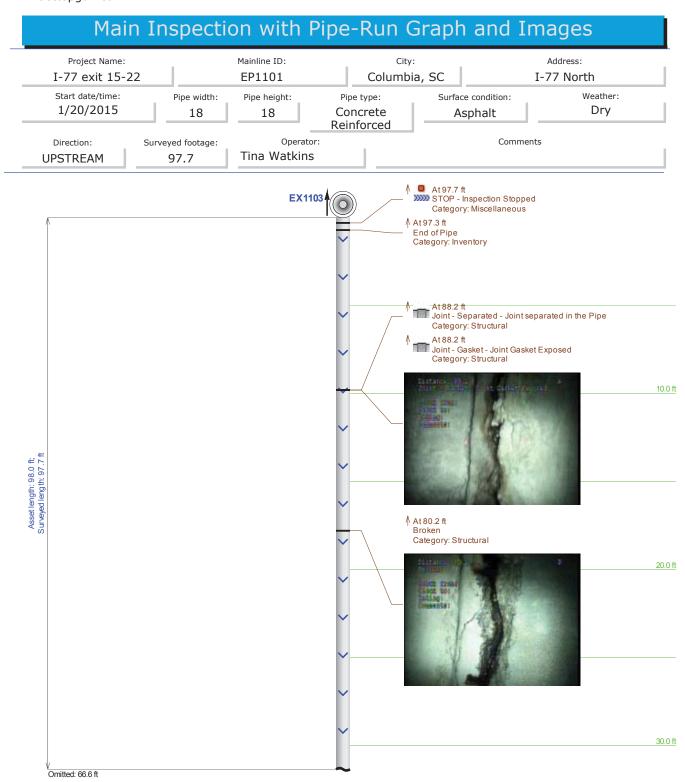
30.0 ft

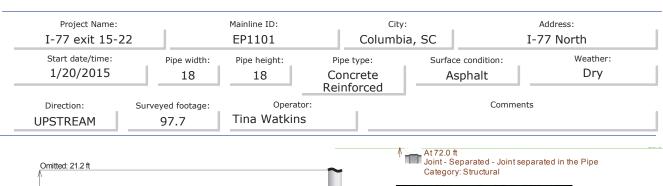


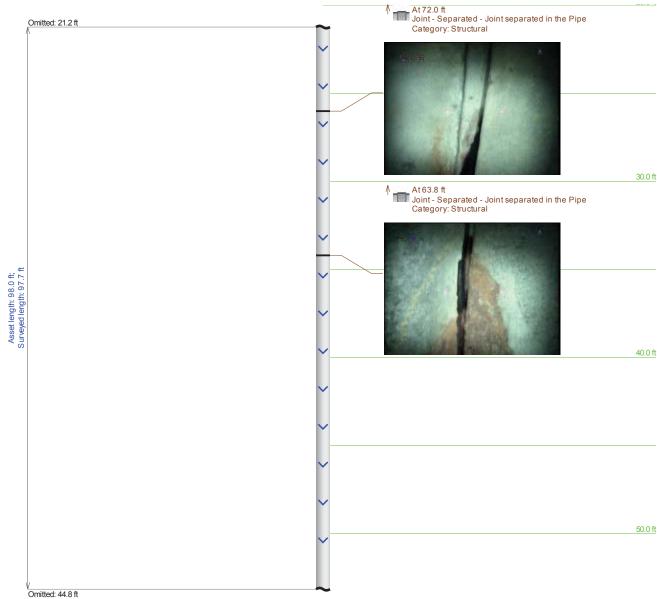


Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1010 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/20/2015 Dry 30 30 Concrete Other Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 384.5



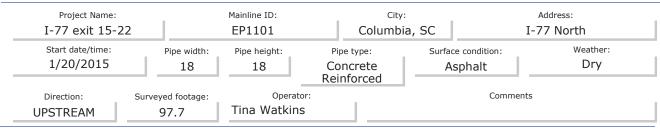




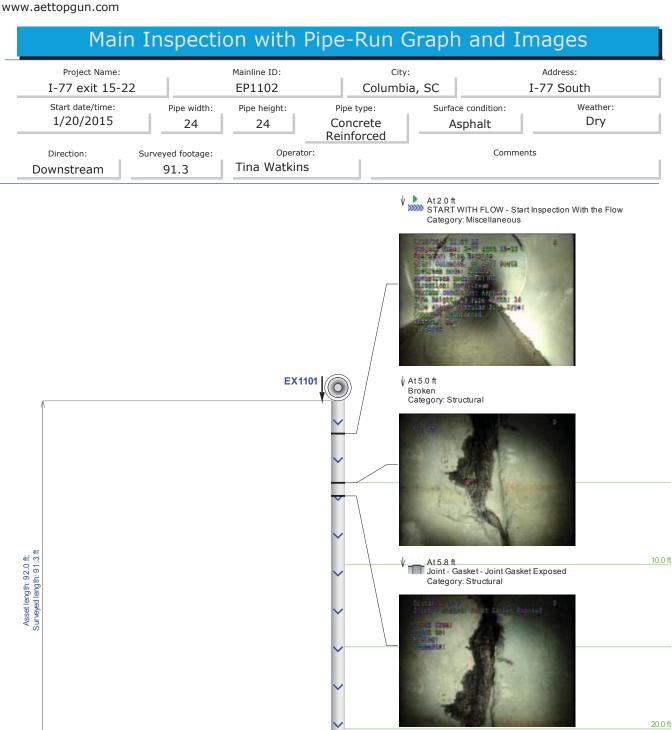




Omitted: 12.2 ft

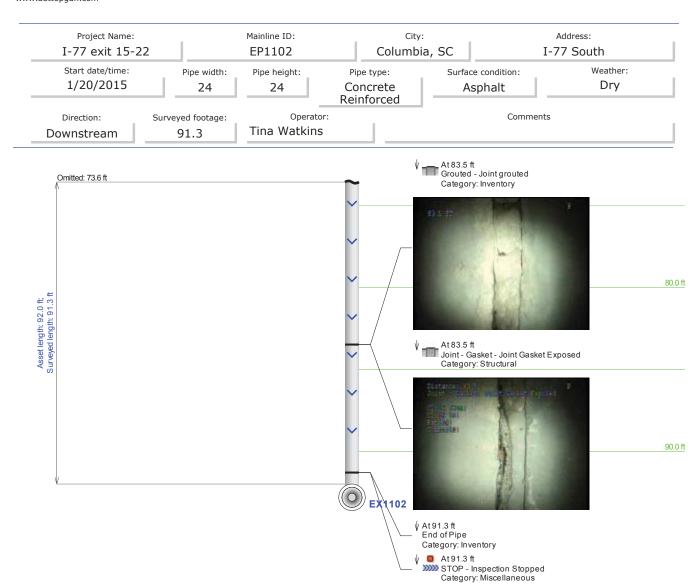


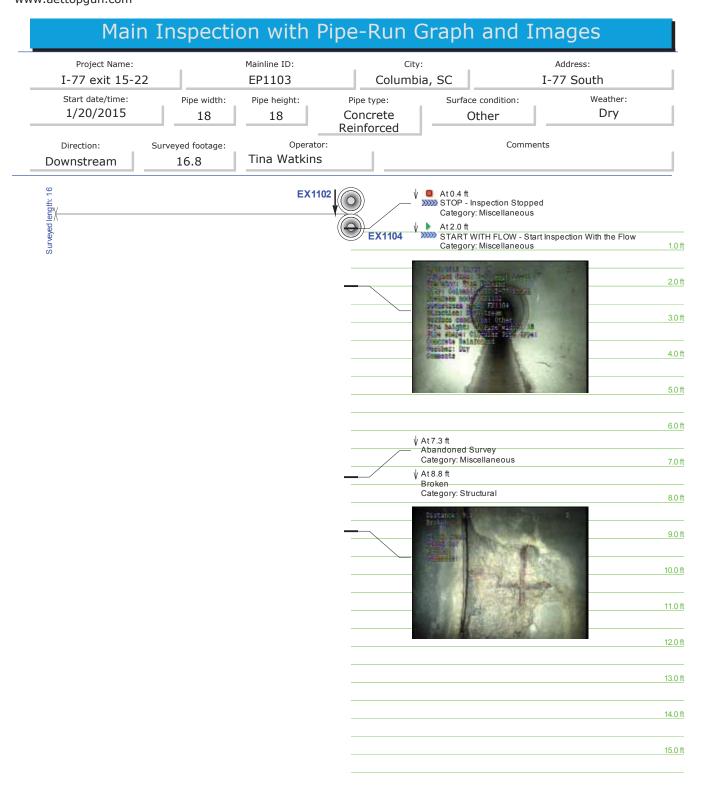




Omitted: 68.9 ft

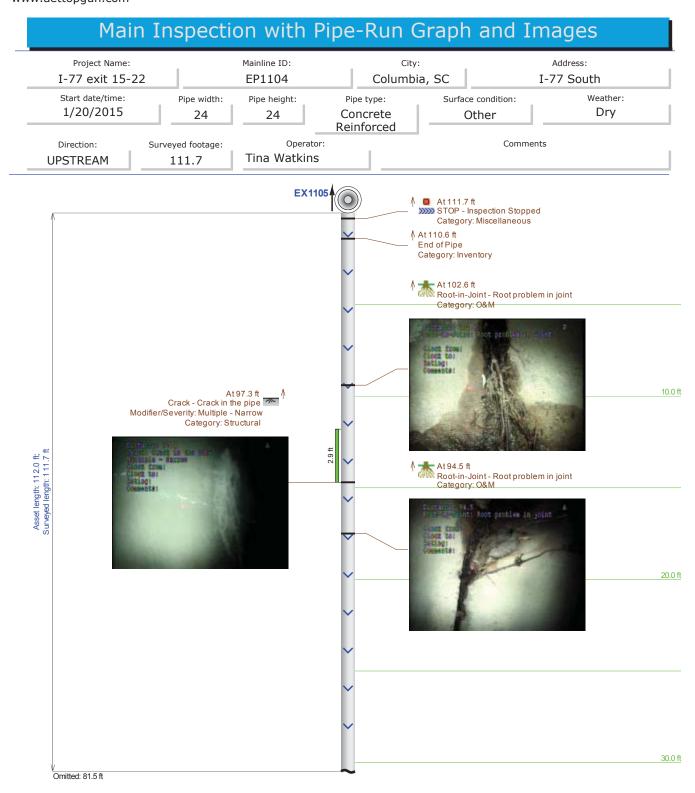


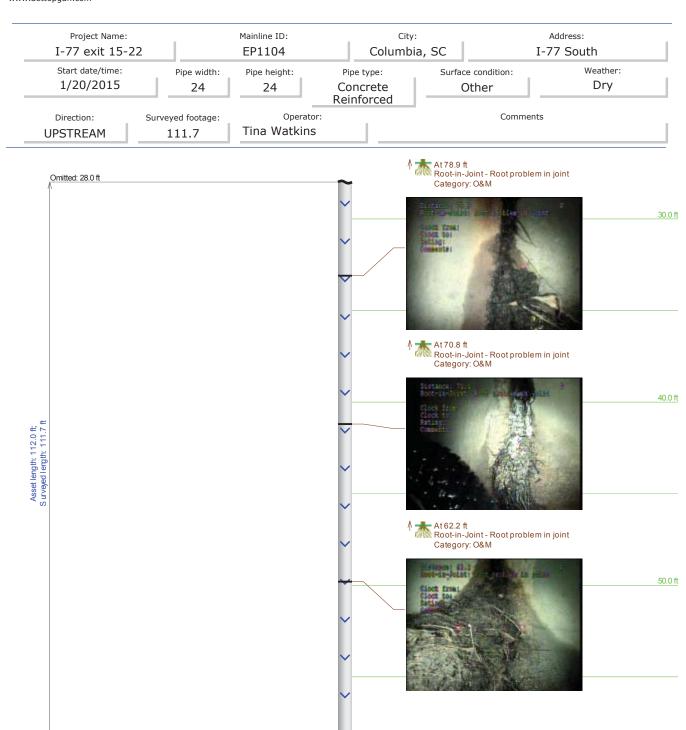




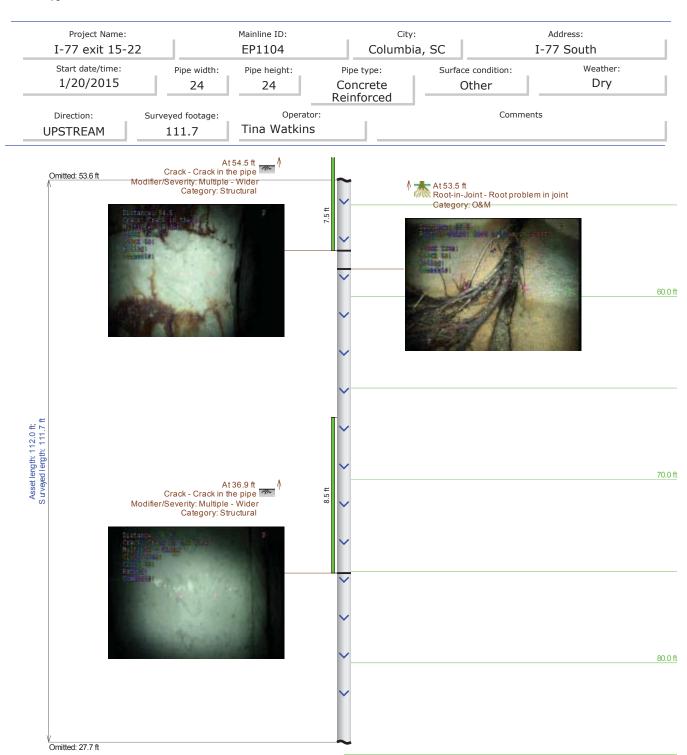
Project Name: I-77 exit 15-22		Mainline ID: EP1103	· · · · · · · · · · · · · · · · · · ·	City: Columbia, SC		Address: I-77 South	
Start date/time: 1/20/2015	Pipe width:	Pipe height:	Pipe type: Concrete Reinforced		ce condition: Other	Weather: Dry	
Direction: Downstream	Surveyed footage: 16.8	Operato Tina Watkins		Comments			

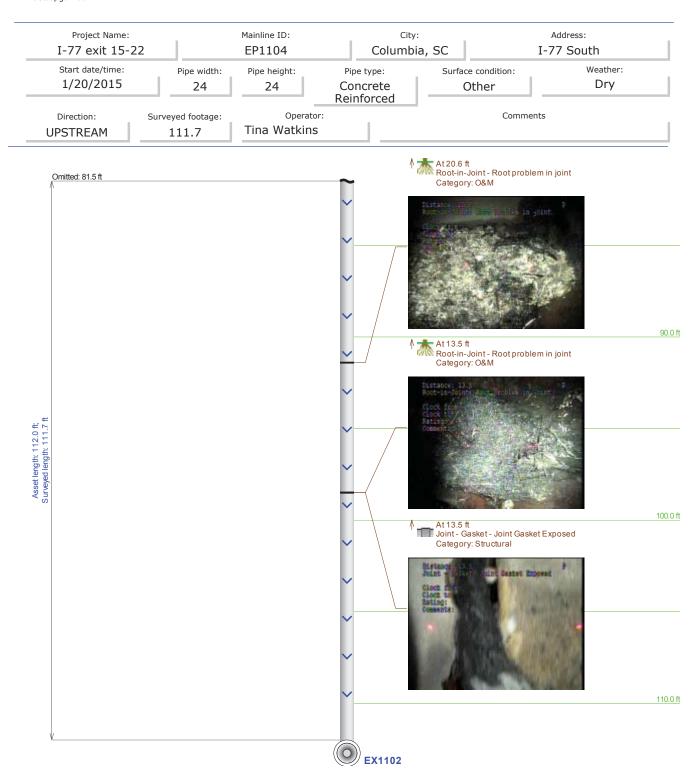


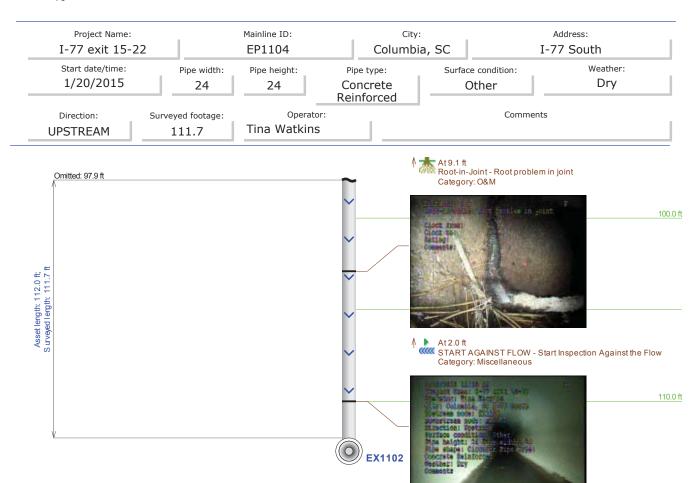


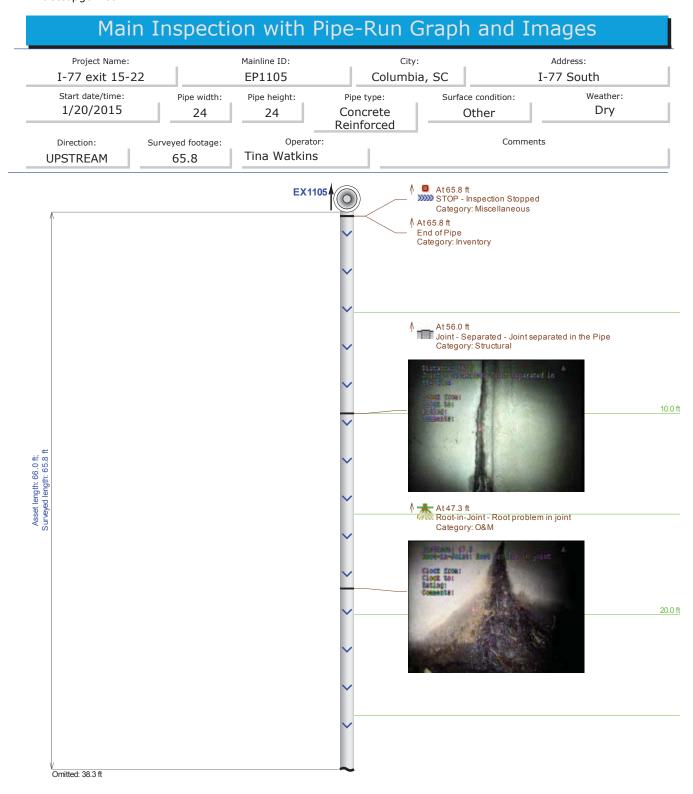


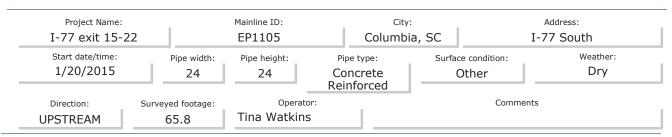
Omitted: 53.3 ft



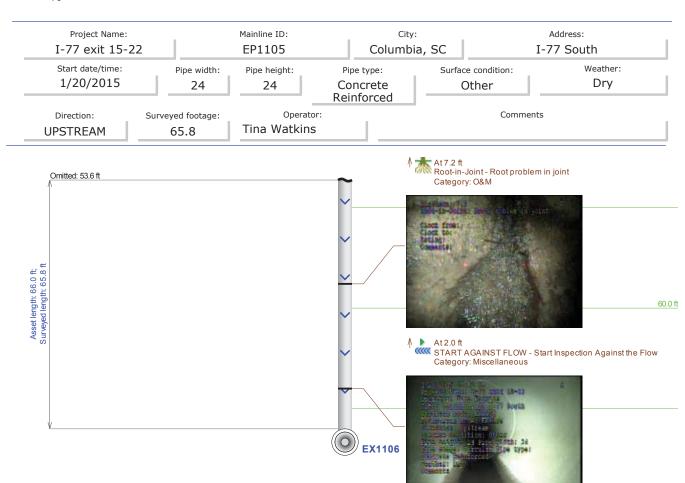


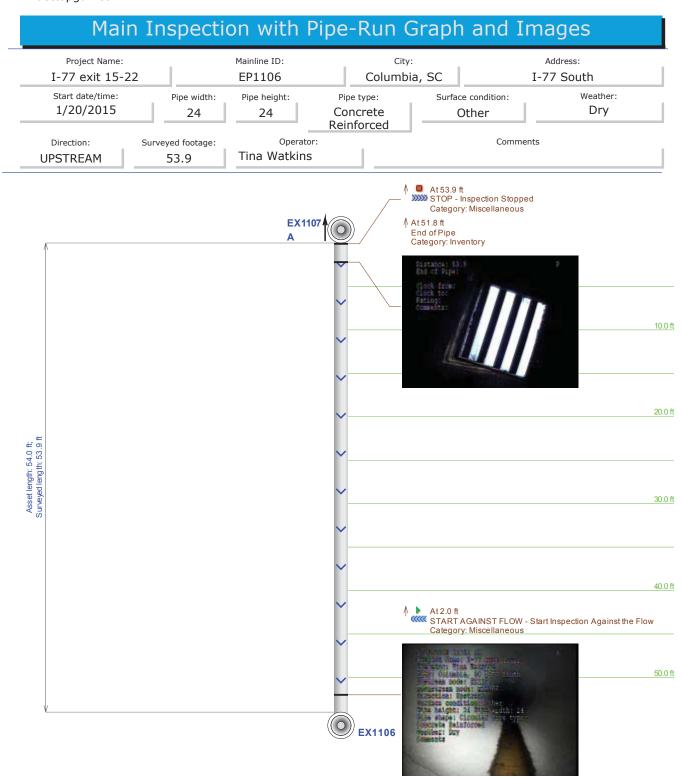


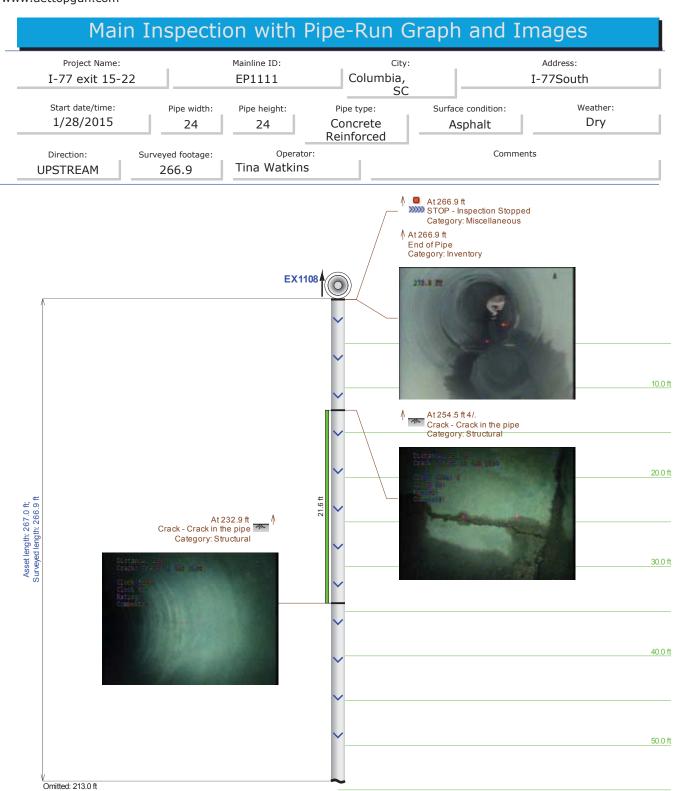


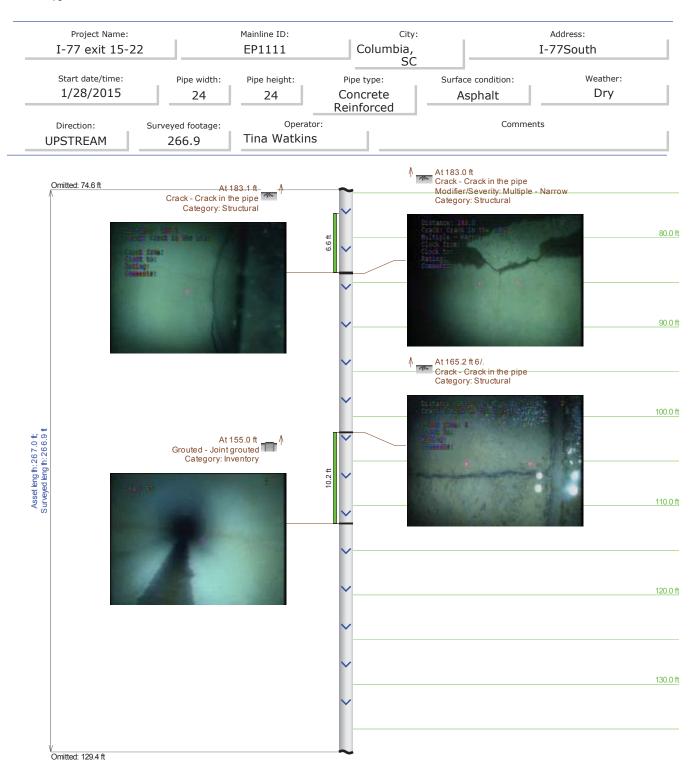


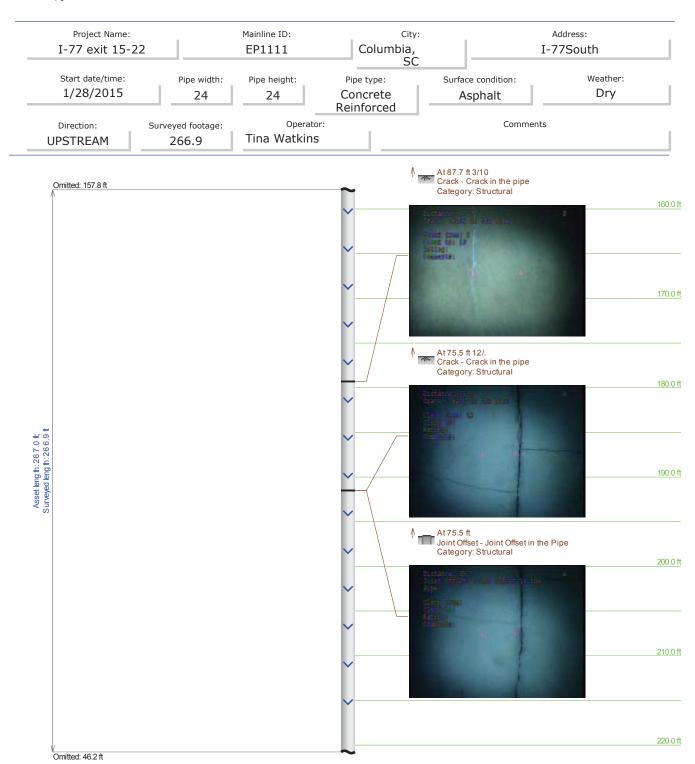


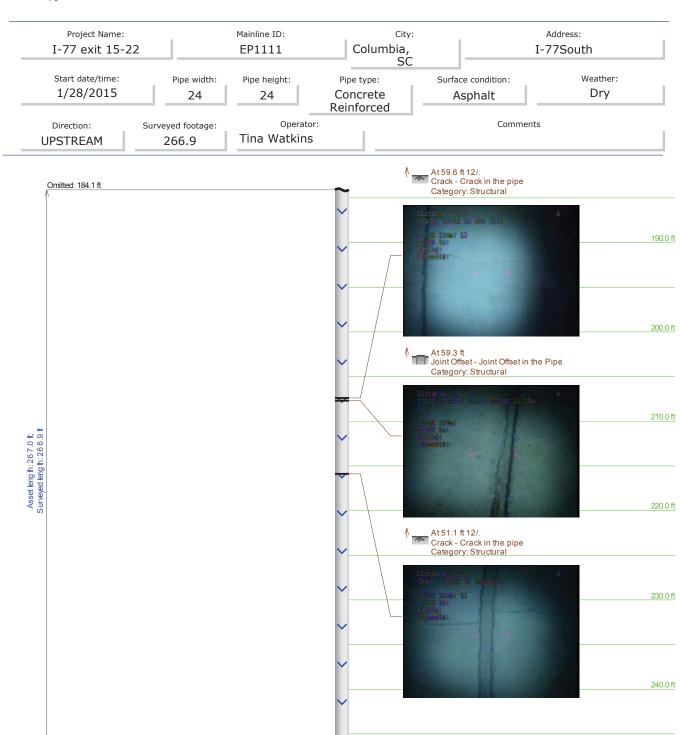












Omitted: 20.0 ft



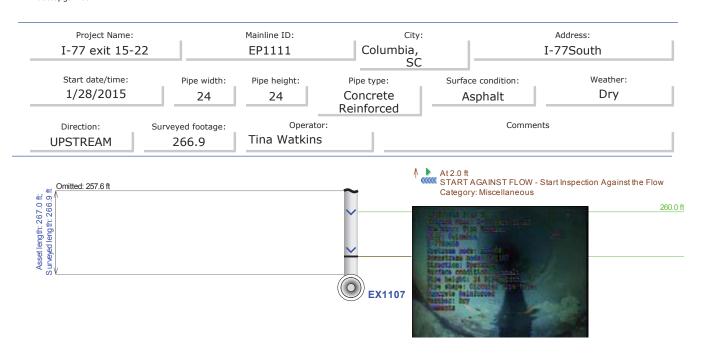
At 7.0 ft
Grouted - Joint grouted
Category: Inventory

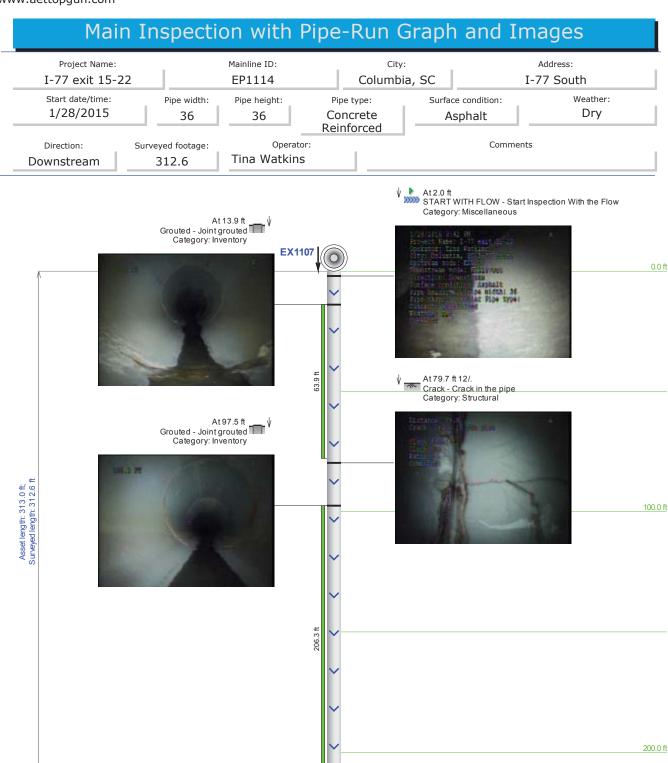
EX1107

At 2.3 ft
Grouted - Joint grouted
Category: Inventory

250.0 ft

260.0 ft





Omitted: 104.7 ft

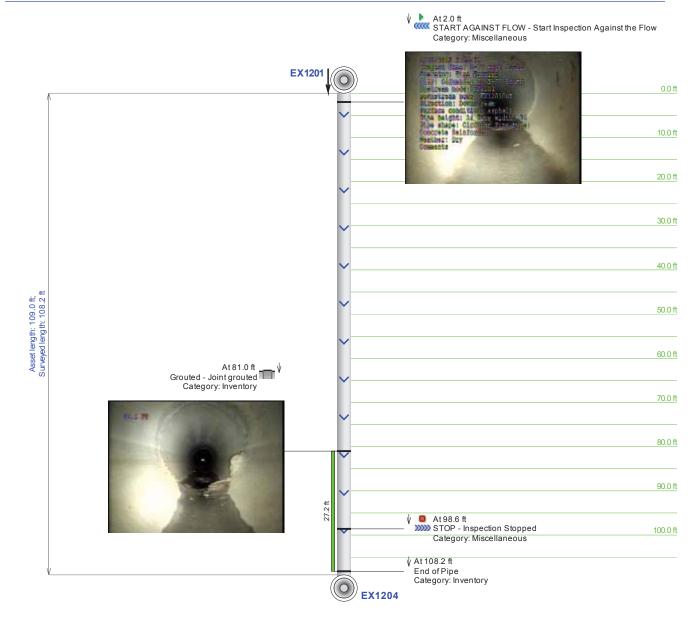


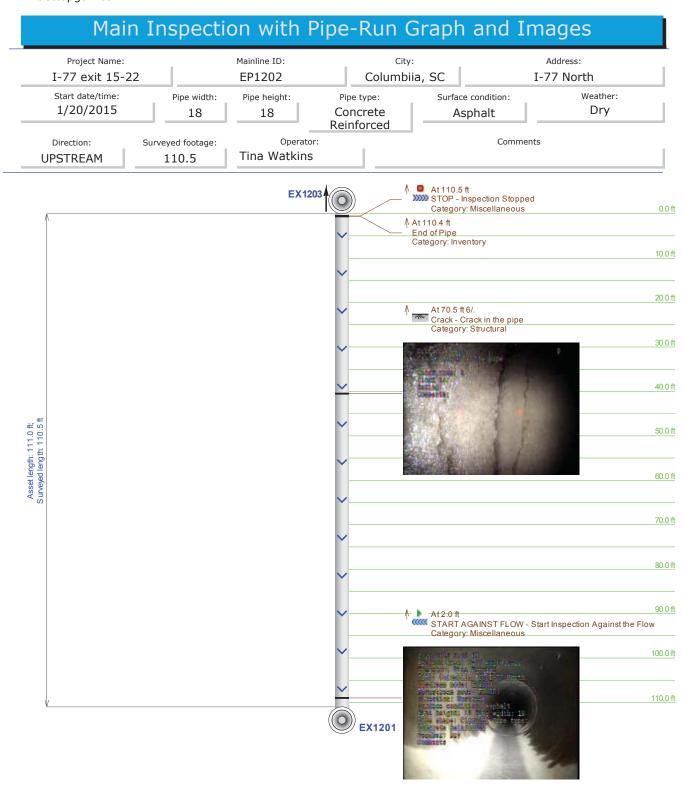
At 312.6 ft

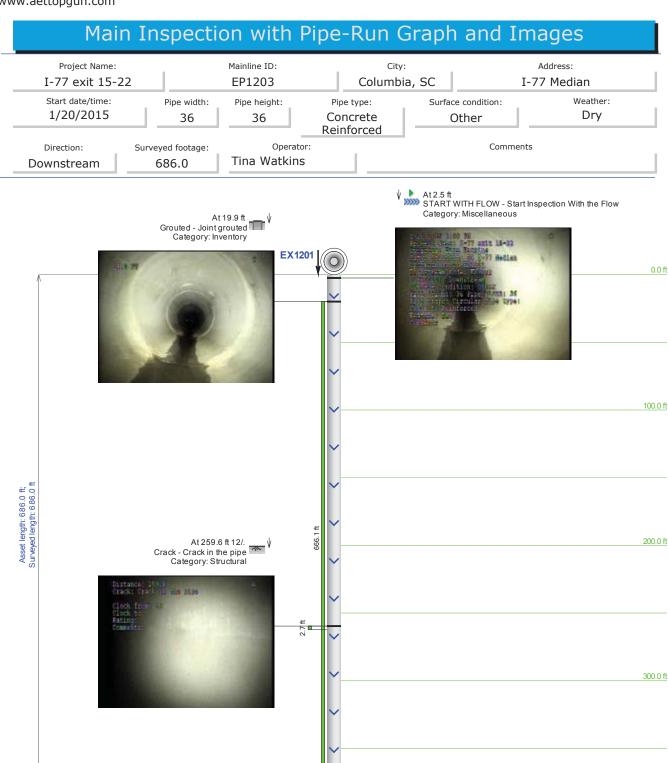
EX1107Outlunction Box

Category: Inventory

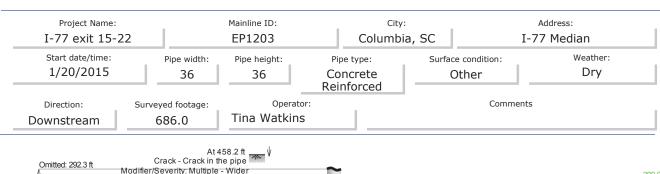
Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP1201 Columbia, SC I-77 South Weather: Start date/time: Pipe width: Pipe height: Surface condition: Pipe type: 1/20/2015 Dry 24 24 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 108.2

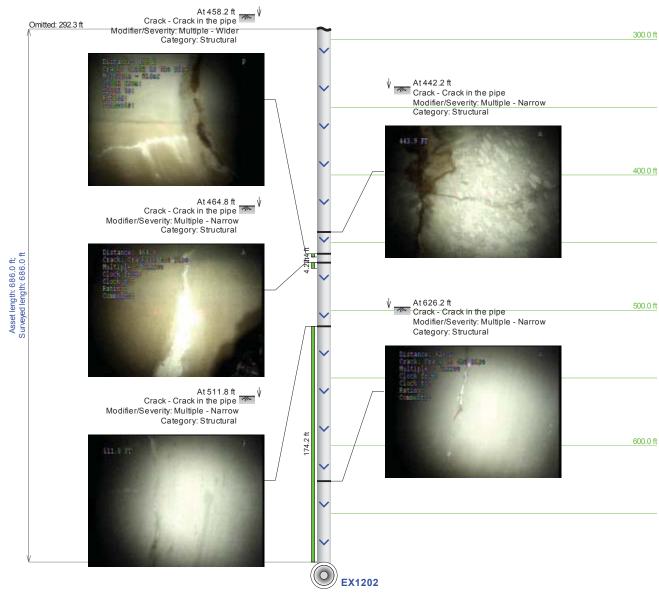


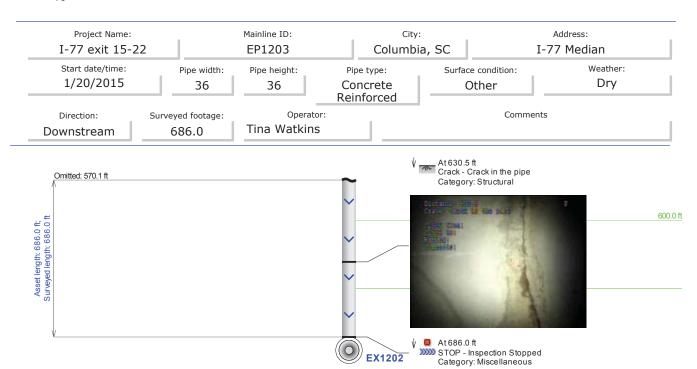


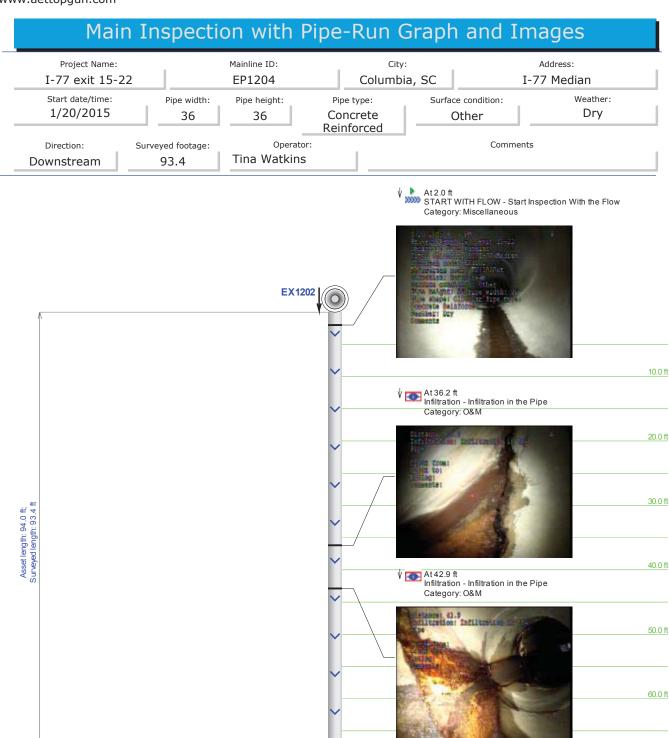


Omitted: 316.9 ft



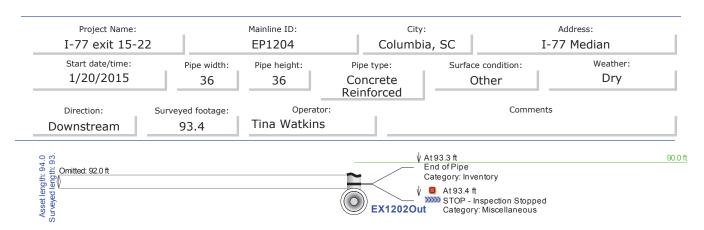


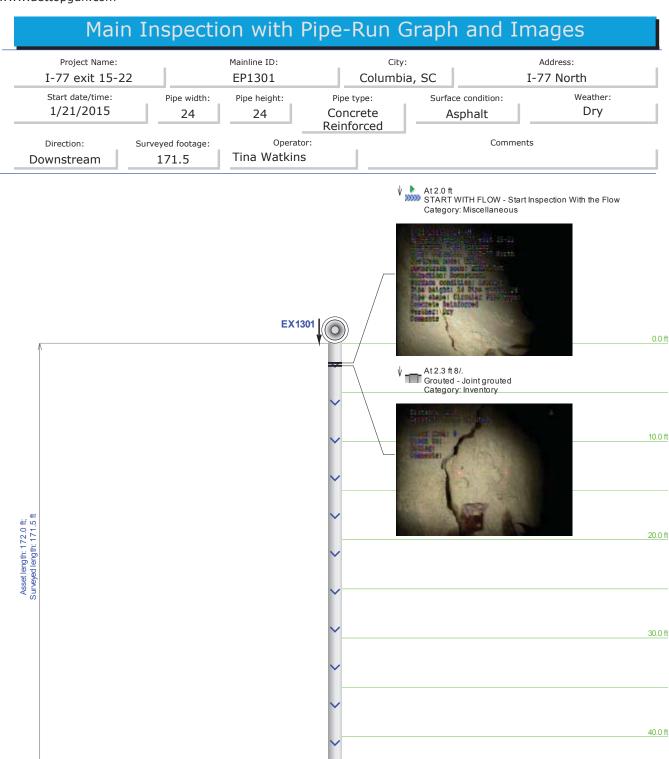




Omitted: 22.6 ft

70.0 ft





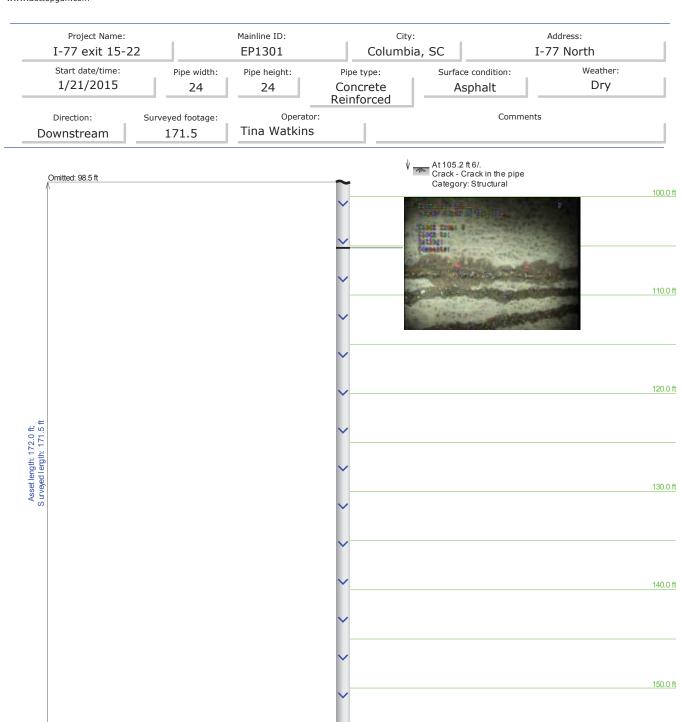
Omitted: 128.5 ft



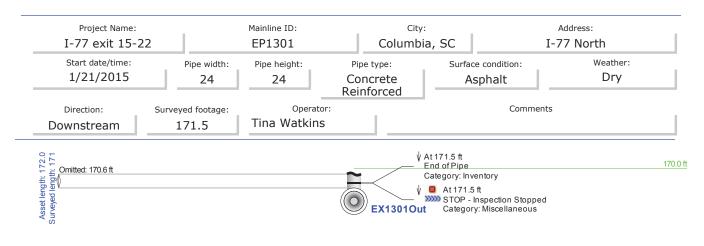
Omitted: 95.7 ft



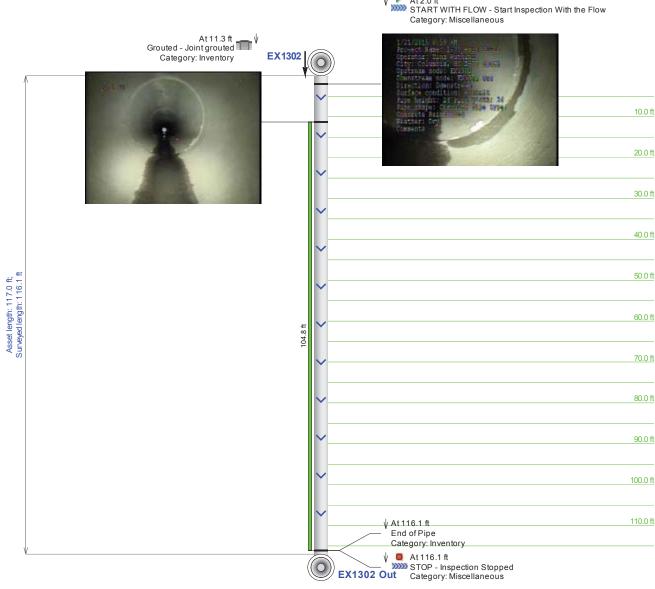
Omitted: 59.4 ft



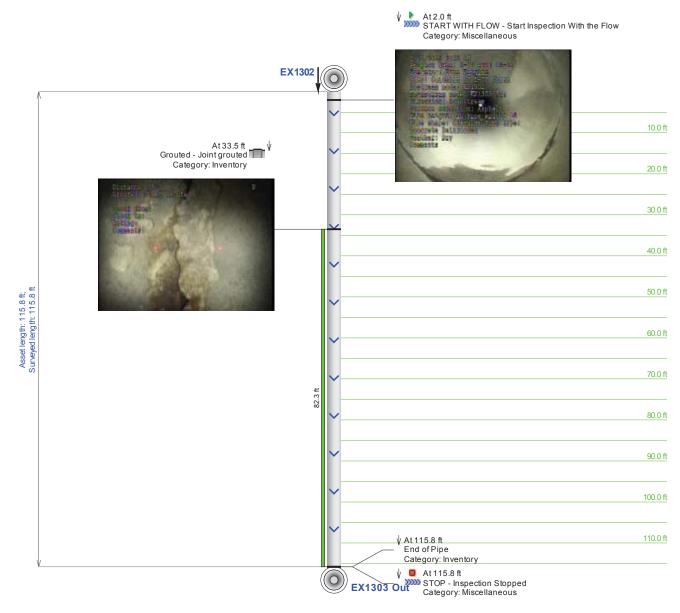
Omitted: 16.3 ft

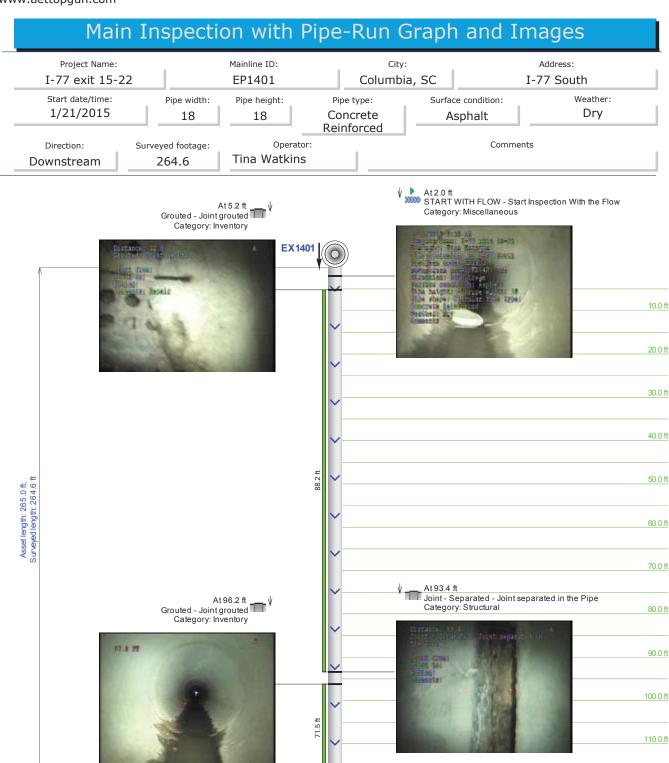


Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1302 Columbia, SC I-77 South Weather: Start date/time: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 24 24 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 116.1 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous

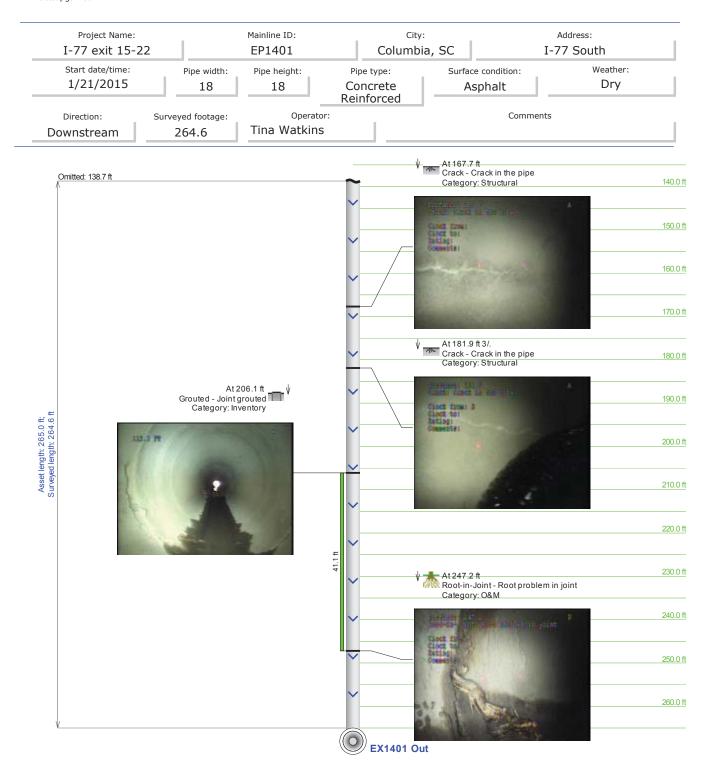


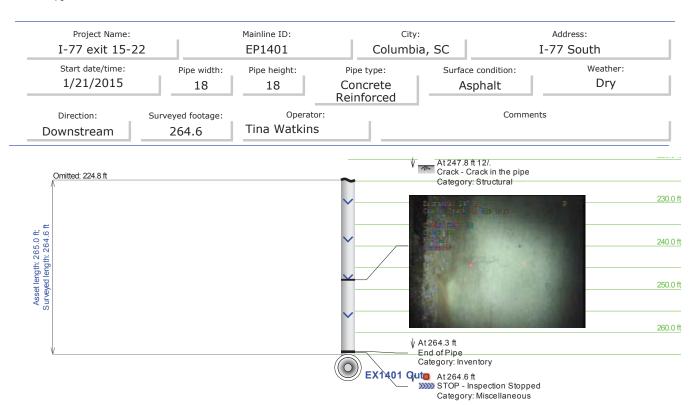
Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1303 Columbia SC I-77 North Weather: Start date/time: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 18 18 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 115.8

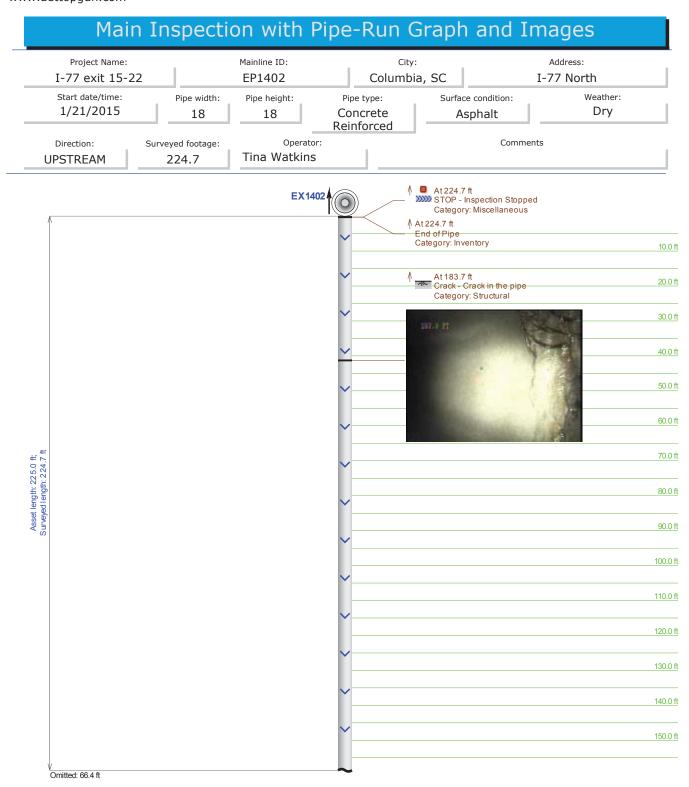


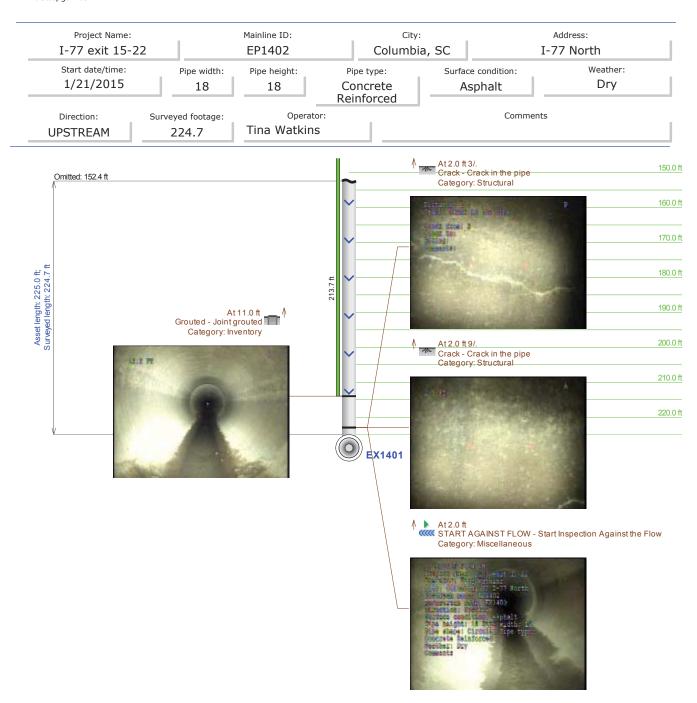


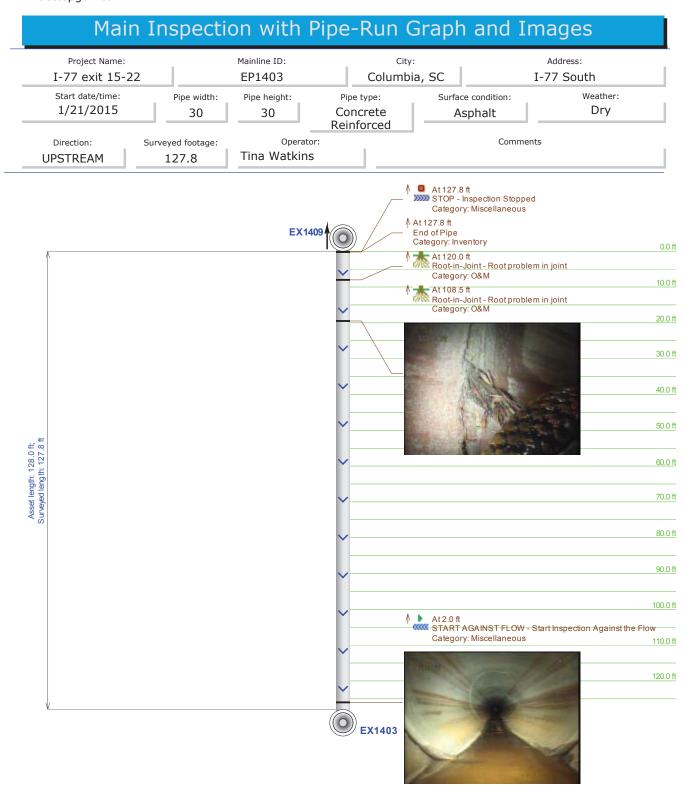
Omitted: 148.4 ft

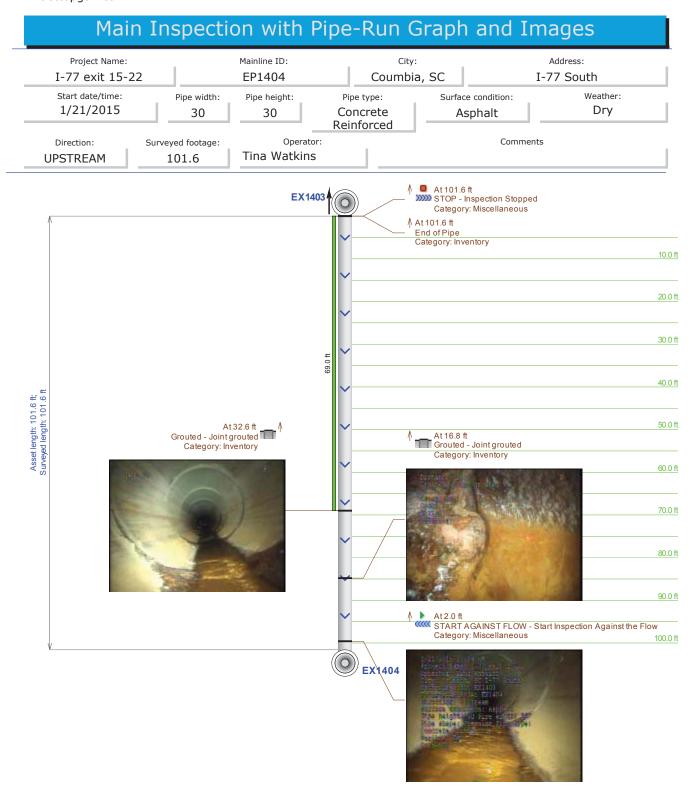


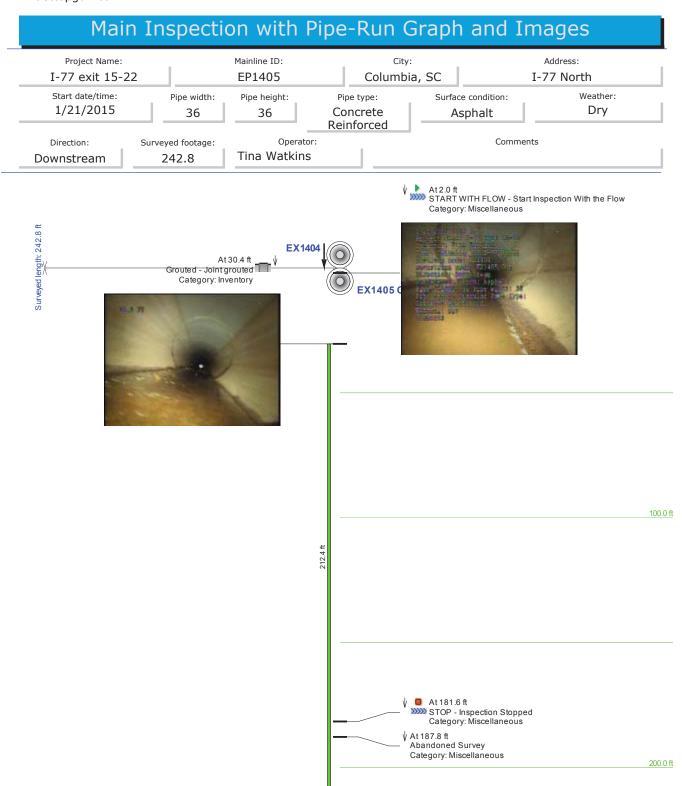


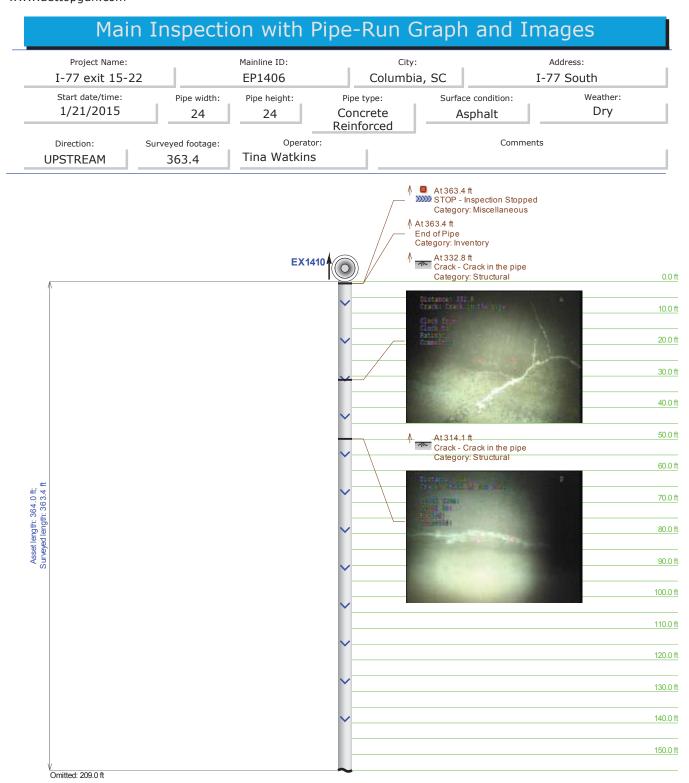


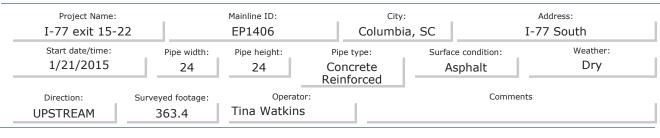


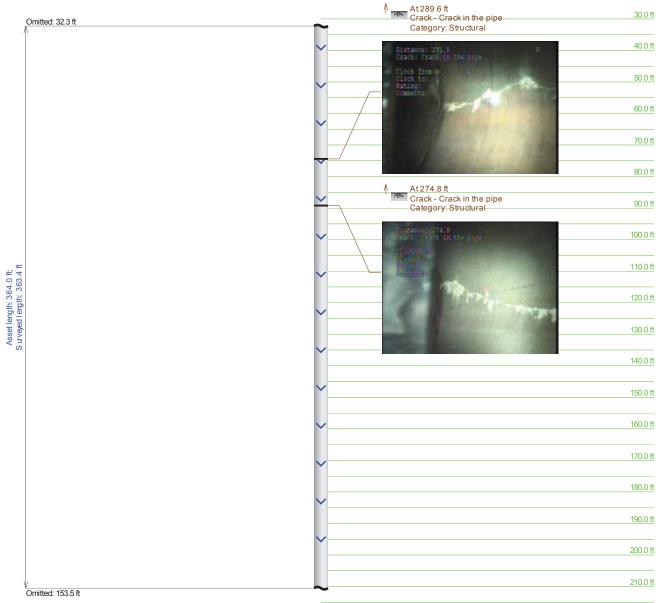


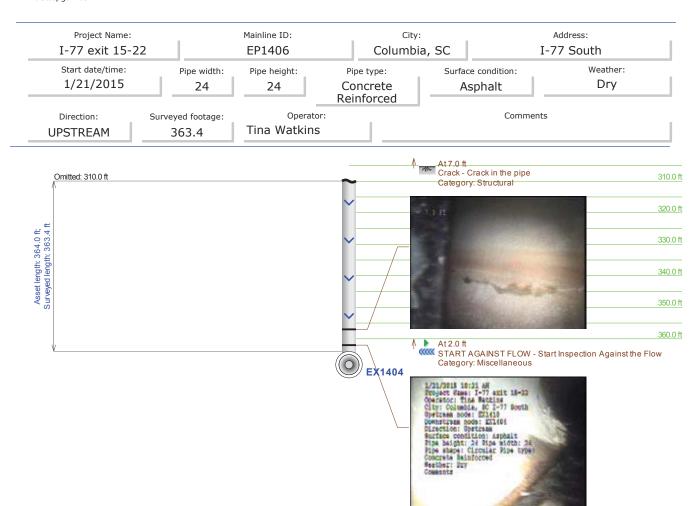














AET Services

Robotic Inspections

EC1407

EC1407 Culvert 440'

Cracking Throughout

Grate uptop 18" pipe Skewed left 135'

Rebar Showing 290'

Floor Crack 394'

www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 EP1408 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 2/11/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 180.5 At 0.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous EX1408 At 14.0 ft Infiltration - Infiltration in the Pipe Category: O&M Joint Separated - Major - Greater Than 1 Category: Structural 20.0 27 10.0 ft 20.0 ft 30.0 ft At 48.5 ft √ Crack - Major - Greater 12" Category: Structural 40.0 ft

Omitted: 105.1 ft

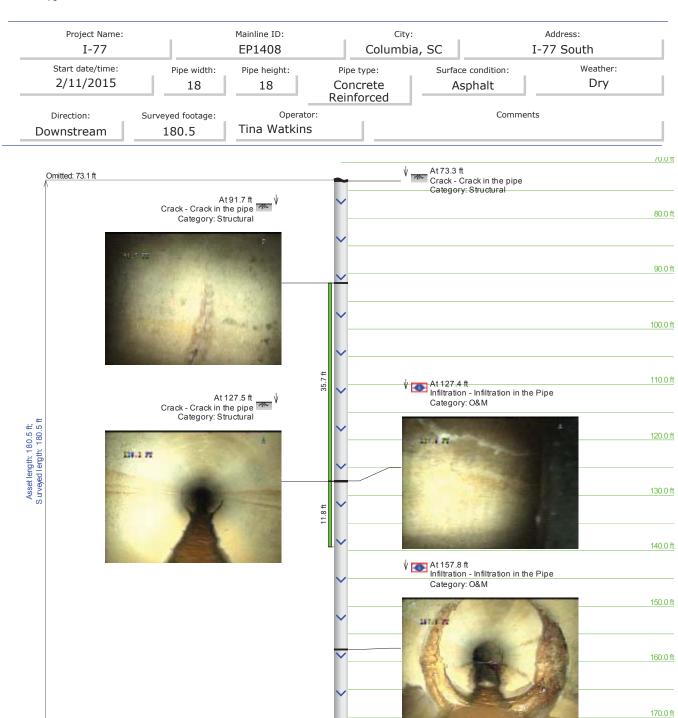
√ At 57.6 ft Crack -Major - Greater 12"

Category: Structural

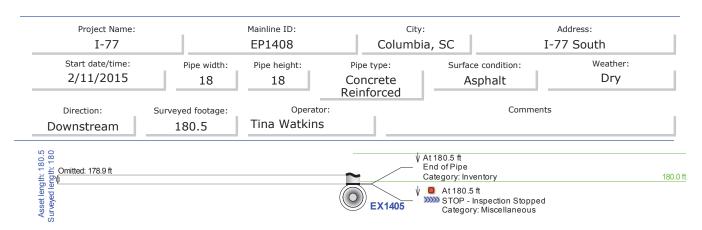
50.0 ft

60.0 ft

70.0 ft

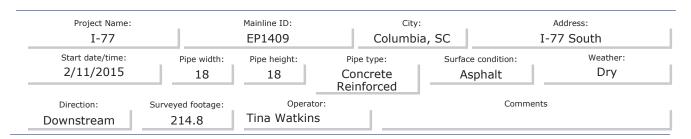


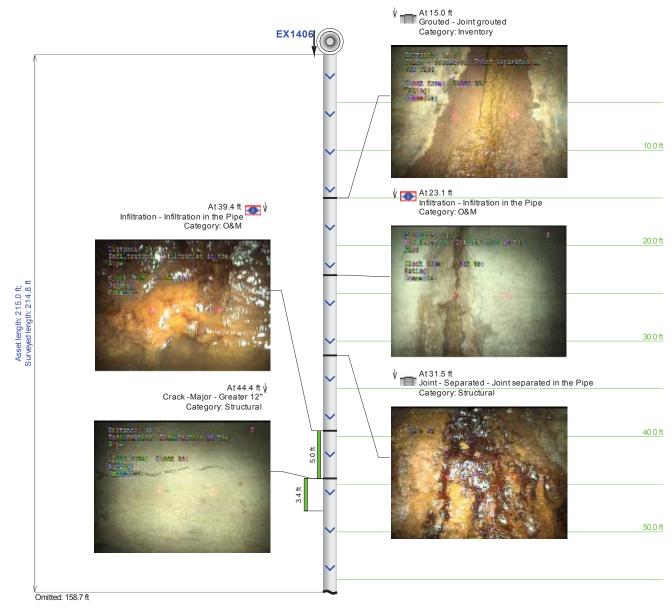
Omitted: 6.0 ft

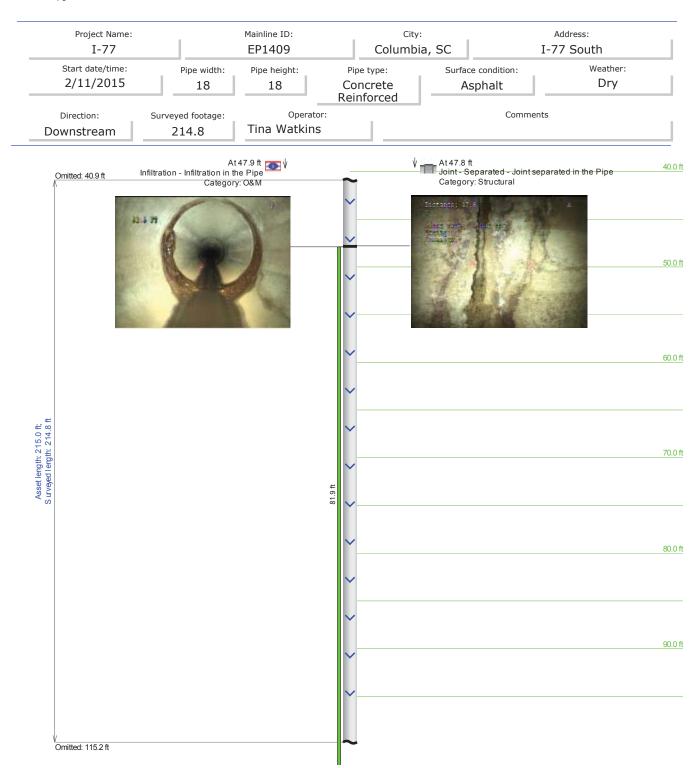


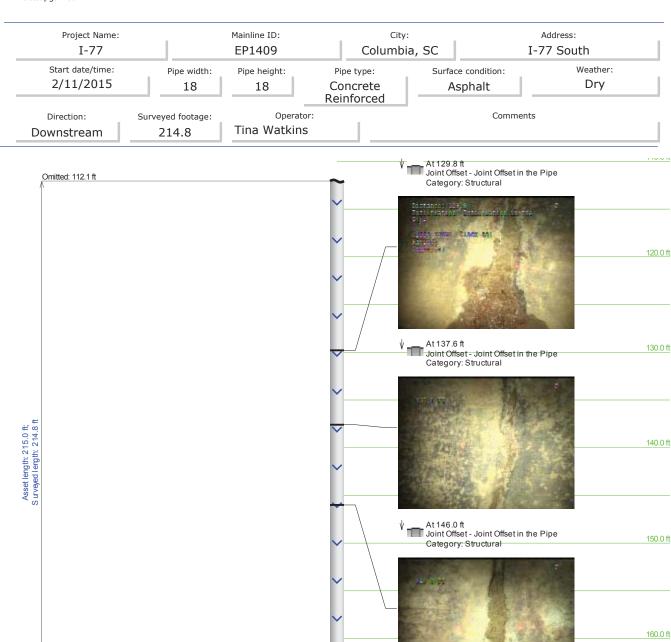


Omitted: 174.4 ft



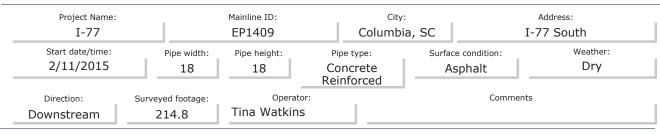


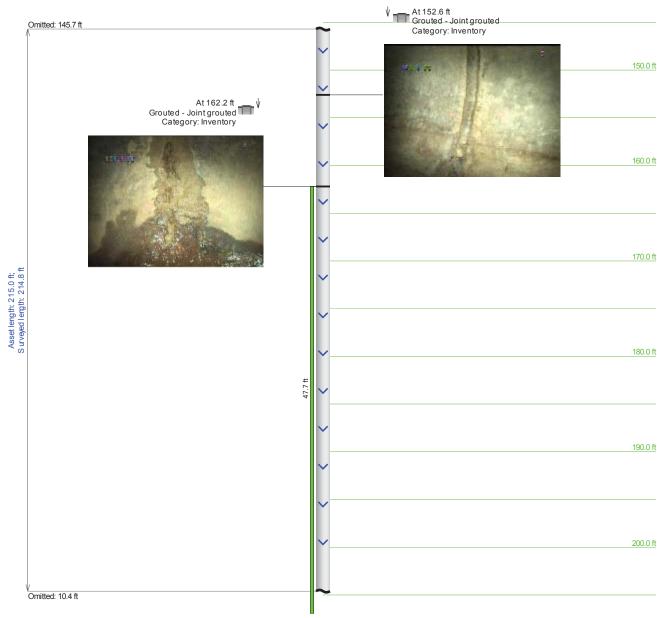


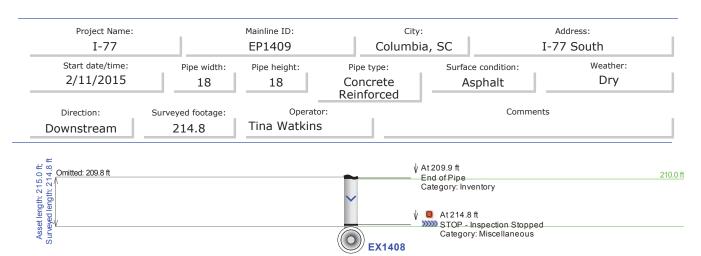


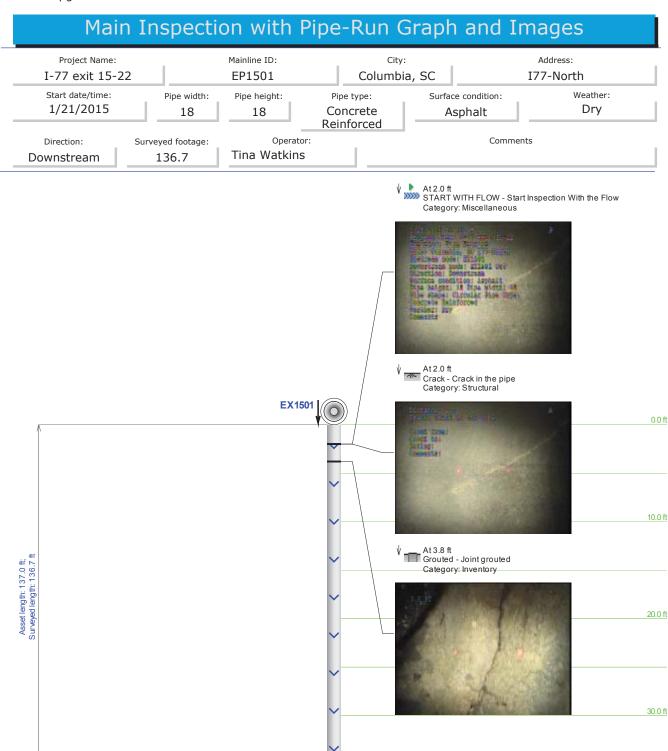
Omitted: 44.0 ft

170.0 ft

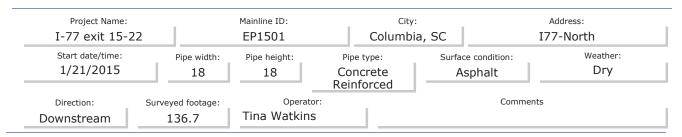


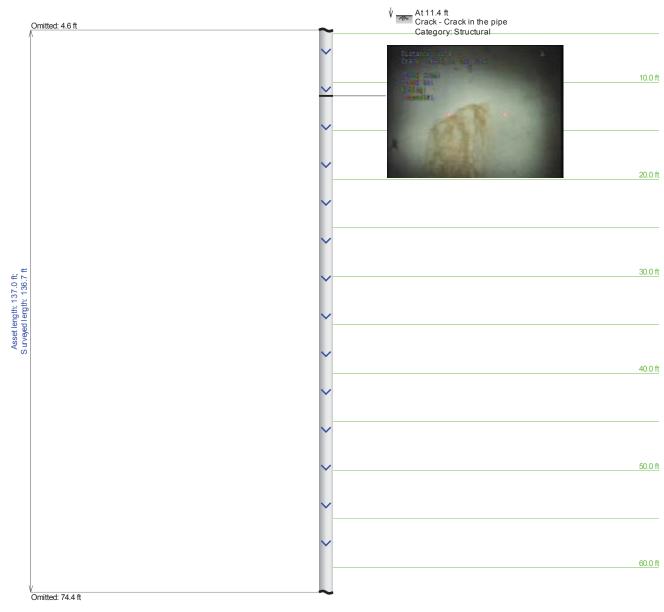


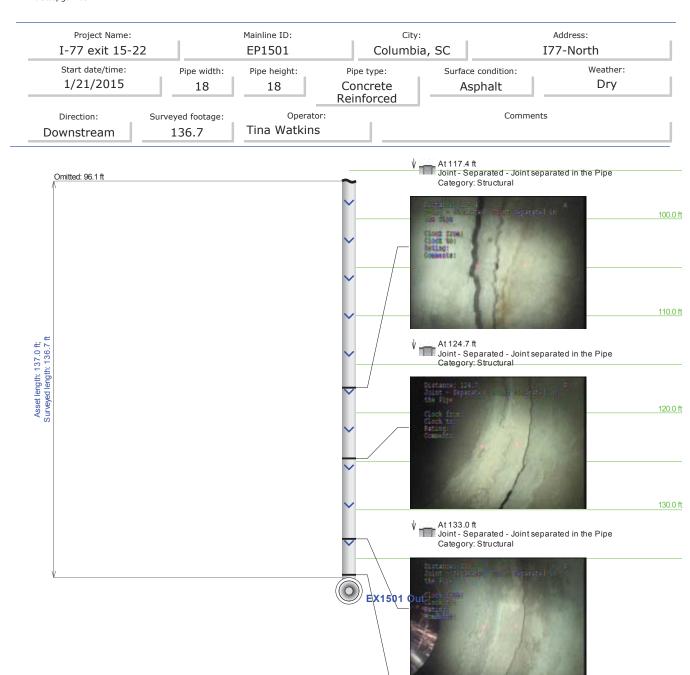




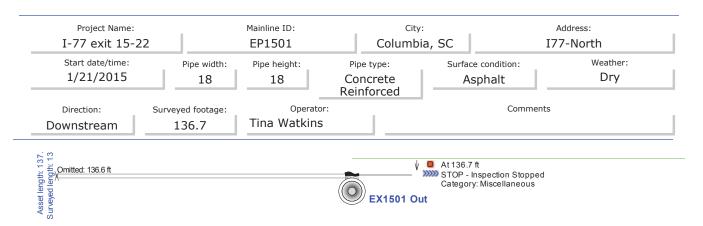
Omitted: 101.6 ft

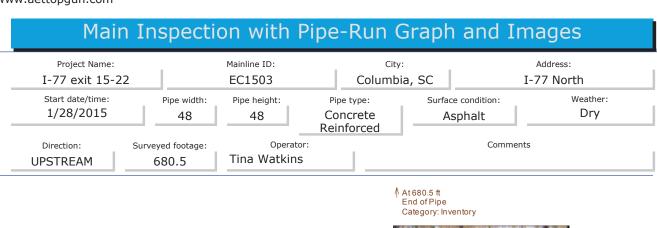


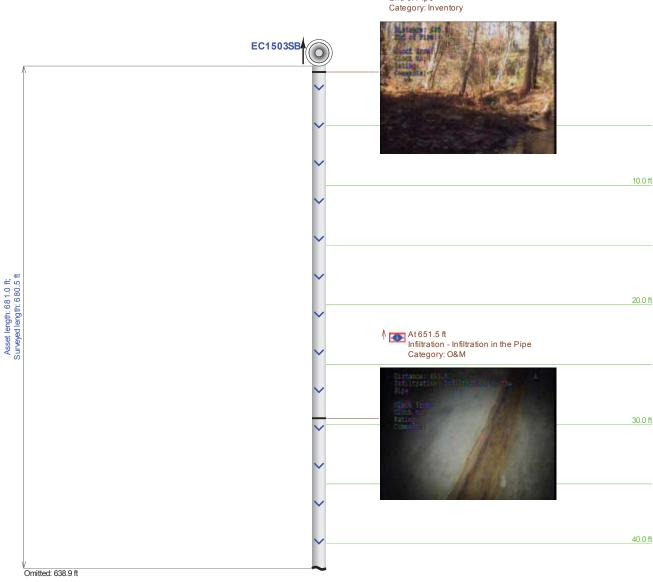


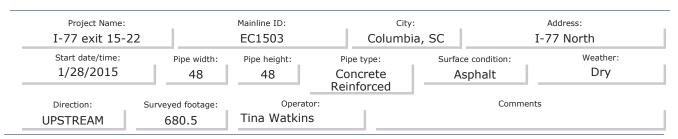


√ At 136.7 ft End of Pipe Category: Inventory

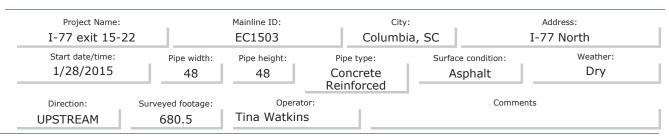


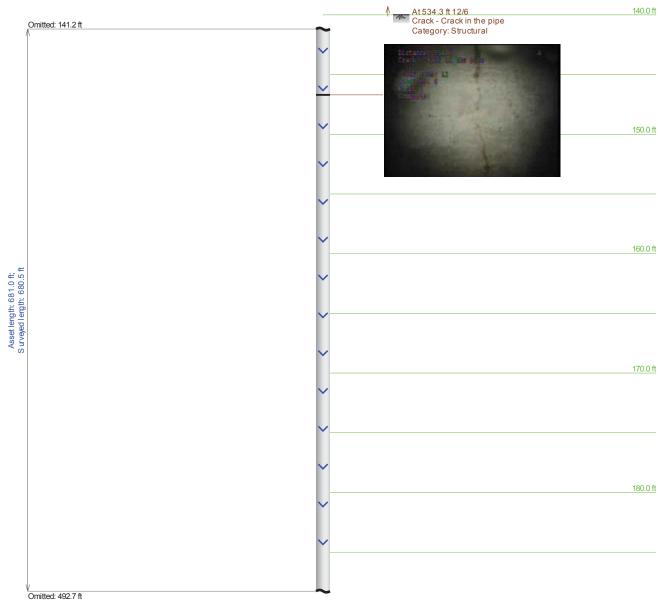


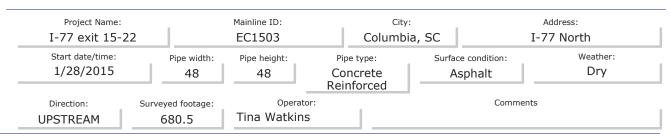


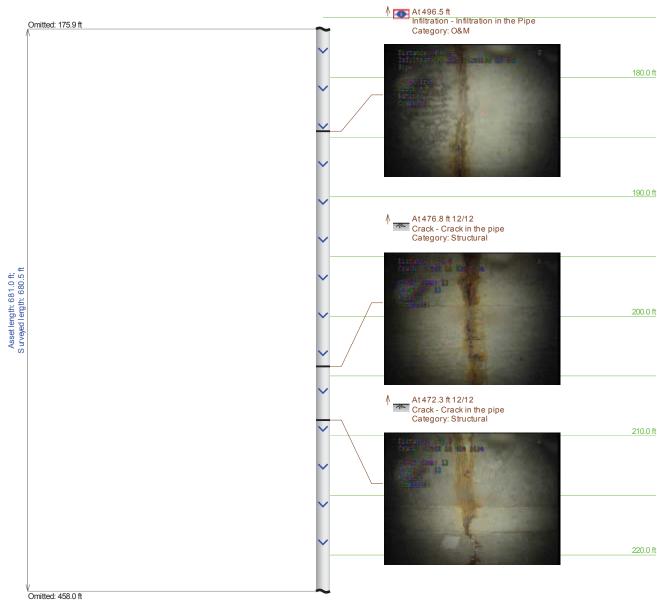


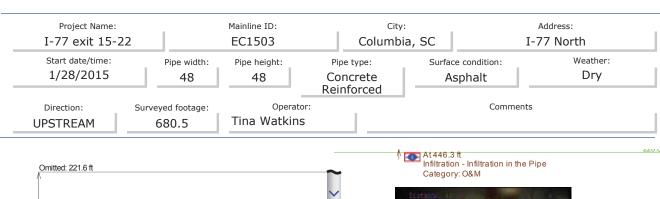


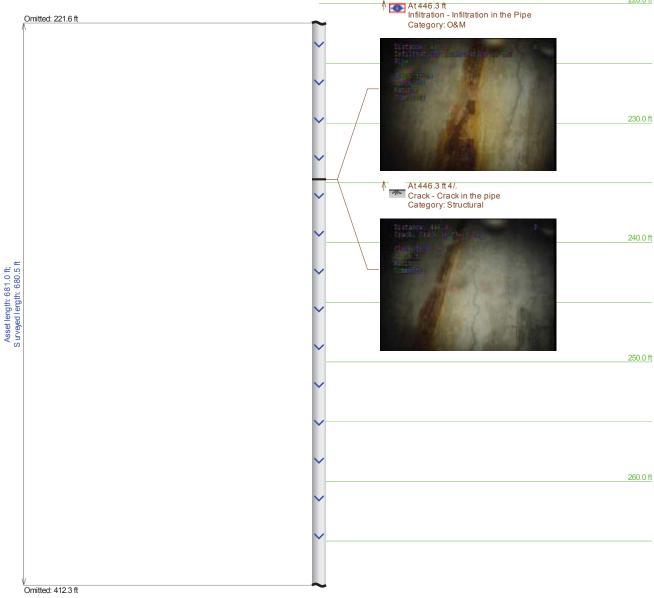


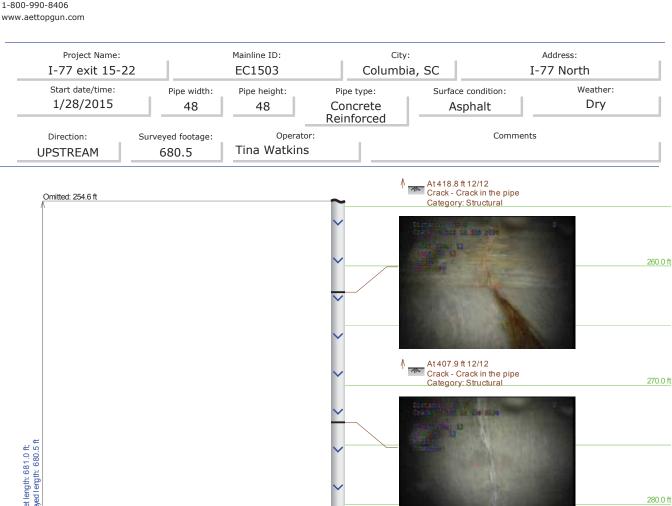








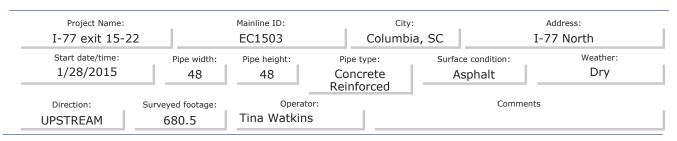


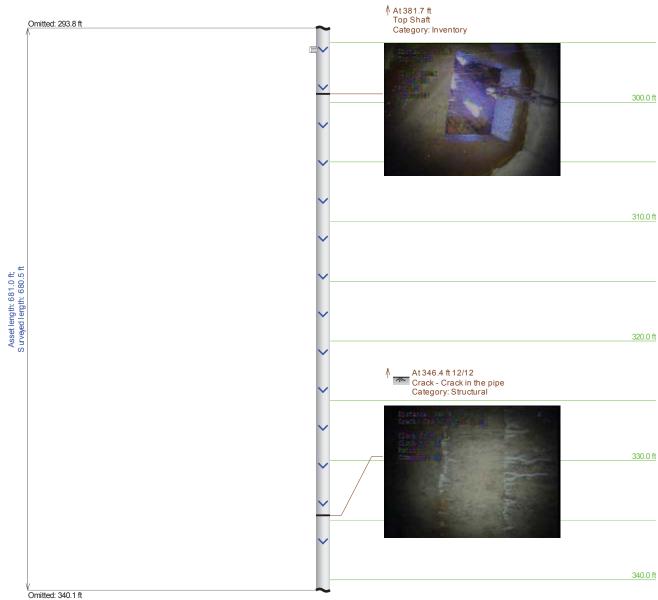


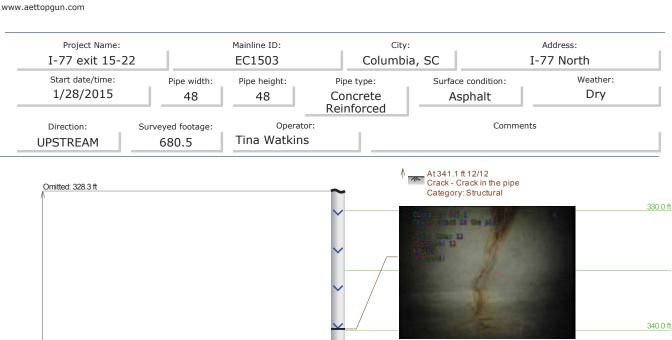
Omitted: 379.4 ft

At 385.4 ft
Lateral Connection Problem
Category: Structural

290.0 ft







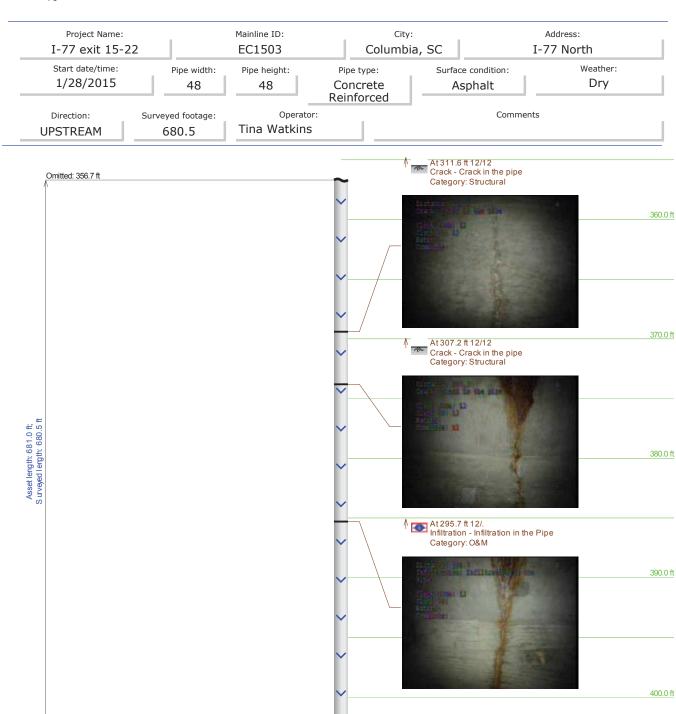
At 330.5 ft 12/12
Crack - Crack in the pipe
Category: Structural

At 322.9 ft 12/12
Crack - Crack in the pipe
Category: Structural

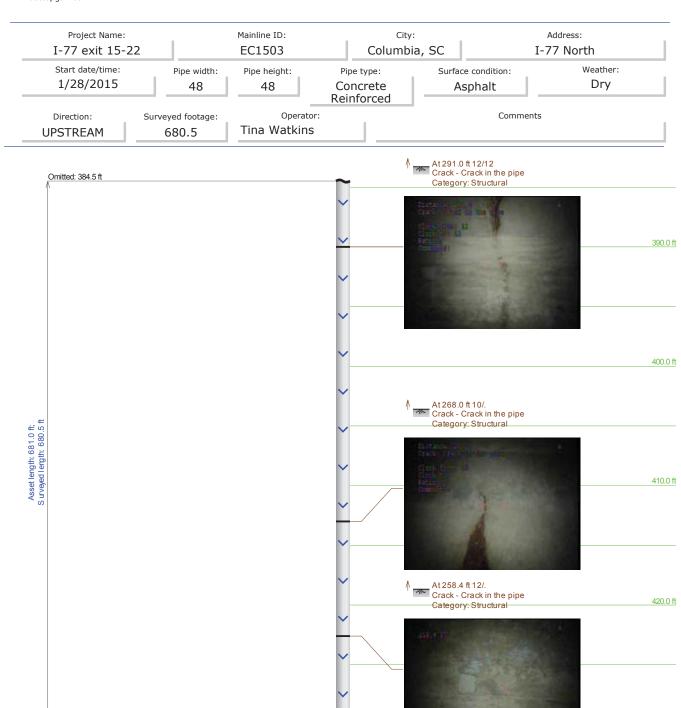
Omitted: 305.6 ft

350.0 ft

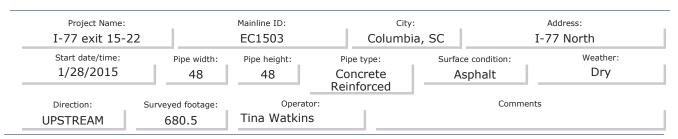
360.0 ft

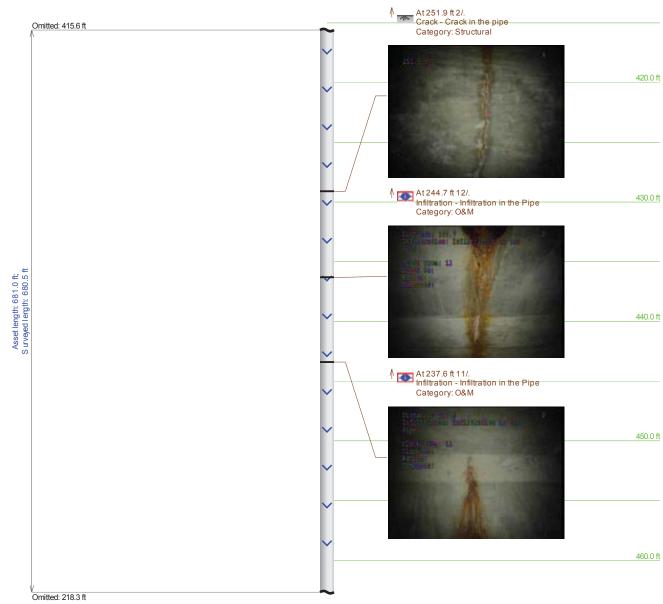


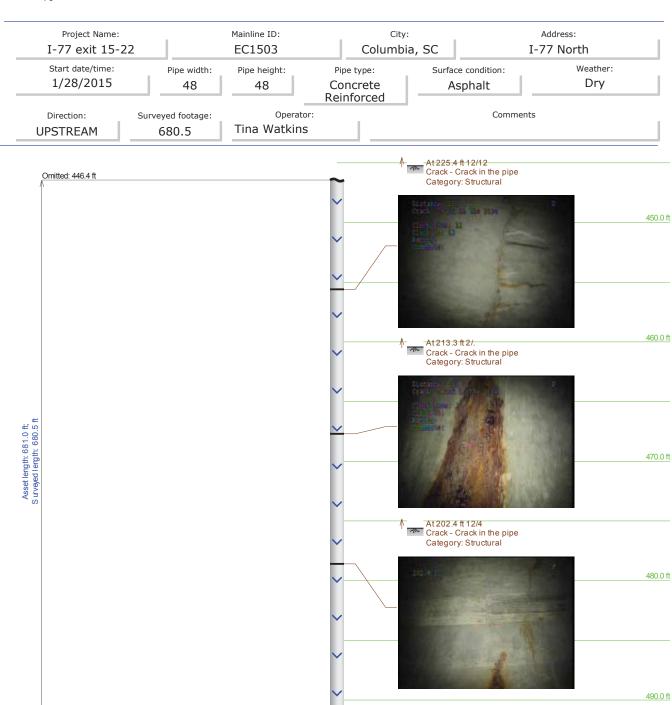
Omitted: 277.2 ft



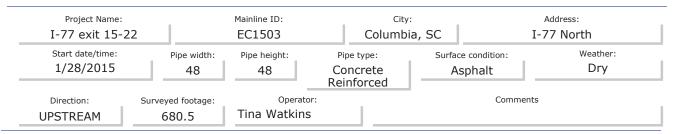
Omitted: 249.4 ft

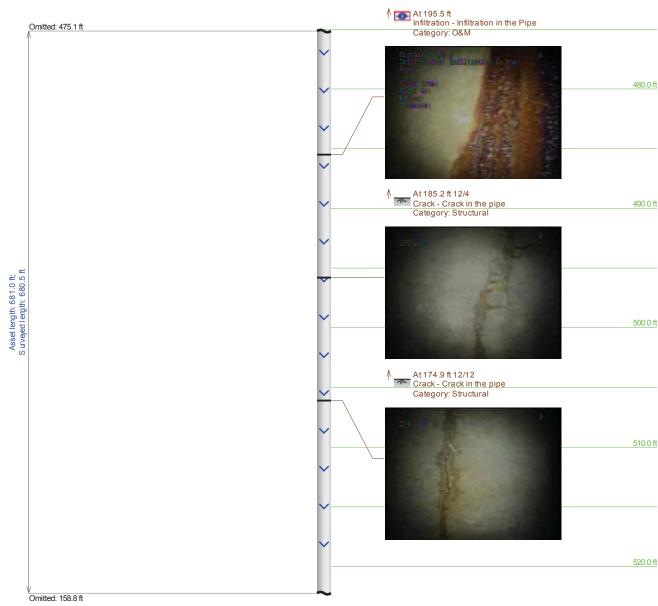


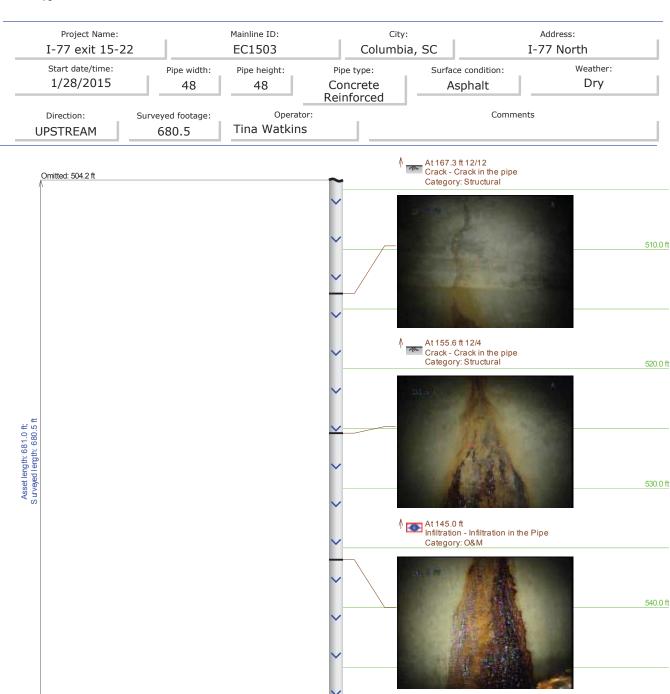




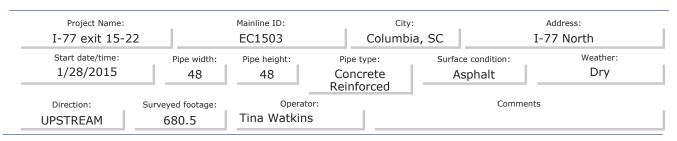
Omitted: 187.5 ft

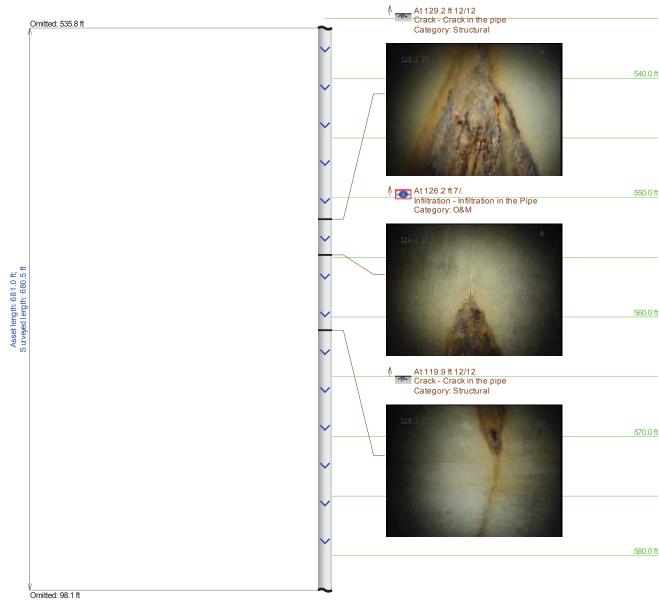


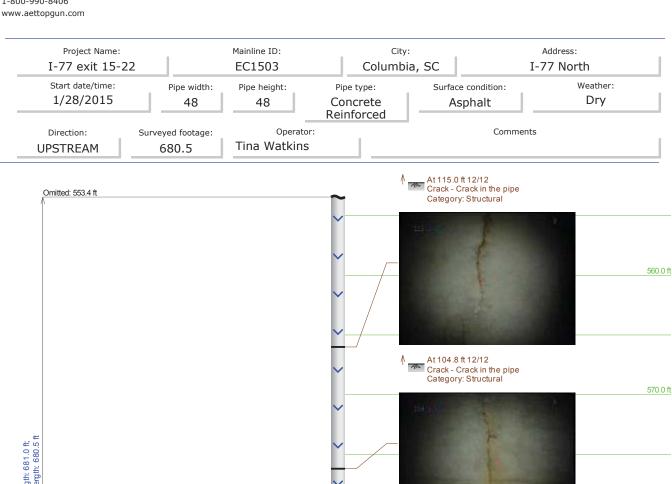




Omitted: 129.7 ft





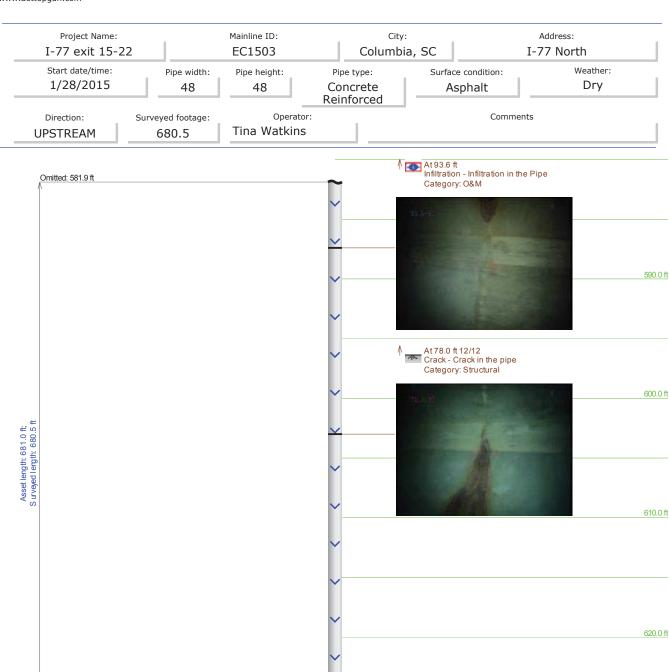


Omitted: 80.5 ft

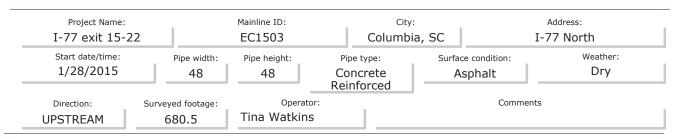
At 98.8 ft 12/12
Crack - Crack in the pipe
Category: Structural

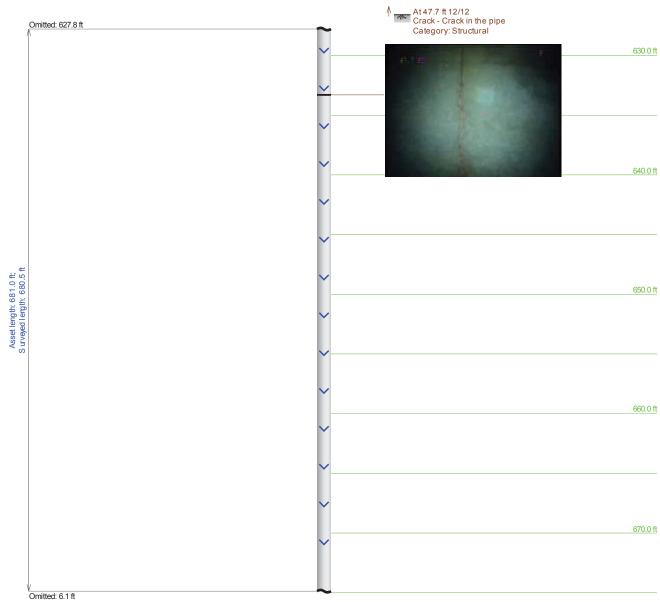
580.0 ft

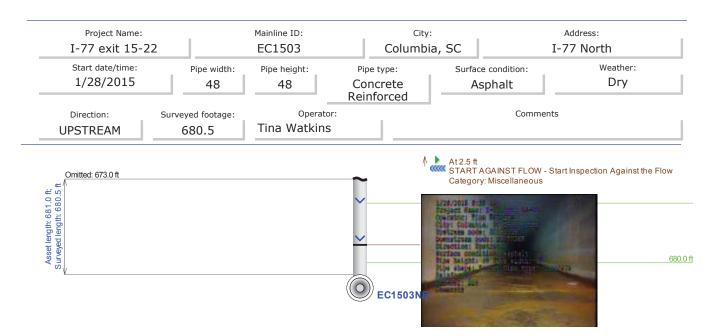
590.0 ft



Omitted: 52.0 ft

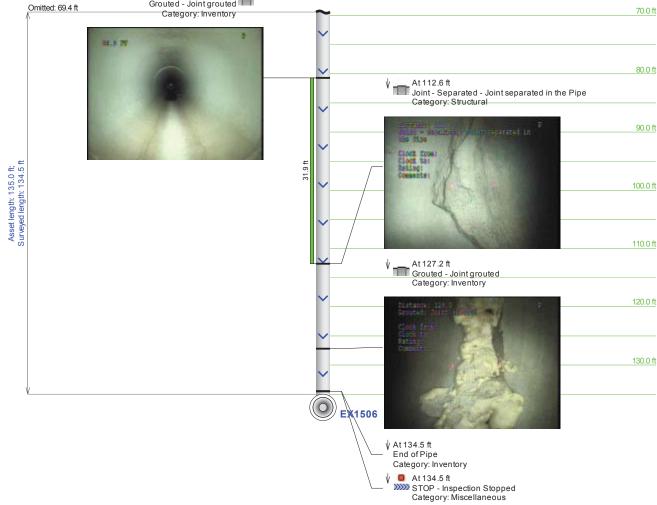




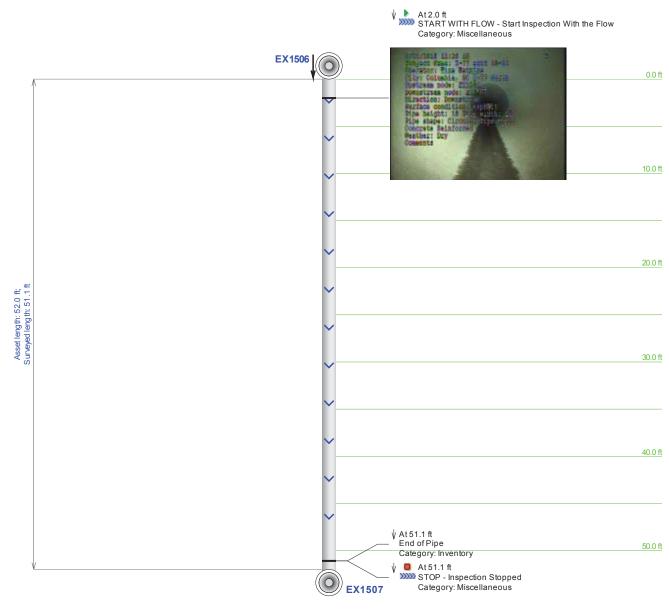


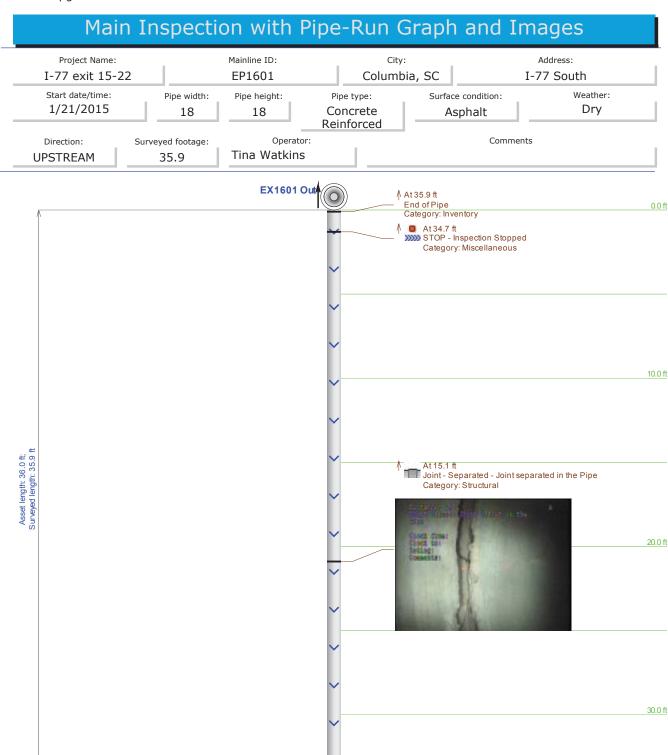
www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1506 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 18 18 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 134.5 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous EX1505 At 14.0 ft Grouted - Joint grouted Category: Inventory 25.5 77



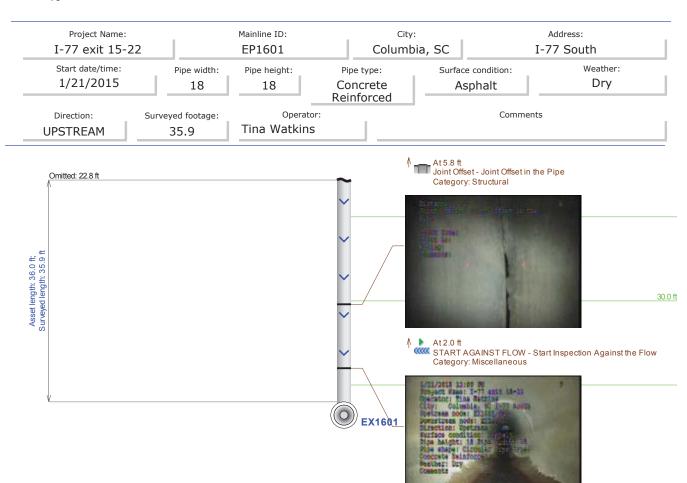


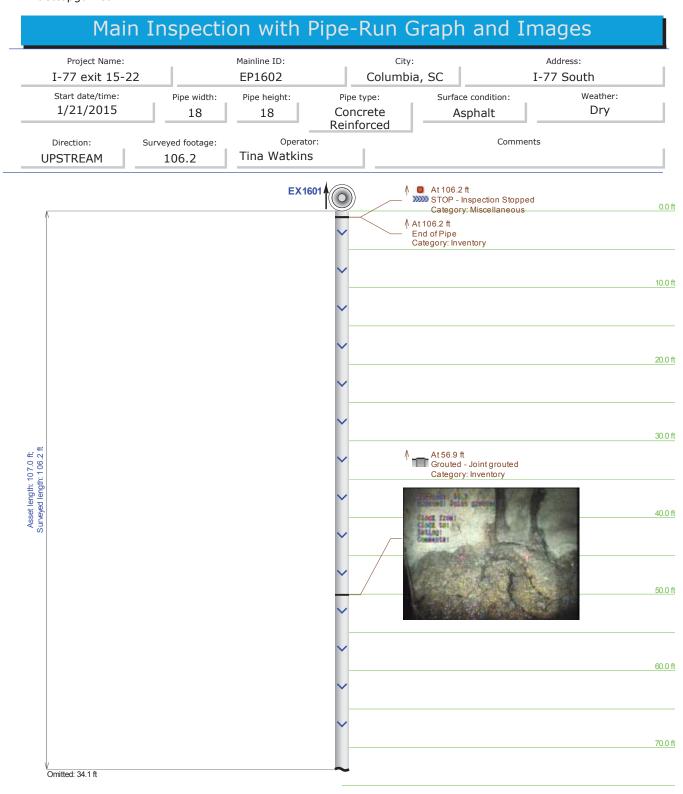
Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1507 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 51.1

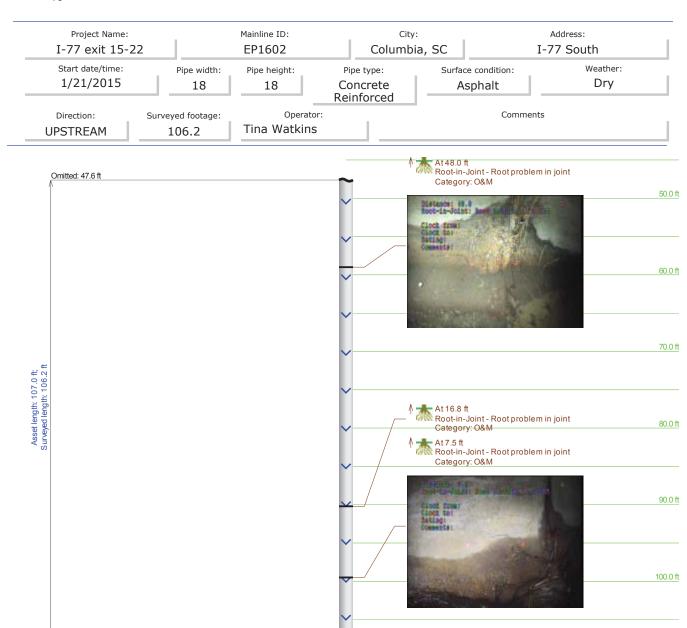




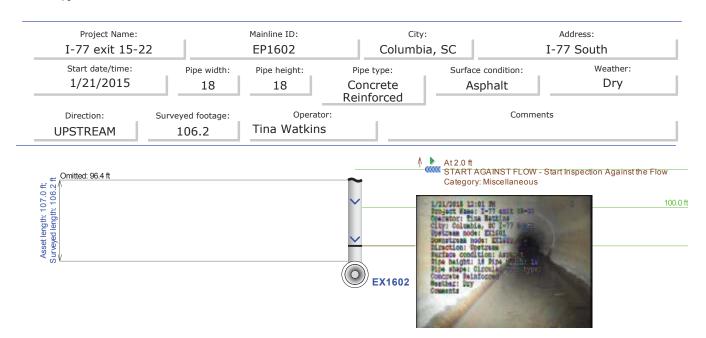
Omitted: 2.8 ft

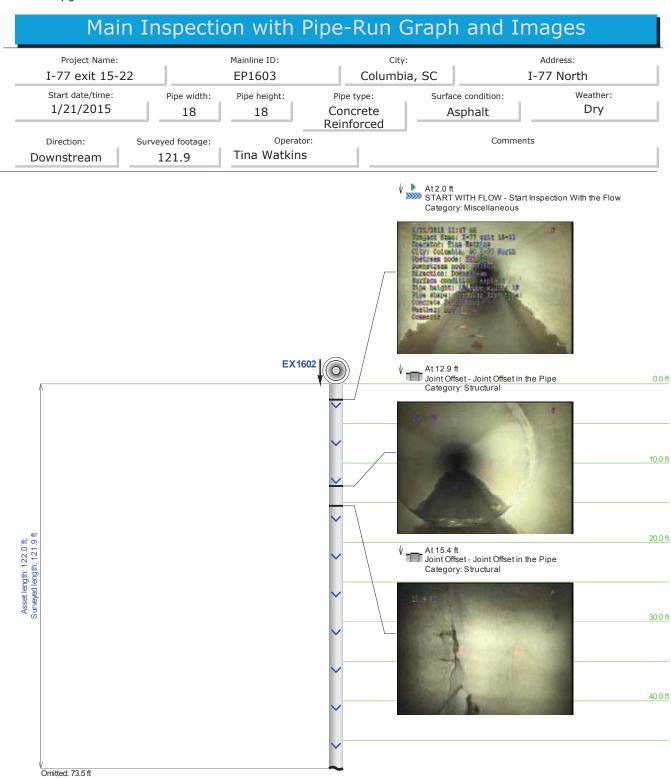






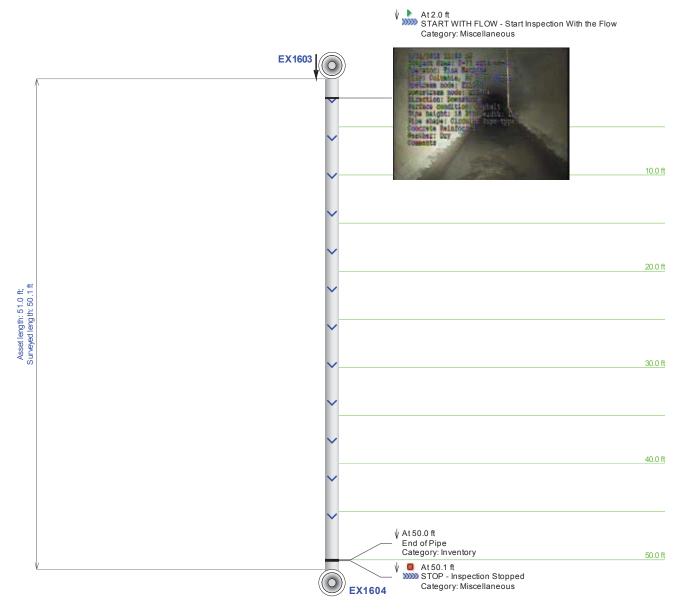
EX1602

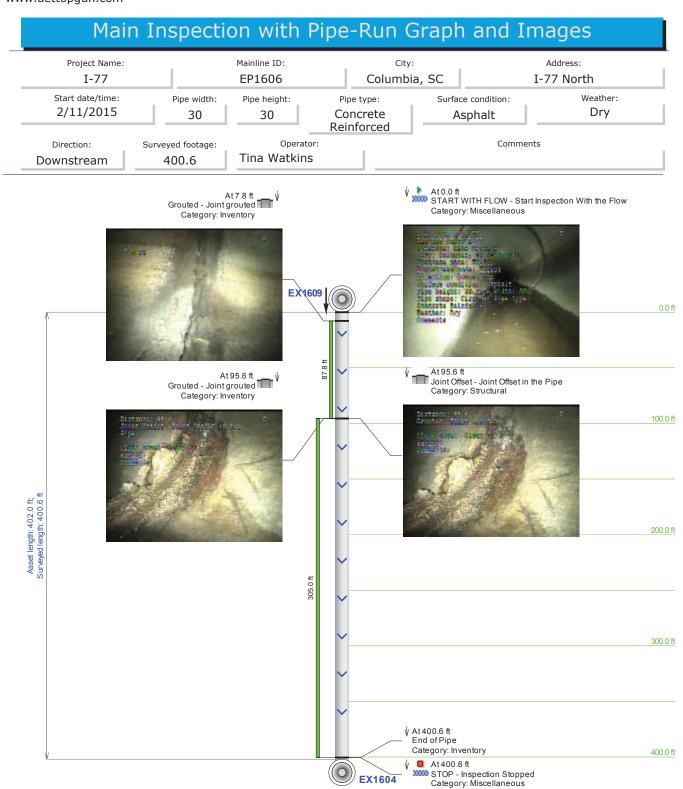


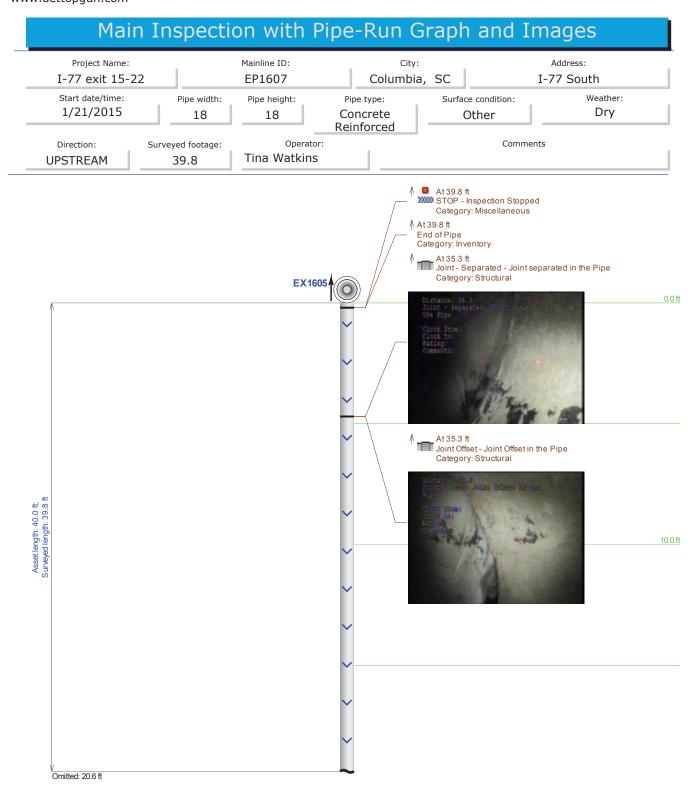


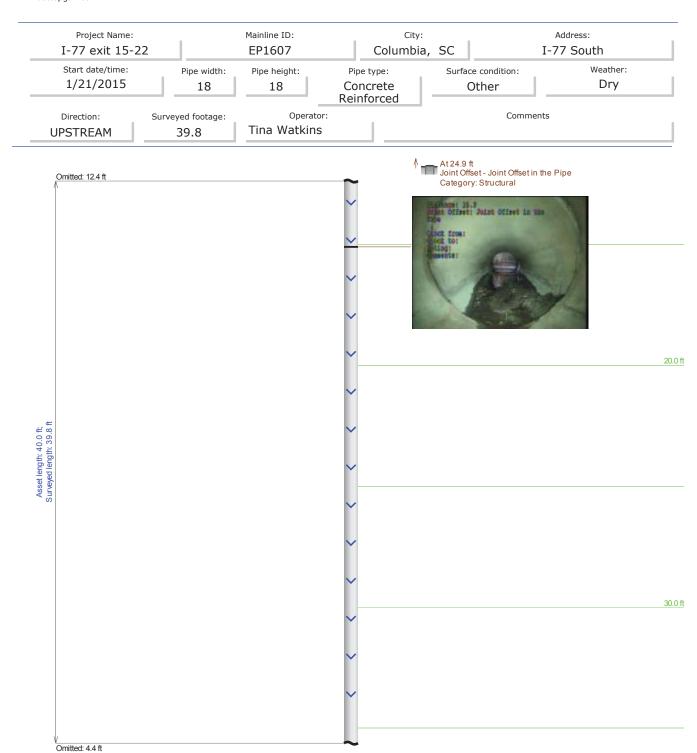


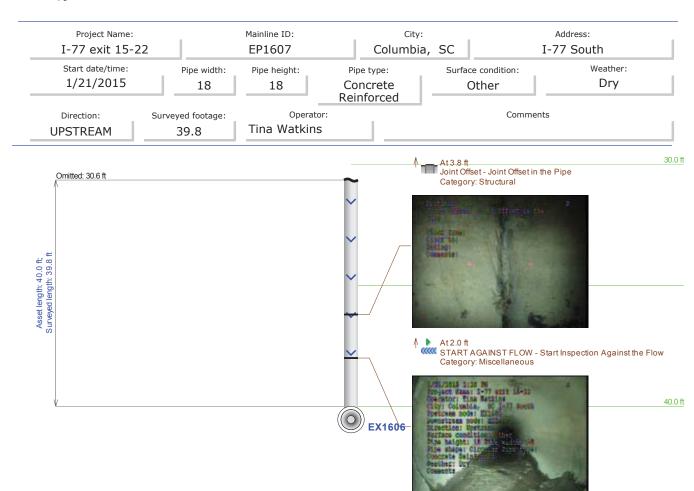
Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1604 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 24 24 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 50.1

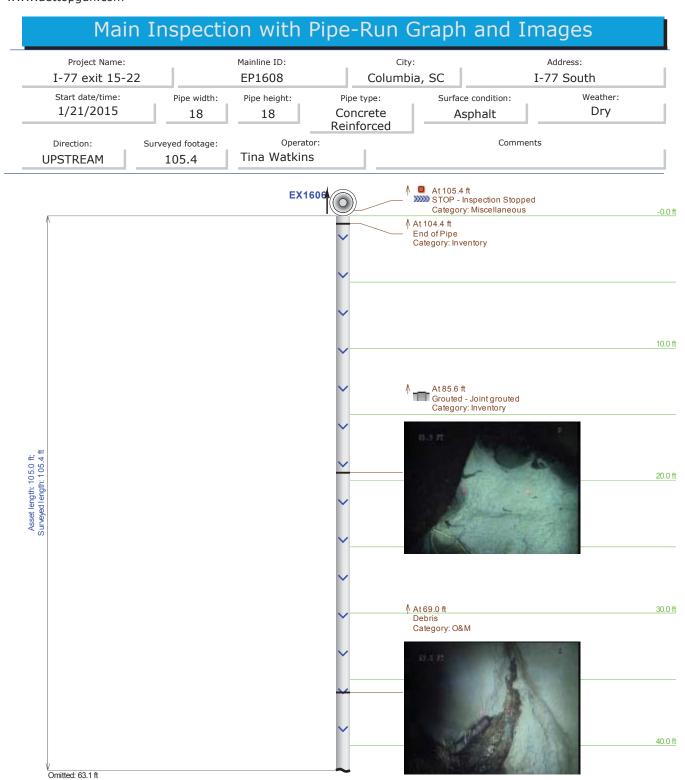


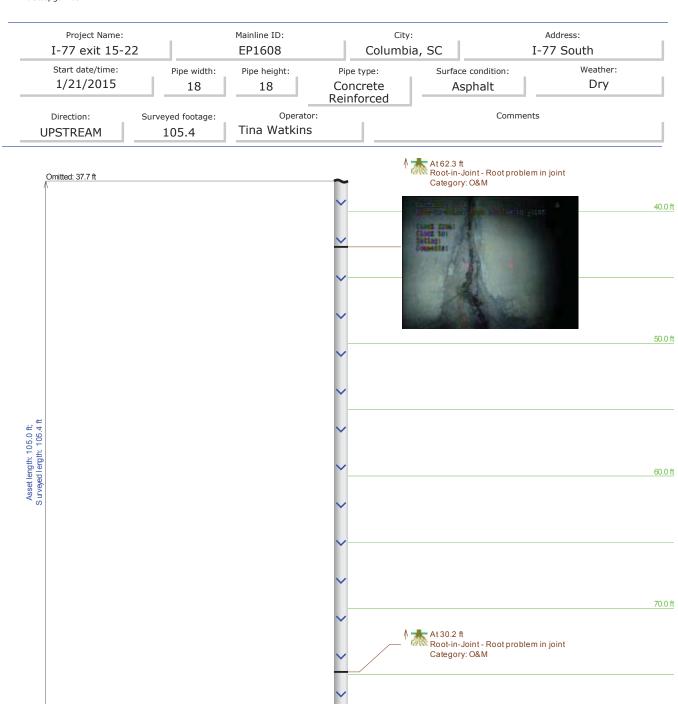






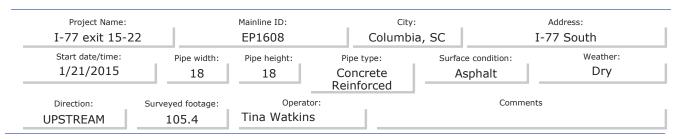


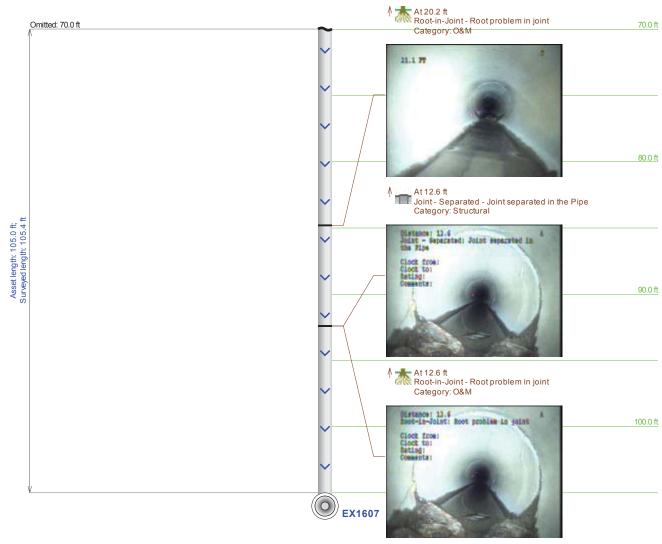


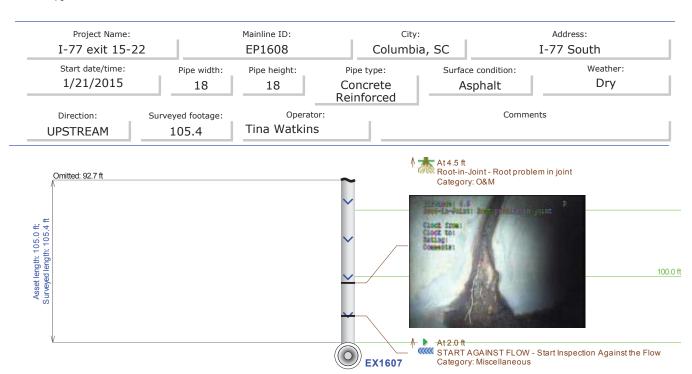


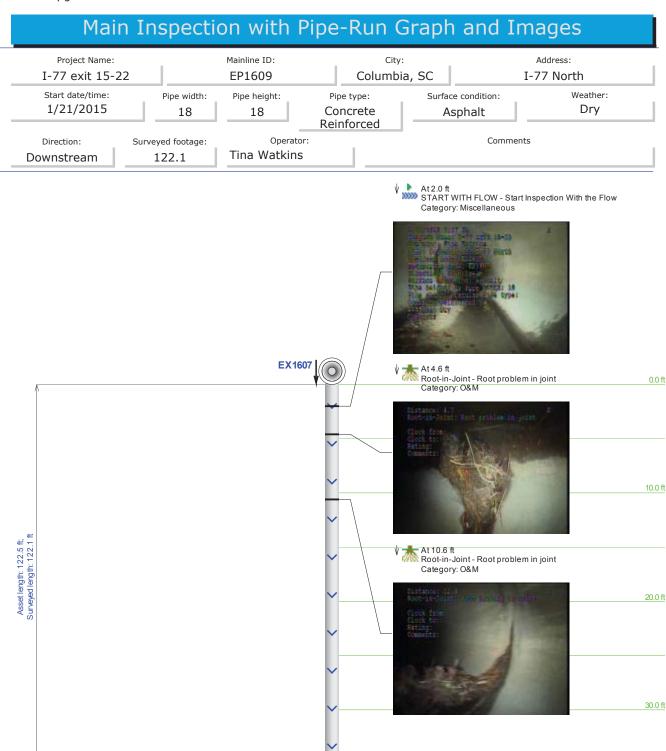
Omitted: 24.8 ft

80.0 ft

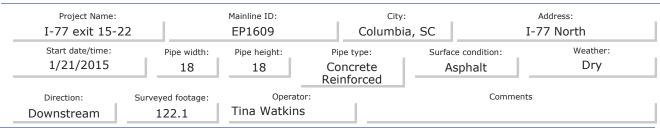


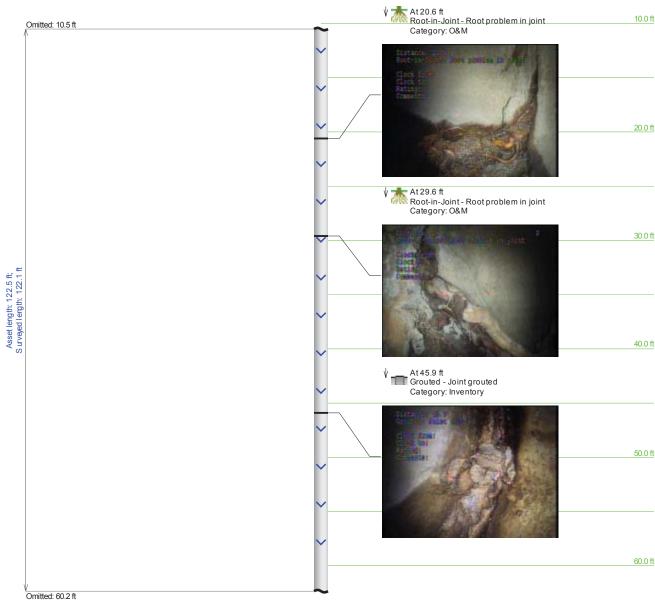


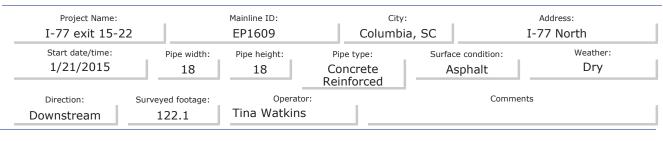


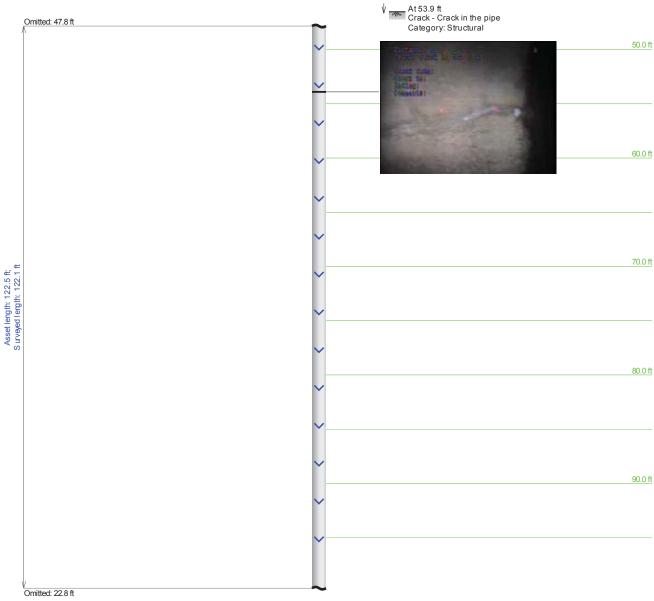


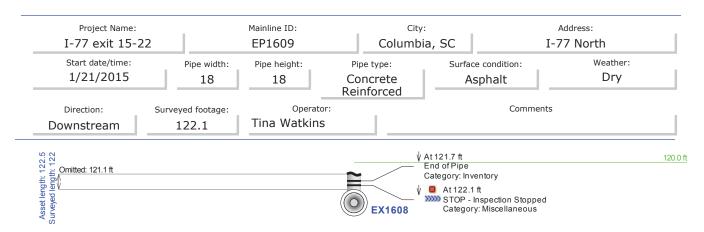
Omitted: 87.1 ft



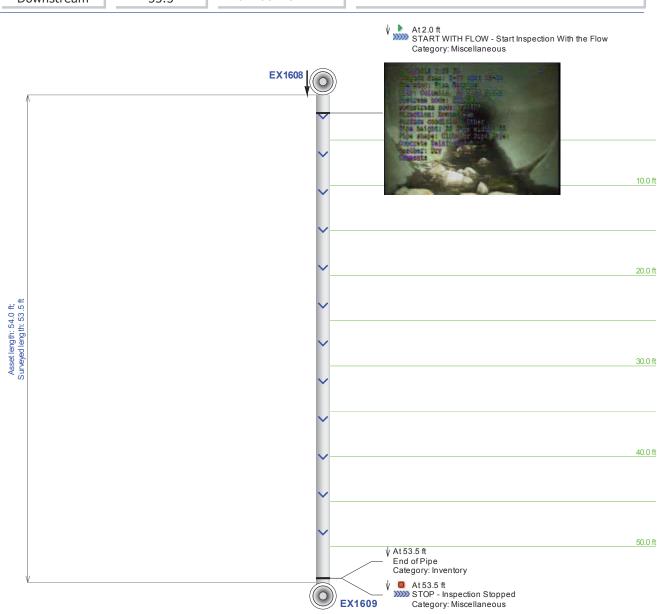




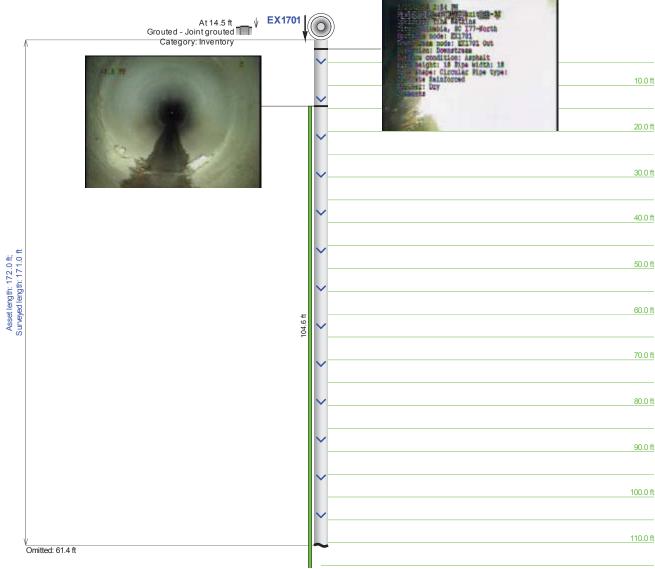




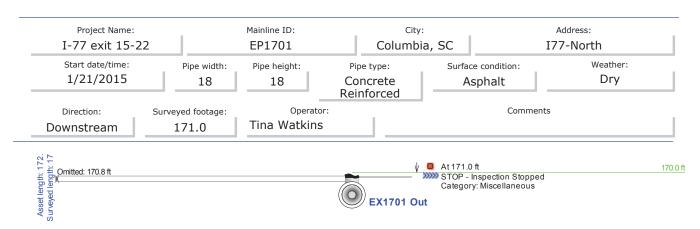
Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1610 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 24 24 Concrete Other Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 53.5



www.aettopgun.com Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP1701 Columbia, SC I77-North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 18 18 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 171.0 At 2.0 ft START WITH FLOW - Start Inspection With the Flow Category: Miscellaneous At 14.5 ft Grouted - Joint grouted EX1701 Category: Inventory









Robotic Inspections

EC1702

EC1702 Culvert 280'

Cracking Throughout

2 Pipes along right side from NB



Robotic Inspections

EC1703

EC1703

Culvert

280'

Cracking Throughout



Robotic Inspections

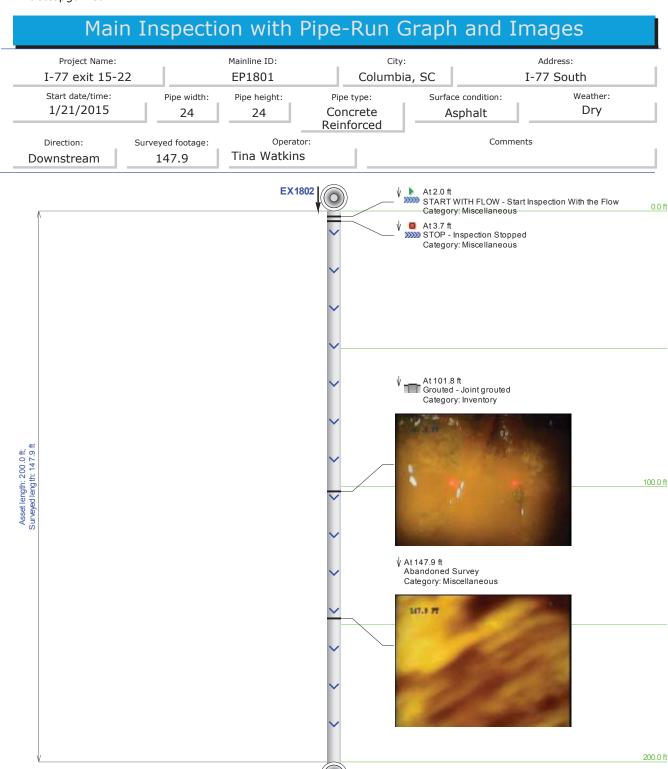
EC1704

EC1704

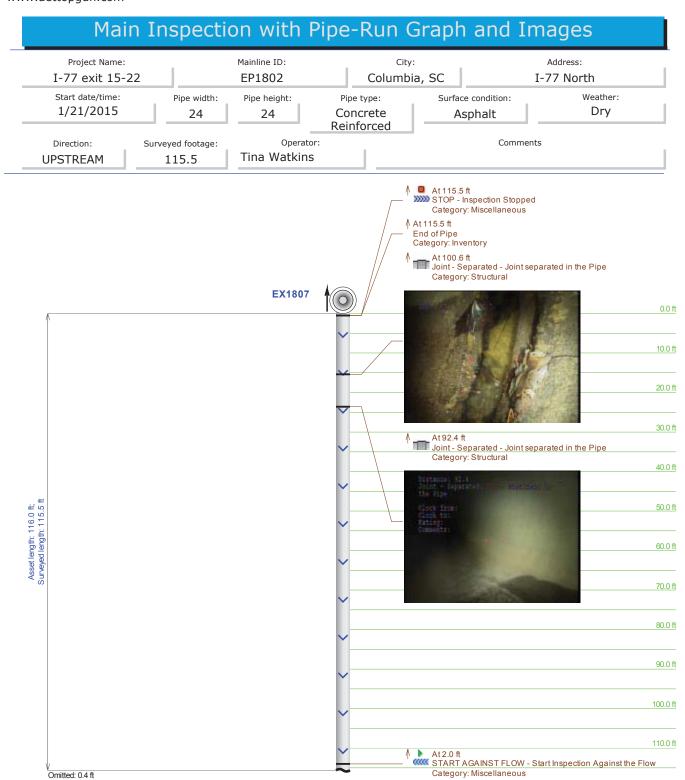
Culvert

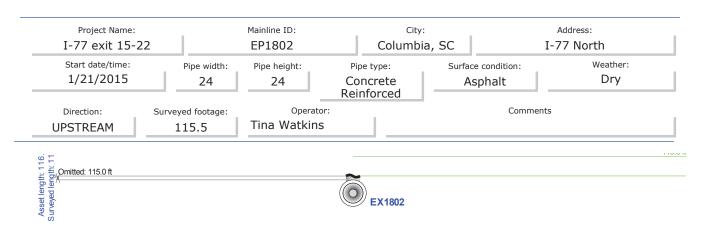
280'

Cracking Throughout

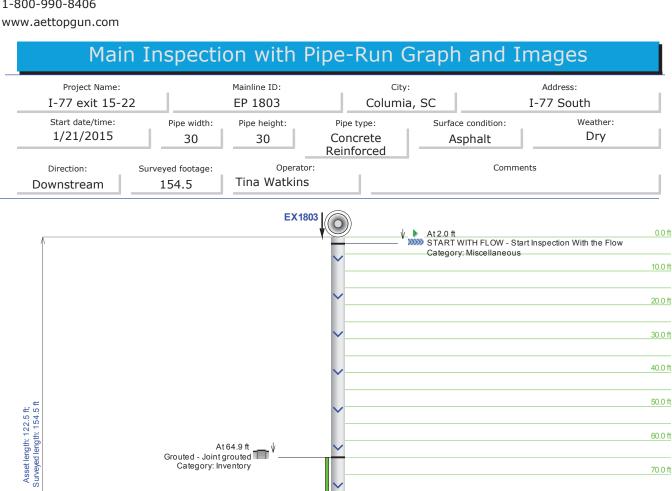


EX1801Out





AET Services 623 Rices Creek Road Liberty,SC 29657 1-800-990-8406



89.6 ft

EX1803Out

V At 154.5 ft End of Pipe

Category: Inventory

At 154.5 ft

STOP - Inspection Stopped Category: Miscellaneous 80.0 ft

90.0 ft

100.0 ft

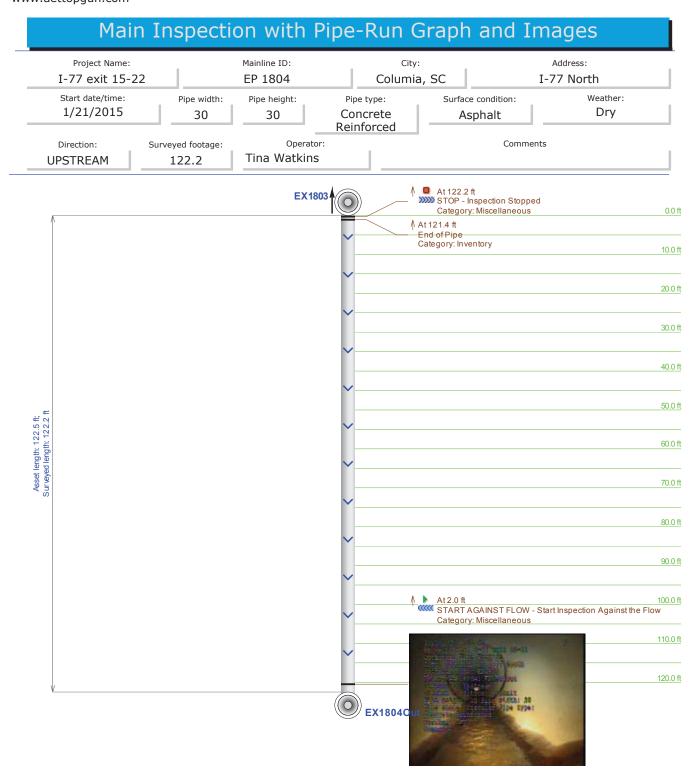
110.0 ft

120.0 ft

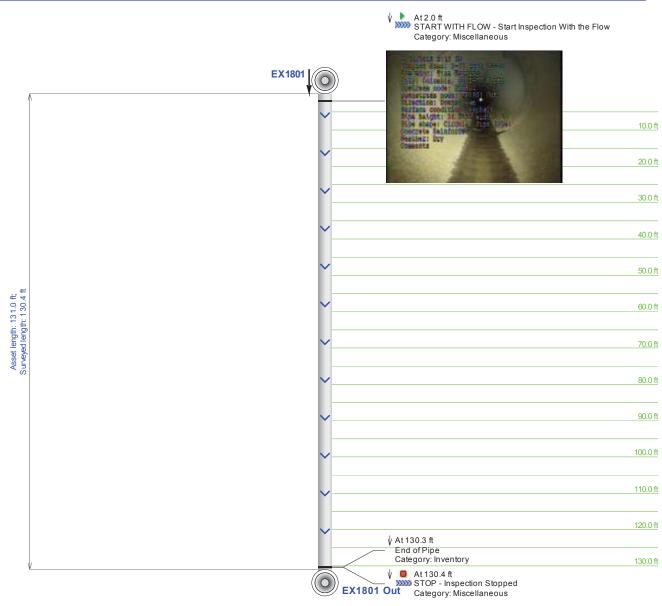
130.0 ft

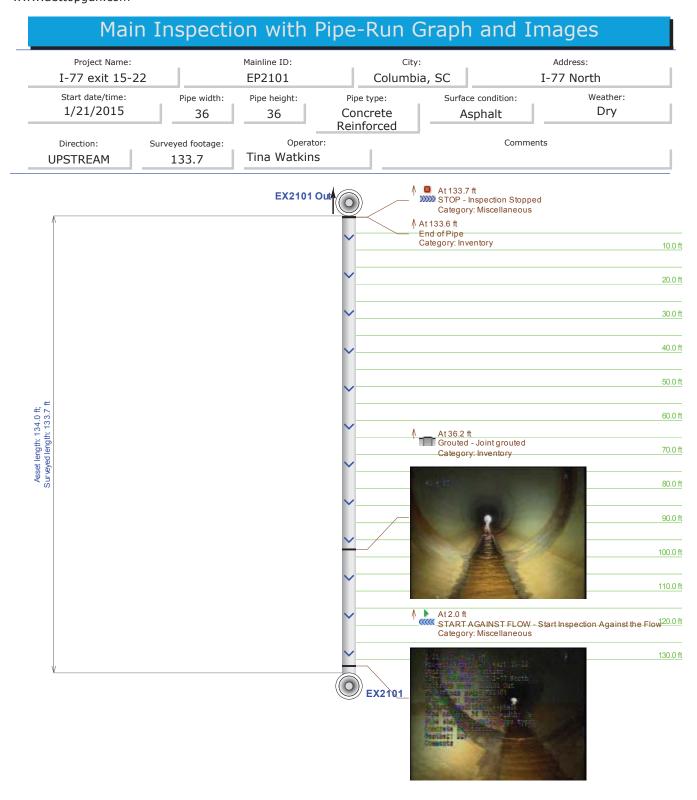
140.0 ft

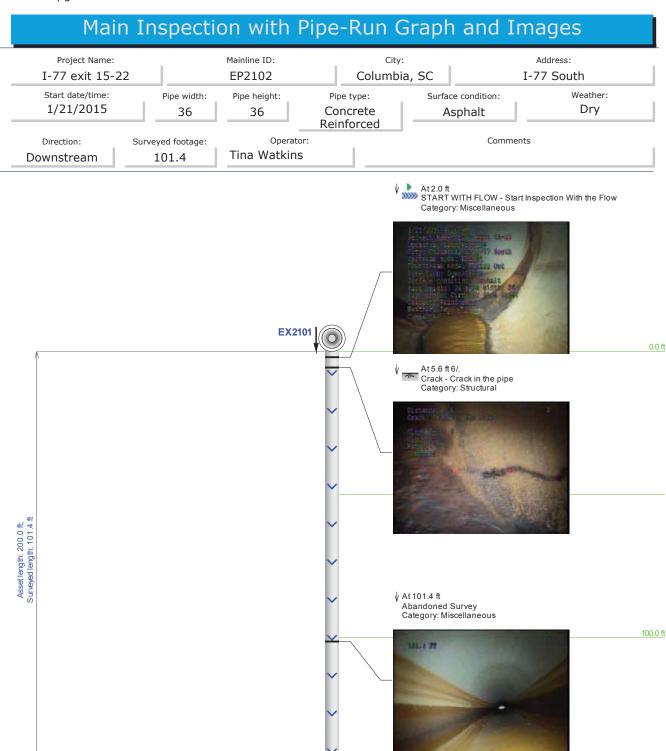
150.0 ft



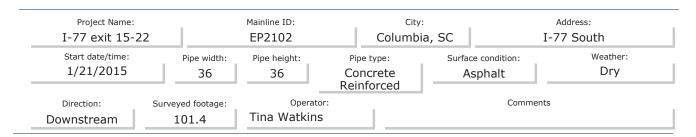
Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP1806 Columbia, SC I-77 North Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 24 24 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 130.4

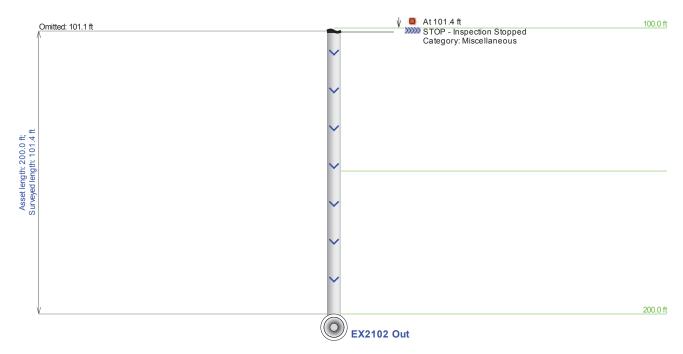






Omitted: 54.3 ft







Robotic Inspections

EC2103

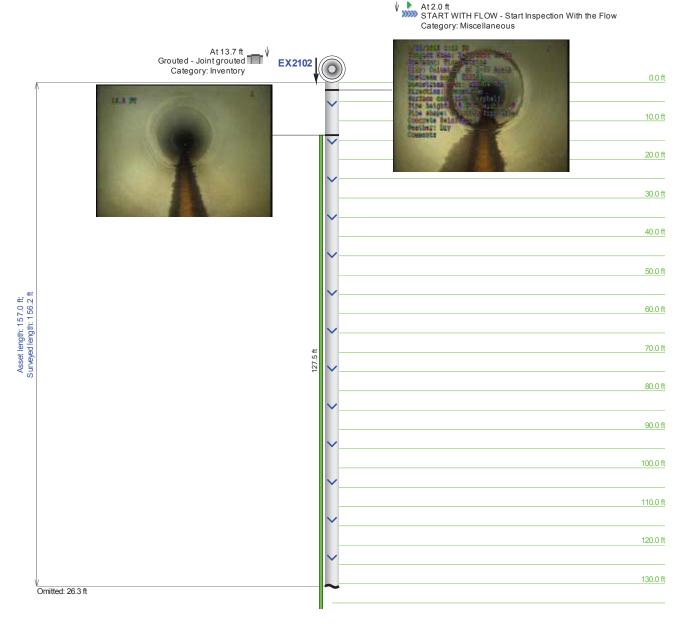
EC2103

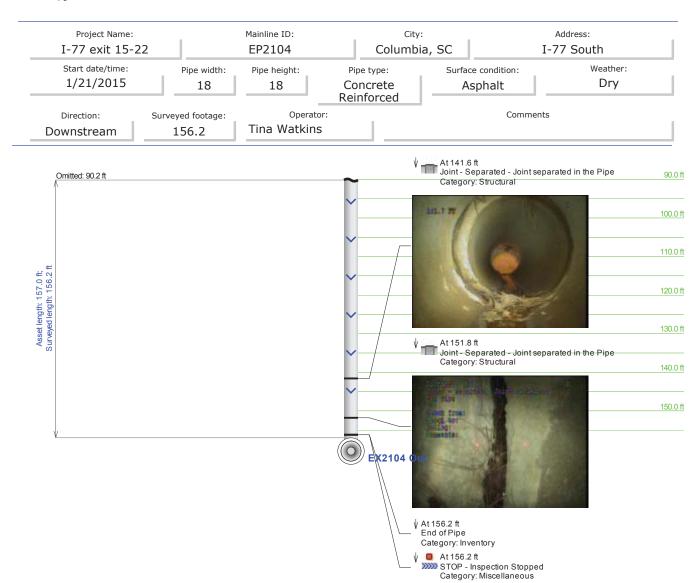
Culvert

288'

Cracking Throughout

Main Inspection with Pipe-Run Graph and Images Mainline ID: Project Name: City: Address: I-77 exit 15-22 EP2104 Columbia, SC I-77 South Start date/time: Weather: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 18 18 Concrete **Asphalt** Reinforced Direction: Surveyed footage: Operator: Comments Tina Watkins Downstream 156.2







Robotic Inspections

EC2105

EC2105

Culvert

288'

Cracking Throughout

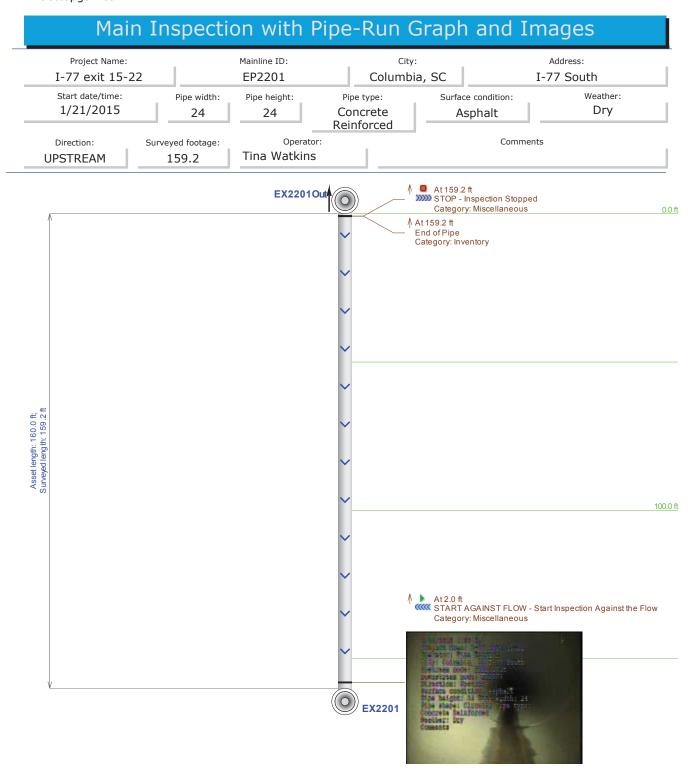


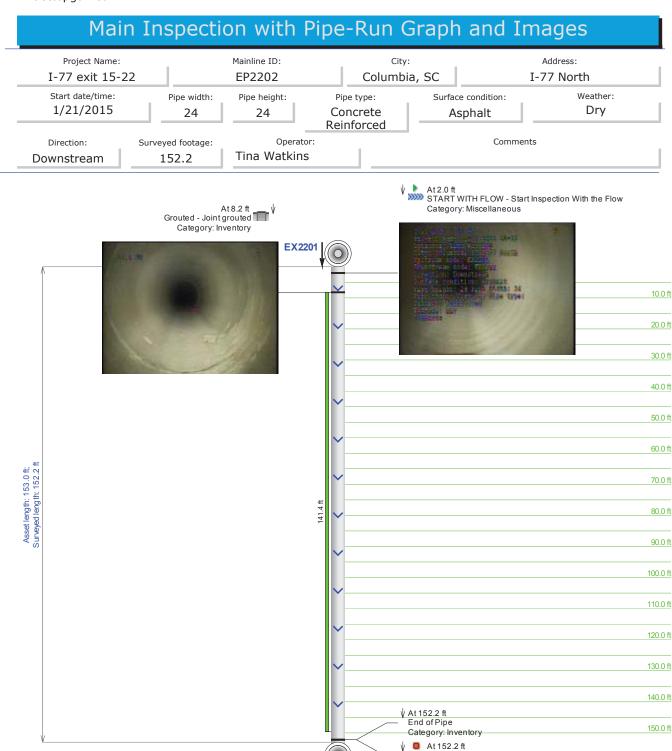
Robotic Inspections

EC2106

EC2106 Culvert 288'

Cracking Throughout

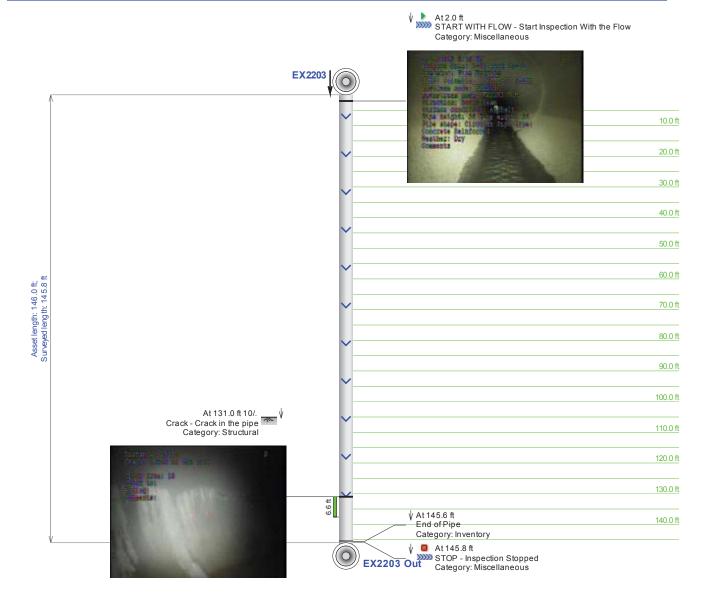


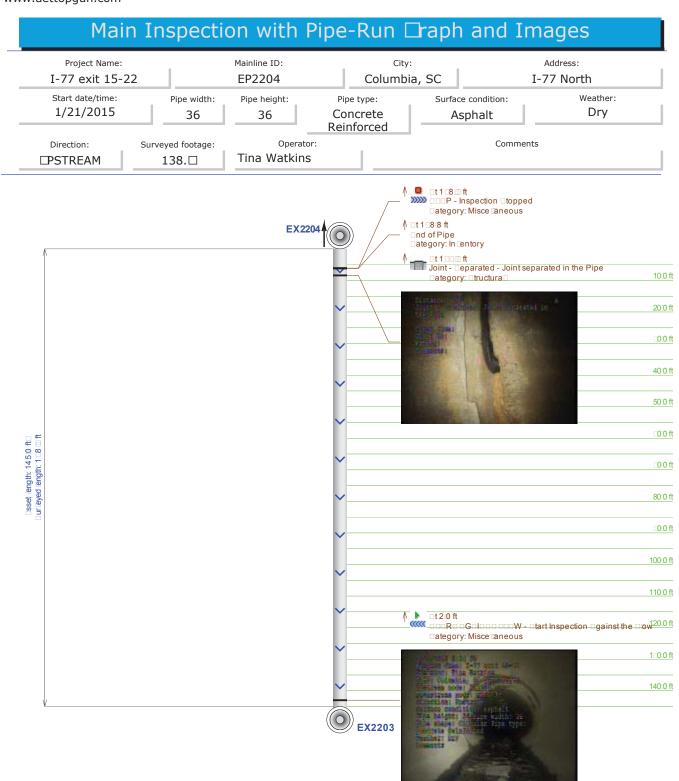


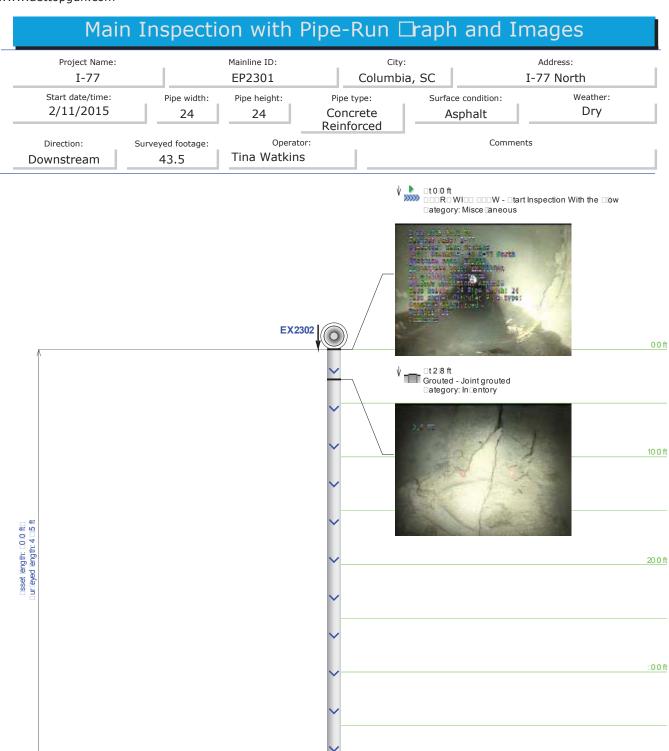
EX2202

STOP - Inspection Stopped Category: Miscellaneous

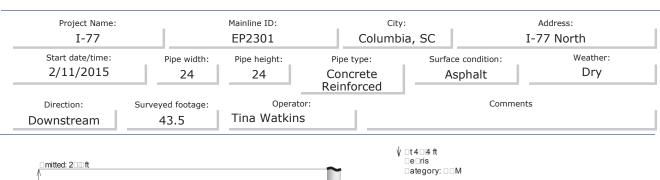
Main Inspection with Pipe-Run Graph and Images Project Name: Mainline ID: City: Address: I-77 exit 15-22 EP2203 Columbia, SC I-77 South Weather: Start date/time: Pipe width: Pipe height: Surface condition: Pipe type: 1/21/2015 Dry 36 36 Concrete **Asphalt** Reinforced Operator: Direction: Surveyed footage: Comments Tina Watkins Downstream 145.8

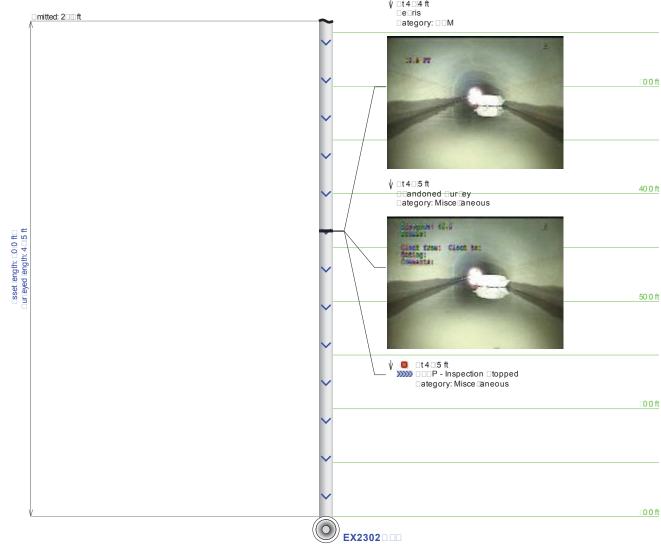


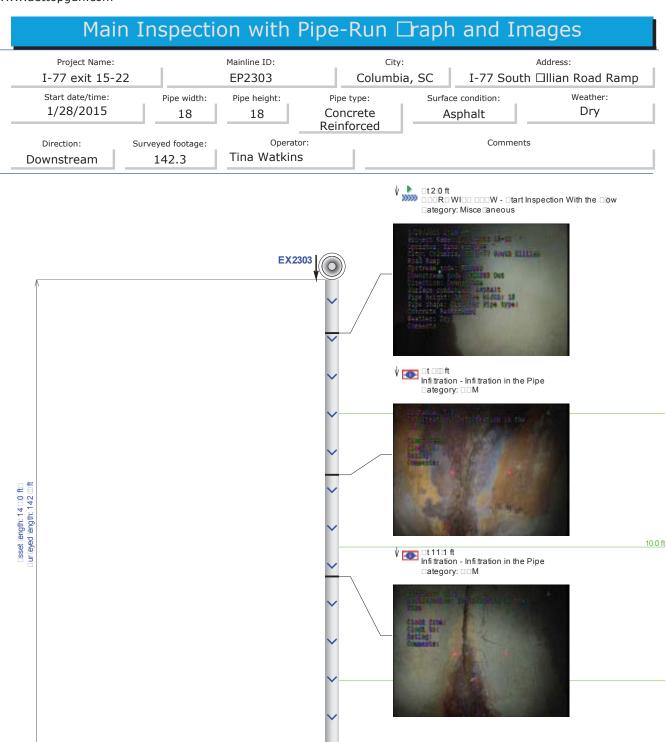




□mitted: □0Ⅲft







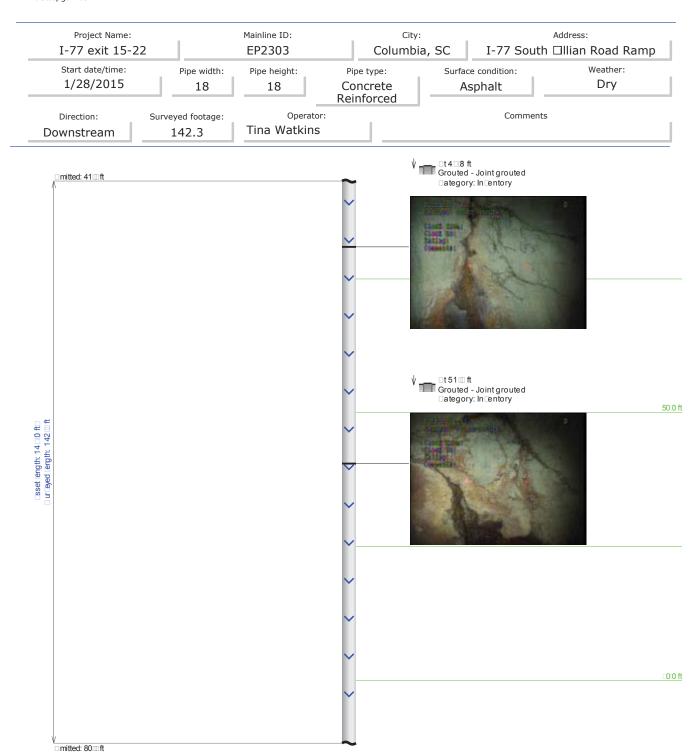
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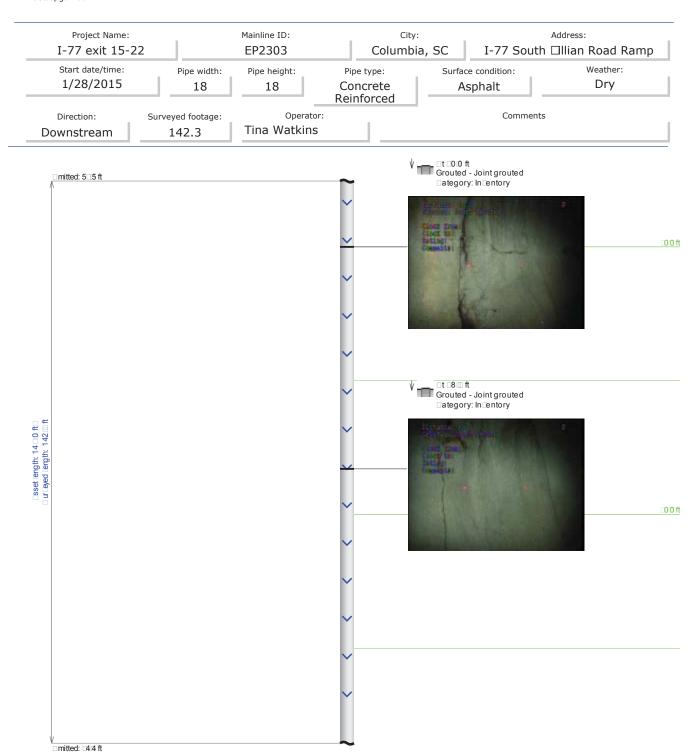


□mitted: 11 □1 ft



□mitted: □□0 ft

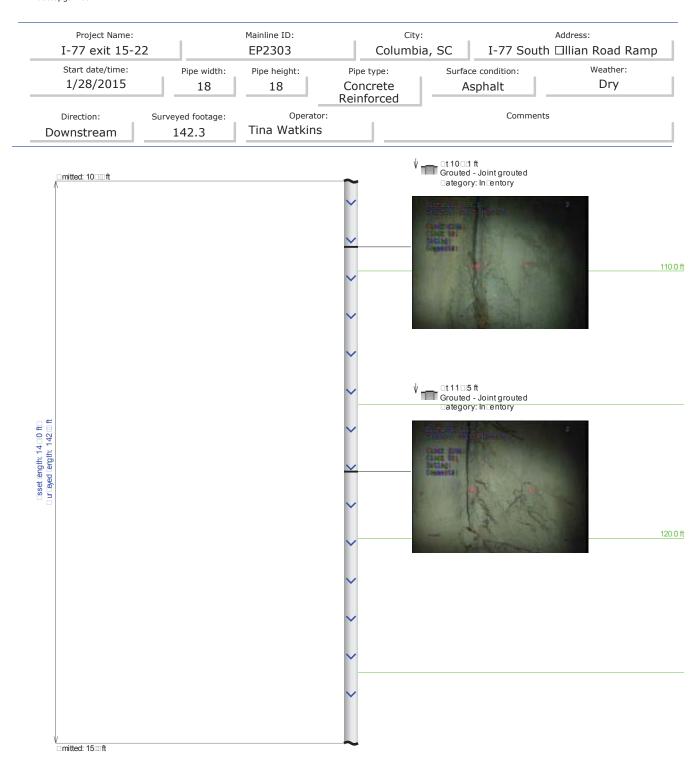








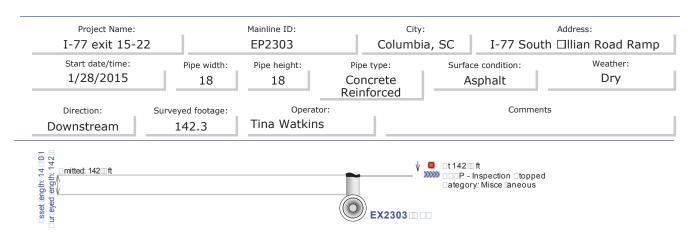
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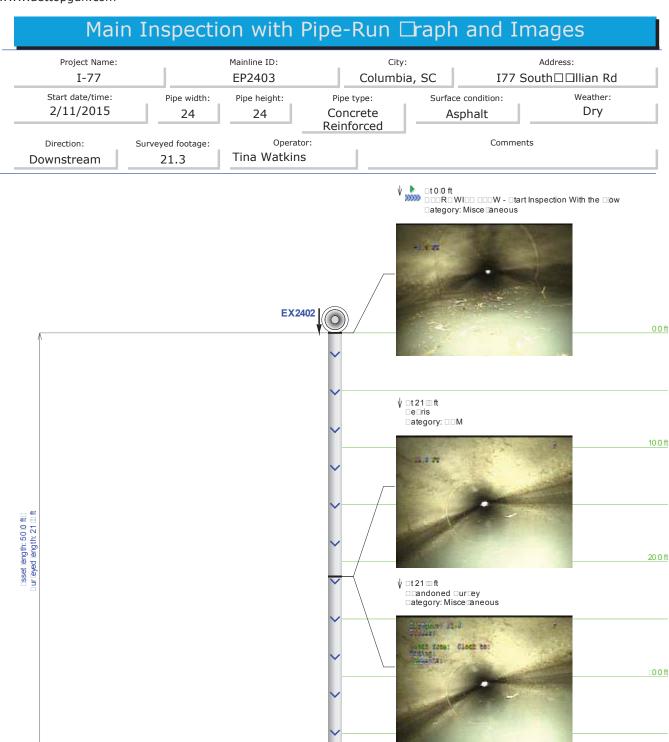
AET Services 623 Rices Creek Road □berty,SC 2□557 1-800-□10-8406 www.aettopgun.com



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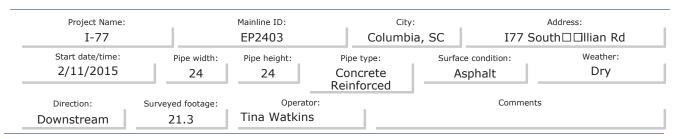


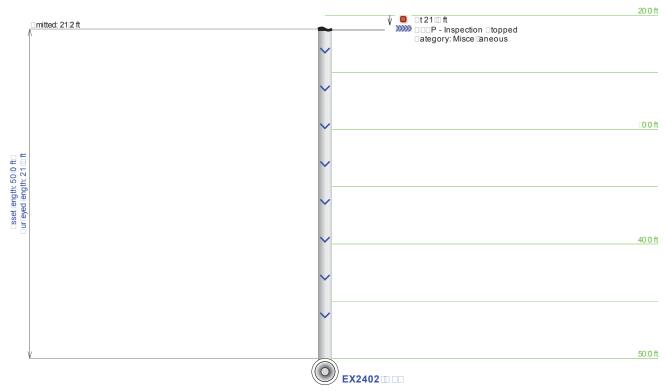
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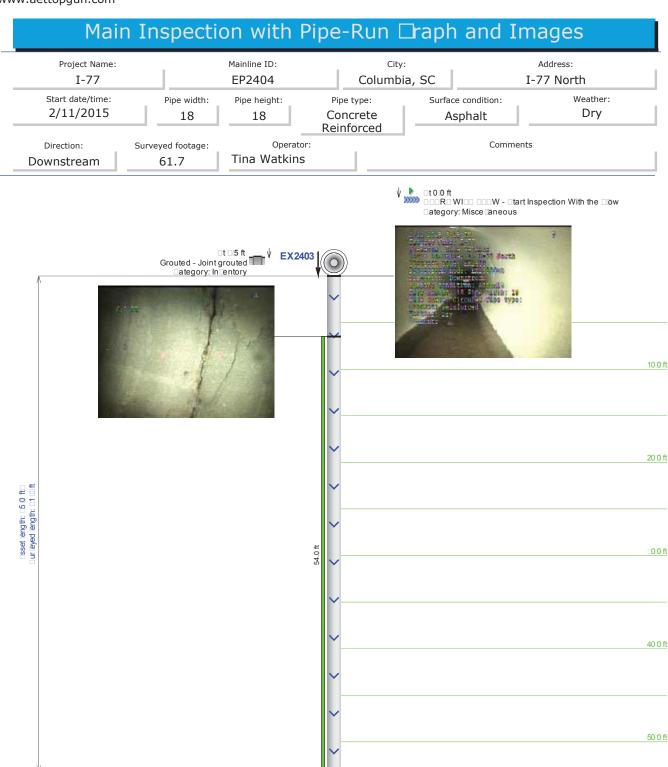
□mitted: 11® ft

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□mitted: 11⊞ft

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